

DIVISION 700 - MATERIALS

GENERAL STATEMENT MATERIALS CERTIFICATION LETTER

For all materials used in the work for which there is no specified acceptance testing by the project Inspectors or the Laboratory, the Contractor shall submit a Materials Certification Letter similar to the following, prior to acceptance as specified in Section 107.9.4.

Company Letterhead

Mr./Mrs. _____, Resident Date _____

Address _____ Project No. _____

Town _____

This is to certify that all materials incorporated into the project for which there is no specified acceptance testing by project inspectors or the laboratory, comply with the pertinent specified material requirements of the contract. Processing, project testing, and inspection control of raw materials shall be in conformity with the applicable drawings and/or standards for all articles furnished.

All records and documents pertinent to this letter and not submitted herewith will be maintained and will be available by the undersigned for a period of not less than three years from the date of completion of the project.

The Materials Certification letter must be signed by a person having legal authority to bind the Contractor.

Materials listed in the above Certificate may be subject to random sampling and testing by the Department at any time. When random verification samples are obtained from the project, they shall be provided by the Contractor at no cost to the Department. Certified materials, which fail to meet specification requirements, may not be accepted and may require replacement with materials that do meet the specifications.

The Contractor may be required to submit to the Resident, for inclusion in the project records, certification and other data from the Manufacturer pertaining to materials used on the project.

The Certificate shall include the actual test results of the material in storage from which the shipments are being made. Certificates shall be supplied for each lot, batch, or blend of each type and grade of material. A new certificate shall be issued at least every 30 days or upon receiving or manufacture of a new material.

The Contractor shall give the supplier sufficient advance notice of orders to permit testing. Material not represented by tests will not be accepted for use on the work.

Deliveries of asphalt materials shall be accompanied by a loading invoice, delivery ticket, or slip, as required under Section 108.1.3 f. The Loading Invoice shall include the applicable certificate number and shall include a printed or stamped statement such as the following:

“THIS IS TO CERTIFY THAT THE ASPHALT MATERIAL REPRESENTED BY THIS LOADING INVOICE CONFORMS TO THE SPECIFICATIONS OF THE PURCHASER FOR THE MATERIAL TYPE AND GRADE STATED THEREON.”

In the event an intermediate hauler of the asphalt material is involved, a copy of their own delivery slip shall be furnished, as well as a copy of the supplier's loading invoice. The hauler's delivery slip and the supplier's loading invoice shall be cross-referenced by use of their respective serial numbers.

All test procedures shall conform to the requirements of AASHTO unless otherwise noted. Gradation tests shall be completed in accordance with AASHTO T 27 except that the sample may be separated on the ½ inch screen. Testing as specified in AASHTO T 11 shall be performed on any materials for which there is a specification limit on the amount of material passing a No. 200 sieve.

The Department may require the Contractor to submit, for inclusion into the project records, certification that new Work Zone Category 1 and Category 2 Devices meet National Cooperative Highway Research Program (NCHRP) Report 350 guidelines. Work Zone Category 1 Devices include plastic drums, cones, and tubular markers. Work Zone Category 2 Devices include portable sign stands (with signs), Type I, Type II, and Type III barricades, vertical panels, intrusion alarms, and other work zone devices under 100 pounds. All Work Zone Category 1 and Category 2 Devices shall meet NCHRP Report 350 and MUTCD guidelines.

Vendors/Contractors will be allowed to self-certify Work Zone Category 1 Devices with a letter of self-certification. A letter of self-certification shall contain at a minimum

- a. A title e.g., “Certificate of Crashworthiness”,
- b. Name and Address of the Vendor making certification,
- c. Unique identification of the certificate (such as a serial number) and of each page and the total number of pages,
- d. Description and unambiguous identification of the item tested,
- e. Identification of the basis for the self-certification process used and to what test level of NCHRP 350,
- f. Signature and title of person(s) accepting responsibility for the content of the certificate and date of issue, and
- g. A statement that the certificate shall not be reproduced except in full.

Crash test information is available on the FHWA Office of Highway Safety's Homepage: http://safety.fhwa.gov/programs/roadside_hardware.htm.

SECTION 701 - STRUCTURAL CONCRETE RELATED MATERIAL

701.01 Portland Cement and Portland Pozzolan Cement Portland cement shall conform to the requirements of AASHTO M 85, Type II.

A Type I, Type II or Type III cement meeting AASHTO M 85 may be used when making precast units.

A Type IP (MS) Portland-pozzolan cement (blended hydraulic cement with moderate sulfate resistance) meeting the requirements of AASHTO M 240, may be used instead of Type II or where Type I Portland cement, meeting the requirements of AASHTO M 85, is allowed. The definitions of the two hydraulic cements mentioned above are as follows: (See ASTM C 219)

Portland cement - a hydraulic cement produced by pulverizing Portland cement clinker, and usually containing calcium sulfate.

Portland-pozzolan cement - a hydraulic cement consisting of an intimate and uniform blend of Portland cement or portland blast furnace slag cement and fine pozzolan produced by intergrinding Portland cement clinker and pozzolan, by blending Portland cement or portland blast furnace slag cement and finely divided pozzolan, or a combination of intergrinding and blending, in which the amount of the pozzolan constituent is within specified limits.

Only one brand of cement shall be used on any one contract unless otherwise permitted, in writing, by the Resident. The cement shall be protected from dampness during storage. Partially set cement or cement that contains caked lumps shall not be used.

The Contractor shall arrange for the cement supplier to provide a Manufacturer's Certification (Mill Test Report) for all cement materials furnished for use on the project. Reports shall include all mandatory and optional requirements specified in AASHTO M 85 or AASHTO M 240 including any processing additions. Reports shall represent the cement incorporated into the project and reports shall be supplied, at a minimum, on a monthly basis. Reports shall be submitted to the Department's Materials Testing Engineer.

701.02 Water Water used in mixing or curing concrete shall be reasonably clean and free of oil, salt, acid, alkali, sugar, vegetation, or substances injurious to the finished product. If required by the Resident, it shall be tested in accordance with the requirements of AASHTO T 26. Water known to be of potable quality may be used without testing. Where the source of water is relatively shallow, the intake shall be so enclosed as to exclude silt, mud, grass, or other foreign materials.

701.03 Air-Entraining Chemical Admixtures Air-entraining admixtures shall conform to the requirements of AASHTO M 154.

The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

701.04 Water Reducing Chemical Admixtures Water reducing admixtures shall conform to the requirements of AASHTO M 194, Type A.

The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

701.0401 High Range, Water Reducing, Chemical Admixture High range, water-reducing admixture, commonly referred to as both high range-water-reducers and/or superplasticizers, shall conform to the requirements of AASHTO M 194, Type F.

The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

701.05 Set-retarding Chemical Admixtures Set-retarding admixtures shall conform to the requirements of AASHTO M 194, Types B or D.

The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

701.06 Curing Materials Sheet materials for curing concrete shall conform to the requirements of AASHTO M 171. Burlap cloth shall conform to the requirements of AASHTO M 182 Class 3, 10 oz/yd. Liquid membrane-forming compounds shall conform to the requirements of AASHTO M 148 and shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

The two types of approved liquid membrane-forming compounds are: (1) Type 1-D, clear or translucent with fugitive dye which must be readily distinguishable for at least 4 hours and must be inconspicuous in 7 days and (2) Type 2, white pigmented.

701.07 Waterstops Waterstops shall be polyvinylchloride and conform to the requirements of US Army Corps of Engineers Specification CRD C-572.

701.08 Smooth Surfaced Asphalt Roll Roofing (formerly called Heavy Roofing Felt) Wherever heavy roofing is called for on the plans an approved standard brand of smooth surface asphalt roofing (organic felt) conforming to ASTM D 224 Type I.

701.10 Fly Ash Fly Ash shall conform to the chemical and physical requirements of AASHTO M 295, Class F fly ash.

The Contractor shall arrange for the fly ash supplier to provide a Manufacturer's Certification (Mill Test Report) for all fly ash materials furnished for use on the project. Reports shall include all mandatory and optional requirements specified in AASHTO M 295 including procedure A in Table 4. Reports shall represent the fly ash incorporated into the project and reports shall be supplied, at a minimum, on a monthly basis. Reports shall be submitted to the Department's Materials Testing Engineer.

701.11 Calcium Nitrite Solution, Chemical Admixture Calcium nitrite solutions shall conform to the requirements of AASHTO M 194, Type C (accelerating admixtures). An approved calcium nitrite based corrosion inhibitor shall be added to the concrete mix as an aqueous solution.

The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

701.12 Silica Fume The silica fume material for use in Portland Cement Concrete shall conform to the chemical and physical requirements of AASHTO M 307 and be one of the products listed on the Maine Department of Transportation's Qualified Products List.

The Contractor shall arrange for the silica fume supplier to provide a Manufacturer's Certification (Mill Test Report) for all silica fume materials furnished for use on the project. Reports shall include all mandatory and optional requirements specified in AASHTO M 307. Reports shall represent the silica fume incorporated into the project and reports shall be supplied, at a minimum, on a monthly basis. Reports shall be submitted to the Department's Materials Testing Engineer.

701.13 Ground Granulated Blast Furnace Slag Ground granulated blast furnace slag shall conform to the chemical and physical requirements for Grade 100 or 120 as listed in AASHTO M 302.

The Contractor shall arrange for the slag supplier to provide a Manufacturer's Certification (Mill Test Report) for all slag materials furnished for use on the project. Reports shall include all mandatory and optional requirements specified in AASHTO M 302 including any processing additions. Reports shall represent the slag incorporated into the project and reports shall be supplied, at a minimum, on a monthly basis. Reports shall be submitted to the Department's Materials Testing Engineer.

SECTION 702 - BITUMINOUS MATERIAL

702.01 Asphalt Cement Performance Graded Asphalt Binder shall conform to the requirements of AASHTO M 320 or AASHTO M 332, whichever is indicated in the contract documents.

For Performance-Graded Asphalt Binder (PGAB), the Contractor shall arrange for the Supplier to furnish the following items to the Department's Materials Testing Engineer.

- a. A Quality-Control Plan for PGAB that conforms to the requirements of AASHTO R 26 “Certifying Suppliers of Performance-Graded Asphalt Binders” and
- b. A CERTIFICATE OF ANALYSIS for all asphalt materials furnished for use on the project. The original of each Certificate of Analysis shall be mailed to the Departments Materials Testing Engineer.

702.04 Emulsified Asphalt Emulsified Asphalt shall conform to the requirements of AASHTO M 140. Cationic emulsified asphalt shall conform to the requirements of AASHTO M 208.

Use of all emulsified asphalt shall comply with all Department of Environmental Protection (DEP) regulations regarding maximum amount of oil distillate, seasonal limitations, etc.

For emulsified asphalts, the Contractor shall arrange for the Supplier to furnish the following item to the Department’s Materials Testing Engineer.

A CERTIFICATE OF ANALYSIS for all asphalt emulsion materials furnished for use on the project. The Certificate shall include the actual test results of the material in storage from which the shipments are being made. Certificates shall be supplied for each lot or batch for each grade/type of emulsion. A new certificate shall be issued at least every 30 days or upon receiving or manufacture of a new material. The original of each Certificate of Analysis shall be mailed to the Department’s Materials Testing Engineer.

Deliveries of emulsion materials shall be accompanied by a loading invoice, delivery ticket, or slip, as required under Section 108.1.3 f. The Loading Invoice shall include the applicable certificate number and shall include a printed or stamped statement such as the following:

“THIS IS TO CERTIFY THAT THE ASPHALT MATERIAL REPRESENTED BY THIS LOADING INVOICE CONFORMS TO THE SPECIFICATIONS OF THE PURCHASER FOR THE MATERIAL TYPE AND GRADE STATED THEREON.”

In the event an intermediate hauler of the asphalt material is involved, a copy of their own delivery slip shall be furnished, as well as a copy of the supplier's loading invoice. The hauler's delivery slip and the supplier's loading invoice shall be cross-referenced by use of their respective serial numbers.

702.05 Temperature Application Range, °F

Type and Grade	Spray	Mix
RS-1	50 to 125	50 to 125
RC 70	80 to 150	80 to 150
RC 250	82 to 175	80 to 150
RC 800	160 to 225	135 to 185
RC 3000	200 to 275	185 to 225

MC 30	50 to 120	50 to 120
MC 70	80 to 150	80 to 150
MC 250	100 to 200	100 to 210
MC 800	185 to 260	200 to 250
MC 3000	230 to 275	200 to 250
All emulsions	50 to 160	50 to 160
Performance-Graded Asphalt Binder (all grades).	As required to achieve a viscosity of 0.15 to 0.31 PA-s.	

702.06 Temperature - Volume Correction Tables All asphalt material shall be corrected for volume by use of the following multipliers to reduce the volume at the observed temperature to the volume at 60°F.

Emulsified Asphalts	Asphalts (ASTM D1250)									
	All Types And Grades	Tem p. °F	Sp. Gr. 0.966 +	Sp. Gr. 0.850	Tem p. °F	Sp. Gr. 0.966 +	Sp. Gr. 0.850	Tem p. °F	Sp. Gr. 0.966 +	Sp. Gr. 0.850
	1	50	1.0035	1.0040	150	.9689	.9647	250	.9352	.9268
	1	55	1.0017	1.0020	155	.9672	.9628	255	.9336	.9249
	1.0000	60	1.0000	1.0000	160	.9655	.9609	260	.9319	.9231
	.9988	65	.9983	.9980	165	.9638	.9589	265	.9302	.9212
	.9975	70	.9965	.9960	170	.9621	.9570	270	.9286	.9194
	.9962	75	.9948	.9940	175	.9604	.9551	275	.9269	.9175
	.9950	80	.9930	.9921	180	.9587	.9532	280	.9253	.9157
	.9938	85	.9913	.9901	185	.9570	.9513	285	.9236	.9138
	.9925	90	.9896	.9881	190	.9553	.9494	290	.9220	.9120
	.9912	95	.9878	.9861	195	.9536	.9475	295	.9204	.9102
	.9900	100	.9861	.9842	200	.9520	.9456	300	.9187	.9083
	.9988	105	.9844	.9822	205	.9503	.9437	305	.9171	.9065
	.9875	110	.9826	.9803	210	.9486	.9418	310	.9154	.9047
	.9862	115	.9809	.9783	215	.9469	.9399	315	.9138	.9029
	.9850	120	.9792	.9763	220	.9452	.9380	320	.9122	.9010
	.9338	125	.9775	.9744	225	.9436	.9361	325	.9105	.8992
	.9825	130	.9758	.9725	230	.9419	.9343	330	.9089	.8974
	.9812	135	.9740	.9705	235	.9402	.9324	335	.9073	.8956
	.9800	140	.9723	.9686	240	.9385	.9305	340	.9057	.8938

.9788	145	.9706	.9666	245	.9369	.9286	345	.9040	.8920
.9775	150	.9689	.9647	250	.9352	.9268	350	.9024	.8902

702.09 Asphalt Filler for Structural Plate Arches Asphalt for filling spaces between the structural plates and the substructure metal connectors of the arch shall conform to the requirements for bituminous material of AASHTO M190 or of AASHTO M320, Table 1, for PG 64-28 or PG 58 -28.

702.12 Emulsified Asphalt Sealing Compound Emulsified asphalt sealing compound shall be an approved commercially prepared product manufactured for specific protective coating, colored as required. It shall contain fillers, pigments and sand or fibrous materials suspended in a suitable emulsified asphalt or tar. It shall be of such consistency that it can be applied at atmospheric temperatures and capable of being easily diluted with the addition of water and mixed by hand stirring at the site of application.

SECTION 703 - AGGREGATES

703.01 Fine Aggregate for Concrete Fine aggregate for concrete shall consist of natural sand or, when approved by the Resident, other inert materials with similar characteristics or combinations thereof, having strong, durable particles. Fine aggregate from different sources of supply shall not be mixed or stored in the same pile nor used alternately in the same class of construction or mix without permission of the Resident.

All fine aggregate shall be free from injurious amounts of organic impurities. Should the fine aggregate, when subjected to the colorimetric test for organic impurities, AASHTO T 21, produce a color darker than the reference standard color solution (laboratory designation Plate III), the fine aggregate shall be rejected.

Fine aggregate shall have a sand equivalent value of not less than 75 when tested in accordance with AASHTO T 176.

Fine aggregate sources shall meet the Alkali Silica Reactivity (ASR) requirements of Section 703.0201.

The fineness modulus shall not be less than 2.26 or more than 3.14. If this value is exceeded, the fine aggregate will be rejected unless suitable adjustments are made in proportions of coarse and fine aggregate. The fineness modulus of fine aggregate shall be determined by adding the cumulative percentages of material by weight retained on the following sieves: Nos. 4, 8, 16, 30, 50, 100 and dividing by 100.

Fine aggregate, from an individual source when tested for absorption as specified in AASHTO T 84, shall show an absorption of not more than 2.3 percent.

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
3/8 inch	100
No. 4	95-100
No. 8	80-100
No. 16	50-85
No. 30	25-60
No. 50	10-30
No. 100	2-10
No. 200	0-5.0

703.02 Coarse Aggregate for Concrete Coarse aggregate for concrete shall consist of crushed stone or gravel having hard, strong, durable pieces, free from adherent coatings and of which the composite blend retained on the 3/8 inch sieve shall contain no more than 15 percent, by weight of flat and elongated particles when performed in accordance with test method ASTM D 4791, Flat Particles, Elongated Particles, or Flat and Elongated Particles in Coarse Aggregate, using a dimensional ratio of 1:5.

The coarse aggregate from an individual source shall have an absorption no greater than 2.0 percent by weight determined in accordance with AASHTO T 85 modified for weight of sample.

The composite blend shall have a Micro-Deval value of 18.0 percent or less as determined by AASHTO T 327 or not exceed 40 percent loss as determined by AASHTO T 96.

Coarse aggregate sources shall meet the Alkali Silica Reactivity (ASR) requirements of Section 703.0201.

Coarse aggregate shall conform to the requirements of the following table for the size or sizes designated and shall be well graded between the limits specified.

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves			
	A	AA	S	LATEX
Grading				
Aggregate Size	1 inch	3/4 inch	1 1/2 inch	1/2 inch
2 inch			100	
1 1/2 inch	100		95-100	
1 inch	95-100	100	-	
3/4 inch	-	90-100	35-70	100
1/2 inch	25-60	-	-	90-100
3/8 inch	-	20-55	10-30	40-70
No. 4	0-10	0-10	0-5	0-15
No. 8	0-5	0-5	-	0-5

No. 16	-	-	-	-
No. 50	-	-	-	-
No. 200	0 - 1.5	0 - 1.5	0 - 1.5	0 - 1.5

703.0201 Alkali Silica Reactive Aggregates. All coarse and fine aggregates proposed for use in concrete shall be tested for Alkali Silica Reactivity (ASR) potential under AASHTO T 303 (ASTM C 1260), Accelerated Detection of Potentially Deleterious Expansion of Mortar Bars Due to Alkali-Silica Reaction, prior to being accepted for use. Acceptance will be based on testing performed by the Department. Sampling will be performed by the Department from stockpiles located at the Contractor's/supplier's ready mixed concrete plants. Aggregate approvals will be performed on a 3-year cycle, unless the source or character of the aggregate in question has changed within 3 years from the last test date.

A list of pre-approved coarse aggregate and aggregate-cement/pozzolan blends is maintained by the Department and will determine the acceptability of concrete mix designs proposed for use.

As per AASHTO T 303 (ASTM C 1260): Use of a particular coarse or fine aggregate will be allowed with no restrictions when the mortar bars made with this aggregate expand less than or equal to 0.10 percent at 30 days from casting. Use of a particular coarse or fine aggregate will be classified as potentially reactive when the mortar bars made with this aggregate expand greater than 0.10 percent at 30 days from casting. Use of this aggregate will only be allowed with the use of cement-pozzolan blends and/or chemical admixtures that result in mortar bar expansion of less than 0.10 percent at 30 days from casting.

Acceptable pozzolans and chemical admixtures that may be used when an aggregate is classified as potentially reactive include the following:

- a. Class F Coal Fly Ash meeting the requirements of AASHTO M 295.
- b. Ground Granulated Blast Furnace Slag (Grade 100 or 120) meeting the requirements of AASHTO M 302.
- c. Densified Silica Fume meeting the requirements of AASHTO M 307.
- d. Lithium Hydroxide Monohydrate (LiOH-H₂O).

Pozzolans or chemical admixtures required to offset the effects of potentially reactive aggregates will be incorporated into the concrete at no additional cost to the Department.

Aggregates classified as potentially reactive by the requirements of this specification may be used if certified test results from an accredited independent laboratory utilizing the current AASHTO T 303 (ASTM C 1260) Accelerated Detection of Potentially Deleterious Expansion of Mortar Bars Due to Alkali-Silica Reaction, indicating an acceptable alkali-aggregate combination, are submitted to the Department.

703.05 Aggregate for Sand Leveling Aggregate for sand leveling shall be sand of hard durable particles free from vegetable matter, lumps or balls of clay and other deleterious substances. The aggregate shall meet the grading requirements of the following table.

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
3/8 inch	85-100
No. 200	0-5.0

703.06 Aggregate for Base and Subbase The following shall apply to Sections (a.) and (c.) below. The material shall have a Micro-Deval...” and replace with “The material shall have a minimum degradation value of 25.0 or less15 as determined by AASHTO T 327. If the Micro-Deval value exceeds 25.0, the Washington State Degradation DOT Test Method T 113T113, Method of Test for Determination of Degradation Value (January 2009 version) shall be performed), except that the test shall be performed on the reported degradation value will be the result of testing a single specimen from that portion of the sample that passes the 1/2 in sieve and is retained on the No. 10 sieve. If the material has a Washington Degradation value of less than 15, the material shall be rejected.

The material, minus any reclaimed asphalt pavement used in Section (b.) below shall have a Micro-Deval value of 25.0 or less as determined by AASHTO T 327. If the Micro-Deval value exceeds 25.0 the material may be used if it does not exceed 25 percent loss on AASHTO T 96, Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine. If AASHTO T 96 is used for acceptance of the material, the material shall be retested at intervals of 25%, 50% and 75% completion of the course.

Recycled Asphalt Pavement (RAP) shall not be used for or blended with aggregate base or subbase.

- a. Aggregate for base, Type A and B shall be crushed ledge or crushed gravel of hard durable particles free from vegetable matter, lumps or balls of clay and other deleterious substances. The gradation of the part that passes a 3 inch sieve shall meet the grading requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves	
	Type A	Type B
1/2 inch	45-70	35-75
1/4 inch	30-55	25-60
No. 40	0-20	0-25
No. 200	0-6.0	0-6.0

At least 50 percent by weight of the material retained on the No. 4 sieve shall have at least one fractured face as tested by AASHTO T 335.

Type A aggregate for base shall only contain particles of rock that will pass the 2 inch square mesh sieve.

Type B aggregate for base shall only contain particles of rock that will pass the 4 inch square mesh sieve.

b. Aggregate for base, Type C shall be crushed ledge or crushed gravel of hard durable particles free from vegetable matter, lumps or balls of clay and other deleterious substances. The material shall meet the grading requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves	
	Type C	
4 inches	100	
3 inches	90-100	
2 inches	75-100	
1 inch	50-80	
½ inch	30-60	
No. 4	15-40	
No. 200	0-6.0	

At least 50 percent by weight of the material coarser than the No. 4 sieve shall have at least one fractured face as tested by AASHTO T 335.

c. Aggregate for subbase shall be sand or gravel of hard durable particles free from vegetable matter, lumps or balls of clay and other deleterious substances. The gradation of the part that passes a 3 inch sieve shall meet the grading requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves	
	Type D	Type E
½ in	35-80	
¼ inch	25-65	25-100
No. 40	0-30	0-50
No. 200	0-7.0	0-7.0

Type D aggregate for subbase gravel may contain up to 50 percent by weight Recycled Concrete Aggregate (RCA). When RCA is used, the portion of the resulting blend of gravel and RCA

retained on a ½” square mesh sieve shall contain a total of no more than 5 percent by weight of other recycled materials such as brick, concrete masonry block, or asphalt pavement as determined by visual inspection.

RCA shall be substantially free of wood, metal, plaster, and gypsum board as defined in Note 9 in Section 7.4 of AASHTO M 319. RCA shall also be free of all substances that fall under the category of solid waste or hazardous materials.

Aggregate for subbase shall not contain particles of rock which will not pass the 6 inch square mesh sieve.

703.07 Aggregates for HMA Pavements Coarse aggregate and fine aggregate for hot mix asphalt pavements shall be of such gradation that when combined in the proper proportions, including filler, if required, the resultant blend will meet the composition of mixture for the type of pavement specified.

Coarse aggregate, that material retained on the No. 4 sieve, shall be crushed stone or crushed gravel and, unless otherwise stipulated, shall consist of clean, tough, durable fragments free from an excess of soft or disintegrated pieces and free from stone coated with dirt or other objectionable matter. Coarse aggregate, shall not exceed an absorption of 2.0 percent by weight as determined by AASHTO T 85.

Fine aggregate, material that passes the No. 4 sieve, shall consist of natural sand, manufactured sand, or a combination of these. It shall consist of hard, tough grains, free from injurious amounts of clay, loam, or other deleterious substances. Fine aggregate, shall not exceed an absorption of 2.3 percent by weight as determined by AASHTO T 84.

The composite blend, minus any reclaimed asphalt pavement used (RAP), shall have a Micro-Deval value of 18.0 percent or less as determined by AASHTO T 327. In the event the material exceeds the Micro-Deval limit, a Washington Degradation test shall be performed. The material shall be acceptable if it has a value of 30 or more as determined by Washington State DOT Test Method T 113, Method of Test for Determination of Degradation Value (January 2009 version) except that the reported degradation value will be the result of testing a single composite specimen from that portion of the sample that passes the ½ inch sieve and is retained on the No. 10 sieve, minus any reclaimed asphalt pavement used.

Aggregates shall also meet the following consensus properties, except that aggregates extracted from RAP will not be included in the sand equivalent test. The Department reserves the right to sample and test the composite aggregate for any of the following properties at any time:

TABLE 3: Aggregate Consensus Properties Criteria

Estimated Traffic, Million 18 kip ESALs	AASHTO T 335 Coarse Aggregate Angularity (minimum)	AASHTO T 304 Method A Uncompacted Void Content of Fine Aggregate (min)	ASTM D 4791 (8.4) Flat and Elongated Particles (maximum)	AASHTO T 176 Clay Content/ Sand Equivalent (minimum)
< 0.3	60/60	40	10	45
0.3 to < 3.0	75/60			
3.0 to < 10	85/80			
10 to < 30	95/90	45		
≥ 30	100/100			50

ASTM D 5821 - 85/80 denotes that 85 percent of the coarse aggregate has one fractured face and 80 percent has two fractured faces.

AASHTO T 304 - Criteria are presented as percent air voids in loosely compacted fine aggregate, (U).

ASTM D 4791 - Criteria are presented as maximum percent by weight of flat and elongated particles (5:1 ratio).

The entire HMA wearing course shall come from the same source of material and the same job mix formula, except when permission is obtained from the Resident to change sources.

703.08 Recycled Asphalt Pavement Recycled asphalt pavement shall consist of salvaged asphalt materials from milled pavements or production waste that has been processed before use to meet the requirements of the job mix formula. It shall be free of winter sand, granular fill, construction debris, or other materials not generally considered asphalt pavement.

703.081 RAP for Asphalt Pavement Recycled Asphalt Pavement (RAP) may be introduced into hot-mix asphalt pavement at percentages approved by the Department according to the MaineDOT Policies and Procedures for HMA Sampling and Testing. If approved by the Department, the Contractor shall provide documentation stating the source, test results for average residual asphalt content, and stockpile gradations showing RAP materials have been sized to meet the maximum aggregate size requirements of each mix designation. The Department will obtain samples for verification and approval prior to its use.

The maximum allowable percent of RAP shall be determined by the asphalt content, the percent passing the 0.075 mm sieve, and Coarse Micro-Deval loss values as tested by the Department. The numerical average of the percent passing the 0.075 mm sieve values will be used for the approval. The maximum percentage of RAP allowable shall be the lowest percentage as determined according to Table 4 below:

TABLE 4: Maximum Percent RAP According to Test Results

Classification	Maximum RAP Percentage Allowed	Asphalt content standard deviation	Percent passing 0.075 mm sieve	Residual aggregate M-D loss value
Class III	10%	N/A	> 10.0	≤ 18
Class II	20%	≤ 0.5	≤ 10.0	
Class I	30%	≤ 0.3	≤ 8.0	

The Department will monitor RAP asphalt content and gradation during production by testing samples from the stockpile at approximately 15,000 T intervals (in terms of mix production). The allowable variance limits (from the numerical average values used for mix designs) for this testing are determined based upon the maximum allowable RAP percentage, and are shown below in Table 5.

TABLE 5: RAP Verification Limits

Classification	Maximum RAP Percentage Allowed	Asphalt content (compared to aim)	Percent passing 0.075 mm sieve (compared to aim)	Percent passing 0.075 mm sieve
Class III	10%	± 1.5	± 2.0	N/A
Class II	20%	± 1.0	± 1.5	≤ 10.0
Class I	30%	± 0.5	± 1.0	≤ 8.0

For specification purposes, RAP will be categorized as follows:

Class III – A maximum of 10.0 percent of Class III RAP may be used in any base, intermediate base, surface, or shim mixture. A maximum of 20.0 percent of Class III RAP may be used in hand-placed mixes for item 403.209.

Class II – A maximum of 20.0 percent Class II RAP in any base, binder, surface, or shim course.

Class I – A maximum of 20.0 percent Class I RAP may be used in any base, intermediate base, surface, or shim mixture without requiring a change to the specified asphalt binder. A maximum of 30.0 percent Class I RAP may be used in in any base or intermediate base mixture provided that a PG 58-28 asphalt binder is used. A maximum of 30.0 percent Class I RAP may be used in any surface or shim mixture provided that PG 58-34 or 52-34 asphalt binder is used. Mixtures exceeding 20.0 percent Class I RAP must be evaluated and approved by the Department.

The Contractor may use up to two different RAP sources in any one mix design. The total RAP percentage of the mix shall not exceed the maximum allowed for the highest classification RAP source used (i.e. if a Class I & Class III used, total RAP must not exceed 30.0%). The blended RAP material must meet all the requirements of the classification for which the RAP is entered (i.e. 10% Class III with 20% Class I, blend must meet Class I criteria). The Department may take belt cuts of the blended RAP to verify the material meets these requirements. If the Contractor elects to use more than one RAP source in a design, the Contractor shall provide an acceptable point of sampling blended RAP material from the feed belt.

In the event that RAP source or properties change, the Contractor shall notify the Department of the change and submit new documentation stating the new source or properties a minimum of 72 hours prior to the change to allow for obtaining new samples and approval.

703.09 HMA Mixture Composition The coarse and fine aggregate shall meet the requirements of Section 703.07. The several aggregate fractions for mixtures shall be sized, graded, and combined in such proportions that the resulting composite blends, including RAP aggregate will meet the grading requirements of the following table:

Aggregate Gradation Control Points

Sieve Designation	Nominal Maximum Aggregate Size---Control Points (Percent Passing)					
	Type 25 mm	Type 19 mm	Type 12.5 mm	Type 9.5 mm	Type 9.5 mm Thin Lift Mixture (TLM)	Type 4.75 mm
Percent By Weight Passing - Combined Aggregate						
37.5 mm	100					
25 mm	90-100	100				
19 mm	-90	90-100	100			
12.5 mm		-90	90-100	100	100	100
9.5 mm		-	-90	90-100	95-100	95-100
4.75 mm		-	-	-90	60-95	80-100
2.36 mm	19-45	23-49	28-58	32-67	40-65	40 - 80
1.18 mm		-	-	-	-	-
600 µm		-	-	-	-	-
300 µm		-	-	-	-	-
75 µm	2.0-6.0	2.0-6.0	2.0-6.0	2.0-7.0*	2.0-7.0*	2.0-7.0

* For 9.5mm nominal maximum aggregate size mixtures, the maximum design aim for the percent passing the 75 µm sieve is 6.5%.

Gradation Classification---- The combined aggregate gradation shall be classified as coarse-graded when it passes below the Primary Control Sieve (PCS) control point as defined in the following table. All other gradations shall be classified as fine-graded.

Gradation Classification

PCS Control Point for Mixture Nominal Maximum Aggregate Size (percent passing)				
Nominal Maximum Aggregate Size	Type 25 mm	Type 19 mm	Type 12.5 mm	Type 9.5 mm
Primary Control Sieve	4.75 mm	4.75 mm	2.36 mm	2.36 mm
PCS Control Point (percent passing)	40	47	39	47

If a Grading “D” mixture is allowed per Special Provision Section 403, it shall meet the following gradation and the aggregate requirements of Section 703.07.

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
½ inch	100
¾ inch	93-100
No. 4	60-80
No. 8	46-65
No. 16	25-55
No. 30	16-40
No. 50	10-30
No. 100	6-22
No. 200	3.0-8.0

703.10 Aggregate for Untreated Surface Course and Leveling Course Aggregate for untreated surface course and leveling course shall be screened or crushed gravel consisting of hard durable particles which are free from vegetable matter, lumps or balls of clay and other deleterious substances. If this item is to be used underneath pavement, it must have a Micro-Deval value of 20.0 or less as determined by AASHTO T 327. If the Micro-Deval value exceeds 20.0, the material may be used if it does not exceed 25 percent loss on AASHTO T 96, Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine. The aggregate shall meet the grading requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves	
	Untreated Aggregate Surface Course	
	Type A	Type B
1 inch	95-100	95-100
¾ inch	90-100	90-100
No. 4	40-65	10-45
No. 10	10-45	10-35

No. 200	0-7.0	0-6.0
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703.11 Aggregate for Shoulder The gradation of that portion passing a 3 inch sieve shall meet the gradation requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves		
	*Paved or Unpaved Lifts 4 inches or Greater	Paved Lifts < 4 inches	Unpaved Lifts < 4 inches
1 inch	-	90-100	90-100
¼ inch	25-70	25-100	25-70
No. 40	5-30	5-50	5-30
No. 200	2.0-10.0	2.0-10.0	2.0-10.0

*Shall not contain particles larger than 6 inches or the thickness of the lift being placed, whichever is less.

703.12 Aggregate for Crushed Stone Surface Crushed stone surface shall be of quarried stone and have a Micro-Deval value of 18.0 percent or less as determined by AASHTO T 327. The aggregate shall meet the grading requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
1 inch	100
¾ inch	60-90
½ inch	10-35
⅜ inch	2-15
No. 4	0-5

703.13 Crushed Stone ¾-Inch Aggregate for Crushed Stone ¾-inch shall be of quarried stone and have a Micro-Deval value of 18.0 percent or less as determined by AASHTO T 327. The aggregate shall meet the grading requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
1 inch	100
¾ inch	90-100
½ inch	20-55
⅜ inch	0-15
No. 4	0-5

703.15 Filler These materials shall conform to the following specification requirements for the designated materials.

Mineral filler shall conform to the requirements of AASHTO M 17.

703.18 Common Borrow Common borrow shall consist of earth, suitable for embankment construction. It shall be free from frozen material, perishable rubbish, peat, and other unsuitable material including material currently or previously contaminated by chemical, radiological, or biological agents unless the material is from a DOT project and authorized by DEP for use.

The moisture content shall be sufficient to provide the required compaction and stable embankment. In no case shall the moisture content exceed 4 percent above optimum, which shall be determined in accordance with AASHTO T 180, Method C or D.

703.19 Granular Borrow Granular borrow shall consist of sand or gravel of hard durable particles free from vegetable matter, lumps or balls of clay, frozen material and other deleterious substances. The gradation of that portion passing a 3 inch sieve shall meet the gradation requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves	
	Material for Underwater Backfill	Material for Embankment Construction
3 inch	100	
No. 40	0-70	0-70
No. 200	0-7.0	0-20.0

Granular borrow shall contain no particles or fragments with a maximum dimension in excess of the compacted thickness of the layer being placed.

703.20 Gravel Borrow Gravel borrow shall consist of well graded granular material having no rocks with a maximum dimension of over 6 inches. The gradation of that portion passing a 3 inch sieve shall meet the gradation requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
¼ inch	0-70
No. 200	0-10.0

703.21 Rock Borrow Rock borrow shall consist of hard durable rock broken to various sizes that will form a compact embankment with a minimum of voids. The maximum size for any rock shall be 3 feet in its greatest dimension.

703.22 Underdrain Backfill Material. Material for underdrain shall be free from organic matter, frozen material and shall conform to the following tables:

Type B material shall conform to the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
1 inch	95-100
½ inch	75-100
No. 4	50-100
No. 20	15-80
No. 50	0-15
No. 200	0-5.0

Type C material shall conform to the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
1 inch	100
¾ inch	90-100
⅜ inch	0-75
No. 4	0-25
No. 10	0-5

703.24 Stone for French Drains Stones for French drains shall consist of hard, durable rock and shall conform to the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
6 inch	90-100
1½ inch	0-40
No. 4	0-5

703.25 Stone Fill Stones for stone fill shall consist of hard, sound, durable rock that will not disintegrate by exposure to water or weather. Stone for stone fill shall be angular and rough. Rounded, subrounded, or long thin stones will not be allowed. Stone for stone fill may be obtained from quarries or by screening oversized rock from earth borrow pits. The maximum allowable length to thickness ratio will be 3:1. The minimum stone size (10 lbs) shall have an average dimension of 5 inches. The maximum stone size (500 lbs) shall have a maximum dimension of approximately 36 inches. Larger stones may be used if approved by the Resident.

Fifty percent of the stones by volume shall have an average dimension greater than 12 inches (200 lbs).

703.26 Plain and Hand Laid Riprap Stone for riprap shall consist of hard, sound durable rock that will not disintegrate by exposure to water or weather. Stone for riprap shall be angular and rough. Rounded, subrounded or long thin stones will not be allowed. The maximum allowable length to width ratio will be 3:1. Stone for riprap may be obtained from quarries or by screening oversized rock from earth borrow pits. The minimum stone size (10 lbs) shall have an average dimension of 5 inches. The maximum stone size (200 lbs) shall have an average dimension of approximately 12 inches. Larger stones may be used if approved by the Resident. Fifty percent of the stones by volume shall have an average dimension greater than 9 inches (50 lbs).

703.27 Stone Blanket Stones for stone blanket shall consist of sound durable rock that will not disintegrate by exposure to water or weather. Stone for stone blanket shall be angular and rough. Rounded or subrounded stones will not be allowed. Stones may be obtained from quarries or by screening oversized rock from earth borrow pits. The minimum stone size (300 lbs) shall have minimum dimension of 14 inches, and the maximum stone size (3000 lbs) shall have a maximum dimension of approximately 66 inches. Fifty percent of the stones by volume shall have average dimension greater than 24 inches (1000 lbs).

703.28 Heavy Riprap Stone for heavy riprap shall consist of hard, sound, durable rock that will not disintegrate by exposure to water or weather. Stone for heavy riprap shall be angular and rough. Rounded, subrounded, or thin, flat stones will not be allowed. The maximum allowable length to width ratio will be 3:1. Stone for heavy riprap may be obtained from quarries or by screening oversized rock from earth borrow pits. The minimum stone size (500 lbs) shall have minimum dimension of 15 inches, and at least fifty percent of the stones by volume shall have an average dimension greater than 24 inches (1000 lbs)

703.29 Stone Ditch Protection Rock used for ditch protection shall consist of sound, durable rock that will not disintegrate by exposure to water or weather. Fieldstone, rough quarry stone, blasted ledge rock or tailings may be used. The rock shall be graded within the following limits or as otherwise approved

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
12 inch	90-100
4 inch	0-15

The size of any stone shall not exceed 18 inches when measured along its longest axis.

703.31 Crushed Stone 2-Inch Crushed stone shall be obtained from rock of uniform quality and shall consist of clean, angular fragments of quarried rock, free from soft disintegrated pieces or other objectionable matter.

The stone, which shall be similar to railroad ballast, shall meet the following gradation requirements:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
2½ inch	100
2 inch	95-100
1 inch	0-30
¾ inch	0-5

703.32 Definitions (ASTM D 2488, Table 1).

Angular: Particles have sharp edges and relatively plane sides with unpolished surfaces

Subrounded: Particles have nearly plane sides but have well-rounded corners and edges

Rounded: Particles have smoothly curved sides and no edges

703.33 Stone Ballast Aggregate for stone ballast shall be clean and graded crushed stone aggregate with a hard, dense angular particle structure providing sharp corners and cubicle fragments with prime consideration for drainage efficiency.

The material retained on the ¾ inch sieve shall contain not more than 5 percent, by weight of flat and elongated particles when performed in accordance with test method ASTM D 4791, Flat Particles, Elongated Particles, or Flat and Elongated Particles in Coarse Aggregate, using a dimensional ratio of 1:5.

The material shall have an absorption no greater than 1.5 percent by weight and a bulk specific gravity of less than 2.60 as determined in accordance with AASHTO T 85 modified for weight of sample.

The material shall not exceed 30 percent loss on ASTM C535, Resistance to Degradation of Large-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine.

The material shall meet the grading requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves	
	Type 4	Type 4A
2 ½ inch		100
2 inch	100	90-100
1 ½ inch	90-100	60-90

1 inch	20-55	10-35
¾ inch	0-15	0-10
3/8 inch	0-5	0-3
No. 200	1.0 max.	1.0 max.

SECTION 704 - MASONRY UNITS

704.01 Clay or Shale Brick Except as modified below, brick shall conform to the requirements of one of the following specifications:

Type of Brick	Specification
Sewer, Catch Basin and Manhole	AASHTO M 91, Grade MS or SM
Building	AASHTO M 114, Grade SW

704.02 Brick for Paving Brick for paving shall conform to the requirements of AASHTO M 114 (C62, Grade SW) for Building brick or shale, with the following modifications:

- a. The absorption limits shall be from 5 percent to 12 percent for the average of 5 bricks.
- b. The compressive strength shall not be less than 6,000 psi.
- c. The modulus of rupture shall not be less than 1,000 psi.
- d. The bricks shall be No. 1, water struck type for paving.

The bricks shall be 2¼ inches by 3¾ inches by 8 inches with permissible variations not to exceed 1/16 inch in depth, 1/8 inch in width and 1/4 inch in length.

Before ordering new brick, samples shall be submitted in whole straps to show color range.

704.03 Concrete Masonry Blocks Concrete masonry blocks may be rectangular or segmented and when specified shall have ends shaped to provide interlock at vertical joints.

Solid blocks for catch basins and manholes shall conform to the requirements of ASTM C 139.

SECTION 705 - JOINT MATERIAL

705.01 Preformed Expansion Joint Filler Preformed expansion joint filler shall be non-extruding and resilient bituminous type and shall conform to the requirements of AASHTO M 213.

705.02 Joint Mortar Joint mortar shall consist of 1 part Portland cement, 2 parts sand and sufficient water to obtain the required consistency. Mortar shall be used within 30 minutes after its preparation.

The cement shall conform to the requirements of Portland cement AASHTO M 85, Type II or IIA.

The sand shall meet the requirements of the following table:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves	
	Joints Thicker Than ½ inch	Joints ½ inch or Thinner
¾ inch	100	-
¼ inch	-	100
No. 4	95-100	-
No. 8	70-95	85-100
No. 16	45-80	60-90
No. 30	25-55	35-70
No. 50	10-30	15-45
No. 100	2-10	0-15
No. 200	0-5.0	0-5.0

When necessary, material retained on the No. 4 sieve may be removed.

The sand shall be subjected to the colorimetric test for organic impurities, AASHTO T 21, and when a color darker than the reference standard color solution (laboratory designation Plate III) is produced, the material shall be rejected.

705.03 Flexible Watertight Gaskets Flexible gaskets, either rubber or plastic, shall conform to AASHTO M 198.

SECTION 706 - NON-METALLIC PIPE

706.02 Reinforced Concrete Pipe This pipe shall conform to the requirements of AASHTO M 170M/M 170. Elliptical pipe shall conform to the requirements of AASHTO M 207M/M 207. Unless otherwise specified, pipe wall design and use of elliptical reinforcement in circular pipe are optional. Pipe arch shall conform to the requirements of AASHTO M 206M/M 206.

Aggregates shall meet the requirements of Section 703.01 and Section 703.02 for fine aggregates and coarse aggregates respectively, except that grading requirements are hereby waived.

Fine and coarse aggregate sources shall meet the Alkali Silica Reactivity (ASR) requirements of Section 703.0201.

Precast reinforced concrete special sections shall conform to the requirements of the cited specifications to the extent to which they apply.

706.06 Corrugated Polyethylene Pipe for Underdrain, Option I, and Option III Culvert Pipe Six inch diameter underdrain pipe and fittings shall conform to the requirements of AASHTO M 252, slot-perforated. Twelve to sixty inch diameter, Option I/III shall conform to the requirements of AASHTO M 294 Type S. Pipe to be used for Underdrain Type C shall meet the requirements of AASHTO M 294, Type SP and have Class I perforations. All polyethylene pipe shall be smooth lined and shall meet the pipe stiffness requirements shown on the Standard Details. The manufacturing plants of polyethylene pipe shall participate annually in the National Transportation Product Evaluation Program (NTPEP) process for plastic pipe and resins which includes audits by the AASHTO Materials Reference Laboratory (AMRL). Plants shall be listed as “Compliant” on the NTPEP website and take immediate corrective action for any deficiencies found during audits. Continued compliance with all elements of the NTPEP/AMRL program and the Maine DOT manufactured Materials Verification Program will be required in order to continue supplying product to the Maine DOT.

706.08 PVC (Polyvinylchloride) Pipe This pipe and fittings shall conform to the requirements of AASHTO M 278. All pipe shall be supplied with gasket type joints.

706.09 PVC (Polyvinylchloride) Perforated Pipe This pipe and fittings shall conform to the requirements of AASHTO M 278 or ASTM F 949 for 6 inch underdrain and AASHTO M 278 for underdrain larger than 6 inch diameter.

706.10 Corrugated Polypropylene Pipe for Option I and Option III Culvert Pipe Option I/III pipe and fittings shall conform to the requirements of AASHTO M 330 Type S (dual wall) or Type D (triple wall). All polypropylene pipe shall be smooth lined and shall meet the pipe stiffness requirements of AASHTO M 330. The manufacturing plants of polypropylene pipe shall participate annually in the National Transportation Product Evaluation Program (NTPEP) process for plastic pipe and resins which includes audits by the AASHTO Materials Reference Laboratory (AMRL). Plants shall be listed as “Compliant” on the NTPEP website and take immediate corrective action for any deficiencies found during audits. Continued compliance with all elements of the NTPEP/AMRL program and the Maine DOT manufactured Materials Verification Program will be required in order to continue supplying product to the Maine DOT.

SECTION 707 - METALLIC PIPE

707.02 Corrugated Steel, Metallic Coated Pipe This pipe and special fittings such as elbows, tees, and wyes shall conform to the requirements of AASHTO M 36/M 36M Type I, IR, II, or III for the specified sectional dimensions, sheet thickness, and coating.

707.05 Corrugated Steel, Metallic Coated Pipe for Underdrain This pipe and special fittings such as elbows, tees, and wyes shall conform to the requirements of AASHTO M 36/M 36M, Type III, Class 1 perforations.

707.06 Corrugated Aluminum Alloy Pipe and Pipe Arches This pipe and special fittings such as elbows, tees, and wyes shall conform to the requirements of AASHTO M 196/M 196M,

Type I, IR, or II. Special sections, such as elbows and metal end sections, shall be of the thickness called for on the plans and shall conform to the applicable requirements of AASHTO M 196/M 196M. Aluminum sheet shall conform to the requirements of AASHTO M 197/M 197M.

707.07 Polymer Precoated, Galvanized Corrugated Steel Pipe and Pipe Arches This pipe and special fittings such as elbows, tees and wyes shall conform to the requirements of AASHTO M 245/M 245M, Type 1 with Grade 250/250 polymer coating. Steel sheet shall conform to the requirements of AASHTO M 246/M 246M.

707.08 Corrugated Aluminum Alloy Pipe for Underdrain This pipe and special fittings such as elbows, tees, and wyes shall conform to the requirements of AASHTO M 196/M 196M, Type III, Class 1 perforations or Type IIIR, Class 4 perforations.

707.09 Steel Structural Plate Pipe, Pipe Arches, Arches, Box Culverts, and Fasteners Plates, bolts, nuts and other accessories shall conform to the requirements of AASHTO M 167/M 167M and the following additional requirements:

- a. All shop welding shall meet the requirements of the latest edition of AWS D1.1, Structural Welding Code - Steel.
- b. Annually, the fabricator shall have quality control tests performed on uncoated random samples of the lightest and heaviest thickness plates produced by welding. The sampling and testing shall be done by a recognized independent testing agency and copies of the test reports, including all welding parameters, shall be submitted to the Fabrication Engineer on an annual basis.
- c. The Fabrication Engineer reserves the right to conduct unannounced inspections of the fabricators facilities and to take random samples of welded plates representative of gages supplied to the Department.
- d. No field welding will be allowed.

707.10 Aluminum Coated (Type 2) Corrugated Steel Pipe This pipe shall conform to the requirements of AASHTO M 36/M 36M using steel sheet conforming to AASHTO M 274.

707.11 Zinc - Coated (Galvanized) Corrugated Steel Pipe This pipe shall conform to the requirements of AASHTO M 36/M 36M using steel sheet conforming to AASHTO M 218.

707.14 Aluminum Alloy Structural Plate Pipe, Pipe Arches, Arches, Box Culverts, and Fasteners Plates for this pipe shall conform to the requirements of AASHTO M 219/M 219M. Bolts and nuts shall conform to the requirements of ASTM F 468M alloy 6061-T6 and F 467 alloy 6061-T6.

SECTION 708 - PAINTS AND PRESERVATIVES

708.01 Exterior Ready Mixed Paint Exterior paint for wood structures should be a good quality house paint approved by the Resident.

708.03 Pavement Marking Paint Paint for final and temporary pavement marking shall meet the requirements of the Maine DOT Maintenance Fast-Dry Water-Based Traffic Paint on file at the Traffic Section in Augusta.

Glass beads shall conform to the requirements of AASHTO M 247, Type I.

708.04 Tree Wound Paint Tree wound paint shall be an approved waterproof, adhesive, and elastic paint, manufactured and customarily used for painting cuts on trees. It shall contain an antiseptic ingredient and be free from kerosene, creosote, coal tar, or any other injurious material.

708.05 Timber Preservative Timber preservatives shall conform to the requirements of AASHTO M 133.

SECTION 709 - REINFORCING STEEL AND WELDED STEEL WIRE FABRIC

709.01 Reinforcing Steel Reinforcing steel, whether plain, galvanized, epoxy coated, or zinc and epoxy dual-coated, shall be deformed bars conforming to the requirements of AASHTO M31 (ASTM A615). Bars shall be Grade 60 unless otherwise specified on the Plans. Epoxy coated reinforcing steel shall also conform to the requirements of AASHTO M284 (ASTM A775). The requirements of AASHTO M284 (ASTM A775), Sections 5.2.1, 5.3, 5.4, and 15.1 shall be mandatory. Zinc and epoxy dual-coated reinforcing steel shall conform to the requirements of ASTM A1055.

Low-carbon, chromium, reinforcing steel shall be deformed bars conforming to the requirements of ASTM A1035. Bars shall be Grade 100 unless otherwise specified on the Plans.

Stainless Steel Reinforcement shall be deformed stainless steel bars conforming to the requirements of ASTM A955. Bars shall be at a minimum Grade 60, unless otherwise specified on the Plans. The chemical composition shall conform to one of the types listed in Table 2 of ASTM A955, unless otherwise specified on the Plans. Heat treatment condition per ASTM A955 is hot rolled. Bars shall be finished in accordance with ASTM A955, section 15.

Fabricated deformed steel reinforcing grids shall conform to the requirements of AASHTO M54 (ASTM A184). Welding and welder qualifications for reinforcing bar grids shall conform to the requirements in Section 503.

709.02 Welded Steel Wire Fabric Welded steel wire fabric shall meet the requirements of AASHTO M 55 Steel Welded Wire Fabric, Plain, for Concrete Reinforcement or AASHTO M 221 Steel Welded Wire Fabric, Deformed, for Concrete Reinforcement as specified.

709.03 Steel Strand Prestressing strand shall meet the requirements of AASHTO M 203 Steel Strand, Uncoated Seven-Wire for Concrete Reinforcement.

Lateral post-tensioned strands for pre-cast/post-tensioned concrete products shall be 0.600 inch diameter AASHTO M 203 strand with a corrosion inhibitive coating covered with an extruded polypropylene sheath.

SECTION 710 - FENCE AND GUARDRAIL

710.01 Barbed Wire Barbed wire shall conform to the requirements of AASHTO M 280. The Design number shall be 12-4-5-14R with, coating Type Z, metallic coating Class 3 if zinc coated or coating Type A, (0.30 oz/ft²) if aluminum coated.

710.02 Woven Wire Wire and stays shall conform to the requirements of AASHTO M 279. The Design number shall be 1047-12-11, Grade 60, coating Type Z, coating Class 3 if zinc coated. The Design number shall be 1047-12-11, Grade 60, coating Type A, if aluminum coated.

710.03 Chain Link Fabric Chain Link fabric shall conform to the requirements of AASHTO M 181, Type I, Class D if zinc coated, or Type II if aluminum coated, or Type IV, Class B if PVC coated. The wire shall be 9 gauge and the mesh shall be 2 inch unless otherwise noted on the plans. Tension wire shall be seven gauge (0.177 inch diameter) steel coil spring wire and have a zinc coating or an aluminum coating. Zinc coating shall be Class 3 and aluminum coating shall be a minimum of 0.4 oz/ft².

710.04 Metal Beam Rail Galvanized steel rail elements shall conform to the requirements of AASHTO M 180, Class A, Type II.

When corrosion resistant steel is specified, rail shall conform to AASHTO M 180, Class A, Type IV. Beams of corrosion resistant steel shall not be painted or galvanized. They shall be so handled and stored that the traffic face of these beams, used in a continuous run of guardrail, shall not show a distinctive color differential.

When metal beam rail is to be installed on a curve having a radius of curvature of 150 ft. or less, the beam sections shall be fabricated on an arc to the required radius and permanently stamped or embossed with the designated radius.

The Department may take one piece of guardrail, a backup plate, and end or buffer section from each 200 pieces in a lot, or from each lot if less than 200 pieces are included therein for determination of compliance with specification requirements. If one piece fails to conform to the requirements of this specification, two other pieces shall be tested. If either of these pieces fails to conform to the requirements of this specification, the lot of material represented by these samples shall be rejected. A lot shall be considered that quantity of material offered for inspection at one time that bears the same heat and coating identification.

710.05 Cedar Rail Fence Rails and posts for cedar rail fence shall be of cedar, reasonably straight, machine or hand peeled and branch stubs cut flush.

710.06 Fence Posts and Braces Wood posts shall be of cedar, white oak, or tamarack, of the diameter or section and length shown on the plans.

Round wood posts shall be of seasoned stock straight and sound and shall have been cut from live growing trees with the outer and inner bark removed and all knots trimmed flush with the surface of the post. Rotted, loose or hollow knots, plugged or open holes will not be permitted. Sound knots will be permitted if the diameter of any one knot or the combined diameter of 2 or more knots occurring in the same cross section is not greater than one-third of the circumference of the post at that cross section. Posts shall be of uniform taper from top to bottom not to exceed 1½ inches of taper in 7 feet.

The minimum diameter of heartwood in line posts at their small end shall be no less than 3 inches. The minimum diameter of line posts at the small end shall be no less than 3 inches. The maximum diameter of line posts at the small end shall be 6 inches. The minimum diameter of heartwood in end, corner, gate, and barway posts shall be 4½ inches at the small end. The minimum diameter of end, corner, gate, and barway posts at the small end shall be 4½ inches. The maximum diameter of end, corner, gate, and barway posts shall be 8 inches.

That portion of wood posts to be set below ground shall be immersed to a depth of 4 feet in a tank or barrel containing an approved commercial timber preservative solution for a minimum of 60 minutes. All wood posts so treated, unless otherwise specified by the Resident, shall have been peeled and air seasoned to permit thorough drying for at least 3 months before treatment. After being treated, the posts shall be piled to permit thorough drying before being set in the ground.

Braces shall be of spruce, eastern hemlock, Norway pine, pitch pine, or tamarack timbers or spruce, cedar, or tamarack round posts of sufficient length to make a diagonal brace between adjacent posts. If other than cedar, white oak, or tamarack, braces shall be treated as described above for posts except that they shall be immersed full length.

Metal posts and braces for woven wire fence shall conform to the requirements of AASHTO M 281. Posts and post assemblies shall be galvanized according to the requirements of AASHTO M 111. Assembly hardware shall be zinc coated in accordance with AASHTO M 232M/M 232. The type of post furnished shall be channel or U unless otherwise noted on the plans.

Metal posts, rails, and bracing for chain link fence shall be as specified in AASHTO M 181. Type I, Grade 1 or Grade 2 posts shall be used with zinc coated chain link fabric. Type II, Grade 1 or 2 shall be used with aluminum coated chain link fabric. End and corner posts shall be 2 inch inside diameter. Line posts shall be 1 ½ inch inside diameter. Top and brace rails shall be 1 ¼ inch inside diameter. Expansion sleeves, turnbuckles and other fittings and hardware shall be galvanized in accordance with the applicable requirements in AASHTO M 181.

710.07 Guardrail Posts Posts shall be of wood or steel.

- a. Wood posts shall be of cedar, tamarack, white oak, Norway pine, southern yellow pine, pitch pine, Douglas fir, maple, beech, birch, red pine, white pine, eastern hemlock, or red oak. They shall be of well-seasoned, straight and sound timber cut from live growing trees, free from loose knots or other structurally weakening defects, including shake, holes and heart rot over 1 inch in diameter. The posts shall be free from season checks that exceed ¼ inch in width. A tolerance of 1 inch in length and ¼ inch in width or thickness is permitted in the dimensions of rectangular posts. They shall be well sawn and have square edges except that wane not more than 1½ inches wide and extending not more than ½ the length of the piece will be allowed on that portion of the post to be placed below ground. Sound, tight, well spaced knots to 2½ inches or less in diameter will be permitted.

Wood posts and offset brackets shall be preservative treated in accordance with the requirements of AASHTO M 133 using pentachlorophenol or chromate copper arsenate preservative. Treatment shall be according to the Standards of AWPA C5.

Composite material blocks tested and meeting the requirements of the National Cooperative Research Program Report 350 and approved by the FHWA may be used as offset brackets.

- b. Galvanized steel posts shall conform to the requirements of AASHTO M 270 Grade 250 (36) if a rolled section or ASTM A 769 Grade 36 if a welded section. Fabrication will be in accordance with Section 504 - Structural Steel. Galvanizing shall be in accordance with Section 506, Shop Applied Protective Coating- Steel.
- c. Corrosion resistant steel posts shall conform to the requirements of ASTM A 769, Grade 50W if a welded section. Fabrication will be in accordance with Section 504 - Structural Steel. The portion of the post that is underground shall meet the galvanized steel post requirements in Section b. above.

710.08 Guardrail Hardware Guardrail hardware shall conform to the applicable standards contained in the latest ARTBA Bulletin No. 268B, "A Guide to Standardized Highway Barrier Rail Hardware", approved by the AASHTO-ARTBA-AGC Joint Cooperative Committee, Technical Bulletin Number 268-B.

All galvanized fittings, bolts, washers, and other accessories shall be in accordance with the requirements of AASHTO M 111, M 232 or AASHTO M 298, Class 50, Type I, whichever applies. All galvanizing shall be done after fabrication.

Hardware for corrosion resistant guardrail shall be in accordance with ASTM A 325M/A 325, Type 3, except the 16 inch bolts and nuts for attaching the metal beam rail to the posts, which may be galvanized as specified above.

SECTION 711 - MISCELLANEOUS BRIDGE MATERIAL

711.01 Steel Pipe Piles, Splices and Tips Steel pipe piles shall conform to the requirements of ASTM A 252. The steel pipe piles shall be Grade 3, $F_y = 45$ ksi, with either straight or spiral butt-welded seams. Lap welded seams are not acceptable. The steel shall be a Prequalified Base Metal from the AWS D1.1 Structural Welding Code - Steel.

Cast steel points, splices and open end cutting shoes shall conform to the requirements of ASTM A 27 Grade 65/35 or ASTM A 148 Grade 90/60. Pipe pile splice backup ring material shall be any steel listed in AWS Structural Steel Welding Code D1.1, Table 3.1, with the exception of 100 ksi minimum yield strength steels

711.02 Gabions Each shipment of gabions to a job site shall be accompanied by a certificate that states that the material conforms to the requirements of this specification. The certificate shall be on manufacturer's letterhead and shall be signed by an officer of the company having legal authority to bind the company.

Mesh openings shall be hexagonal in shape, measuring approximately 3 inches by 4 inches and shall be uniform in size. Double twist mesh joints shall be flexible with each pair of wires twisted three half turns, commonly called triple twisted, to prevent unraveling. Steel wire and galvanizing shall meet the requirements of ASTM A 641M/A 641 and ASTM A 90, shall have a Class 3 coating, and shall be soft temper. The wire mesh shall have a sufficient elasticity to permit elongation of the mesh equivalent to a minimum of 10 percent of its length.

a. Galvanized Gabions without Polyvinylchloride Coating The diameter of the steel wire mesh shall be 0.1181 inches after galvanizing. The diameter of the selvedge wire, running through all the edges (perimeter wire), shall be 0.1535 inches after galvanizing. The diameter of the wire for assembling and lacing the gabion units shall be 0.0866 inches after galvanizing. The above wire sizes shall have a tolerance of +/- 2.5 percent.

b. Polyvinylchloride (PVC) Coated Gabions When specified on the plans, all galvanized steel wire shall be coated with a minimum of 0.015 inches of gray or green PVC, which shall be suitable to resist the destructive effects of immersion in acidic, salt, or polluted water, exposure to ultra violet light and abrasion and retain these characteristics after a period of not less than 3,000 hours when tested in accordance with ASTM A975. The diameter of the steel wire mesh shall be 0.1063 inches after galvanization and 0.1363 inches overall, core wire plus PVC coating.

The diameter of the selvedge wire running through all the edges (perimeter wire) shall be 0.1338 inches after galvanizing and 0.1638 inches overall, core wire plus PVC coating. The diameter of the wire for assembling and lacing the gabion units shall be 0.0866 inches after galvanizing and 0.1166 inches overall, core wire plus PVC coating. All wire sizes shall have a tolerance of +/- 2.5 percent, the thickness of the PVC excluded.

The mesh shall be capable of withstanding the test described below:

An uncut section of mesh 6 feet long and of a minimum width of 3 feet, including all selvedge bindings, shall have the ends securely clamped for 3 feet along the width of the sample. When the width of the section under test exceeds 3 feet, the clamps shall be placed in the middle portion of the width and the excess width shall be allowed to fall free on each side of the clamped section. The sample shall then be subjected to sufficient tension to cause 10 percent elongation of the sample section between clamps. After elongation and while clamped as described above (and otherwise unsupported), the section shall be subjected to a load applied to an area of 1 ft² located approximately in the center of the sample section between the clamps and in a direction perpendicular to the direction of the tensile force. The sample shall withstand without rupture of any wire or opening of any mesh fastening, an actual load, so applied, equaling or exceeding 6,000 pounds. The ram head used in the test shall be circular with its edges beveled or rounded to prevent cutting the wires.

711.03 Stones for Gabions Stones to fill gabions shall be of clean, hard, and durable rock with a minimum dimension of 4 inches in all directions and a maximum dimension of 12 inches.

Stones to fill hand filled gabions shall be of clean, hard, durable, crushed ledge or quarried rock with a minimum dimension of 4 inches in all directions and a maximum of 12 inches.

711.04 Bridge Drains Bridge drains shall be fabricated in conformance with the details shown in the Contract documents, and to the requirements of Section 504 - Structural Steel. All bridge drain parts shall be hot-dip galvanized in accordance with the requirements of Section 506, Shop Applied Protective Coating-Steel.

711.06 Stud Shear Connectors, Anchors and Fasteners Shear connectors shall meet the dimensional tolerances of Figure 7.1 of the ANSI/AASHTO/AWS D1.5 Bridge Welding Code (D1.5 Code). Shear connectors, anchors and fasteners shall meet the material requirements of Section 7 of the D1.5 Code. Shear connectors shall meet the mechanical property requirements of Table 7.1, Type B of the D1.5 Code. Anchors and fasteners shall meet the mechanical property requirements of Table 7.1 of the D1.5 Code, Type A.

711.07 Mattresses Each shipment of mattresses on a job site shall be accompanied by a certificate that states that the material conforms to the requirements of this specification. The certificate shall be on the manufacturer's letterhead and shall be signed by an officer of the company having legal authority to bind the company.

Mesh openings shall be hexagonal in shape, measuring approximately 2½ by 3¼ inches and shall be uniform in size. Double twist mesh joints shall be flexible with each pair of wires twisted three half turns (triple twisted) to prevent unraveling. Steel wire and galvanizing shall meet the requirements of ASTM A 641M/A 641 and ASTM A 90, shall have a Class 3 coating, and shall be soft temper. The wire mesh shall have a sufficient elasticity to permit elongation of the mesh equivalent to a minimum of 10 percent of its length ±.

a. Galvanized Mattresses without Polyvinylchloride Coating The diameter of the steel wire mesh shall be 0.0866 inches after galvanizing. The diameter of the selvedge wire, running through all the edges (perimeter wire), shall be 0.1063 inches after galvanizing. The diameter of the wire for assembling and lacing the units shall be 0.0866 inches after galvanizing. The above wire sizes shall have a tolerance of ± 2.5 percent.

b. Polyvinylchloride (PVC) Coated Mattresses When specified on the plans, all galvanized steel wire shall be coated with a minimum thickness of 0.015 inches of gray or green PVC, which shall be suitable to resist the destructive effects of immersion in acidic, salt or polluted water, exposure to ultra violet light and abrasion and retain these characteristics after a period of not less than 3,000 hours when tested in accordance with ASTM A975. The diameter of the steel wire mesh shall be 0.0866 inches after galvanization and 0.1166 inches overall minimum, core wire plus PVC Coating.

The diameter of the selvedge wire running through all the edges (perimeter wire) shall be 0.1050 inches after galvanizing and an overall minimum diameter of 0.1350 inches, core wire plus PVC coating. The diameter of the wire for assembling and lacing the units shall be 0.0866 inches after galvanizing and 0.1166 inches nominal overall, core wire plus PVC coating. All wire sizes shall have a tolerance of ± 2.5 percent, the thickness of the PVC excluded.

The mesh shall be capable of withstanding the test described below:

An uncut section of mesh 6 feet long and a minimum width of 3 feet, including all selvedge bindings, shall have the ends securely clamped for 3 feet along the width of the sample. When the width of the section under test exceeds 3 feet, the clamps shall be placed in the middle portion of the width and the excess width shall be allowed to fall free on each side of the clamped section. The sample shall then be subjected to sufficient tension to cause 10 percent elongation of the sample section between clamps. After elongation and while clamped as described above (and otherwise unsupported), the section shall be subjected to a load applied to an area of 1 ft² located approximately in the center of the sample section between the clamps and in a direction perpendicular to the direction of the tensile force. The sample shall be able to withstand without rupture of any wire or opening of any mesh fastening, an actual load, so applied, equaling or exceeding 6,000 pounds. The ram head used in test shall be circular with its edges beveled or rounded to prevent cutting the wires.

711.08 Stones for Mattresses Stones to fill mattresses shall be of clean, hard, and durable rock with a minimum dimension of 3 inches in all directions and a maximum dimension of 6 inches.

711.09 Neoprene Pads The neoprene shall be either chloroprene or natural polyisoprene of 50 +/-5 Shore A durometer hardness and shall conform to the requirements of Sections 18.10.2 of AASHTO LRFD Bridge Construction Specifications, where applicable.

711.10 H-Beam Piles, Splices and Tips Steel H-beam piles shall be ASTM A 572, Grade 50 minimum.

Steel H-beam pile splicers shall be ASTM A 572, Grade 50 minimum.

H-Beam pile tips shall consist of pile points equipped with cutting teeth. The slope forming the point shall not be steeper than 1¾ : 1. Material for H-Beam pile tips shall consist of plain cast steel pile points conforming to the requirements of AASHTO M 103 (ASTM A 27), Grade 65/35 or ASTM A 148, Grade 80/50. The use of pile tips fabricated by welding sections of plate in an "H" configuration will not be allowed.

711.11 Elastomer Elastomer for bearings shall conform to AASHTO LRFD Bridge Construction Specification Section 18.2.3 except that the elastomer compound shall be Grade 3 or 4 or 5, unless otherwise noted on the Plans. A higher grade of elastomer may be substituted for a lower one.

711.12 Stainless Steel Stainless steel shall conform to the requirements of ASTM A 167 Type 308 or ASTM A 240, Type 304.

711.13 Polytetrafluoroethylene (PTFE) The PTFE, filled or unfilled, shall conform to the requirements of Section 18.8 of AASHTO, LRFD Bridge Construction Specifications. PTFE resin shall conform to the requirements of ASTM D 4894 or D 4895.

SECTION 712 - MISCELLANEOUS HIGHWAY MATERIAL

712.02 Calcium Chloride Calcium chloride shall conform to the requirements of AASHTO M 144.

712.04 Stone Curbing and Edging Stone for curbing and edging shall be approved granite from acceptable sources. The stone shall be hard and durable, predominantly gray in color, free from seams that impair its structural integrity and of smooth splitting character. Such natural variations may include bands or clusters of mineral or both of mineral crystallization that do not impair the structural integrity of the curb stone. The dimensions shape and other details shall be as shown on the plans.

The exposed face of stone curb shall be free from indications of drill holes. Half drill holes not larger than ¾ inch diameter will be permitted in the arris line in the plane of the back.

- a. Vertical Curb shall have a top surface sawed or dressed to an approximate true plane with no depression or projection on that surface of over ⅛ inch. The top front arris line shall

be pitched straight and true with no variations from a straight line greater than $\frac{1}{4}$ inch. The top back arris line shall meet the same requirement except that indentations of a maximum of $\frac{3}{8}$ inch will be allowed. There shall be no projection or depression on the back face that would exceed a batter of 1 horizontal on 3 vertical for a distance from the top of 3 inches.

The front face shall be at right angles to the top and shall be smooth split and have no projections greater than 1 inch or depressions greater than $\frac{1}{2}$ inch, measured from the vertical plane of the face through the top arris line for a distance down from the top of 8 inches. The remainder of the face shall have no projections or depressions greater than 1 inch from the plane of the face.

The ends of the curb shall be approximately square with the planes of the top, back and face and so finished that when the sections are placed end to end with the required minimum spacing of $\frac{1}{4}$ inch no more than $\frac{5}{8}$ inch space shall show in the joint for the full width of the top surface and for the entire exposed front face. The remainder of the end may extend back no more than 8 inches from the plane of the joint.

Drill holes through the curb will be allowed providing they are at least 9 inches below the top and are mortared full with Portland cement mortar before placing the stone.

b. Miscellaneous Stone Curb When a depressed or modified section of curb is called for on the plans or ordered by the Resident, for driveways, crossings, closures, transitions or for other reasons, the Contractor shall furnish curbing with the required modifications.

c. Curb Inlets Inlets used at catch basins shall conform to the applicable requirements of Vertical Curb, Type 1 and to the shape, dimensions, and details as shown on the Standard Detail

Natural grain size and color variations characteristic of the source deposit will be permitted.

d. Dimensions The stone curb units shall be of the dimensions indicated on the plans and shall be cut in lengths of not less than 4 feet nor greater than 10 feet. Random lengths of curb less than 4 feet in length may be obtained if the Resident determines it necessary to meet field conditions. All curb to be set on a radius of 60 feet or less shall be cut .

e. Vertical Bridge Curb shall conform to the requirements above, except as indicated on the plans and as follows:

1. The back face of the curb stones shall have no projections or depressions greater than 1 inch, measured from the vertical plane of the back face through the arris or pitch line down to the bottom of the stone. The front face shall be finished as required for Vertical Curb, Type 1, except that it shall be finished the full distance down on the face. Bottoms of curb stones shall be approximately parallel to the top and sawed or dressed to lay with a bedding of approximately 1 inch for the full length of the stone.

2. Anchor holes shall be provided in the back of the stones, pitched down as shown on the plan, a maximum of 18 inches from each end of the stone and spaced horizontally at a maximum of 4 feet apart. A minimum of 2 anchor holes shall be provided in each stone.

3. The ends of stones at expansion joints between spans and at ends of the bridge shall be cut to present a vertical face when set in position, beveled to the skew angle, if any and the entire end finished in the same manner as the top.

4. The exposed edges of the stones at intermediate joints shall be trimmed square with the planes of the top and front face so that a neat, parallel joint, free from drill holes is formed between the stones. Length of stones shall be so scheduled that joints will be uniform in width along any run of curb. Joints shall be $\frac{1}{2}$ inch $\pm \frac{1}{8}$ inch. A joint shall be provided at each curb and sidewalk contraction joint of the bridge.

5. Stones set transversely at ends of a bridge, when the grade exceeds 2 percent, shall have the top beveled to fit the grade of the bridge.

6. Mortar for bedding shall be composed of 1 part Portland cement and 2 parts sand with sufficient water to form a workable mix. Cement, sand, and water shall conform to Section 502 - Structural Concrete.

7. Mortar for pointing shall be composed of equal parts sand and Portland cement with sufficient water to form a workable mix and shall conform to Section 502 - Structural Concrete.

8. Portland cement grout shall be made the same as mortar for pointing, except that consistency shall be such that it will flow readily.

f. Curb Type 5 The exposed face shall be smooth split to an approximate true plane having no projections or depressions which will allow over 1 inch to show between a 2 foot straightedge and the face when the straightedge is placed as closely as possible on any part of the face. Half drill holes not more than 3 inches in length and $\frac{3}{4}$ inch in diameter will be permitted along the bottom. The arris line, top front shall be straight and true with no variation from a straight line greater than $\frac{1}{8}$ inch. The arris lines at the bottom of the face shall be straight and true so that not over 1 inch shall show between the stone and a straightedge for the full length of the stone. The ends shall be square to the length at the face and so finished that when the stones are placed end to end, no space more than $1\frac{1}{2}$ inches will show in the joint for the width of the face.

When Type 5 Curbing is required on a curve, the pieces shall be shaped as described in the table on the Standard Detail plans.

712.05 Preformed Plastic

Type of Material This section covers reflectorized plastic materials preformed into rolls or ribbons of various lengths, pliability, and widths suitable for use as reflecting pavement markings on Portland cement concrete or hot mix asphalt pavement.

General Characteristics The preformed marking materials shall consist of white or yellow films with pigments selected and blended to provide the appropriate highway colors for traffic markings. Glass or ceramic beads shall be incorporated to provide immediate and continuing retroreflection. The size, quality, and refractive index of the beads shall be such that the performance requirements of this specification shall be met.

The edges of the preformed material shall be clean cut and true. The preformed plastic material may be supplied complete with a precoated, factory applied adhesive for immediate pavement application without the use of heat, solvent, or other types of adhesive for immediate pavement application without the use of heat, solvent, or other types of adhesive operations or it may be furnished with separate adhesives as recommended by the manufacturer.

The affixed material shall be capable of molding itself to the pavement contoured by the action of traffic and maintain its original dimensions and placement under normal traffic conditions at the pavement temperatures, which could occur within the State. After application, the markings shall be immediately ready for traffic.

Physical Requirements - Color Pigments shall be selected and blended to conform to standard highway colors throughout the expected life of the material. When tested by Federal Test Method Standard 141 Method 4232, the white shall be no darker than Color Number 37778 of Federal Standard Number 595 and the yellow shall conform to Color Number 33538 of Federal Standard Number 595 (Highway Yellow Color PR#1).

Retro-Reflectivity The retro-reflective preformed film shall have a layer of reflective spheres bonded to the top surface. The white and yellow film shall have the following initial minimum retroreflectance values at 0.2° and 0.5° observation angles and 86° entrance angle as measured in accordance with the photometric testing procedures of ASTM D 4061.

Retroreflectance values shall be expressed as specific luminance in millicandelas per square meter per lux ($\text{mcdm}^{-2}\text{lx}^{-1}$) [millicandelas per square foot/foot candle ($\text{mcd ft}^{-2}\text{fc}^{-1}$)]

Observation Angle	White		Yellow	
	0.2°	0.5°	0.2°	0.5°
SL [$\text{mcdm}^{-2}\text{lx}^{-1}$] [$\text{mcd ft}^{-2}\text{fc}^{-1}$]	550	380	410	250

The test distance shall be 50 feet and the sample size a 2 by 2½ foot rectangle. The angular aperture of both the photoreceptor and light projector shall be 10 minutes of arc. The reference center of the sample and the reference axis shall be taken perpendicular to the test sample.

Bead Retention When tested with a 2 by 6 inch sample bent over a ½ inch diameter mandrel with the 2 inch dimension perpendicular to the mandrel axis, microscopic examination of the arc on the mandrel shall show no more than 10 percent of the beads are entrapped in the binder and less than 40 percent of the surface of the bead.

Application The preformed plastic material shall be capable of application to non-defective pavement surfaces that are dry and free from dirt or other foreign matter. For normal application, the pavement temperature should be at least 60°F and rising.

Special instructions should be supplied by the vendor for application to be made at pavement temperatures below 60°F. Application shall be according to manufacturer's recommended procedures. Plastic pavement marking materials shall only be applied to surfaces with temperatures within the range specified by the manufacturer for optimum adhesion.

Adhesive, activators or special coatings for various types of pavement surfaces shall be provided with the preformed plastic material. Detailed information must be supplied with the material outlining required application procedures for such adhesives, activators, or special coating.

Preformed plastics shall be capable of being applied to new asphalt pavement immediately prior to the final rolling of the new surface and of being rolled into place with conventional pavement and highway rollers. The plastic material and adhesives used in such applications shall be of the type that water used on the roller to prevent asphalt pickup shall not be harmful to the successful application of the plastic.

Special equipment necessary for the successful installation of any preformed plastic material shall be available from the manufacturer of the plastic material on a lease, loan, or purchase basis.

Longitudinal lines shall be offset at least 2 inches from construction joints of Portland cement concrete pavement. When directed by the Resident, opening of 6 inch lengths shall be left at 20 foot intervals in edge lines not inlaid into the pavement surface that are placed on the inside of superelevated curves so as to prevent the ponding of water on the pavement surface.

712.06 Precast Concrete Units Precast concrete units shall conform to the plan dimensions and shall meet the requirements of AASHTO M 199 except as modified below.

Approved structural fibers may be used as a replacement of 6 x 6 #10 gauge welded wire fabric when used at an approved dosage rate for the construction of manhole and catch basin units. The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List of Structural Fiber Reinforcement.

Cement shall conform to Section 701. An approved air-entraining admixture shall be added to obtain the required air content.

Aggregates shall meet the quality requirements of Section 703.01 - Fine Aggregate for Concrete and Section 703.02 - Coarse Aggregate for Concrete except that limitations on grading and the fineness modulus may be omitted.

The concrete mix design shall be approved by the Department. Concrete shall contain 6.0 percent air content, plus or minus 1.5 percent tolerance when tested according to AASHTO T 152. All concrete shall develop a minimum compressive strength of 4000 psi in 28 days when tested according to AASHTO T 22. The absorption of a specimen, when tested according to AASHTO T 280, Test Method "A", shall not exceed 9.0 percent of the dry mass.

712.061 Structural Precast Concrete Units Structural precast concrete units shall conform to the dimensions shown on the Plans, the requirements of this Specification and with the reviewed Working Drawings.

Materials Materials for concrete shall conform to the requirements of Standard Specification Section 502.03, Materials. Cement shall be Type I, Type II, or Type III. Coarse aggregate for concrete shall conform to Class A, AA or Latex. The maximum water cement ratio shall be 0.40. For the purpose of calculating water cement ratios, one U.S. gallon of water shall be considered to weigh 8.34 pounds. Concrete shall contain a minimum of 3 gallons per cubic yard of calcium nitrite solution. The minimum 28 day compressive strength shall be 5,000 psi, unless otherwise stated in the Contract.

Material for reinforcing shall meet the requirements of Standard Specification Section 709.01 - Reinforcing Steel or Section 709.02 - Welded Steel Wire Fabric.

Only one mat of steel is required for concrete member thicknesses of seven inches or less; two mats of steel, one at each face, are required for concrete member thicknesses greater than seven inches.

Grout, concrete patching material, and geotextiles shall be one of the products listed on the Department's Qualified Products List (QPL).

Quality Control and Quality Assurance Quality Control (QC) is the responsibility of the Contractor. The Quality Control Inspector (QCI) shall inspect all aspects of the work.

The Contractor shall generate a nonconformance report (NCR) for materials and/or workmanship that is nonconforming. The NCR shall describe the nonconformance and the proposed corrective action. Copies of the NCR shall be provided to the Department's Quality Assurance Inspector (QAI) and the Fabrication Engineer for review. Nonconforming material and/or workmanship shall be corrected or replaced.

In the event that an item fabricated under this Specification does not meet the Contract requirements but is deemed suitable for use by the Department, it may be accepted in accordance with Section 106.8, Non-Conforming Work, of the Standard Specifications.

Acceptance is the prerogative of the Department. The Department will conduct Quality Assurance (QA) in accordance with Standard Specification Subsection 106.5. Testing deemed necessary by the Department that is in addition to the minimum testing requirements will be scheduled to minimize interference with the production schedule. The QAI will witness or review documentation, workmanship and testing to assure the Work is being performed in accordance with the Contract Documents.

The QAI has the authority to reject materials and products that do not meet the Contract requirements, including Work rejected due to denial of access or the lack of adequate notice of the beginning of production. The acceptance of material or workmanship by the QAI will not prevent subsequent rejection, if the Work is later found to be unacceptable.

The Contractor shall provide a private office at the fabrication plant for the QAI in accordance with the Facilities for Inspection requirements in Standard Specification Section 535.

Construction The precast units shall be manufactured at a facility that has had a minimum of five years of experience in producing similar type products. The plant shall meet the requirements of AASHTO M-157. Facilities that are certified by the Precast/Prestressed Concrete Institute (PCI) or the National Precast Concrete Association (NPCA) will be considered pre-qualified.

The Contractor shall notify the Department of the planned start of production date of the precast units a minimum of two weeks prior to beginning production for in-Maine work and a minimum of three weeks prior to beginning production for out-of-Maine work. If the production schedule changes, notify the Fabrication Engineer no less than three Working Days prior to the initial production start-up date. Any Work done without the QAI present will be rejected. Advise the Fabrication Engineer of the production schedule and any changes to it. If Work is suspended on a project, the Fabrication Engineer will require 72 hours of notice prior to the resumption of Work.

The Contractor shall calibrate all production equipment on the following schedule, unless there is reason to believe calibration should be performed sooner: Water gauges- every 90 days; admixture dispensers- every 90 days; concrete cylinder compression testing machine- annually; batch plant scales- every six months. Use proving rings, load cells and solid standard weights, as applicable. The calibration shall be performed by a testing laboratory acceptable to the Department, using calibration equipment the accuracy of which is traceable to a National Institute of Standards and Technology (NIST) standard. Provide calibration certifications to the QAI prior to beginning fabrication.

All precast members for a distinct system or Contract Pay Item, including end blocks, steps, caps, box culverts, arch sections, wall units, or other elements, shall be manufactured from the same material sources of aggregates, brand and type of cement and color pigment.

Gradations of coarse and fine aggregates shall be furnished to the QAI prior to beginning work and at least once per week thereafter.

Concrete mix designs shall be submitted for review and approval by the Department prior to use.

Reinforcing steel shall be fabricated, handled, and placed in accordance with Standard Specification Section 503, Reinforcing Steel, and the reviewed Working Drawings.

Forms shall be sufficiently rigid and accurate to maintain the member's dimensions. All forms shall be well built, substantial and unyielding, securely braced, strutted and tied to prevent motion and distortion while concrete is being placed in them. Corners within the forms shall be fitted with consistent width chamfer strips of 5/8 inch \pm 1/8 inch mitered at their intersections; all exposed corners shall be chamfered. Forms shall be treated with a bond breaking agent; they shall be clean and free of foreign material.

Recess inserts one inch, unless noted otherwise in the Contract. The QAI is not responsible for verifying the location of inserts or other hardware installed for the convenience of the Contractor.

Curing The units shall be cured by one of the following methods:

1. Curing by Moisture Retention: Cure the concrete in accordance with the latest edition of the Precast/Prestressed Concrete Institute Manual for Quality Control for Plants and Production of Structural Precast Concrete Products (MNL 116), Section 4.20. Moist cure the concrete until it has reached design strength.

Do not use membrane-forming curing compounds without the approval of the Fabrication Engineer. If membrane-forming curing compounds are authorized, follow the requirements of MNL 116 and the curing compound manufacturer's published recommendations.

2. Accelerated Curing of Concrete: Cure the concrete in accordance with MNL 116, Section 4.19, except as modified herein.

After initial set, the temperature gain of the concrete shall not exceed 40°F per hour. Initial set shall be determined in accordance with ASTM C403, Standard Test Method for Time of Setting of Concrete Mixtures by Penetration Resistance; a strength gain of 500 psi indicates initial set. The maximum allowable concrete temperature shall be 160°F. Concrete temperature shall be measured during curing with recording thermometers. In order to qualify for accelerated cure, the concrete temperature shall attain a minimum temperature of 120°F; that temperature shall be maintained for a minimum of 8 hours and the concrete shall achieve a minimum of 80 percent of design strength.

The accelerated curing cycle shall be considered complete when the method of supplying heat is stopped and/or the concrete temperature drops below 120°F. Two cylinders shall be tested immediately upon completion of the accelerated cure cycle. Products that have

not achieved all of the above criteria shall be moist cured until the concrete has achieved design strength.

If the precast units have achieved 80 percent of design strength during the accelerated curing cycle, no further curing will be required.

The forms shall remain in place until the concrete attains a minimum compressive strength of 3,000 psi.

Concrete Testing Acceptance of structural precast units, for each day's production, will be determined by the Department, based on compliance with this specification and satisfactory concrete testing results. Process control test cylinders shall be made and tested in accordance with the following standards:

AASHTO T 22 (ASTM C39) Test Method for Compressive Strength of Cylindrical Concrete Specimens

AASHTO T23 (ASTM C31) Practice for Making and Curing Concrete Test Specimens in Field

AASHTO T141 (ASTM C172) Practice for Sampling Freshly Mixed Concrete

AASHTO T152 (ASTM C231) Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method

AASHTO T196 (ASTM C173) Standard Test Method for Air Content of Freshly Mixed Concrete by the Volumetric Method

ASTM C1064 Test Method for Temperature of Freshly mixed Portland Cement Concrete

ASTM C1611 Standard Test Method for Slump Flow of Self-Consolidating Concrete

Tested concrete shall conform to the following:

- a. Temperature shall be from 40° F to 85° F.
- b. Air content shall be 5.5% to 7.5%; concrete with higher air content may be used, at the Contractor's option, provided extra cylinders are made to demonstrate that stripping strength and design strength are achieved.
- c. Slump flow shall be homogenous with no sign of segregation.
- d. Visual stability index (VSI) of SCC shall not exceed 1; if a mortar paste halo is present it shall not exceed an average of 0.25 inch.

The Contractor shall cast a minimum of 8 concrete test cylinders for each continuous concrete placement, for QC purposes: 2 cylinders shall be standard cured in accordance with AASHTO T23 (ASTM C31) to be tested for 28 day strength; and a minimum of 6 cylinders shall be field cured under the same conditions as the units and tested for stripping and design strength. Unit identification, entrained air content, water-cement ratio, slump flow and temperature of the sampled concrete shall be recorded at the time of cylinder casting. The Contractor shall perform all testing in the presence of the QAI. The QAI will designate the loads to be tested.

At least once per week, the Contractor shall make 2 concrete cylinders (6 cylinders when the Contract includes permeability requirements) for use by the Department; cylinders shall be standard cured in accordance with AASHTO T23 (ASTM C31).

If the Contractor fails to make enough cylinders to demonstrate that the product meets the Contract requirements, the product will be considered unacceptable.

The compressive strength of the concrete will be determined by averaging the compressive strength of two test cylinders made from the same sample. For the purpose of determining design strength, the average of two cylinders shall meet or exceed the design strength, and the difference in strength between the two shall be no more than 10 percent of the higher strength cylinder.

Perform compressive strength testing to determine transfer and design strength in the presence of the QAI. Cylinder tests not witnessed by the QAI will not be acceptable.

All QC Inspectors performing concrete testing shall hold a current ACI Field Testing Technician Grade I Certification, or other equivalent certification.

Surface Finish and Repairs Exposed surfaces shall be finished and repaired in conformance with the referenced specification. If the finish is not specified, then surfaces shall have a uniform appearance; only minor repairs to remove and blend fins, patch minor spalls, tie holes and handling device recesses and to repair small, entrapped air pockets, shall be permitted

Tolerances Dimensional tolerances shall be in conformance with the applicable referenced specification or the established industry standards for the product being produced.

Documentation The producer of the structural precast units shall keep accurate records of aggregate gradations, concrete batching, testing, curing, and inspection activities, verifying that forms, reinforcing and unit dimensions conform to these requirements. Copies of reports shall be furnished to the Department.

Marking The date of manufacture, production lot number and type of unit shall be clearly and indelibly inscribed by the Contractor on a rear or unexposed portion of each unit.

Handling, Storage and Shipping The Contractor shall handle, store, and ship units in such a manner as to prevent chipping, cracking, fractures, and excessive bending stresses. Damaged units shall be repaired or replaced.

712.07 Catch Basin and Manhole Frames, Grates and Covers, Drainage, Sewer, Utility, and related castings shall conform to the plan dimensions and shall be gray iron or ductile iron castings conforming to the requirements of AASHTO M 306.

712.08 Corrugated Metal Units The units shall conform to plan dimensions and the metal to AASHTO M36/M36M. Bituminous coating, when specified, shall conform to AASHTO M190 Type A.

712.09 Catch Basin and Manhole Steps Steps for catch basins and for manholes shall conform to ASTM C478M [ASTM C478], Section 13 for either of the following material:

(a) Aluminum steps-ASTM B221M, [ASTM B211] Alloy 6061-T6 or 6005-T5.

(b) Reinforced plastic steps Steel reinforcing bar with injection molded plastic coating copolymer polypropylene. Polypropylene shall conform to ASTM D 4101.

712.23 Flashing Lights Flashing Lights shall be power operated or battery operated as specified.

(a) Power operated flashing lights shall consist of housing, adapters, lamps, sockets, reflectors, lens, hoods and other necessary equipment designed to give clearly visible signal indications within an angle of at least 45 degrees and from 10 to 300 ft under all light and atmospheric conditions.

Two circuit flasher controllers with a two-circuit filter capable of providing alternate flashing operations at the rate of not less than 50 nor more than 60 flashes per minute shall be provided.

The lamps shall be 650 lumens, 120 volt traffic signal lamps with sockets constructed to properly focus and hold the lamp firmly in position.

The housing shall have a rotatable sun visor not less than 7 inches in length designed to shield the lens.

Reflectors shall be of such design that light from a properly focused lamp will reflect the light rays parallel. Reflectors shall have a maximum diameter at the point of contact with the lens of approximately 8 inches.

The lens shall consist of a round one-piece convex amber material which, when mounted, shall have a visible diameter of approximately 8 inches. They shall distribute light and not diffuse it. The distribution of the light shall be asymmetrical in a downward direction. The light distribution of the lens shall not be uniform, but shall consist of a small high intensity portion with narrow distribution for long distance throw and a larger low intensity portion with wide distribution for short distance throw. Lenses shall be marked to indicate the top and bottom of the lens.

(b) Battery operated flashing lights shall be self-illuminated by an electric lamp behind the lens. These lights shall also be externally illuminated by reflex-reflective elements built into the lens to enable it to be seen by reflex-reflection of the light from the headlights of oncoming traffic. The batteries must be entirely enclosed in a case. A locking device must secure the case. The light shall have a flash rate of not less than 50 nor more than 60 flashes per minute from minus 20 °F to plus 150 °F. The light shall have an on time of not less than 10 percent of the flash cycle. The light beam projected upon a surface perpendicular to the axis of the light beam shall produce a lighted rectangular projection

whose minimum horizontal dimension shall be 5 degrees each side of the horizontal axis. The effective intensity shall not have an initial value greater than 15.0 candelas or drop below 4.0 candelas during the first 336 hours of continuous flashing. The illuminated lens shall appear to be uniformly bright over its entire illuminated surface when viewed from any point within an angle of 9 degrees each side of the vertical axis and 5 degrees each side of the horizontal axis. The lens shall not be less than 7 inches in diameter including a reflex-reflector ring of ½ inch minimum width around the periphery. The lens shall be yellow in color and have a minimum relative luminous transmittance of 0.440 with a luminance of 2854° Kelvin. The lens shall be one-piece construction. The lens material shall be plastic and meet the luminous transmission requirements of this specification. The case containing the batteries and circuitry shall be constructed of a material capable of withstanding abuse equal to or greater than 1.21 mm thick steel [No. 18 U.S. Standard Gage Steel]. The housing and the lens frame, if of metal shall be properly cleaned, degreased and pretreated to promote adhesion. It shall be given one or more coats of enamel which, when dry shall completely obscure the metal. The enamel coating shall be of such quality that when the coated case is struck a light blow with a sharp tool, the paint will not chip or crack and if scratched with a knife will not powder. The case shall be so constructed and closed as to exclude moisture that would affect the proper operation of light. The case shall have a weep hole to allow the escape of moisture from condensation. Photoelectric controls, if provided, shall keep the light operating whenever the ambient light falls below 215 lx [20 foot candles]. Each light shall be plainly marked as to the manufacturer's name and model number.

If required by the Resident, certification as to conformance to these specifications shall be furnished based on results of tests made by an independent testing laboratory. All lights are subject to random inspection and testing. All necessary random samples shall be provided to the Resident upon request without cost to the Department. All such samples shall be returned to the Contractor upon completion of the tests.

712.32 Copper Tubing Copper tubing and fittings shall conform to the requirements of ASTM B88M Type A [ASTM B88, Type K] or better.

712.33 Non-metallic Pipe, Flexible Non-metallic pipe and pipe fittings shall be acceptable flexible pipe manufactured from virgin polyethylene polymer suitable for transmitting liquids intended for human or animal consumption.

712.34 Non-metallic Pipe, Rigid Non-metallic pipe shall be Schedule 40 polyvinylchloride (PVC) that meets the requirement of ASTM D1785. Fittings shall be of the same material.

712.341 Metallic Pipe Metallic pipe shall be ANSI, Standard B36.10, Schedule 40 steel pipe conforming to the requirements of ASTM A53 Types E or S, Grade B. End plates shall be steel conforming to ASTM A36/A36M.

Both the sleeve and end plates shall be hot dip galvanized. Pipe sleeve splices shall be welded splices with full penetration weld before galvanizing.

712.35 Epoxy Resin Epoxy resin for grouting or sealing shall consist of a mineral filled thixotropic, flexible epoxy resin having a pot life of approximately one hour at 50°F. The grout shall be an approved product suitable for cementing steel dowels into the preformed holes of curb inlets and adjacent curbing. The sealant shall be an approved product, light gray in color and suitable for coating the surface.

712.36 Hot Mix Asphalt Curb The performance graded asphalt binder for hot mix asphalt curb shall meet a PG 58-28 or a PG 64-28. The aggregate shall conform to the requirements of Subsection 703.07. The coarse aggregate portion retained on the No. 8 sieve may be either crushed rock or crushed gravel.

The mineral constituents of the hot mix asphalt shall be sized and graded and combined in a composite blend that will produce a stable durable curbing with an acceptable texture.

Hot Mix Asphalt for curb shall meet the requirements of Section 403 - Hot Mix Asphalt Pavement.

712.37 Precast Concrete Slab Portland cement concrete for precast slabs shall meet the requirements of Section 502 - Structural Concrete, Class A.

The slabs shall be precast to the dimension shown on the plans and cross section and in accordance with the Standard Detail plans for Concrete Sidewalk Slab. The surface shall be finished with a float finish in accordance with Subsection 502.14(c). Lift devices of sufficient strength to hold the slab while suspended from cables shall be cast into the top or back of the slab.

712.38 Stone Slab Stone slabs shall be of granite from an acceptable source, hard, durable, predominantly gray in color, free from seams which impair the structural integrity and be of smooth splitting character. Natural color variations characteristic of the deposit will be permitted. Exposed surfaces shall be free from drill holes or indications of drill holes. The granite slabs in any one section of backslope must be all the same finish.

The granite slabs shall be scabble dressed or sawed to an approximately true plane having no projections or depressions over ½ inch under a 2 ft straightedge or over 1 inch under a 4 ft straightedge. The arris at the intersection of the top surface and exposed front face shall be pitched so that the arris line is uniform throughout the length of the installed slabs. The sides shall be square to the exposed face unless the slabs are to be set on a radius or other special condition which requires that the joints be cut to fit, but in any case shall be so finished that when the stones are placed side by side no space more than 3/4 inch shall show in the joint for the full exposed height.

Lift pin holes in all sides will be allowed except on the exposed face.

SECTION 713 - STRUCTURAL STEEL AND RELATED MATERIAL

713.01 Structural Steel Highway bridge steel shall meet the requirements of AASHTO M 270. The grade of steel shall be as specified on the plans.

Main load-carrying components subject to tensile stresses or stress reversal shall meet the notch toughness requirements in AASHTO M 270M, Table 10, Zone 2, for non-fracture critical steel or Table 11, Zone 2 for fracture critical steel. Frequency of tension tests shall comply with the requirements of S1.

Impact test sampling and testing procedures shall be in accordance with AASHTO T.

Steel for ancillary bridge products and steel structures shall conform to AASHTO M 270 or one or more of the following:

- ASTM A 36
- ASTM A 572
- ASTM A 588
- ASTM A 53
- ASTM A 500
- ASTM A 595 Grade C
- ASTM A 786
- ASTM A 847
- ASTM A 992

Ancillary bridge products shall be as described below:

- (a) bearings
- (b) drainage components
- (c) expansion devices (gland seal, compression seal, finger joint)
- (d) modular expansion devices
- (e) steel bridge rail
- (f) catwalks and inspection walkways

713.02 High Strength Bolts Bolts shall conform to the requirements of ASTM A 325, Type 1 or Type 3. Type 3 bolts shall be supplied for all structures utilizing unpainted AASHTO M 270M weathering steel.

Nuts shall meet the requirements of AASHTO M 291 or AASHTO M 292.

Circular and beveled washers shall conform to the requirements of AASHTO M 293.

Direct Tension Indicators (DTI'S) shall conform to the requirements of ASTM F 959. DTI's for use with painted steel shall have a plain "as fabricated" finish. DTI's for use with unpainted steel shall be galvanized to the requirements of AASHTO M 298 Class 50, Type I and have a fusion-bonded epoxy coating. DTI's used with galvanized steel, metalized steel and steel coated with a zinc-rich primer shall be galvanized to the requirements of AASHTO M 298 Class 50, Type I.

“Twist Off” Type Tension Control Structural Bolt/Nut/Washer Assemblies shall meet the requirements of ASTM F1852. They shall meet the chemical and mechanical requirements of ASTM A 325.

Bolts, nuts and washers specified to be galvanized may be galvanized by either hot dip galvanizing to the requirements of AASHTO M 232 Class C or mechanically galvanized to the requirements of AASHTO M 298 Class 50, Type I.

All fastener (bolts and nuts), whether black or galvanized, shall be coated with a suitable lubricant. Galvanized nuts shall be lubricated with a lubricant containing a visible dye.

Each lot of bolts, nuts, washers and DTI’s shall be tested by the manufacturer in accordance with the tests tabulated in Table 1 - Test Schedule. The testing frequency for bolts, nuts and washers from each shipping lot of fasteners shall be as specified in the applicable AASHTO/ASTM Standard Specifications. The testing frequency for each production lot of DTI’s shall be as specified in ASTM F 959.

TABLE 1 - Test Schedule*

Bolts	Tensile Strength (Wedge Test) Proof Load Hardness Coating Thickness	ASTM F 606 ASTM F 606 ASTM F 606 AASHTO M 298
Nuts	Proof Load Hardness Coating Thickness	ASTM F 606 ASTM F 606 AASHTO M 298
Washers	Hardness Coating Thickness	ASTM F 606 AASHTO M 298
DTI’s	Coating Thickness Compression Load	AASHTO M 298 ASTM F 959

*The supplier(s) shall submit test reports for all testing required in this Table. Test reports shall contain, in addition to the test results, the name and address of the testing agency, the manufacturer, lot tested, and Mill Test Reports for all steel used in the manufacture of the fastener assemblies and DTI’s.

The supplier shall perform, or cause to be performed a Rotational Capacity Test (RCT) for every production lot combination of bolts, washers and nuts. Each combination shall be designated with a unique RCT lot number. The test results shall be furnished to the Engineer.

713.03 Preformed Pads Preformed pads shall be made with new unvulcanized rubber and unused fabric fibers and shall be approximately 1/8 inch thick after compression and vulcanizing with a proportion of fiber content sufficient to maintain strength and stability. The surface hardness shall be 85 to 95 Shore A Durometer. The ultimate breakdown limit of the pad under compressive loading shall be no less than 10,000 psi.

713.04 Bronze or Copper-Alloy Bearing and Expansion Plates Bronze bearing and expansion plates shall conform to the requirements of AASHTO M 107 (ASTM B 22), Alloy No. 911 and copper alloy bearing and expansion plates shall conform to the requirements of AASHTO M 108, (ASTM B 100) Alloy No. 510 or 511, unless otherwise specified. The bearing surface(s) subject to sliding action shall be provided with trepanned recesses (not grooves) filled with a lubricating compound. The lubricating compound shall consist of graphite and metallic substances with a lubricating binder capable of withstanding the atmospheric elements. The compound shall be pressed into the recesses to form dense, non-plastic lubricating inserts.

The lubricating area shall comprise between 25 percent and 35 percent of the total area of the plate subject to sliding action. The sliding surface(s) shall be planed parallel to the prevailing direction of movement of the structure and subsequently polished, unless detailed otherwise.

713.05 Cold-finished Carbon Steel Shafting Cold-finished carbon steel shafting shall conform to the requirements of AASHTO M 169. Grade Designation 1021-1030 inclusive, cold drawn, either semi-killed or fully-killed, shall be furnished unless otherwise specified.

713.06 Castings Gray iron castings, for use other than for drainage castings, (Section 712.07) shall conform to the requirements of AASHTO M 105. Class Number 35 shall be furnished unless otherwise specified.

Malleable iron castings shall conform to the requirements of ASTM A 47. Grade Number 32510 shall be furnished unless otherwise specified.

Carbon-Steel Castings for General Applications shall conform to the requirements of AASHTO M 103. The grade 70-36 of steel shall be used unless otherwise specified.

Castings shall be true to pattern in form and dimensions, free from pouring faults, sponginess, cracks, blow holes and other defects in positions affecting their strength and value for the service intended.

Castings shall be boldly filleted at angles and the arrises shall be sharp and perfect.

All castings must be sand blasted or otherwise effectively cleaned of scale and sand to present a smooth, clean, and uniform surface.

713.07 Metal Bin Type Retaining Wall The metal for bin type retaining wall members shall be galvanized and shall conform to the applicable requirements of AASHTO M 36.

When fiberglass, aramid or carbon graphite fiber coating is specified, the galvanized metal sheets used to form the retaining wall, except the base plates and connecting channels, shall additionally be coated on both sides with a layer of fiberglass, aramid or carbon graphite fibers applied in sheet form by pressing it into the molten spelter. Immediately after the metallic bond

has solidified the fibers shall be thoroughly saturated with a bituminous saturant conforming to the following requirements:

Penetration at 77 °F, 3.5 oz, 5 sec.	0.14 - .018 inches
Loss on heating at 325°F, 1.75 oz, 5 hrs	Not more than 1.5 percent
Flash point (open cup)	Not less than 440°F
Penetration at 77 °F, 3.5 oz, 5 sec., of residue after heating at 325°F, as compared with penetration of asphalt before heating	Not less than 70 percent
Insoluble in carbon disulfide	Not more than 2 percent

Testing shall be in accordance with methods specified in AASHTO M 20.

Whenever possible in the manufacture of the units, a minimum forming radius of 1 inch is to be maintained. All units that are formed with less than 1 inch radius shall be hot-dipped galvanized after forming.

Bolts shall conform to the requirements of ASTM F 568 Class 4.6 (ASTM A_307) and galvanized in accordance with AASHTO M 232.

713.08 Steel Extrusions Material for steel extrusions for expansion devices shall be ASTM A 36/A 36M, ASTM A 588/A 588M, or ASTM A 242/A 242M, except that ASTM A 242/A 242M shall not be used for extrusions that are to be welded.

SECTION 714 - JOINT SEALS

714.01 Elastomer for Seal Elements The preformed elastomeric polychloroprene joint seal elements, both compression and gland type, shall conform to the requirements of AASHTO M 297.

714.02 Fabric for Seal Elements Fabric used for reinforcement in a seal element shall be a non-wicking fabric conforming to the requirement of ASTM D 578.

714.03 Lubricant Adhesive The lubricant-adhesive shall be a 1 part, moisture curing, polyurethane and aromatic hydrocarbon solvent mixture and shall have the following physical properties:

Solids content	60-80 percent by weight
Service Range	5°F to 120°F minimum
Film Strength (ASTM D 412)	1,200 psi minimum
Elongation at Break	250 percent minimum

Each lot of lubricant-adhesive shall be delivered in sealed containers plainly marked with the manufacturer's name or trademark and the date of manufacture. Maximum shelf life shall not exceed 6 months.

714.04 Sealant The sealant shall be a one part, moisture curing, polyurethane base, non-sag, elastomeric product, conforming to the requirements of Federal Specification TT-S-0023OC(2), Type II, Class A or ASTM C920, Type S, Grade NS, Class 25.

Each lot of sealant shall be delivered in sealed containers plainly marked with the manufacturer's name or trademark and the date of manufacture. Maximum shelf life shall be as recommended by the manufacturer.

714.05 Compression Seals Compression seals shall be multi-channel extruded shapes made of material conforming to the requirements of Section 714.01 - Elastomer for Seal Elements, and in a configuration as determined by each particular manufacturer and as shown in the contract documents. The seal shall be marked on the top surface with the manufacturer's name or trademark, the lot number and the size designation at intervals of 5 feet or less. Actual seal dimensions shall not differ from the nominal dimensions by more than $\frac{1}{16}$ inch/inch of depth or width, or a maximum of $\frac{1}{4}$ inch whichever is less.

The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

714.06 Gland Type Seals Gland type seals shall be single membrane extruded or molded shapes, made of material conforming to the requirements of Section 714.01 - Elastomer for Seal Elements and of a configuration as determined by each manufacturer and as shown in the contract documents. The seals shall be marked on the top surface with the manufacturer's name or trademark, the lot number and the size designation at intervals of 5 feet or less. If fabric is used to reinforce the seal, it shall conform to the requirements of Section 714.02 - Fabric for Seal Elements.

The material used shall be one of the products listed on the Maine Department of Transportation's Qualified Products List.

SECTION 715 - LIGHTING MATERIAL

715.02 Steel Conduit Galvanized steel conduit shall be of uniform thickness with scale-free, smooth circular bore to permit cutting of clean, true threads.

Steel conduit and couplings shall be Schedule 40 galvanized steel pipe conforming to the requirements of ASTM A 53. Threaded couplings shall be conduit type permitting the end of conduits to fully abut each other squarely within the coupling. Other fittings for metal conduit, exclusive of bushings shall be threaded malleable iron conforming to the requirements of ASTM A 338 and shall be galvanized in accordance with the requirements of AASHTO M 232.

All bushings shall be threaded, insulated grounding type.

715.03 Non-Metallic Conduit Non-metallic conduit shall be rigid unplasticized polyvinylchloride conduit, suitable for Type II or Type III installations, whichever is required and shall conform to the standards of the NEMA or the UL.

715.04 Prewired Conduit Prewired conduit will no longer be accepted

715.07 Secondary Wiring Secondary wiring cables, including neutrals and grounding conductors, shall be 600 volt cables and shall consist of single conductor, stranded, soft-drawn or annealed copper wire, insulated with flame retardant, moisture and heat resistant thermoplastic material. The cable shall be UL approved and listed as THW, THWN or XHHW.

Wire shall be of sufficient size to allow a maximum voltage drop of 5 percent from source of power to the most remote luminaire. Phase identification shall be made by factory-applied color coding.

Terminal lugs shall be cast copper alloy, solderless, mechanical type.

All conduit connections in above ground junction boxes and light standards shall be made by connector kits, fused or nonfused, as indicated on the plans. Splices for the roadway lighting shall be made by straight through or wye connector kits as required. Where these connector kits cannot be used as verified by the Resident, connections on cables in junction boxes shall be made by splicing as described elsewhere. The connector kits shall be a quick disconnect type. Double connector kits shall be used where there is more than one phase conductor.

Fused "wye" connectors shall be composed of a "wye" line side housing assembled with a load side and fuse terminal housing. The housing shall be formed from water-resistant synthetic rubber. Each housing shall provide a water seal around the cables and when fully assembled shall form a watertight connector.

The interior shall be arranged to receive and retain line side wiring and the fuse contacts. The fuse contacts shall be spring-loaded copper designated for 30 amperes, 600 volts, shall have 90 percent minimum conductivity and shall be suitable for gripping 5 amperes or as designated on the plans, 600 volt cartridge type midget fuse approximately $1\frac{3}{32}$ inch in diameter and $1\frac{1}{2}$ inches long. The contacts shall be fully annealed. The load side conductors shall be connected by crimping and the line side conductors shall be connected with screws. The connector shall be of the non-locking type that will break off under extreme tensile stress leaving no exposed metal contacts on the line side of the connector.

The cable diameter used will determine the size of each housing. The load side housing shall retain the fuse when disconnected.

Non-fused connectors shall be similar to the fused "wye" connectors. The cable diameter will determine the size of each housing of each connector.

Fuses for connectors shall be rated at 5 amperes or as indicated on the plans.

715.08 Luminaire LED fixtures

The luminaire shall be designed for the wattage rating and voltage indicated on the plans and for operating on a multiple circuit. All luminaries shall be new and be the product of the same manufacture

The LED shall have a minimum of 100,000 hours and have a ten year warranty on the fixture and led modules.

Light color temperature shall be 5000-5500 k. Each fixture shall have a minimum of at least 2 separate light sources. Lenses or refractors shall be guaranteed not to yellow for a period of 20 years. Fixtures shall be IES full cutoff. Lm 80 test data shall be supplied in the bid package

The LED fixtures must be equipped with a Smart Driver. The Smart Driver shall provide current regulation for individual light clusters. Current accuracy over the LED operating temperature range must be +/- 3%. It shall have over temperature protection, shut down at critical temperature and resume operation temperature. Under voltage lockout for power off or brownout.

The line side of each fixture/ retrofit must have surge suppression that meets or exceeds ANSI/IEEE C62.41-2002 Category C-.

Led fixtures shall have a lamp lumen depreciation of no more than (LLD) 15% over 52,000 hours, 12 years.

On any contract supplying at least 10 light fixtures 1 additional spare fixture for each 10 fixtures shall be supplied to Maine Department of Transportation.

715.09 Luminaire, Lamp and Ballast for High Mast Lighting Shall conform to 715.08.

The luminaire shall provide ANSI-IES Type I, Type IV, or Type V distributions.

715.10 Photo Electric Control The control shall meet the following minimum requirements:

a. Unit Design The photoelectric unit shall consist of a light sensitive element connected directly to a control relay without intermediate amplifications. The unit shall be zenith sensing type.

b. Housing The photoelectric control shall be housed in a weatherproof housing.

c. Operating Levels The operating levels shall be factory set to turn on at approximately 2.0 foot candles and off at approximately 6.0 foot candles.

d. Supply Voltage The control shall be capable of operation on a supply voltage of 105 to 285 volts.

e. Base The base of the unit shall be provided with a 3-prong, EEI-NEMA standard twist lock plug mounting.

f. Directional Design The control shall be oriented in a northerly direction according to the manufacturer's recommendation.

g. Surge Protection The unit shall have a built-in surge protective device for protection from induced high voltage and follow through currents.

715.11 Service Equipment The service pole or service rack and other entrance equipment shall be as detailed on the plans. All service equipment new or upgraded shall be marked with an appropriate arc flash plaque or decal with the following information :

Flash hazard boundary
Cal/cm² hazard at 18 inches
PPE level
Shock hazard when cover is
Limited approach boundary
Restricted approach boundary
The prohibited approach boundary

This shall be located on the outside of the equipment and shall be visible, weatherproof, and fade resistant, and not easily removed.

All new controller cabinets, traffic signal or lighting, shall be configured to eliminate arc flash. All electrical equipment will be dead front, no open terminals, bus bars, breakers, or exposed terminal strips.

All live parts over 50 volts shall be covered with Lexan or a suitable barrier to eliminate the possibility of an arc flash.

The control cabinet shall be fabricated from cast aluminum, sheet aluminum, galvanized steel, or stainless steel. The following are required:

- a. Hinged cover with weather protected hasp for padlocking. The lock will be provided by others.
- b. Mounting brackets.
- c. Suitable bossed and threaded holes in the case wall for conduit installation.
- d. Independent single pole magnetic trip circuit breakers.
- e. Manual control switch.
- f. Lightning arresters in load and line side - rated 650 Volts RMS, indoor type.

- g. Contactor - One double pole, single throw solenoid (shall be non-mercury) contactor with contacts rated at the voltage and amperes shown on the plans. The coil shall be capable of operating at the voltage shown on the plans, 60 hertz. The contactor shall be normally open unless otherwise specified. The contacts shall be mercury.
- h. Ground rods shall be copperclad steel or galvanized, $\frac{5}{8}$ inch diameter, 8 feet long, complete with ground clamp and square head bolt.

Dry-Type transformers shall be designed for indoor and outdoor installation. The following are required:

- a. 25 KVA rating, 120/240 volt primary, 240/480 volt secondary, single phase 3 wire system, if single phase service is supplied.
- b. Frequency - 60 hertz.
- c. Key-hole mounting slots and lifting groove.
- d. Insulation system for 239°F rise at 104°F ambient.
- e. Wiring compartment located on bottom front of unit. Access through a single cover.
- f. Heat barrier to protect connecting cables.
- g. Connecting leads to extend 6 inches from box and identified with metal tags.
- h. Core and coils to be contained within a non-ventilated weatherproof enclosure.
- i. Conduit knockouts to be located on bottom, back and sides of wiring compartment.
- j. Maximum sound level to be 40 decibels.

Rack-mounted circuit breakers shall be enclosed in NEMA 3R enclosures with rain-tight hubs. The breakers shall be rated for 3 pole, 125 amperes, 600 volt, 4 wire service. Lugs for padlocking shall be supplied.

The service entrance rack shall be constructed as shown on the plan. Minor modifications will be permitted, if approved by the Resident to accommodate variations in equipment dimensions. Lumber shall be as shown on the plan. Bolts and hardware shall be hot-dipped galvanized steel.

715.12 Lowering System for High Mast Lighting Each pole shall be furnished with a mechanical lowering system operated by cables and an electrically operated winch that will permit servicing of the luminaires and associated electrical and mechanical apparatus from the ground. Lowering systems shall permit lowering of the complete assembly, including

luminaires, ballasts, fuses, and other apparatus, which may require periodic inspection or servicing, to a height of 5 feet or less above the pole base plate.

At the top of the pole shaft there shall be mounted a detachable head assembly which remains fixed in position during the raising and lowering operation. The head assembly shall consist of 3 or more symmetrically located fixed support arms, which will carry the weight of the lowering assembly. The fixed head assembly shall incorporate no moving parts except for the necessary pulleys, rollers, or sheaves that guide the lowering cables and electrical cable during the lowering operation of the assembly. The fixed head assembly and luminaire lowering ring shall be galvanized steel. All required pulley rollers or sheaves and associated bearings, bushings and shafts shall be constructed of highly corrosion resistant materials not relying upon plating of the parent material for corrosion protection. All pulleys and rollers attached to the head assembly shall have permanently lubricated bearings or bushings.

All parts of the head assembly shall be protected with covers, screens, shields, as necessary, to prevent entrance of dirt, moisture, ice accumulation, nesting of insects or birds or other contaminants harmful to the operation of the lowering device.

All miscellaneous fittings, fasteners, or hardware shall be fabricated from corrosion resistant materials that do not rely on plating for their corrosion protection.

A lightning rod of approved design shall be attached to the top of each pole and shall be firmly attached to the pole shaft or head assembly to provide good electric bonding to the pole shaft.

The entire luminaire lowering ring assembly shall be raised and lowered by three or more symmetrically placed stainless steel aircraft type cables located inside the pole shaft and extending through the head assembly support arms and attached to the lowering ring. Electrical cable supplying energy from the base of the pole to the luminaire ring shall be rough service mining type cable consisting of 3-600 volt conductors assembled with a messenger into a single cable. No electrical disconnect shall be permitted at the top of the pole. In addition, a stainless steel guide cable or equivalent shall be attached to the inside of the pole shaft to prevent the twisting of the lowering and electrical cables during the raise-lower operation. Each of the stainless steel lowering cables shall be capable of supporting the entire lowering assembly.

When the luminaire ring assembly is in the fully raised position, a mechanism for securing the luminaire ring to the head assembly shall be provided. Such mechanism shall provide support for the lowering ring and shall latch the ring to the head assembly thus permitting the removal of all tension on the cables. All mechanisms shall be designed to provide the operator with a positive means of ascertaining when the raising operation is complete and the ring assembly is in the proper resting position.

The inner portion of the lowering ring shall be equipped with a protective bumper or roller system which will prevent damage to the pole shaft surface and preclude excessive swing during the lowering-raising operation.

Winches shall be operated by an appropriately geared 120 volt electric drive motor with adjustable torque limiter that can be easily hand-transported. One drive motor assembly shall be furnished which will operate all units. A rubber covered heavy-duty type "SO" rated 600 volt cable with connectors shall be provided to test the luminaires when they are in a fully lowered position.

An approved junction box shall be installed in the hand hole of each pole that will accommodate the terminations of the underground cable with the cable in the pole serving the luminaires and to include a 480 volt grounded receptacle.

Secondary lightning arresters shall be rated for 650 volts RMS and shall be designed for outdoor use. The arresters shall be installed in each phase conductor to ground and shall be attached in or on the luminaire lowering device in a location accessible for inspection and servicing when the device is lowered.

SECTION 716 - STRUCTURAL ALUMINUM AND RELATED MATERIAL

716.01 Aluminum Railings

a. Aluminum Extrusions Traffic rails, hand rails, splice bars, and pales brackets shall conform to the requirements of ASTM B 221, Alloy 6061-T6 or 6351-T5. Post and post bases shall conform to the requirements of ASTM B 221, Alloy 6061-T6. Pales shall conform to the requirements of ASTM B429 Alloy 6063-T5. Washers shall conform to the requirements of ASTM B 209, Alloy Alclad 2024-T4.

b. Aluminum Rivets Rivets shall conform to the requirements of ASTM B 316, Alloy 6061-T6 (cold heading). Rivets shall have a button type manufactured head. Self-plugging, aluminum blind fasteners for pale panels shall meet the following requirements: 1) Sleeve-ASTM B 211, Alloy 5056 (Stabilized), 2) Pin - ASTM B 211, Alloy 2017 (Naturally Aged). The driven fastener shall meet the requirements for ultimate shear and tensile strength of Military Specification MIL-R-7885.

c. Miscellaneous Aluminum Parts Rail caps shall be either sand cast or permanent-mold cast and shall conform to the requirements of ASTM B 26 or ASTM B 108, Alloy A 356-T6. All aluminum bars and plates shall conform to the requirements of ASTM B 209, Alloy 6061-T6. Standard structural shapes conform to the requirements of ASTM Specification B 308.

d. Steel Anchor Assembly Steel spacers for post anchors shall conform to the requirements of ASTM A 36. Nuts embedded in concrete shall conform to the requirements of ASTM A 307.

Anchor bolts, exposed nuts and washers shall conform to the requirements of ASTM A 449 or ASTM F 1554, Grade 55 and shall be hot dipped galvanized in accordance with ASTM A 153 or ASTM B 695, Class 50, Type 1.

e. Stainless Steel Parts Cap screws, for fastening clamp bars and set screws for pale-panel brackets shall conform to the requirements of ASTM F 593, Alloy Group 1, Condition CW.

SECTION 717 - ROADSIDE IMPROVEMENT MATERIAL

717.01 Fertilizer Fertilizer shall be commercial fertilizer having available elements in conformity with the standards of the Association of Official Agricultural Chemist. The fertilizer shall be furnished in unopened bags with the weight, contents, and guaranteed analysis shown there on or on a securely attached tag.

(a) Grass Seed Fertilizer shall be

Property	Specification	Breakdown of Property
Nitrogen (N)	23 percent	At least 50 % from slow release sources
Phosphorus (P)	0 percent	No phosphorus
Pottasium (K)	12 percent	40% from potassium magnesium sulphate
Iron (Fe)	1 percent	From iron sulphate

(b) Slow Release Fertilizer tablets - Planting Tablets shall be a long lasting 20-10-5 plus minors ¾ ounce tablet.

(c) Water Soluble Fertilizer shall be:

Property	Specification
Nitrogen	20 percent
Phosphorus	10 percent
Potassium	20 percent

Completely water soluble, non-corrosive, without chlorides or carbonates, and containing a color tracer dye.

717.011 Humic Acid Soil Conditioner

Humic Acid Soil Conditioner shall come from naturally occurring dry granular unaltered oxidized lignite, also called Humate, or equivalent product approved by the department's landscape architect, crushed to a particle size which passes a US standard 7 mesh screen [1/8

inch]. Humate or equivalent product shall have a Humic Acid equivalent content of 70% (by weight).

717.02 Agricultural Ground Limestone Agricultural ground limestone shall have the following mechanical analysis:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
No. 10	100
No. 20	90-100
No. 100	50-100
Total Carbonates	>=80

Agricultural ground limestone may be shipped in containers or in bulk. Packaged material shall be delivered in the manufacturer's standard containers. The containers shall be new and so constructed to assure safe arrival at the site. The net weight of the contents shall not exceed 100 pounds per container. The manufacturer's name, a guarantee analysis, and the net weight shall appear on each container. Bulk shipments shall be accompanied by certificates stating manufacturer's name, weight, and guarantee analysis.

Liquid lime may be substituted for agricultural ground limestone when seeding hydraulically. Liquid lime shall be water soluble and contain the following analysis:

Property	Specification
Elemental Calcium (Ca)	19 percent
Nitrogen (N)	15.5 percent of which 1% is from ammoniac nitrogen and 14.5 /5 is from Nitrate Nitrogen

Liquid lime shall be delivered in the manufacturer's unopened containers. The manufacturer's name, a guaranteed analysis, and the quantity shall appear on each container.

717.03 Seed All seed shall be certified as to mixture, germination, purity, and live seed.

Each variety shall conform to the following:

Property	Specification
Germination	>80 percent
Pure Live Seed	>85 percent
Purity	>85 percent

Weed Seed	<1.0 percent
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All seed shall be from the current year's crop unless recent tests by an approved testing agency demonstrate that older seed meets the above requirements.

Seed Mixtures shall consist of seed proportioned percent by weight as follows:

Method #1 - Park Mixture	
Property	Specification
Creeping Red Fescue	45% +/- 4%
Kentucky Bluegrass	25 +/- 4%
Chewings Fescue	15 +/- 4%
Perennial Ryegrass	10 +/- 2%
Annual Ryegrass	5 +/- 2%

Method #2 - Roadside Mixture #2	
Property	Specification
Red Fescue	35 % +/- 4 %
Sheep Fescue	35 % +/- 4 %
Red Top	5 % +/- 2 %
White Clover	6 % +/- 2 %
Annual Rye	19 % +/- 2 %

Method #3 - Roadside Mixture #3	
Property	Specification
Crown Vetch	50 % +/- 4%
Perennial Lupine	25 % +/- 4%
White Clover	4 % +/- 4%
Perennial Rye Grass	21% +/- 4%

717.04 Mulch

(a) Hay mulch shall consist of long fibered hay, reasonably free from weeds and other undesirable material. No material shall be used which is so wet, decayed or compacted as to inhibit even and uniform spreading. No chopped hay, grass clippings, or other short fibered material shall be used unless directed.

(b) Straw mulch shall consist of long fibered straw derived from oats, wheat, rye or other cultivated grains, reasonable free from weeds and other undesirable material. No material shall be used which is so wet, decayed or compacted as to inhibit even and uniform

spreading. No chopped hay, grass clippings, or other short fibered material shall be used unless directed.

(c) Cellulose fiber mulch shall consist of elongated wood fibers from virgin or recycled sources and post-consumer newsprint. The woods fibers shall be tested to show no lead, asbestos or other heavy metals exceeding EPA toxic levels. Cellulose fiber mulch shall be free of refuse, physical contaminants, and material toxic to plant growth. Cellulose fiber shall not contain more than 30 percent post-consumer newsprint.

(d) Bark mulch shall consist of soft wood bark fragments that have been aged for at least 6 months. Bark mulch shall be free of refuse, physical contaminants, material toxic to plant growth, and reprocessed wood products. Bark mulch shall be a well-graded material conforming to the following:

Property	Specification
pH	4.0 to 8.0
2 inch Sieve Designation	100 percent passing
Soluble Salts Content	<4.0 mmhos/cm

(e) Erosion control mix shall be an organic substance of source separated materials, separated at the point of waste generation that may include; forest residues, bark, paper mill flume grit, stump grindings and aged wood waste. Erosion control mix shall be free of refuse, physical contaminants, material toxic to plant growth, and reprocessed wood products. Erosion control mix may contain rocks less than 4 inches in diameter and shall be a well graded material conforming to the following:

Property	Specification
pH	5.0 to 8.0
6 inch Sieve Designation	100 percent passing
¾ inch Sieve Designation	75 to 85 percent passing
Soluble Salts Content	<4.0 mmhos/cm
Organic Matter	20 to 100 percent, dry weight basis

(f) Stone mulch shall be clean native stone free of refuse, physical contaminants, material toxic to plant growth, and limestone. Stone mulch shall conform to the following:

Sieve Designation	Percentage by Weight Passing Square Mesh Sieves
¾ inch	100
¼ inch	0-10

717.05 Mulch Binder Shall consist of a commercially developed product for the tacking of hay or straw. Binder shall be free of refuse, physical contaminants, material toxic to plant growth, or asphalt. Paper fiber mulch may be used as a binder at the rate of 5 lb/unit. Paper fiber

mulch shall consist of 100 percent post-consumer newsprint processed to be applied hydraulically.

717.061 Erosion Control Blankets Shall consist of a machine produced rolled blanket of biodegradable organic fibers, evenly distributed over the entire area of blanket, of a consistent thickness, sewn into a biodegradable mesh on the top and bottom surface using a cotton blend thread. The blanket shall remain in place when subject to shear stress of 1.55 lb/ft². The blanket shall remain intact until grass is established. See Section 618.10 - Seeding, Maintenance and Acceptance.

717.063 Ground Anchors Shall consist of metal staples or biodegradable stakes as recommended by the manufacturer of the erosion control blanket to be used.

717.07 Herbicide The herbicide shall be an approved chemical registered in the State of Maine for the required treatment.

717.09 Humus Humus shall be an organic substance meeting the following:

Property	Specification
pH	4.5 to 8.0
1 inch Sieve Designation	100 percent passing
Soluble Salts Content	<4.0 mmhos/cm
Organic Matter	35 percent, minimum
Dewar Self Heating	>5 stability

Humus may be a natural peat from sedge, sphagnum or reed origin, or compost from source separated materials that may include leaf and yard trimmings, food scrapes, food processing residues, manure and other agricultural residuals, or biosolids. Humus shall contain no visible admixture of refuse or other physical contaminants or any material toxic to plant growth.

SECTION 718 - TRAFFIC SIGNALS MATERIAL

718.01 Vehicular Signal Indications

a. Vehicular signal heads for traffic signals and flashing beacons shall conform to or exceed the current edition of the ITE "Standard for Adjustable Face Vehicle Traffic Control Signal Heads". Each housing section shall be complete with a one-piece, hinged door mounting for the lens and other parts of the optical system, watertight gaskets, and simple door-locking device. The optical system shall be mounted so that the various parts may be swung open for ready access or removal. The sections shall be interchangeable and constructed so that sections can be removed or added. All new traffic signal (vehicular and/or pedestrian) heads shall have light emitting optical assemblies for all colors.

There shall be a round opening in the top and bottom of each head to receive 1½ inch supporting pipe frame. All parts of the housing, including the doors and end plates shall be of die cast aluminum free from flaws, cracks, blow holes or other imperfections or polycarbonate.

All exposed bolts, screws, hinge pins, and door-locking devices shall be stainless steel. All interior screws and fittings shall be stainless steel or approved nonferrous, corrosion-resistant material.

All gaskets, including door, optical assembly, exclusive of lampholder gaskets, shall be of neoprene. Lampholder gaskets shall be of a material unaffected by heat.

All light emitting diode optical assemblies shall be wired so that a white wire will be connected to the ground and black or colored wire to the terminal of the LED optical assembly. The wires shall in turn be connected to the terminal block mounted inside at the back of the housing. The terminal block shall have sufficient screw type terminals to terminate all field wires and lamp wires with separate screws. The terminals to which field wires are attached shall be permanently identified or the wiring shall be color coded to facilitate fieldwork. Each LED assembly shall be provided with a removable visor hood unless tunnel hoods or louvered hoods are specified on the plans. Hoods for 8 inch sections shall be 7 inches long, hoods for 12 inch sections shall be 9½ inches long.

All heads to be modified shall be retrofitted with light emitting diode optical assemblies for all colors.

When 2 or more vehicular signal heads or a combination of vehicular signal heads and pedestrian signal heads are installed on 1 pole, only 1 conduit riser shall be used. The signal heads shall not be connected together by the use of liquid tight flexible metal conduit and terminal fittings.

All new vehicular signal faces installed at any one intersection shall be of the same make and type.

LED optical assemblies for the 8 inch units shall be 650-lumen minimum initial output, 120 volt, 100,000 hour rated life, clear traffic signal lamps. Lamps for the 12 inch units shall be 1900-lumen minimum initial output, 120 volt, 100,000 hour rated life.

The intensity and distribution of light from each illuminated signal LED optical assembly shall conform to the latest revisions of the ITE "Standard for Adjustable Face Vehicle Traffic Control Signal Heads", and the "Standard for Traffic Signal LED".

b. Programmed Visibility Vehicular Indications The programmed visibility traffic signal vehicular indication shall optically determine the visibility zone of indication without the use of hoods or louvers. The projected signal may be visible or selectively veiled anywhere within 15° of the optical axis.

The signal head shall be adjustable to various angles between 9° above and below horizontal. No indication shall result from external illumination and each indication shall be illuminated separately. The visibility of the signal indication shall be adjustable within the signal head to fit the lane or lanes in which traffic is to be controlled.

The illumination lamp shall be a nominal 150 watt, 115 volt AC, 3 prong, sealed beamed type, having an integral reflector and an average rated life of 6,000 hours. A dimming device shall be provided to reduce the candela at each signal head for nighttime operation to approximately 15 percent of the candlepower for daytime operation.

A circular reflector with a specular inner surface shall mate the lamp to a diffusing element.

An internal imaging surface shall be provided to permit an effective veiling system to be applied as determined by the desired visibility zone. The Contractor shall notify the Resident 48 hours prior to the application of the veiling system. The optical limited-diffuser shall be provided with positive indexing means and shall be composed of heat resistant glass.

The objective lens shall be a high resolution planar incremental lens thermally sealed within a flat laminate of weather resistant acrylic. The lens outline shall be symmetrical. Lens colors shall conform to the latest ITE transmittance and chromaticity standards.

The signal shall be housed in cast aluminum, conforming to the latest ITE alloy and tensile requirements. Each section shall have a sun visor. The cast aluminum shall have a chromate preparatory treatment before the application of green or yellow baked enamel prime and finish. The lens cover and the interior of the case shall be flat black. Hinge and latch pins shall be stainless steel. All access openings shall be sealed with weather resistant rubber gaskets.

The lamp fixture housing shall be readily accessible and lamp replacement shall not require special tools nor necessitate major disassembly.

Electrical connections between the case and lamp holder shall be an interlock assembly that disconnects the lamp holder when open. Number 16 wire shall be used to connect the lamp receptacle to the signal head terminal.

The signal head shall be capable of being mounted to standard 1½ inch fittings as a signal head section, as a multiple section face or in combination with other signal heads.

The signal section shall be provided with a rigid connection that permits tilting from at least 9° above or below the horizontal while maintaining a common vertical line through couplers and conduit. No special tools should be needed for servicing or mounting.

c. All exposed wiring for traffic signals, beacons, solar panels, etc shall be imsa 19-1 rated and be of sufficient size to carry the full load unless approved by the resident engineer

718.02 Pedestrian Signal Indications All pedestrian signal head indications shall be displayed within a rectangular background and shall consist of symbolized messages (Upraised Hand and Walking Person).that shall be at least 6 inches high. All pedestrian heads shall be LED optical assemblies and shall have countdown displays.

LED optical assemblies shall be 1,900-lumen minimum initial output, 120-volt, 100,000 hour rated life.

When 2 or more pedestrian signal heads are installed on 1 wood pole, only 1 conduit riser shall be used. The pedestrian signal heads shall not be connected by the use of liquid tight flexible metal conduit.

Pedestrian signal heads which use fiber optic bundles to form the message shall achieve the color of the messages by filters between the light source and the optical bundles. The legends shall be 5 inches minimum in height with a $\frac{5}{8}$ inch stroke. The housing materials shall conform to the requirements for conventional pedestrian signal housings. The message shall be illuminated by a light source designed to operate on a 120-volt source and shall be rated for 100,000 hour average life. The flashing message shall be accomplished by use of a solid-state flasher required for conventional pedestrian signal indications.

718.03 Signal Mounting All trunnions, brackets, and suspensions used for assembling and mounting signal control faces shall be entirely weather tight. The inside area of the cross section of the tubular arms shall not be less than the inside area of 1½ inch IPS pipe to permit the signal control wires or cable to be inserted through them.

After final adjustment, all vehicular and pedestrian signal heads, regardless of mounting arrangement, shall be fastened by a positive locking device acceptable to the Resident. This device shall prevent any deviation from the position set, but shall allow for readjustment of the signal head later in the same installation or in another installation without the necessity of damaging any part of the signal head.

718.04 Vehicular Loop Detectors Vehicle detectors shall consist of wire loops and self-contained detection equipment capable of registering independently the presence or passage of any vehicle passing over the loop at any speed up to 70 mph and at any temperature between -35 and +165°F. Each loop detector shall contain its own integral power supply and shall operate between 95 VAC and 135 VAC. The input power shall be protected by fuse or resettable circuit breaker.

The detector amplifiers shall be self-tuning, solid state construction except for the output relay. Printed circuit design shall allow the components to be removed and replaced without permanent damage to the printed circuit boards or tracks.

Detector amplifiers installed in a common cabinet shall have a frequency difference and shall not interfere with the operation of other detector amplifiers installed in the same cabinet.

All input and output circuits for each amplifier shall enter by a single connector provided with a threaded shell. All controls, indicator lights, meters, fuseholders, circuit breakers and connectors shall be mounted on the front panel of the detector amplifier. All controls shall be adjustable without the use of tools and the controls shall be clearly and permanently identified.

The detector unit shall show a visible indication of vehicle calls. After a power interruption, the units shall return to normal operation within 30 seconds. If any vehicle stops over a portion of the loop registering a call, the detector shall be capable of detecting additional vehicles traversing the loop after approximately 15 seconds.

Detectors shall detect vehicles by lanes of traffic and shall not detect traffic moving away from the intersection when properly positioned in normal travel lanes. Detection must be positive and not erratic under all actual operating conditions with the exception of storm damage to the detector.

All detectors shall be capable of detecting all four-wheeled vehicles for all lengths of lead-in up to 750 feet for single detection loops and for a combined lead-in length of 750 feet in the case of multiple loops.

Detector loop wire shall be number 14 THWN stranded wire, moisture and heat resistant. Lead-in cable shall conform to the detector manufacturer's recommendations. The wire shall be encapsulated in vinyl tubing over its entire length.

No damage shall occur in the detector if the pavement loop or lead-in becomes short-circuited.

718.05 Microwave Detectors Microwave detectors shall work on an operational frequency of 10.525 GHZ. The detection method shall be microwave with adjustable patterns with a response time of 165 milliseconds and an adjustable hold time of 0.5 to 5 seconds. It shall be powered from 10 VAC to 24 VAC. All contacts shall be form C, 5 amp rated. The detector shall have a fail-safe microprocessor circuit so that if the unit fails it will place the controller in recall on the apparent phase.

718.06 Pedestrian Detectors Pedestrian push button detectors shall be weatherproof and constructed to eliminate the possibility of electrical shock in all weather conditions.

The pedestrian push button switch shall be a phenolic enclosed precision snap-acting type, switching unit, single-pole, double-throw, with screw type terminals, rated 15 amperes at 125 volts, AC and shall have the following characteristics:

- a. The switching unit shall have a stainless steel plunger actuator and shall be provided with U-frame to permit recessed mounting in push button housing.

- b. Where a pedestrian push button is attached to a pole, the housing shall be shaped to fit the curvature of the pole and secured to provide a rigid installation. When required, saddles shall be provided to make a neat fit.

718.07 Controllers All controllers shall be solid state menu driven keyboard units meeting NEMA standards and capable of operating in fixed time, semi-actuated and actuated modes or as designated on the plans. Controller shall have a programmable "EE prom" chip with an internal real-time clock/calendar capable of daily, weekly, and yearly events time programming. The controller shall be designed to provide the number of intervals shown on the plans without any auxiliary equipment. Solid state controllers shall be provided with conflict monitors as specified in Section 718.11.

All equipment inputs, outputs, and terminals shall be identified by the phase designations shown on the plans.

The reliability of the equipment shall be demonstrated by test performance that will confirm that the controller unit, fully wired cabinet, and auxiliary equipment meet the operational and functional requirements of the plans and specifications.

The Contractor shall be responsible for providing all information describing the operation of the equipment necessary to facilitate the completion of the tests. All schematic wiring diagrams of the controllers and auxiliary equipment, all cabinet diagrams and all operation manuals shall be submitted to the Resident at the time the controllers are delivered for testing. These diagrams shall show in detail all circuits and parts. Such parts shown thereon shall be identified by name or number and in such a manner as to be readily interpreted.

The controller shall be delivered with all documentation manuals as per Section 643.17 to the Maine DOT Sign Shop, with all internal connections made and ready for testing. The test will be performed under simulated field loads or manufacturer's design loads, whichever are greater. Testing will be performed by the Department.

The Contractor shall allow 21 days for the testing of each controller, commencing on the day the controller is delivered to the test site, exclusive of the number of days necessary for the Contractor to respond to defect notices and the number of days the Contractor requires to correct the defective equipment.

The test shall consist of not less than five days of continuous, satisfactory operation. If unsatisfactory performance of the controller develops during the test, the Contractor shall remove the defective equipment for repair within five working days after notification, correct the deficiency and the controller shall be retested, until the 5 days of continuous satisfactory operation are obtained. If repeated failures occur, the entire controller may be rejected, requiring the Contractor to submit a new controller for testing. Delays to the contract resulting from unsatisfactory test performance due to continued equipment failure will not be considered as a valid reason to justify extension of the contract time.

The Contractor will be notified when testing of the traffic signal equipment has been completed. It shall be their responsibility to transport the equipment to the work site.

Each traffic controller unit, flasher and all other current interrupting devices shall be equipped with a suitable radio interference suppressor installed at the input power point. Interference suppressors shall be designed to minimize interference in both broadcast and aircraft frequencies. Suppressors shall be designed for 125 percent of the total connected load and shall meet standards of the UL and the EIA.

The type of controller, auxiliary equipment and other operational features shall be as noted on the plans.

All equipment, except pedestrian push buttons, shall be designed to operate on 120 volt, 60 hertz. Operation shall be satisfactory at voltages from 105 to 130. The voltage for pedestrian push buttons shall not exceed 18 volts.

a. Auxiliary Functions All controllers shall be capable of providing flashing operation of the signal lights, as indicated on the plans. Transfer from flashing operation or to flashing operation shall conform to the MUTCD. The clock for auxiliary functions shall be a solid state time clock or module.

If noted on the plans, preempt circuits shall be provided for emergency vehicles and/or railroad crossings. The clearance and preempt indications shall be as noted on the plans. Preempt circuits shall function during stop and go and flashing operation unless otherwise noted. The duration of clearance and preempt intervals shall be adjustable over the range noted on the plans and shall be labeled according to function. The railroad preempt circuit shall be designed to operate as a fail-safe loop through a normally made contact on the railroad's control relay in the railroad's control cabinet. Railroad preemption shall have precedence over all preemption intervals for other purposes.

Actuated vehicle phases and actuated pedestrian phases shall be served in that interval of the cycle indicated on the plans. Time for an actuated interval shall be taken from the non-actuated phase(s) as noted on the plans.

Automatic transfer to or from flashing operation shall conform to the MUTCD. Manual advance of the intervals by use of hand cord control shall cause the controller to advance to the next programmed interval only upon pulse signal from the hand cord circuit, interval timing shall hold the interval for the minimum programmed amber and red clearance intervals.

b. Solid State Controllers The controller unit shall be enclosed in a sheet metal case with protective painted finish, designed to permit easy access to the interior and removal of printed circuit boards and modules without the use of special tools. All program controls, fuses, and indicator lights shall be mounted on the front panel and shall be clearly and

permanently labeled. Modules of unlike function shall be mechanically keyed or electrically interlocked to prevent placement in the wrong location. Each module shall be identified with the symbol shown on the plans with an embossed, color contrasting, plastic label. All components shall be marked for identification compatible with the maintenance manual for the controller unit, including components mounted on printed circuit boards.

When on manual operation, all phases shall be called regardless of vehicle detection and the controller shall be advanced upon pulse from the hand cord circuit, except yellow and red clearance intervals shall be timed for the duration programmed.

An exclusive pedestrian phase shall not extend or recycle until a vehicle phase has been serviced. When on manual operation, all phases shall be called regardless of vehicle detection and the controller shall be advanced upon pulse from the hand cord circuit except yellow and red clearance intervals shall be timed for the duration programmed. Automatic transfer from or to flashing operation shall conform to the MUTCD.

Pedestrian phases shall not be extended by actuations, during the walk or clearance interval. Actuations during the clearance intervals shall be placed in memory.

718.08 Controller Cabinet The traffic signal control equipment shall be enclosed within a dust and moisture-proof aluminum or stainless steel housing with an auxiliary door in door feature. The door hinge pins shall be made of stainless steel. The cabinet shall be installed with the back toward the nearest line of traffic unless otherwise directed by the Resident. All new controller cabinets, traffic signal or lighting, shall be configured to eliminate arc flash. All electrical equipment will be dead front, no open terminals, busbars, breakers, or exposed terminal strips.

All live parts over 50 volts shall be covered with Lexan or a suitable barrier to eliminate the possibility of an arc flash.

The controller cabinet shall be of sufficient size to accommodate all control equipment including temperature control equipment. It shall be designed to be attached to the type of pole indicated in the contract plans. If attached to a traffic signal post, it shall be integral with the post giving the appearance of the signal post passing directly through the cabinet. If the controller cabinet is to be ground mounted, details of the installation will be shown on the plans. Piano type hinges on controller cabinet doors shall be fabricated of stainless steel with a stainless steel hinge pin.

All manual control switches, push button control, flashing switch, signal switch and any other specified switches shall be located to be accessible within the outside door, without exposing the controller mechanism.

The flashing mechanism for flashing beacon installations must be enclosed within a dust and moisture proof aluminum cabinet with a hinged door.

The flasher cabinet shall be of sufficient size to accommodate a 2 circuit solid state flashing mechanism and other necessary equipment. It shall be designed to attach to the type of pole indicated in the contract plans. The flasher cabinet shall be vented to prevent excessive heat build-up.

The locks for the door of the flasher cabinet shall be the type shown on the plans. Two keys shall be furnished with each control lock.

All cabinets housing solid-state traffic signal control equipment shall be provided with a thermostatically controlled ventilating fan and throwaway glass fiber air filters. The electric fan shall have ball or roller bearings and shall have a capacity of 100 ft³ per minute. The ventilating system shall be designed to prevent the entrance of rain, snow, dust, and insects. The fan and vents shall be arranged in such a manner that the air intake is at the cabinet bottom and the exhaust is at the cabinet top. The filter shall be firmly held in place such that cracks and openings are eliminated to ensure that all air is filtered. The fan shall be thermostatically controlled with an adjustable upper limit of 100°F to 140°F and a differential of not more than 10°F.

The locks for the switch compartment door of the controller cabinet shall be the type shown on the plans. The main door shall be a lock of the tumbler type. Two keys shall be furnished with each control lock.

All traffic signal controller cabinets shall be supplied with a convenience outlet, a standard 3-wire grounding duplex receptacle, 20 amp capacity and a lamp socket and 1900 lumen lamp.

A police panel shall be provided behind the auxiliary door and shall contain a switch to select "flash-automatic" function, "automatic-manual" function, "signals on-off" function and a manual control cord. Switch terminals on the rear of the main cabinet door shall be insulated so that no live posts are exposed. The "signal on-off" switch shall allow the signal indications to be de-energized, but power to all other control circuits shall not be disrupted. Switches shall be labeled and rated for load current. Traffic signal controller cabinet main doors over 6 ft² in area shall be provided with a stop to limit door opening to both 90° and 180° ±10°. The stop shall be provided with a catch that can be operated when the door reaches these 2 positions and will hold the door open securely until released. Controller cabinets designated on the plans to be ground mounted shall have a pliable seal composed of caulking compound or mastic placed between the cabinet base and the concrete foundation to prevent dust and dirt from entering the cabinet. The bottom of the controller cabinet shall be mounted on an aluminum non-breakaway transformer base as shown on the plans.

The cabinet shall be supplied with sufficient shelf space for all control units. Receptacles for relays, shelf spaces below all removable control equipment, contactors, switches, fuses, circuit breakers and terminal blocks shall be identified with plastic labels embossed with the symbol used on the plans to indicate the related function.

718.09 Flasher The flasher shall be a two circuit solidstate device with no contact points or moving parts, producing between 50 and 60 flashes per minute with a 50 percent to 67 percent duty cycle. The flasher mechanism shall be mounted on a plug-in base with a plug-in mounting. The flasher relay shall energize the flasher and transfer signal light circuits from the controller unit to the flasher. The flasher shall be capable of breaking and carrying 10 amps on each circuit at 125 volts. All amber indications shall be on one circuit and all red indications shall be on the other circuit. The flasher shall be protected from lightning damage by a device intended for use with solid state equipment. The flashing mechanism shall be independent of the controller unit and shall remain in operation upon shutdown of the controller or removal of the controller unit from the cabinet. The pedestrian indications shall be flashed with a separate solid-state flasher.

718.10 Program Selection The weekly program selection unit shall be capable of automatically supervising the operation of cycle 1, cycle 2, cycle 3, split 1, split 2, split 3, reset 1, reset 2, reset 3, and flashing operation. The weekly program selection unit shall allow selection and/or omission of these functions to be varied on a daily basis. The weekly program selection unit shall enable function transfers to be made as often as 15 minutes.

Solid state devices used to automatically select dial, reset and flashing modes shall meet the applicable functional requirements of mechanical devices. They shall maintain the preset program during power interruption and shall continue timing functions using a reserve power source.

718.11 Contacts and Relays All contacts used in connection with interval indications shall be of pure coin silver or its equivalent and shall be capable of breaking and carrying at least 15 amperes at 120 volts AC. The Contractor is directed to arrange the internal wiring and number of circuits so that the contact rating is not exceeded.

All actuated controllers shall be equipped with external type signal light relays.

Relays shall not be used in connection with any automatic non-flashing red, yellow or green indication in installations having pre-timed electro-mechanical equipment, without the approval of the Resident.

Relays shall be designed for continuous duty. Relays shall be designed to operate at ambient temperatures from -30°F to +158°F.

Each relay shall be mounted on a plug-in base with a plug-in mounting. Coils shall have a power consumption of 10 volt-amperes maximum and shall be designed for continuous duty on 120 volts, AC.

A leakage resistor, which shall permit current to pass through the relay coil if the contacts should remain closed after the coil circuit is opened, shall be installed with each external signal light relay to overcome residual magnetism effects.

All relays shall be of a rating sufficient to carry the electrical loads imposed upon them. A sufficient number shall be provided so that the total load is distributed among the various circuits in such a manner that the rating of each relay is 150 percent of the load.

The monitor unit shall be connected to the field terminals of the signal light circuit to provide protection against conflicting green, yellow or walk indications being simultaneously energized as a result of controller failure, relay or solid state switch failure, short circuited field wiring or other failures.

When a conflict is detected, the monitor unit shall cause the signal system to commence flashing operation; energize the stop-timing circuit of the controller while controller power shall remain on; lock-in flashing operation until manual actuation of the momentary contact reset push button; remove power from the signal light circuit; disable all functions of the "Flash-Automatic" and "Automatic Manual" switches in the police panel.

Each circuit of the solid state switching devices shall have a minimum rating of 1,000 watts for tungsten lamp load at 120 volts, AC. The solid state switching devices shall be plug- in mounted to a base. Solid state switching devices shall be protected from transient voltages and lightning by components especially designed for use with solid state devices.

Circuit breakers shall be approved and listed by the UL. The operating mechanism shall be enclosed and shall be trip-free from operating handle under load and shall be trip-indicating. All circuit breakers shall be quick-make, quick-break on either automatic or manual operation. Contacts shall be silver alloy enclosed in an arc quenching chamber. Overload tripping of breakers shall not be influenced by an ambient temperature range of from 0 to +158°F.

718.12 Conductors The number and size of conductors required in each cable is indicated on the plans. All conductors shall be stranded copper conductors. Multiconductor cables shall conform to the latest revisions of IMSA Specification Number 19-1 or 20-1. The service ground rod shall be 8 foot by 5/8 inch copperclad rod.

The service wiring shall be single conductor number 6 AWG THW stranded copper black insulated and number 6 AWG THW stranded copper white insulated rated 600 volts.

All circuits for the timer and each auxiliary control unit shall terminate in a multiple contact connector. Conductors shall be attached to all pins of the connector and cabled. Conductors of the cable, except spares, shall be fitted with terminal ends compatible with the terminal block and shall have identifying bands. The ends of all spare conductors shall be taped.

SECTION 719 - SIGNING MATERIAL

719.01 Reflective Sheeting The reflective sheeting shall consist of a retro-reflective lens system having a smooth outer surface. The sheeting shall have a precoated adhesive on the back side, protected by an easily removable liner.

The reflective sheeting and its components shall conform to all the requirements of FHWA "Standard Specifications for Construction of Roads and Bridges on Federal Highway Projects", FP-92, Section 718.01. Engineering grade reflective sheeting shall meet the reflective intensity requirements for Type II sheeting as shown in Table 718-1, Section 718.01 - Vehicular Signal Indications, FP-92. High intensity prismatic reflective sheeting shall meet the reflective requirements for Type III prismatic sheeting as shown in Table 718-3, Section 718.01 - Vehicular Signal Indications, FP-92.

Reflective sheeting, used in sign construction, shall have been manufactured within the six months immediately prior to fabrication of each sign. Upon delivery at the job site of each shipment of signs, a letter of certification shall be provided by the Contractor that the reflective sheeting conforms to the requirements contained herein.

Super high intensity fluorescent retro-reflective sheeting, ASTM D 4956 - Type VII, Type VIII, or Type IX (prismatic), is required for all construction signs.

For Type I Guide Signs, all reflective sheeting shall be color matched on each sign unit.

719.02 Demountable High Intensity Prismatic Reflectorized Letters, Numerals, Symbols and Borders Demountable reflectorized letters, numerals, symbols and borders shall consist of cut out high intensity prismatic sheeting, conforming to FHWA, "Standard Specifications for Construction of Roads and Bridges on Federal Highway Projects", 1992, FP-92, Section 718.01(d).

719.03 Aluminum Extrusions The extruded aluminum planks shall be bolted type with dimensions, holes, lengths, and cross sections as detailed on the plans. The extruded aluminum molding for edging of the extruded aluminum sign panels shall be of the cross section as detailed on the plans. Extruded aluminum planks shall conform to ASTM B 221, 606-T6, 6005-T5, or 6061-T6. The extruded aluminum planks and molding shall be free from all corrosion and dirt and the face and edges shall be true, smooth, and free from burrs and breaks.

a. Degreasing Required on aluminum plank by either of the following methods:

1. Vapor Degreasing shall be by total immersion of the plank in a saturated vapor of trichloroethylene or perchloroethylene. Trademark printing shall be removed with lacquer thinner or controlled alkaline cleaning system.

2. Alkaline Degreasing - Planks shall be immersed in a tank containing alkaline solutions, controlled and titrated to the solution manufacturer's specification. Immersion time shall depend upon the amount of soil present and the gauge of the metal.

b. Etching

1. Alkaline Etch The pre-cleaned aluminum surface shall be well etched in an alkaline etching material that is controlled by titration, use time, temperature and

concentration specified by the solution manufacturer and rinsed thoroughly. Smut shall be removed with an acidic, chromium compound type solution as specified by the solution manufacturer and then thoroughly rinsed.

2. Alodine 500 or 1,200 is acceptable.

c. Drying Material may be air-dried or oven dried. Metal shall not be handled between all cleaning and etching operations and the application of Reflective Sheeting, except by device or clean gloves. There shall be no opportunity for metal to be exposed to grease, oils, or other contaminants before application of Reflective Sheeting.

d. Fabrication All fabrication shall be completed before metal degreasing.

719.04 Aluminum Sheets All blanks shall be made of 5052-H38 or 6061-T6 aluminum. The Contractor shall guarantee the material to be free of buckles, warp, dents, cockles, burrs and defects resulting from fabrication.

a. Degreasing Required on sheet aluminum by either of the following methods:

1. Vapor Degreasing Sign blanks shall be totally immersed in a saturated vapor of trichloroethylene or perchloroethylene. Trademark printing shall be removed with lacquer thinner or controlled alkaline cleaning system.

2. Alkaline Degreasing Sign blanks shall be totally immersed in a tank containing alkaline solutions, controlled and titrated to the solution manufacturer's specifications. Immersion time shall depend upon the amount of soil present and the gauge of the metal.

b. Etching

1. Alkaline Etch The pre-cleaned aluminum surface shall be well etched in an alkaline etching material that is controlled by titration, use time, temperature and concentration specified by the solution manufacturer, and rinsed thoroughly. Smut shall be removed with an acidic, chromium compound type solution as specified by the solution manufacturer and then thoroughly rinsed.

2. Alodine 500 or 1,200 is acceptable.

c. Drying Material may be air-dried or oven dried. Metal shall not be handled between all cleaning and etching operation and packaging, except by device or clean gloves. There shall be no opportunity for metal to come in contact with grease, oils, or other contaminants prior to application of the packaging and shipping.

d. Fabrication All fabrication, including shearing, cutting, and punching of holes, shall be completed before metal degreasing. Fabrication of all metal parts shall be accomplished in a uniform and skillful manner. The surface of all sign panels shall be flat.

The minimum sheet thickness shall be 0.08 inches for signs of an area of 12 ft² or less and shall be 0.125 inches for signs over 12 ft² unless otherwise specified.

e. Chromate Treatment Treatment shall be in accordance with ASTM B 449 Class I.

719.05 Plywood The plywood shall conform to the following requirements:

a. Face stock Face veneers shall be Grade A.

b. Core and Cross Veneers Core and crossband veneers shall be Grade B or better and shall be solid jointed.

c. Glue The entire area of each contacting veneer surface shall be bonded with a waterproof adhesive that meets the test requirements for exterior type.

d. Overlay The overlay shall be of the high-density type. It shall be a minimum of 60 lb/1,000 ft² surface, shall be at least 0.009 inches thick, and have a minimum resin content of 40 percent based on the dry weight of the impregnated fiber. It shall consist of at least 2 sheets of resin-impregnated fiber of sufficient resin content to bond itself to the plywood. Manufacturing precautions shall be taken to prevent overlay surfaces from coming into contact with any substance that would inhibit adhesion of paint or reflective sheeting. The overlay shall be natural color.

e. Thickness The thickness of plywood shall be $\frac{5}{8}$ inch.

f. Testing The plastic overlay shall not delaminate from the plywood after being subjected to the exterior boiling test for glue line durability.

719.06 Demountable Reflectorized Delineators Delineators shall be diamond reflectors approximately 3 inch square or shall be rectangular, adhesive coated reflective sheeting permanently adhered to a sheet aluminum backing. All delineators on a project shall be the same type. Single delineators shall be clear or silver-white; double and triple delineators shall be amber.

a. Single delineators shall have one $\frac{1}{4}$ inch square hole for center mounting. Double and triple delineators shall have two $\frac{1}{4}$ inch square mounting holes on the vertical centerline.

Single delineators shall be 3 by 3 inches diamonds with $\frac{3}{4}$ inch radius corners and two $\frac{1}{4}$ inch square mounting holes, 3 inches on center.

Double delineators shall be 3 by 6 inch rectangles with $\frac{3}{4}$ inch radius corners and two $\frac{1}{4}$ inch square mounting holes 5 inches on center.

The aluminum shall be 6061-T6, ASTM B 209 or 6063-T6 or 6005-T5 0.063 inches thick sheet properly degreased and etched or treated with a light, tight amorphous chromate type coating.

The reflective sheeting shall be applied to properly treated base panels with mechanical equipment in a manner specified by the sheeting manufacturer.

b. General Requirements and Packaging. The finished delineators shall show careful workmanship, be free of burrs, scratches or damaged reflective surface.

Delineators shall be packaged in such a manner as to insure their arrival at destination in undamaged condition. Delineators shall not become wet in storage or shipment.

719.07 Assembly and Mounting Hardware - General The attachment of signs shall be in accordance with the contract documents and the appropriate hardware prescribed in this Section. Requests for substitution for all specified material shall be submitted in writing with full documentation, including but not limited to specifications and mill certification reports, enabling the Department to evaluate the proposal promptly.

719.071 Aluminum Planking The bolt assembly required to fasten the extruded aluminum planks together shall conform to the designs used in standard commercial processes for the selected type of extruded aluminum panels. Guidance for bolt hole punching and typical plank-to-plank attachment is provided in the contract documents.

719.072 Overhead Signing Sign panels mounted to independent sign support structures and support structure components mounted to bridges passing over the highway are considered to be overhead signing. Overhead signing shall be mounted on W6 by 9 steel beams conforming to the requirements of ASTM A 992/A 992M, galvanized in accordance with AASHTO M 111, or the same size aluminum beams conforming to ASTM B 221M, alloys and tempers of 6061-T6, 6063-T6 or 6005-T5. These components shall be horizontally spaced a maximum of 5¼ feet on center, extending from the bottom of sign panel to the top. If supplemental signs are included in the contract, these beams will extend from the bottom of the main sign panel to the top of the supplemental sign panel. The maximum distance from the edge of the sign to the center of the W6 by 9 shall not exceed approximately 3¼ feet.

On independent sign support structures, these W6 by 9 beam components shall be fastened to chords with a pair of appropriately sized U-bolts on each side of the web at each fastening location. A similar pair of U-bolt assemblies shall be used in attaching each chord of an overhead component to upright supports. U-bolts for steel support structures shall conform to ASTM A 449, Type 1 or 2. The U-bolt hardware, which includes nuts, flat washers and helical lock washers, shall be galvanized in accordance to AASHTO M 232 (ASTM A 153 or B 695, Class 50, Type 1). Washers shall conform to the requirements of ASTM F 436. The U-bolt material for aluminum support structures, or a combination of steel and aluminum structural components, shall be stainless steel conforming to the requirements of ASTM F 593, alloy group 1, with a minimum yield

strength of 45 ksi. Steel support structures may also utilize stainless steel hardware assemblies as an alternative to galvanized steel. Nuts shall be of the locking type with nylon inserts. Washers shall conform to the requirements of ASTM A 276, Type 302. Flat washers, without helical lock washers, will be acceptable in this stainless steel assembly.

On bridge mounted structures, the fastener configurations shall be depicted in the contract documents.

719.073 Post Clip Hardware For Overhead Signing Signs mounted steel or aluminum W shape beam components shall be attached using post clip hardware as described in this Section as well as the contract documents. Overhead signing shall have post clip assemblies fastened in pairs, one on each side of the web of the W shape beam, at all locations on the backside of the extruded plank panels that provide a groove accommodating a post clip bolt and assembly. Post clips shall be 356-T6 aluminum conforming to the requirements of ASTM B 108. The post clip bolt material for overhead signing shall be stainless steel conforming to the requirements of ASTM A 193/A 193M, AISI Type 304, Grade B8. The post clip bolt nut shall be stainless steel material conforming to the requirements of ASTM A 194/A 194M, AISI, Type 303, Grade 8F and of the locknut type with nylon inserts. Flat washers of these assemblies shall be stainless steel material conforming to the requirements of ASTM A 276, Type 302.

719.074 Post Clip Hardware For Roadside Signing Signs mounted on other than overhead locations may be mounted using aluminum hardware. The aluminum post clips shall be 356T-6 aluminum conforming to the requirements of ASTM B 108. The post clip bolts, washers and nuts shall conform to the requirements of aluminum alloy 2024-T-4 (bolts and washers) and alloy 6061-T6 or 6262-T9 (lock nuts).

719.075 Roadside Signing Aluminum signs mounted on U-channel posts shall be fastened with $\frac{5}{16}$ by $1\frac{1}{2}$ inch stainless steel bolts, washers, and self-locking type nuts. The bolts shall conform to the requirements of ASTM F 593. A washer, either a white nylon or neoprene or stainless steel ASTM F 593 shall be used between the head of the bolt and the face of the sign.

Plywood signs mounted on U-channel posts shall be fastened with $\frac{5}{16}$ by $2\frac{1}{2}$ inch stainless steel cap screws and hex nuts conforming to ASTM F 593, and a washer either white nylon or neoprene or stainless steel ASTM F 593 shall be used between the head of the bolt and the face of the sign.

Delineator assembly hardware shall consist of $\frac{1}{4}$ by $2\frac{1}{4}$ inch stainless steel bolts, washers, and self-locking type nuts. The bolt assembly shall conform to the requirements of ASTM F 593.

SECTION 720 - STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS

720.01 Aluminum Supports Extrusions for aluminum supports shall conform to ASTM B 221 or ASTM B 429, Alloy 6061-T6 or 6063-T6. Castings for use with aluminum supports shall conform to ASTM B 26 or B 108, Alloy 356.0-T6, except that castings for parts having a non-structural application, such as pole caps or bolt covers, may be temper F. The T6 tempers specified for Alloys 6063 and 356.0 shall be obtained by the proper heat treatment of the assemblies after all welding for the base and if required, for the handhole reinforcement has been completed. No welding will be allowed on the shaft other than as required for the attachment of the shoe base, handhole reinforcement and bracket arm. All welding for aluminum light standards shall conform to the current edition of AWS Structural Welding Code - Aluminum D1.2.

The exterior of all parts shall have a satin brushed or satin etched finish. The assemblies shall be free of bulges, dents, and cracks and on external surfaces, discoloration and scratches. The presence of any of these defects or any other imperfection detrimental to strength or appearance may be cause for rejection by the Resident. All assemblies shall be tire wrapped for protection during shipment, storage, and handling.

All ends of shafts open to the weather shall be fitted with a cast-aluminum or formed aluminum cap secured in place with set screws.

All assemblies shall be permanently marked on the edge of the base plate or flange, indicating alloy and temper of base plate/flange and shaft, as well as the diameter and wall thickness of the shaft.

a. Light Standards, Mast Arm Poles, Strain Poles and Dual Purpose Poles Shafts shall be round, tapered and seamless and shall be fabricated as a single continuous unit without splices, except that shafts with a length of over 40 feet may be fabricated with one splice at approximately mid-height and dual purpose poles may be fabricated with a splice immediately above the mast arm attachment. The minimum wall thickness of the shaft shall be 0.188 inches. A hand hole of approximately 25 in² in area, reinforced to maintain the full design strength of the shaft, shall be provided with the handhole center approximately 18 inches above the base. Provisions for internal grounding shall be incorporated in the handhole reinforcement. A hole, fitted with a rubber grommet, shall be provided in the shaft to match the wire-way of the bracket arm or mast arm, except that strain poles shall be provided with a wire inlet as shown on the standard details. Bases shall be shoe type and shall be supplied with suitable covers for the anchor bolts. Anchor bolt covers shall be securely fastened to the base by means of one or more stainless steel Phillips or hex head screws with a minimum size of ¼ inch. Bases shall be welded to the shaft with both an internal and external continuous fillet weld. The use of sleeve type bases or other bases not requiring welding of the shaft to the base and the use of reinforcing sleeves will not be allowed.

Dual-purpose poles shall be provided with a pull wire for the luminaire.

b. Colonial Light Standards Shafts shall be round, tapered and seamless and shall be fabricated as a single continuous unit without splices. The minimum wall thickness shall be 0.125 inches and the minimum diameter at the base shall be 5 inches. The length of the shaft plus the base shall be 14¼ feet, with a 4 inch long by 3 inch OD straight section at the top to accept the pole top luminaire. The entire assembly of pole and base shall be black anodized. Bases shall be handhole type with a handhole of approximately 25 in² in area and equipped with an approved locking device on the handhole cover. The bases shall have internal flanges capable of accepting four ¾ inch anchor bolts, equally spaced on a 10 inch diameter bolt circle. The bases shall be welded to the shaft with both an internal and an external continuous fillet weld.

c. Pedestal Poles Shafts shall be round, tapered and seamless and shall be fabricated as a single continuous unit without splices. The minimum outside diameter at the base shall be 6 inches and the minimum wall thickness shall be ⅛ inch. The length of the pole plus the base shall be 10 feet, except that the length of poles supporting only pedestrian heads shall be 8 feet. Bases shall be transformer type with suitable covers for the anchor bolts. The bases shall be welded to the shaft with an external and an internal fillet weld.

d. Bridge, Cantilever and Butterfly Type Sign Support Structures The configuration of the foundations, bases, shafts, and trusses shall be of the Contractor's design, as approved by the Engineer, and shall use only material as specified above. Sleeve type bases or other bases not requiring welding to the shaft and the use of reinforcing sleeves will not be allowed.

720.02 Aluminum Mast Arm and Bracket Arm Mast arms and bracket arms shall be of the same materials as the matching pole and have a similar finish. Member cross sections shall be either round or elliptical and have a minimum wall thickness of ⅛ inch. Internal diameters, bends, joints, and attachments shall permit internal wiring in the upper member of the arms. Fixtures for attaching the arms to the poles shall be either castings or extrusions, sized to meet the design requirements, and shall be designed to prevent rotation of the arms about the poles. Any mechanical means used to prevent rotation shall completely penetrate both the fixture and the shaft and the use of set screws will not be allowed. All welding for aluminum light standards shall conform to the current edition of AWS Structural Welding Code - Aluminum D1.2.

a. Mast Arms for Signals Mast arms shall be of the tapered tube truss type design, consisting of an upper and a lower member with vertical struts, welded to form an integral unit or single member tapered arm. Mast arms shall be equipped with sturdy signal hangers and/or appropriate tenons for mounting the signal heads and shall have weatherproof wire inlets located close to the suspended signal heads.

b. Bracket Arms for Luminaires Bracket arms shall be of the single member or truss type. Single member type bracket arms shall be of the tapered upsweep design. Truss type bracket arms shall be of a tapered tube design, consisting of an upper and a lower member and a single vertical strut, welded to form an integral unit. Arms shall be equipped with an appropriate tenon for the attachment of the luminaire.

720.03 Steel Supports Tapered shafts for steel supports shall conform to ASTM A 595, Grade A or approved equal. Straight shafts for steel supports shall conform to ASTM A 53, Grade B, ASTM A 500, Grade A and B, or an approved equal. Base plates and flanges shall be fabricated of steel plate conforming to ASTM A 709, Grade 36 or 50 and sized to transmit the full design load of the shaft. Steel shapes shall conform to the requirements of ASTM A 992. Flange chord splice plates and base plates are considered main load carrying members and shall comply with the requirements of Section 713.01 - Structural Steel. All work shall conform to the applicable provisions of Section 504 - Structural Steel.

The interior and exterior of all support structure components shall be hot-dip galvanized in conformance with AASHTO M 111.

Chord flange splice fastener assemblies shall conform to ASTM A 325, Type 1, and galvanized in accordance with AASHTO M 232 (ASTM A 153 or B 695, Class 50, Type 1). Other fastener assemblies shall be as specified in Section 719.07, or as approved by the Engineer.

All ends of shafts open to the weather shall be fitted with an appropriate cast aluminum or galvanized cast iron cap secured in place with stainless steel set screws conforming to the requirements of ASTM F 593.

All assemblies of each structure shall be permanently marked on the edge of the base plate or flange indicating steel specification, type and grade of base plate/flange and shaft, as well as the diameter and wall thickness of the shaft.

a. Light Standards, Mast Arm Poles, Strain Poles and Dual Purpose Poles Shafts shall be round, unless otherwise specified in the contract plans, and either tapered or of uniform cross section and shall be fabricated as a single continuous unit without splices, except that shafts with length over 40 feet may be fabricated with one splice at approximately mid-height and dual purpose poles may be fabricated with a splice immediately above the mast arm attachment. The minimum wall thickness of the shafts shall be number 7 gauge. A hand hole of approximately 25 in² in area, reinforced to maintain the full design strength of the shaft, shall be provided with the hand hole center approximately 18 inches above the base plate. Provisions for internal grounding shall be provided in a location accessible through the hand hole. A hole, fitted with a rubber grommet, shall be provided in the shaft to match the wire-way of the bracket arm or mast arm, except that strain poles shall be provided with a wire inlet as shown on the standard details. On dual-purpose poles, a pull wire shall be provided for the luminaire.

b. Pedestal Poles Shafts shall be 4 inch nominal ID, schedule 40 pipe without splices. The length of the pole plus the base shall be 10 feet, except that the length of poles supporting only pedestrian heads shall be 8 feet.

c. Bridges, Cantilever, and Butterfly Type Sign Support Structures The configuration of the foundations, bases, shafts, and trusses shall be of the Contractor's design as approved by the Resident and shall use only material specified above.

720.04 Steel Mast Arm and Bracket Arm Material for mast arms and bracket arms shall be as specified in Section 720.03. Internal diameters, bents, joints, and attachments shall permit internal wiring in the upper member of the arms. Arms shall be hot dipped galvanized, both inside and outside, in conformance with AASHTO M 111. All work shall conform to the applicable provisions of Section 504 - Structural Steel.

a. Mast Arms for Signals Mast arms may be of the single member or the truss type. Single member type mast arms shall be a single, straight or tapered, round member and may incorporate a maximum of 2 telescopic splices. Truss type mast arms shall be of a tapered design consisting of an upper and a lower member connected by vertical struts welded to form an integral unit. Mast arms shall be equipped with sturdy signal hangers and/or appropriate tenons for mounting the signal heads and shall have weatherproof wire inlets located close to the suspended signal heads.

b. Bracket Arms for Luminaires Bracket arms may be of the single member or the truss type. Single member type bracket arms shall be of the tapered upsweep design. Truss type bracket arms shall be of a tapered design consisting of an upper and lower member connected by a single vertical strut, welded to form an integral unit. Bracket arms shall be equipped with an appropriate tenon for the attachment of the luminaire.

720.05 High Mast Light Standard High mast light standards shall have a cross section that is either round or polygonal with not less than 12 sides, and shall have a uniform taper from the base to the top, except that an expanded base section may be used, if required, to accommodate the electrical and mechanical equipment. All work shall conform to the applicable provisions of Section 504 - Structural Steel.

For unpainted high mast structures, material for the shaft, base and attachments shall conform to the requirements ASTM A 709 Grade 50W or ASTM A 595, Grade C.

The base plate and reinforcing components of high mast poles shall be considered main load carrying members and shall comply with the requirements of Table A, Section 713.01 - Structural Steel. If applicable, the Contractor shall submit a proposed coating specification for approval by the Fabrication Engineer.

A Certificate of Compliance shall be provided for all material in accordance with the requirements of the General Statement of Division 700 - Materials.

720.06 Steel H-beam Posts Steel H-beam Posts shall conform to the requirements of ASTM A 992. All work shall conform to the applicable provisions of Section 504 - Structural Steel. Steel shall be hot-dip galvanized in accordance with AASHTO M 111. All steel hardware for

use with H-beam poles shall be hot-dip galvanized in accordance with AASHTO M 232 (ASTM A 153 or B 695, Class 50, Type 1).

720.07 Anchor Bolts Anchor bolts and nuts supplied for aluminum and/or steel supports shall conform to ASTM A 449, Type 1, or ASTM F 1554, Grade 55, both with a minimum yield strength of 55 ksi. Anchor bolts shall be supplied with 2 heavy hex nuts and 2 hardened washers and unless otherwise specified the anchor bolts shall have a 90° bend with a 6 inch minimum leg length at the lower end. The anchor bolts, nuts and hardened washers shall be hot-dip galvanized in accordance with AASHTO M 232 or AASHTO M 298 Class 50, Type 1. The bolt shall be zinc-coated 12 inches from the exposed end, unless otherwise specified. If the anchor bolts are to be used with breakaway devices incorporating the function of a nut, for example, longitudinally grooved breakaway couplings, nuts or washers will not be required.

Alternate materials, grades, and designs may be used for the anchor bolts subject to approval of the Engineer.

720.08 U-Channel Posts Except as otherwise authorized, Rib Back U-Channel posts for signs of less than 4 ft² in area, shall be fabricated of steel weighing 2.5 lb/ft, and shall not be doubled-up. Aluminum U-channel posts having the same strength characteristics and shape as steel U-channel posts may be used, subject to the approval of the Resident. The steel U-channel posts shall be powder coat green in accordance with AASHTO M 111.

720.09 Wood Ornamental Light Standard Material for wood light standards shall be Western Red Cedar (*Rhuja Plicata*) or other species with equal or better decay resistance, approved by the Resident .

The wood cross section shall be 10 by 10 inches minimum. The mounting height and depth of burial shall be as shown on the plans.

The following items shall be supplied, a 2 inch slipfitter of corrosion resistant material of adequate length to support the luminaire, a sideplowed wire-way covered by a suitable flush filler strip to accommodate three, number 12 conductors, a pair of two-piece plinths for conduit entrance and splice box.

All bolts shall be hot-dip galvanized in accordance with AASHTO M 232 or AASHTO M 298 Class 50, Type 1.

All parts of the wood light standard shall be prefitted and predrilled, holes shall be counterbored to conceal bolts and filled with mastic compound.

The wood light standard surface finish shall be as indicated on the plans.

720.10 Wood Utility Pole Wood Utility poles shall be Douglas Fir or Southern Yellow Pine, conforming to ANSI Standard Specification 05.1 and of the class and length as indicated on the plans.

Poles shall not have more than 180° twist in grain over the full length. Sweep shall be no more than 4 inches. Tops of poles shall be beveled.

Wood Utility poles shall be pressure treated, after fabrication, with creosote, pentachlorophenol (oil borne), pentachlorophenol liquefied petroleum gas solution or copper naphthenate (oil borne), in accordance with the latest AWWA Specification C4 for pressure treated wood poles.

720.11 Mast Arms for Wood Utility Poles All mast arms for wood Utility pole attachment shall be standard 2 inch diameter pipe of specified length and shall include a mast arm head, universal joint, insulated wire inlet, tie rods, cross arm and mounting brackets. The vertical distance between the mast arm and point of attachment of the cross arm shall not be less than 40 percent of the mast arm length. All attachments for mast arm assemblies shall be designed to withstand stresses due to the mast arm and signal weights and wind loads generated by a 100 mph wind.

720.12 Wood Sign Posts Wood sign posts shall be rectangular, straight and sound timber, cut from live growing native spruce, hemlock, cedar or Douglas Fir trees, free from loose knots or other structurally weakening defects of importance, such as shake or holes and heart rot. A tolerance of 1 inch in length and ¼ inch in width or thickness is permitted in the dimensions of rectangular posts. They shall be sawn true and planed 4 sides. Nominal dimensions of rectangular posts shall be as given in the Contract documents.

Breakaway requirements 4 by 6 inch posts shall have two 1½ inch diameter holes drilled perpendicular to traffic, one hole centered at 4 inches above ground level and one centered at 18 inches above ground level (posts shall be installed with the 6 inch length parallel to the roadway); 6 by 6 inch posts shall have two 2 inch diameter holes drilled perpendicular to traffic, one hole centered at 4 inches above ground level and one centered at 18 inches above ground level; 4 by 4 inch posts need not be modified.

When pressure treated wood sign posts are called for on the plans, the wood shall be Yellow Pine, Number 2 or better, .40 CCA, D4 S. The pressure treated wood shall meet AWWA Standard P-5 or Federal Standard TT-W-550. The treating process shall meet Federal Specification TT-W-571, or AWWA Commodity Standards as applicable.

SECTION 721 - BREAKAWAY DEVICES

721.01 Breakaway Devices Breakaway devices shall be capable of supporting all design loads and shall conform in all respects to the requirements of the AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals" and all applicable commentary. Breakaway Support Certification of both breakaway and structural

adequacy shall be provided by the Manufacturer. Design calculations or test data of production samples to support certification shall be provided. Breakaway support components shall provide the same or greater structural strength as the support post or pole utilizing the breakaway device.

SECTION 722 - GEOTEXTILES

722.00 General Geotextile Property Requirements This is a materials specification covering geotextile fabrics for use in stabilization/reinforcement, drainage, erosion control and separation applications. This specification sets forth a set of physical, mechanical and endurance properties that must be met, or exceeded, by the geotextile being manufactured. This specification is intended to assure both good quality and performance of geotextiles but is possibly not adequate for the complete specification in a specific situation. Additional tests, or more restrictive values for the tests indicated, may be necessary under conditions of a particular application. This specification is based on geotextile survivability from installation stresses. This Section is intended for use in conjunction with Section 620 - Geotextiles.

Table 1 provides strength properties for all three geotextile classes. The geotextile shall conform to the properties in Table 1 based on the geotextile class required in the application specific sections which follow.

All Table 1 property values represent Minimum Average Roll Value (MARV) in the weakest principal direction and shall meet or exceed the values stated below. The geotextile properties required for each class are dependent on geotextile elongation.

Table 1 - Geotextile Strength Property Requirements

	Test Method ^a	Class 1 ^b		Class 2 ^b		Class 3 ^b	
		Elongation		Elongation		Elongation	
		<50% ^c	≥□50%	<50% ^c	≥50% ^c	<50% ^c	≥□50% ^c

			c				
Grab Strength - lbs	D 4632	315	202	247	157	180	112
Sewn Seam Strength ^d - lbs	D 4632	283	182	223	142	162	101
Tear Strength - lbs	D 4533	112	79	90 ^e	56	67	40
Puncture Strength - lbs	D 6241	618	433	495	309	371	223
Other	Test Method ^a						
Conformance	D 4759						
Sampling and Testing	D 4354						
Storage and Handling	D 4873						

^aASTM test method.

^bRequired geotextile class is designated in Tables 2, 3, 4, 5, or 6 for the indicated application. The severity of installation conditions for the application generally dictates the required geotextile class. Class 1 is specified for more severe or harsh installation conditions where there is a greater potential for geotextile damage, and Classes 2 and 3 are specified for less severe conditions.

^cAs determined in accordance with ASTM D 4632. The strengths specified in the columns labeled “<50%” and “≥50%” refer to the elongation at which the geotextile material was tested. For example; if a fabric is tested at 15% elongation then it must meet or exceed the minimum strength shown in the “<50%” column. Submittals must include the percent elongation at which the material was tested.

^dWhen sewn seams are required.

^eThe required MARV tear strength for woven monofilament geotextiles is 56 pounds.

722.01 Stabilization/Reinforcement Geotextile The Stabilization/Reinforcement geotextile shall meet the requirements of Table 2. All property values, with the exception of Apparent Opening Size (AOS), represent Minimum Average Roll Value (MARV) in the weakest principal direction and shall meet or exceed the values stated below. Values for AOS represent maximum average roll values.

Both woven and nonwoven geotextiles are acceptable; however no “slit-film” woven fabrics will be permitted and woven geotextiles will not be permitted for subgrade stabilization/reinforcement under pavement structural sections

Table 2 - Stabilization/Reinforcement Geotextile Property Values

Other Properties	Test Method	Requirements
Geotextile class		Class 1 from Table 1 ^a
Permittivity	ASTM D 4491	0.05/sec ^b

Apparent Opening Size (AOS)	ASTM D 4751	0.43mm (maximum)
Ultraviolet Stability (Retained Strength)	ASTM D 4355	50% after 500 hours of exposure

^aDefault geotextile selection. Class 2 or 3 may be specified based on survivability or field testing.

^bPermittivity of the geotextile should be greater than that of the soil. The Resident may also require the permeability of the geotextile to be greater than that of the soil.

Both woven and non-woven geotextiles are acceptable: however, no “slit film” woven fabrics will be permitted and woven geotextiles will not be permitted for subgrade stabilization/reinforcement under pavement structure sections.

722.02 Drainage Geotextile The geotextile shall have property values expressed as Minimum Average Roll Value (MARV) in the weakest principal direction, which meet or exceed the values stated below. Sampling and conformance testing shall be in accordance with ASTM D 4354. Geotextile product acceptance shall be based on ASTM D 4759. Geotextile Storage and Handling requirements shall be based on ASTM D 4873.

Both woven and nonwoven geotextiles are acceptable, however, no "slit-film" woven fabrics will be permitted. The geotextile must meet the following requirements:

Table 3 - Drainage Geotextile Property Values

Other Properties	Test Method	Requirements		
		Percent In-Situ Soil Passing #200 (0.075 mm) ^a		
		<15	15 to 50	>50
Geotextile class		Class 2 from Table 1 ^b		
Permittivity ^{c, d}	ASTM D 4491	0.5/sec	0.2/sec	0.1/sec
Apparent Opening Size (AOS) mm (maximum) ^{c, d}	ASTM D 4751	0.43	0.25	0.22 ^e
Ultraviolet Stability (Retained Strength)	ASTM D 4355	50% after 500 hours of exposure		

^aBased on grain size analysis of in situ soil in accordance with AASHTO T 88.

^bDefault geotextile selection. Class 3 may be specified based on survivability, field testing or specific drain construction requirements.

^cThese default filtration property values are based on predominant particle sizes of the in situ soils. In addition to the default permittivity value, the Resident may require geotextile permeability and/or performance testing based on engineering design for drainage systems in problematic soil environments.

^dSite specific geotextile design should be performed especially if one or more of the following problematic soil environments are encountered: unstable or highly erodable soils such as non-cohesive silts; gap graded soils; alternating sand/silt laminated soils; dispersive clays; and/or rock flour.

^eFor cohesive soils with a plasticity index greater than 7, geotextile maximum average roll value for apparent opening size is 0.30mm.

722.03 Erosion Control Geotextile The Erosion Control geotextile shall meet the requirements of Table 4. All property values, with the exception of Apparent Opening Size (AOS), represent Minimum Average Roll Value (MARV) in the weakest principal direction and shall meet or exceed the values stated below. Values for AOS represent maximum average roll values.

Both woven and non-woven geotextiles are acceptable; however, no "slit-film" woven fabrics will be permitted.

Table 4 - Erosion Control Geotextile Property Values

Other Properties	Test Method	Requirements Percent In-Situ Soil		
		Passing #200 (0.075 mm ^a)		
		<15	15 to 50	>50
Geotextile class:				
Woven monofilament geotextiles		Class 2 from Table 1 (see below)		
All other geotextiles		Class 1 from Table 1 (see below)		
Permittivity ^{a, b}	ASTM D 4491	0.7/sec	0.2/sec	0.1/sec
Apparent Opening Size (AOS) mm (maximum) ^{a, b, c, d}	ASTM D 4751	0.43	0.25	0.22 ^{c, d}
Ultraviolet Stability (Retained Strength)	ASTM D 4355	50% after 500 hours of exposure		

^aBased on grain size analysis of in situ soil in accordance with AASHTO T 88.

^bThese default filtration property values are based on predominant particle sizes of the in situ soils. In addition to the default permittivity value, the Resident may require geotextile permeability and/or performance testing based on engineering design for drainage systems in problematic soil environments.

^cSite specific geotextile design should be performed especially if one or more of the following problematic soil environments are encountered: unstable or highly erodable soils such as non-cohesive silts; gap graded soils; alternating sand/silt laminated soils; dispersive clays; and/or rock flour.

^dFor cohesive soils with a plasticity index greater than 7, geotextile maximum average roll value for apparent opening size is 0.30mm .

Woven Monofilament Erosion Control Geotextiles require Class 2 geotextile class designation. All other Erosion Control Geotextiles require Class 1 geotextile class designation.

The Erosion Control Geotextile class selection is appropriate for conditions of equal or less severity than either of the following:

- a. Armor layer stone weights do not exceed 220 pounds, stone drop height is less than 3 feet, and no aggregate bedding layer is required.
- b. Armor layer stone weighs more than 220 pounds, stone drop height is less than 3 feet, and the geotextile is protected by a 6 inch thick aggregate bedding layer designed to be compatible with the armor layer. More severe applications require an assessment of geotextile survivability based on a field trial section and may require a geotextile of higher strength properties.

The Resident may specify a Class 2 geotextile based on one or more of the following:

- a. The Resident has found Class 2 geotextiles to have sufficient survivability based on field performance of the geotextile.
- b. The Resident has found Class 2 geotextiles to have sufficient survivability based on laboratory testing and visual inspection of a geotextile sample removed from a field test section constructed under anticipated field conditions.
- c. Armor layer stone weighs less than 220 pounds, stone drop height is less than 3 feet, and the geotextile is protected by a 6 inch thick aggregate bedding layer designed to be compatible with the armor layer.
- d. Armor layer stone weights do not exceed 220 pounds and stone is placed with a zero drop height.

Note: 703.25 Stone Fill has stones up to 500 pounds, 703.26 Plain and Hand Laid Riprap has stones up to 200 pounds, 703.27 Stone Blanket has stones up to 3,000 pounds, 703.28 Heavy Riprap has stones up to 1000 pounds.

722.04 Separation Geotextile The Separation geotextile shall meet the requirements of Tables 5 and 6. All property values, with the exception of Apparent Opening Size (AOS), represent Minimum Average Roll Value (MARV) in the weakest principal direction and shall meet or exceed the values stated below. Values for AOS represent maximum average roll values. Both woven and non-woven geotextiles are acceptable.

Table 5 - Separation Geotextile Property Values

Other Properties	Test Method	Requirements
Geotextile class		See Table 6
Permittivity	ASTM D 4491	0.02/sec ^a
Apparent Opening Size (AOS)	ASTM D 4751	0.60 mm(maximum)
Ultraviolet Stability (Retained Strength)	ASTM D 4355	50% after 500 hours of exposure
^a Permittivity of the geotextile should be greater than that of the soil. The Resident may also require the permeability of the geotextile to be greater than that of the soil.		

Table 6 - Required Degree of Survivability as Function of Subgrade Conditions, Construction Equipment, and Lift Thickness^a

Required Degree of Survivability as Function of Subgrade Conditions, Construction Equipment, and Lift Thickness ¹	Low ground pressure equipment ≤3.6 psi	Medium ground pressure equipment >3.6 to ≤7.3 psi	High ground pressure equipment >7.3 psi
Subgrade has been cleared of all obstacles except grass, weeds, leaves, and fine wood debris. Surface is smooth and level so that any shallow depressions and humps do not exceed 18 inches in depth or height. All larger depressions are filled. Alternately, a smooth working table may be placed.	Low (Class 3)	Moderate (Class 2)	High (Class 1)

Subgrade has been cleared of obstacles larger than small to moderate -sized tree limbs and rocks. Tree trunks and stumps should be removed or covered with a partial working table. Depressions and humps should not exceed 18 inches in depth or height. Larger depressions should be filled.	Moderate (Class 2)	High (Class 1)	Very High (Class 1+)
Minimal site preparation is required. Trees may be felled, delimbed, and left in place. Stumps should be cut to project not more than 6 inches above subgrade. Geotextile may be draped directly over the tree trunks, stumps, large depressions and humps, holes, stream channels, and large boulders. Items should be removed only if placing the geotextile and covering material over them will distort the finished roads surface.	High (Class 1)	Very High (Class 1+)	Not Recommended

^aRecommendations are for 6 to 12 inch initial lift thickness. For other initial lift thicknesses:

- 12 to 18 inches: reduce survivability requirement one level;
- 18 to 24 inches: reduce survivability requirement two levels;
- >24 inches: reduce survivability requirement three levels.

For special construction techniques such as prerutting, increase the geotextile survivability requirement one level. Placement of excessive initial cover material thickness may cause bearing failure of the soft subgrade.

Class 1+ (extra high strength) geotextile properties are higher than Class 1 and shall meet the requirements of Table 7.

Table 7- Class 1+ Geotextile Strength Property Requirements

Class 1+ - Extra High Strength Geotextile	Test Method	Class 1+
Grab Strength - lbs	ASTM D 4632	375
Sewn Seam Strength ^d - lbs	ASTM D 4632	335
Tear Strength - lbs	ASTM D 4533	135
Puncture Strength - lbs	ASTM D 6241	745

