

WIN 23657.00
TRACY BRIDGE
(ROUTE 125/PINKHAM
BROOK ROAD)
OVER MEADOW BROOK,
DURHAM,
ANDROSCOGGIN
COUNTY, MAINE

HYDROLOGIC AND HYDRAULIC
REPORT

July 2021

PREPARED FOR

MAINEDOT

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Preliminary Hydrologic and Hydraulic Report

Tracy Bridge (Route 125/Pinkham Brook Road) over Meadow Brook

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1.0 Introduction

The existing Tracy Bridge carries Route 125 (Pinkham Brook Road) over Meadow Brook and is to be replaced and a portion of Meadow Brook is to be realigned. The existing bridge deck is located approximately 27' upstream of the confluence between Meadow Brook and the Androscoggin River. Figure 1 shows the bridge in aerial view.



Figure 1: Existing Bridge

The existing bridge spans over Meadow Brook with a total length of approximately 18' from abutment to abutment and has an out-to-out width of 25.50'. The existing structure features concrete abutments that are parallel to the streamflow. The low chord of elevation of the existing bridge is 81.17', and the existing structure has a hydraulic opening of 251.31 square feet.

The centerline of the roadway will be shifted approximately 16' upstream and will be widened compared to the existing structure, as a result the upstream fascia will extend approximately 35' further upstream. The proposed structure will have a total length of 75', a clear span of 62.48' perpendicular to streamflow and an out-to-out width of 35.40'. The structure will consist of 36" F-beams, topped with 8" Structural concrete and 1" of integral wearing surface. The low chord will be 82.91', approximately 1.74' higher than the existing bridge. The hydraulic opening will increase to 366.15 square feet. The proposed design also includes an upstream stream

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realignment and surrounding bank regrade which was accounted for in the proposed model and is reflected in the cross sections presented in the Appendix.

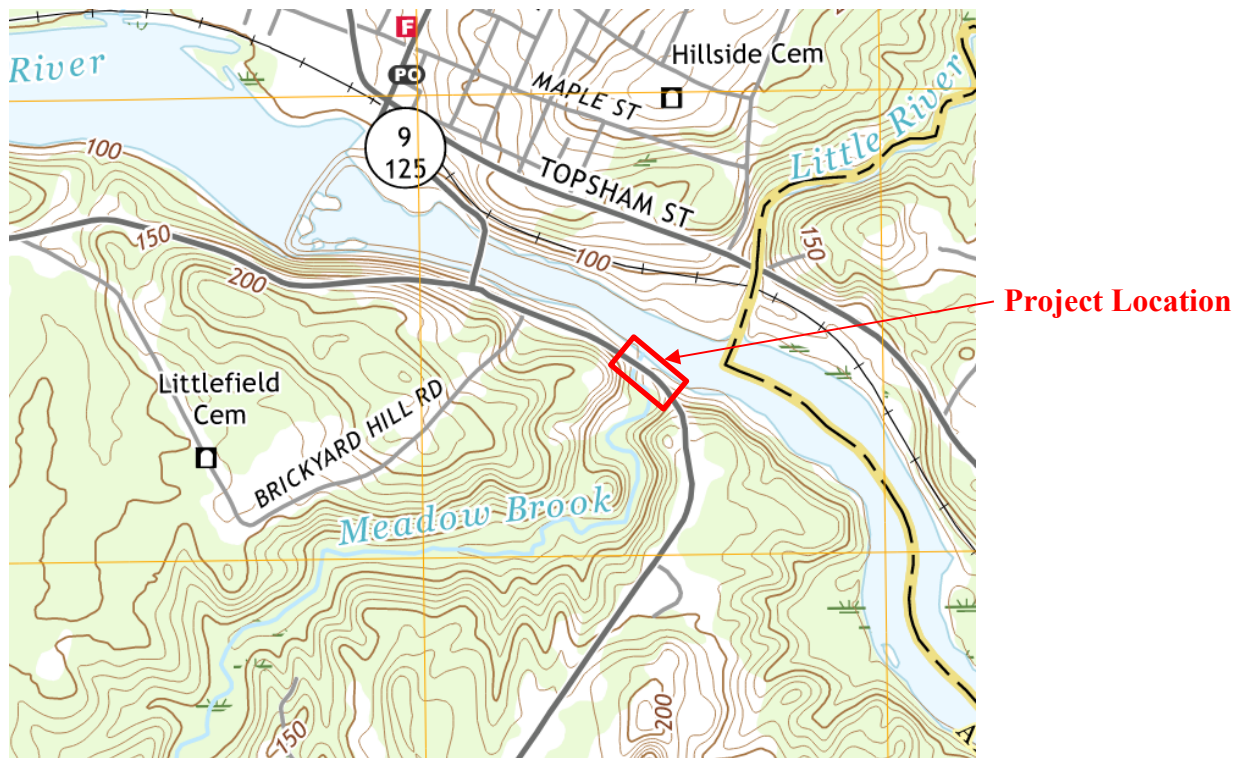


Figure 2: Project Location

The closest upstream structure is located approximately 1.78 miles upstream of the existing bridge. The Androscoggin River is located immediately downstream, and the water surface of the Androscoggin River directly impacts the water surface elevations at the existing bridge.

This report details hydrologic, hydraulic and scour elevations associated with the design of a replacement bridge for Tracy Bridge (Route 125/Pinkham Brook Road) over Meadow Brook. This design report specifically compares the proposed bridge's hydraulic conditions to the existing bridge's hydraulic conditions.

2.0 Review of Existing Data

Existing data was reviewed, including but not limited to:

- Androscoggin Flood Insurance Study by FEMA
- Historical Flood data published by USGS
- Topographic maps
- Existing bridge as-built plans

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The low chord of the existing bridge was confirmed to be 81.17' using site photos and surveyed seat elevations. The flood profiles for the confluence between Meadow Brook and the Androscoggin River were taken from the most recent FEMA Flood Insurance Study (FIS) for Androscoggin County, Volume 2 of 3 (23001CV002A). The Flood profiles from said study showed the 100-year and 500-year events at water surface elevations of 83.50' and 89.80' respectively. This profile is included in the Appendix.

3.0 Hydrology

Flows for Meadow Brook were calculated using USGS Regression equations, and the drainage area for this project was determined to be 3.80 mi². The peak flow calculation sheets can be found in the Appendix D. The peak flow data used is summarized below in Table 1. Flows taken from the previously referenced FIS can also be found in Table 1 for comparison.

Return Interval, years	Peak Flow (cfs)	FIS Flow (cfs)
1.1	55.5	---
10	216.9	197
25	284.5	---
50	327.6	300
100	383.6	349
500	510.2	497

Table 1: Summary of Peak Flows

4.0 Hydraulics

U.S. Army Corps of Engineers' software HEC-RAS version 5.0.7 was used for the hydraulic modeling and analysis of both the existing Tracy Bridge and Meadow Brook, along with the proposed realigned Meadow Brook and redesigned Tracy Bridge. The boundary condition employed the use of known downstream water surface elevations, and the resultant data is displayed in Table 2. Due to no known water surface elevations for the 1.1 year design storm, the boundary conditions for this storm event were set to the normal depth boundary condition. The model was run in the sub-critical flow regime as it is necessary to run in subcritical for the known water surface elevation boundary condition. Ineffective flow areas were modeled due to the contraction and expansion of the bridge opening. Manning's coefficients of 0.04 and 0.075 were used for the channel and the overbanks respectively. These values were taken from the FEMA FIS 23001CV001A and the values can be found in Appendix C. For the bridge modeling approach, the energy method was used for the low flow condition and the pressure and/or weir method was used for the high flow condition. Please note that due to the redesign, the cross sections had to be cut twice for the model. Once along the existing stream alignment and once along the proposed realignment. Due to this the cross section labeled 112.72 in the existing model is homogenous with the cross section labeled 131.70 in the proposed model. It should also be noted that the Androscoggin River was not modeled during this analysis.

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Summary of Hydraulic Data-Tracy Bridge over the Meadow Brook in Durham, Maine	Existing Conditions (RS 112.72)	Proposed Conditions (RS 131.70)
Low Chord	81.17	82.91
Floodplain width at Q100, ft	91.99	235.58
Floodplain width at Q500, ft	146.93	461.54
Width at Banks, ft	21.35	28.27
Headwater Q10, ft	77.72	77.72
Headwater Q50, ft	81.53	81.52
Headwater Q100, ft	83.54	83.52
Headwater Q500, ft	89.82	89.81
Discharge Velocity Q10, fps	0.83	0.69
Discharge Velocity Q50, fps	0.92	0.70
Discharge Velocity Q100, fps	0.94	0.65
Discharge Velocity Q500, fps	0.61	0.42
Ordinary High-Water Elevation (Q1.1), ft	68.10	68.45
Discharge Velocity Q1.1, fps	2.06	1.72
Clearance at Q10, ft	3.45	5.19
Clearance at Q50, ft	0.00	1.39
Clearance at Q100, ft	0.00	0.00
Clearance at Q500, ft	0.00	0.00
Bridge Opening Area, ft ²	251.31	366.15
Flow area at Q100, ft ²	409.69	1132.44
Flow area at Q500, ft ²	1548.14	3057.99

Table 2: Hydraulic Analysis Summary with FEMA FIS Water Surface Elevations

Known water surface elevations of the Androscoggin River during storm events taken from the aforementioned FEMA FIS (23001CV002A) were used to examine the backwater effects which are the controlling water surface factor due to the proximity of the confluence between the Androscoggin River and Meadow Brook. The known water surface elevations are provided in Table 3. As can be seen in Table 2, the resultant water surface elevations from our model are nearly identical to the known water surface elevations presented in the FIS.

Return Interval, years	Water Surface Elevation (ft)
10	77.70
50	81.50
100	83.50
500	89.80

Table 3: Summary of Water Surface Elevations

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5.0 Scour

A scour analysis was conducted using the resultant hydraulic data from the FEMA FIS Water surface elevation model. The D_{50} of the streambed material was assumed to be approximately 0.03936 feet. The D_{50} will be updated as soon as the geotechnical analysis is completed. It was determined that for the 100-year storm the near abutment had 5.78 feet of total scour and the far abutment had 3.13 feet of scour. For the 500-year storm the near abutment had 3.38 feet of scour and the far abutment had 8.65 feet of scour.

	100 - year storm	
	Abutment 1	Abutment 2
Aggradation/ Degradation (ft)	0.00	0.00
Contraction/Expansion Scour (ft)	0.00	0.00
Local Scour (ft)	5.78	3.13
Pressure Flow Scour (ft)	7.16	7.16
TOTAL SCOUR (ft)	12.94	10.29

	500-year storm	
	Abutment 1	Abutment 2
Aggradation/ Degradation (ft)	0.00	0.00
Contraction/Expansion Scour (ft)	0.00	0.00
Local Scour (ft)	3.38	8.65
Pressure Flow Scour (ft)	6.18	6.18
TOTAL SCOUR (ft)	9.56	14.83

Table 4: Scour Executive Summary Table

The project proposed to protect against scour by placing riprap at the abutments. The velocities of the Q100 and Q500 events are acceptable for the placement of plain riprap at the abutments of the proposed structure.

6.0 Summary

The existing Tracy bridge carrying Pinkham Brook Road over Meadow Brook is to be replaced and a portion of Meadow Brook is to be realigned. The existing bridge deck is located 27' upstream of the confluence between Meadow Brook and the Androscoggin River. The proposed bridge provides a greater hydraulic opening while maintaining the existing water surface elevations. The proposed bridge also provides greater clearance.

Appendix A

HEC-RAS Analysis

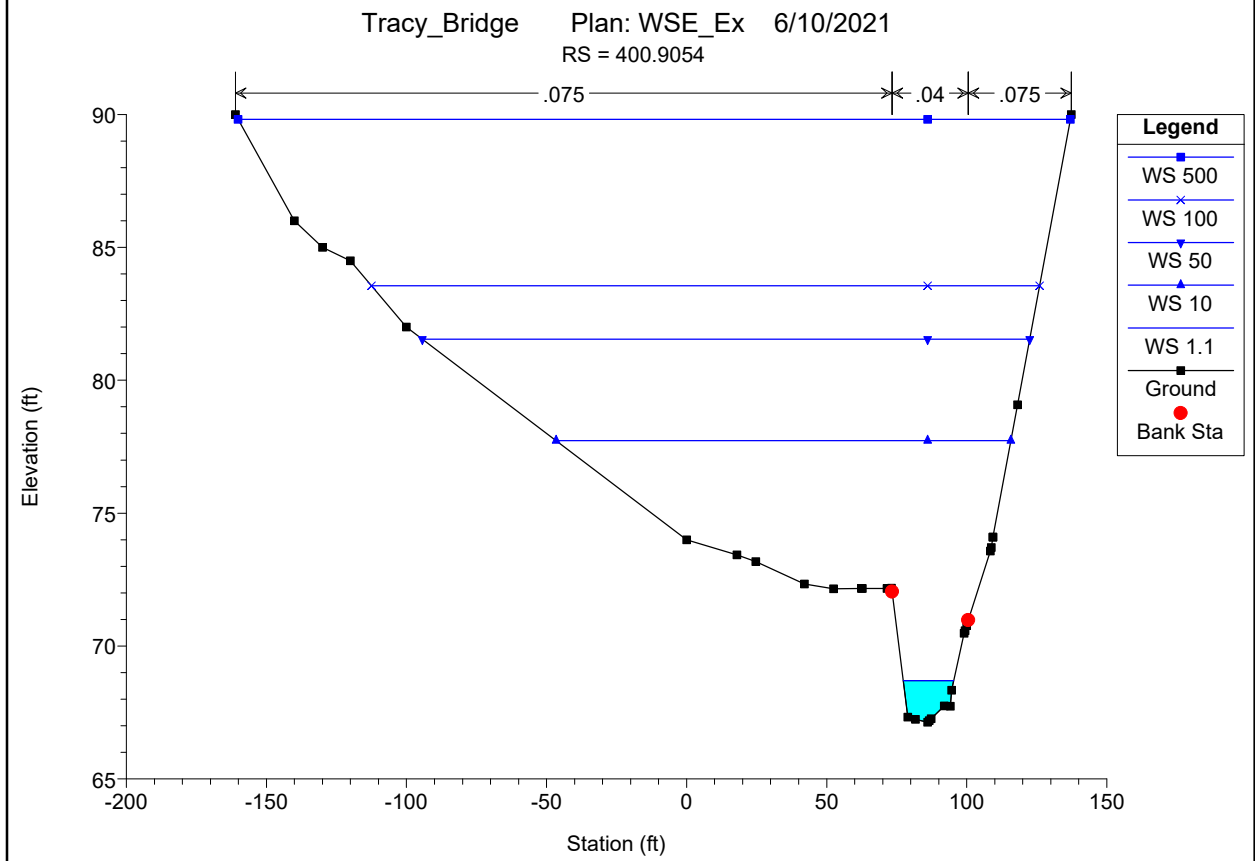
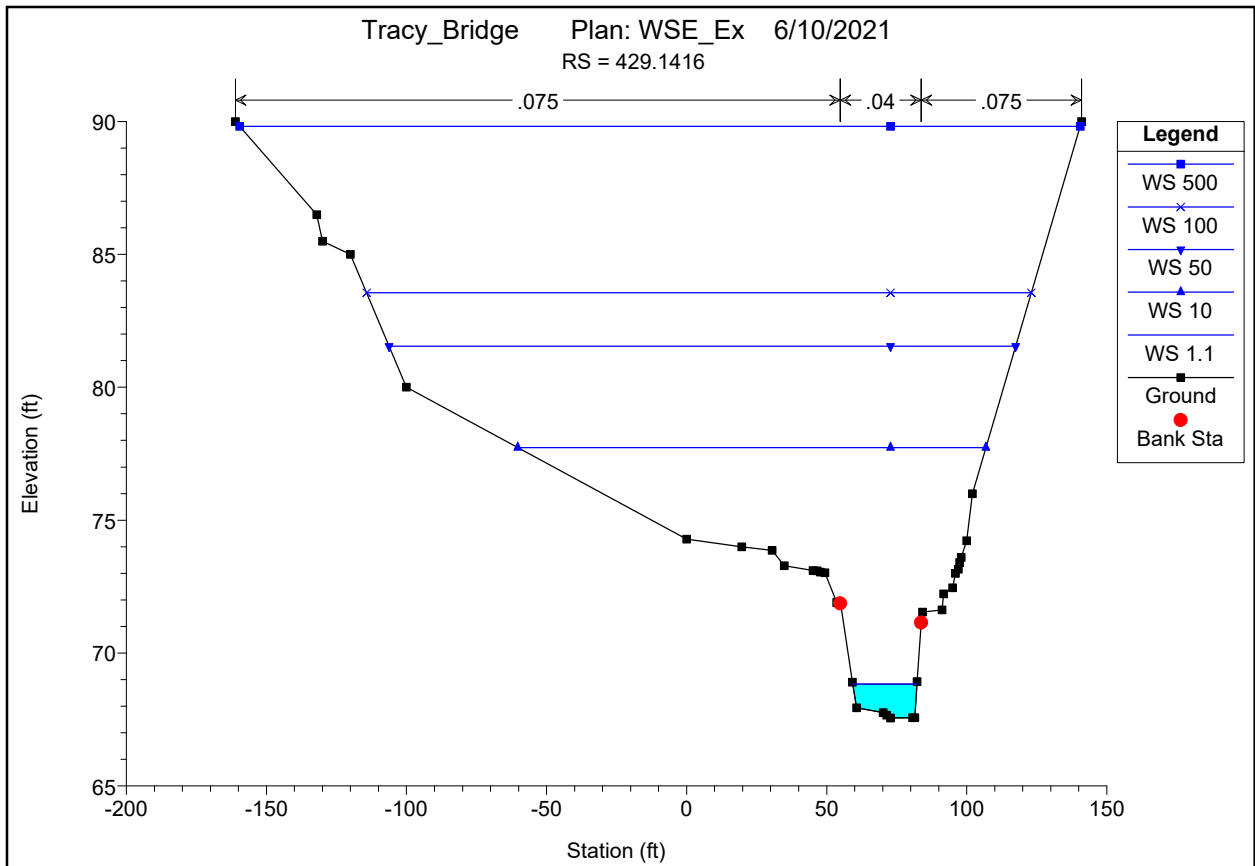
Existing HEC-RAS

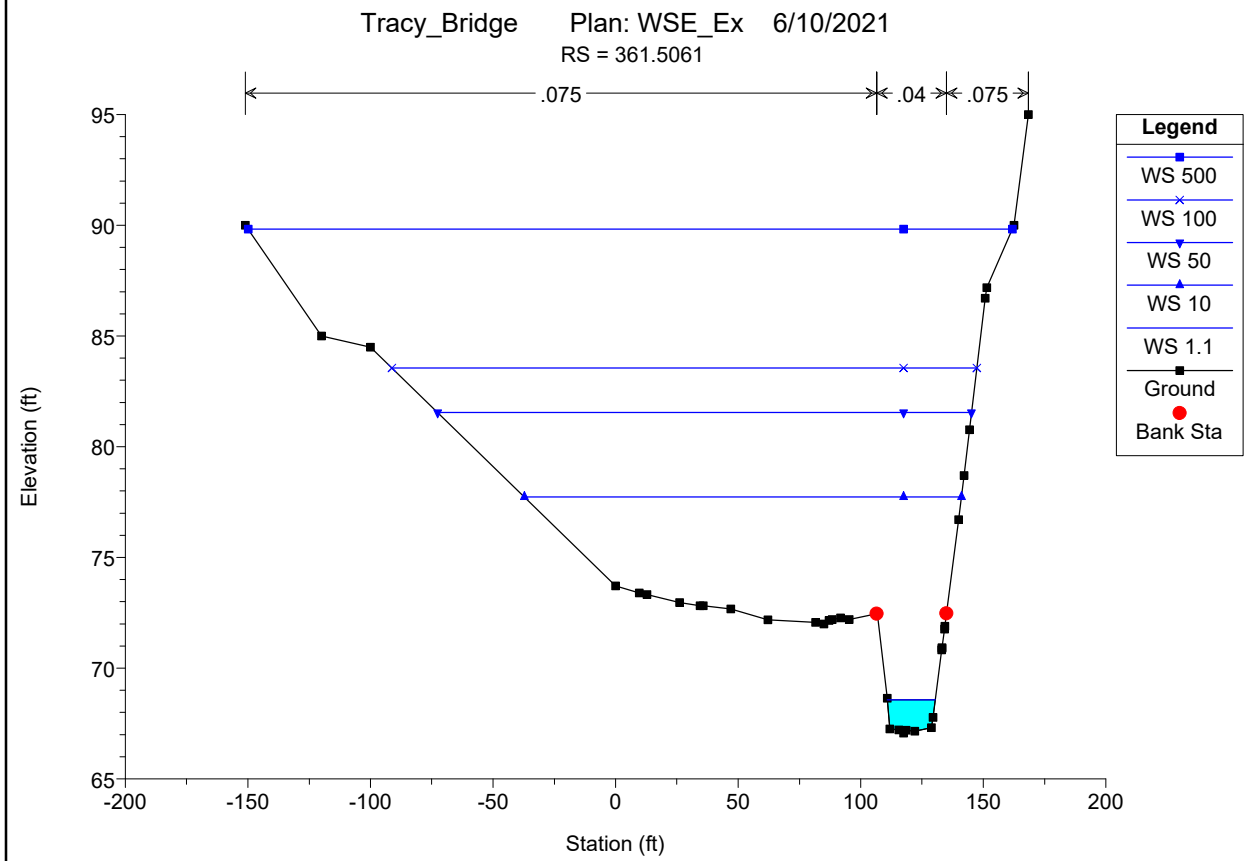
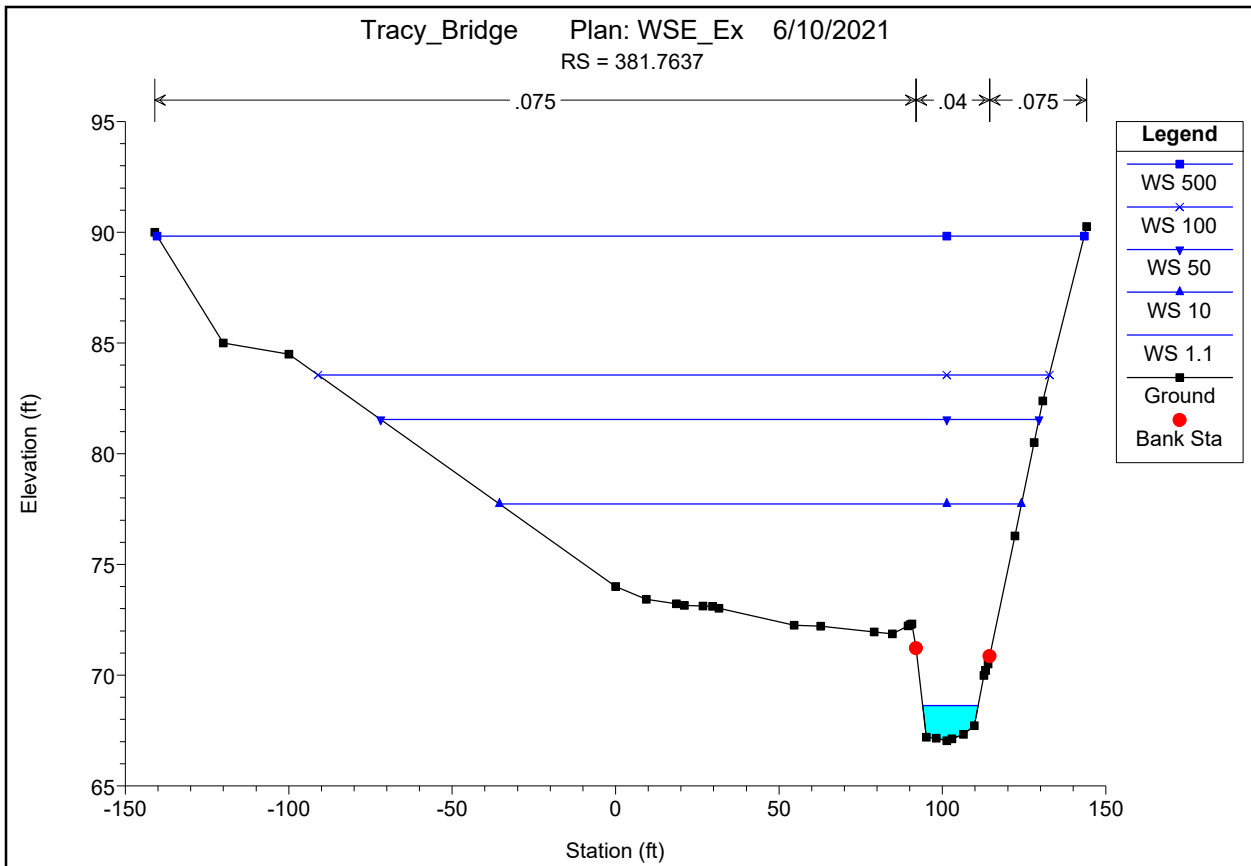
HEC-RAS Plan: WSE-EX River: Meadow_Brook Reach: Meadow_Brook

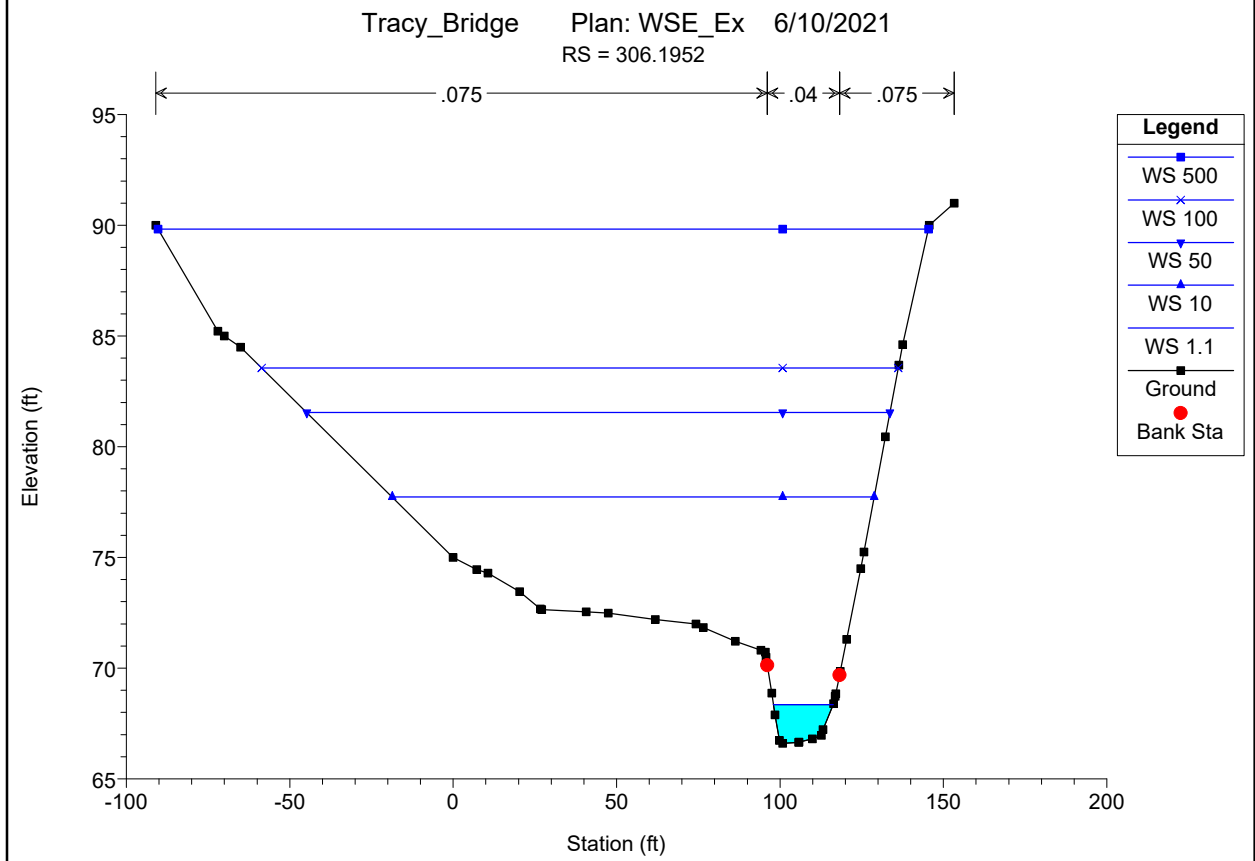
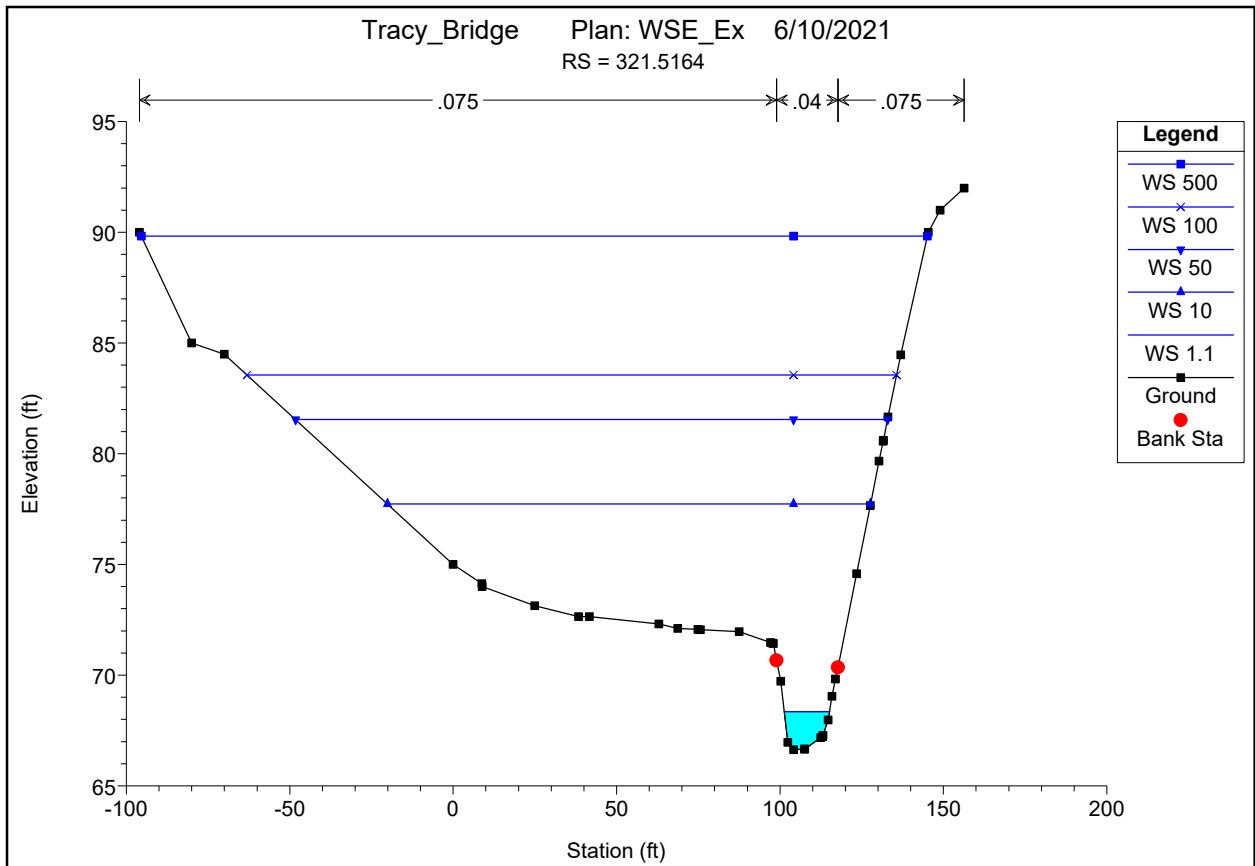
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Meadow_Brook	429.1416	1.1	55.50	67.55	68.83		68.91	0.003623	2.27	24.44	22.90	0.39
Meadow_Brook	429.1416	10	216.90	67.55	77.73		77.73	0.000013	0.56	700.38	167.12	0.03
Meadow_Brook	429.1416	50	327.60	67.55	81.54		81.55	0.000005	0.45	1469.28	223.62	0.02
Meadow_Brook	429.1416	100	383.60	67.55	83.56		83.56	0.000004	0.40	1933.75	237.30	0.02
Meadow_Brook	429.1416	500	510.20	67.55	89.83		89.83	0.000001	0.30	3606.46	300.06	0.01
Meadow_Brook	400.9054	1.1	55.50	67.13	68.70		68.80	0.004038	2.57	21.58	18.03	0.41
Meadow_Brook	400.9054	10	216.90	67.13	77.73		77.73	0.000012	0.53	761.13	162.35	0.03
Meadow_Brook	400.9054	50	327.60	67.13	81.54		81.55	0.000005	0.45	1484.13	216.74	0.02
Meadow_Brook	400.9054	100	383.60	67.13	83.56		83.56	0.000004	0.41	1944.43	238.46	0.02
Meadow_Brook	400.9054	500	510.20	67.13	89.83		89.83	0.000001	0.30	3649.17	297.09	0.01
Meadow_Brook	381.7637	1.1	55.50	67.04	68.62		68.72	0.003788	2.58	21.51	16.92	0.40
Meadow_Brook	381.7637	10	216.90	67.04	77.73		77.73	0.000012	0.54	785.65	159.69	0.03
Meadow_Brook	381.7637	50	327.60	67.04	81.54		81.55	0.000005	0.46	1474.32	201.39	0.02
Meadow_Brook	381.7637	100	383.60	67.04	83.56		83.56	0.000004	0.42	1902.59	223.77	0.02
Meadow_Brook	381.7637	500	510.20	67.04	89.83		89.83	0.000001	0.31	3540.54	283.69	0.01
Meadow_Brook	361.5061	1.1	55.50	67.07	68.57		68.65	0.002807	2.23	24.89	19.56	0.35
Meadow_Brook	361.5061	10	216.90	67.07	77.73		77.73	0.000009	0.45	903.43	178.34	0.03
Meadow_Brook	361.5061	50	327.60	67.07	81.54		81.55	0.000004	0.38	1658.91	217.80	0.02
Meadow_Brook	361.5061	100	383.60	67.07	83.56		83.56	0.000003	0.36	2118.88	238.66	0.02
Meadow_Brook	361.5061	500	510.20	67.07	89.83		89.83	0.000001	0.27	3870.08	311.78	0.01
Meadow_Brook	321.5164	1.1	55.50	66.64	68.36		68.50	0.004978	3.01	18.41	13.87	0.46
Meadow_Brook	321.5164	10	216.90	66.64	77.73		77.73	0.000013	0.58	749.58	147.80	0.03
Meadow_Brook	321.5164	50	327.60	66.64	81.54		81.55	0.000006	0.49	1376.93	181.11	0.02
Meadow_Brook	321.5164	100	383.60	66.64	83.56		83.56	0.000005	0.46	1759.75	198.76	0.02
Meadow_Brook	321.5164	500	510.20	66.64	89.82		89.83	0.000002	0.34	3162.48	240.52	0.01
Meadow_Brook	306.1952	1.1	55.50	66.61	68.35		68.42	0.002594	2.24	24.75	18.30	0.34
Meadow_Brook	306.1952	10	216.90	66.61	77.73		77.73	0.000010	0.53	793.08	147.47	0.03
Meadow_Brook	306.1952	50	327.60	66.61	81.54		81.54	0.000005	0.47	1414.71	178.42	0.02
Meadow_Brook	306.1952	100	383.60	66.61	83.56		83.56	0.000004	0.44	1790.78	194.77	0.02
Meadow_Brook	306.1952	500	510.20	66.61	89.82		89.83	0.000001	0.34	3153.84	235.73	0.01
Meadow_Brook	286.9647	1.1	55.50	66.23	68.36		68.39	0.000540	1.22	45.52	25.66	0.16
Meadow_Brook	286.9647	10	216.90	66.23	77.73		77.73	0.000008	0.46	807.23	144.14	0.02
Meadow_Brook	286.9647	50	327.60	66.23	81.54		81.54	0.000004	0.43	1419.88	177.05	0.02
Meadow_Brook	286.9647	100	383.60	66.23	83.56		83.56	0.000003	0.41	1794.25	194.44	0.02
Meadow_Brook	286.9647	500	510.20	66.23	89.82		89.83	0.000001	0.32	3186.36	253.43	0.01
Meadow_Brook	265.7286	1.1	55.50	66.07	68.32		68.37	0.001089	1.71	32.49	18.11	0.22
Meadow_Brook	265.7286	10	216.90	66.07	77.73		77.73	0.000012	0.57	708.31	134.05	0.03
Meadow_Brook	265.7286	50	327.60	66.07	81.54		81.54	0.000006	0.50	1280.87	166.08	0.02
Meadow_Brook	265.7286	100	383.60	66.07	83.56		83.56	0.000005	0.47	1632.69	183.10	0.02
Meadow_Brook	265.7286	500	510.20	66.07	89.82		89.83	0.000002	0.36	2936.45	222.67	0.01
Meadow_Brook	225.3258	1.1	55.50	66.55	68.24		68.31	0.002118	2.06	26.95	19.26	0.31
Meadow_Brook	225.3258	10	216.90	66.55	77.73		77.73	0.000013	0.57	642.76	119.92	0.03
Meadow_Brook	225.3258	50	327.60	66.55	81.54		81.54	0.000007	0.52	1165.13	153.65	0.02
Meadow_Brook	225.3258	100	383.60	66.55	83.56		83.56	0.000005	0.49	1494.42	173.25	0.02
Meadow_Brook	225.3258	500	510.20	66.55	89.82		89.83	0.000002	0.37	2690.86	208.84	0.01
Meadow_Brook	195.1151	1.1	55.50	66.12	68.22		68.26	0.000918	1.53	36.33	21.99	0.21
Meadow_Brook	195.1151	10	216.90	66.12	77.73		77.73	0.000010	0.55	614.85	96.69	0.03
Meadow_Brook	195.1151	50	327.60	66.12	81.54		81.54	0.000007	0.54	1032.04	123.39	0.03
Meadow_Brook	195.1151	100	383.60	66.12	83.56		83.56	0.000006	0.54	1303.02	149.80	0.02
Meadow_Brook	195.1151	500	510.20	66.12	89.82		89.83	0.000002	0.42	2460.54	208.73	0.02
Meadow_Brook	171.7323	1.1	55.50	65.30	68.22		68.24	0.000358	1.07	51.89	26.29	0.13
Meadow_Brook	171.7323	10	216.90	65.30	77.72		77.73	0.000011	0.58	493.55	72.81	0.03
Meadow_Brook	171.7323	50	327.60	65.30	81.54		81.54	0.000008	0.59	814.47	94.55	0.03
Meadow_Brook	171.7323	100	383.60	65.30	83.56		83.56	0.000007	0.59	1016.76	109.86	0.03
Meadow_Brook	171.7323	500	510.20	65.30	89.82		89.83	0.000003	0.49	1877.70	160.29	0.02
Meadow_Brook	151.9101	1.1	55.50	65.39	68.19	66.89	68.23	0.001037	1.67	33.14	18.86	0.22
Meadow_Brook	151.9101	10	216.90	65.39	77.72	68.01	77.73	0.000021	0.78	320.14	69.19	0.04
Meadow_Brook	151.9101	50	327.60	65.39	81.53	68.56	81.54	0.000016	0.84	455.18	87.72	0.04
Meadow_Brook	151.9101	100	383.60	65.39	83.55	68.82	83.56	0.000014	0.85	526.53	94.78	0.04
Meadow_Brook	151.9101	500	510.20	65.39	89.82	69.30	89.83	0.000004	0.56	1737.17	164.22	0.02
Meadow_Brook	112.7206	1.1	55.50	66.32	68.10	67.32	68.17	0.002209	2.06	26.95	20.19	0.31
Meadow_Brook	112.7206	10	216.90	66.32	77.72	68.32	77.73	0.000025	0.83	262.79	55.25	0.05
Meadow_Brook	112.7206	50	327.60	66.32	81.53	68.82	81.54	0.000020	0.92	358.90	71.31	0.04
Meadow_Brook	112.7206	100	383.60	66.32	83.54	69.03	83.56	0.000018	0.94	409.69	91.99	0.04
Meadow_Brook	112.7206	500	510.20	66.32	89.82	69.48	89.82	0.000005	0.61	1548.14	146.93	0.02
Meadow_Brook	80.1124	1.1	55.50	66.13	67.87	67.43	68.04	0.006942	3.32	16.74	14.33	0.54
Meadow_Brook	80.1124	10	216.90	66.13	77.71	68.66	77.73	0.000040	1.07	203.47	48.38	0.06
Meadow_Brook	80.1124	50	327.60	66.13	81.52	69.24	81.54	0.000033	1.18	277.47	59.53	0.06

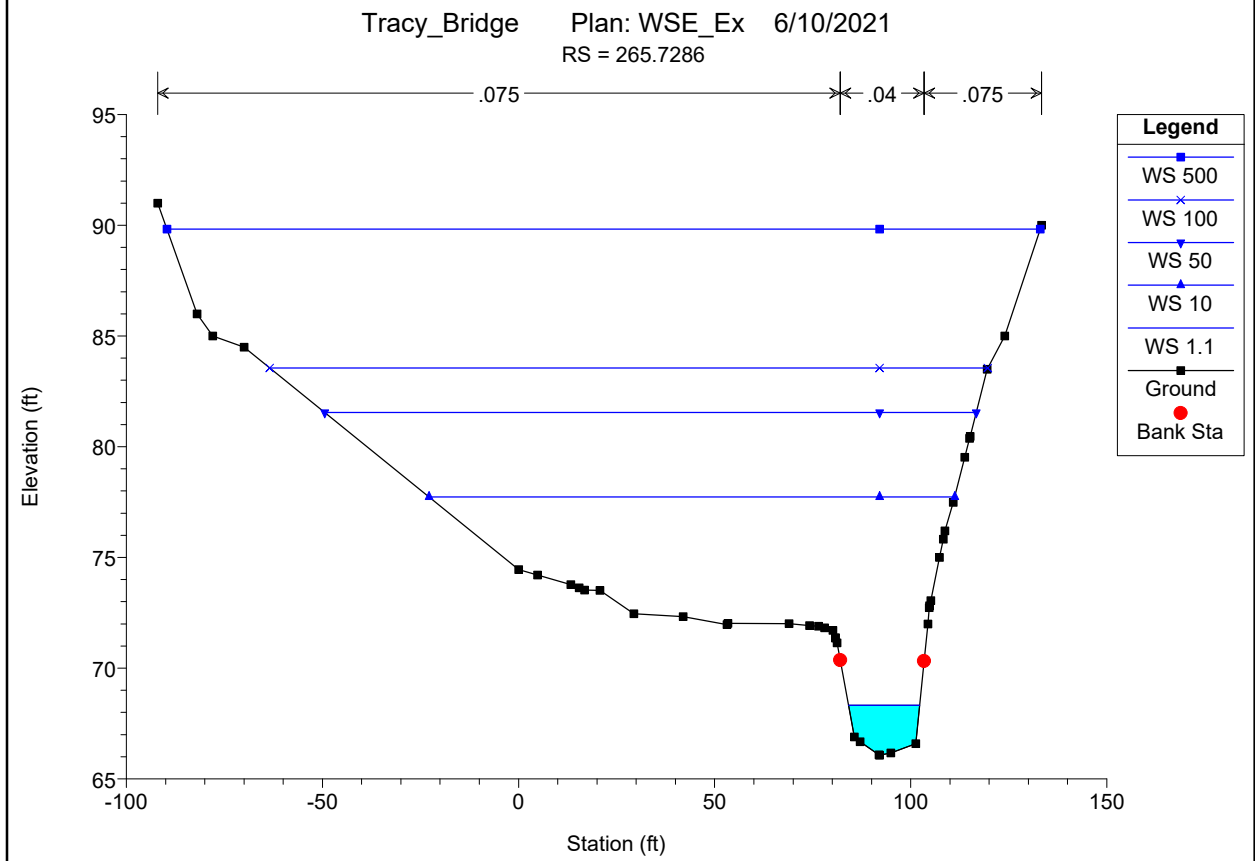
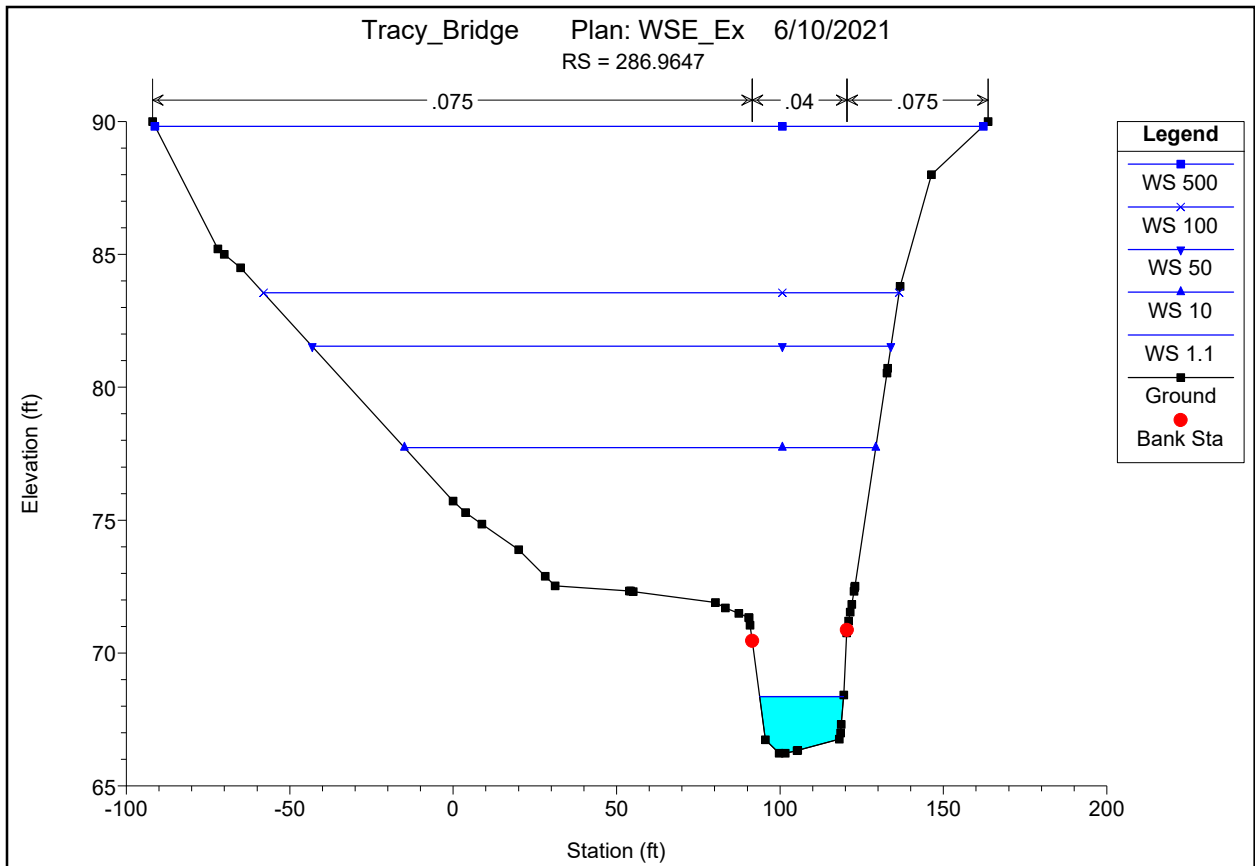
HEC-RAS Plan: WSE-EX River: Meadow_Brook Reach: Meadow_Brook (Continued)

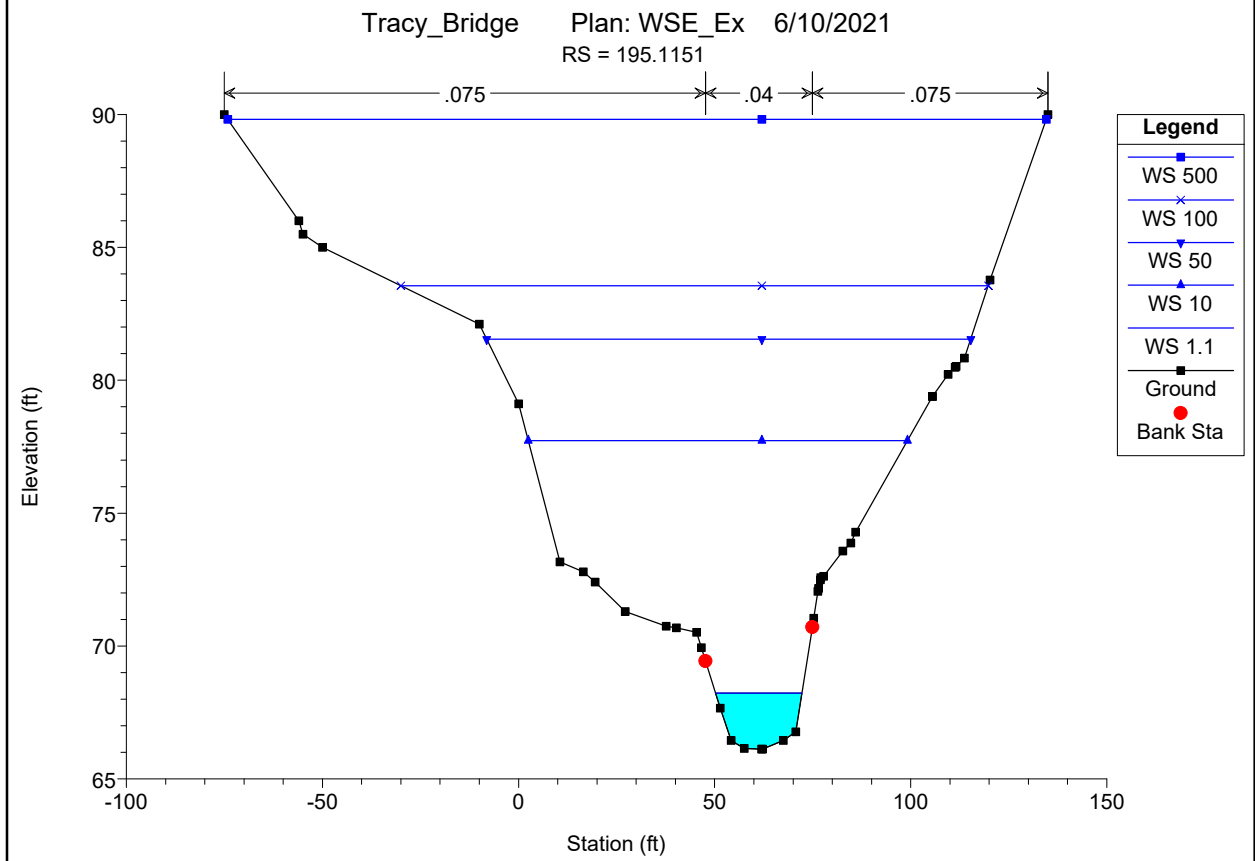
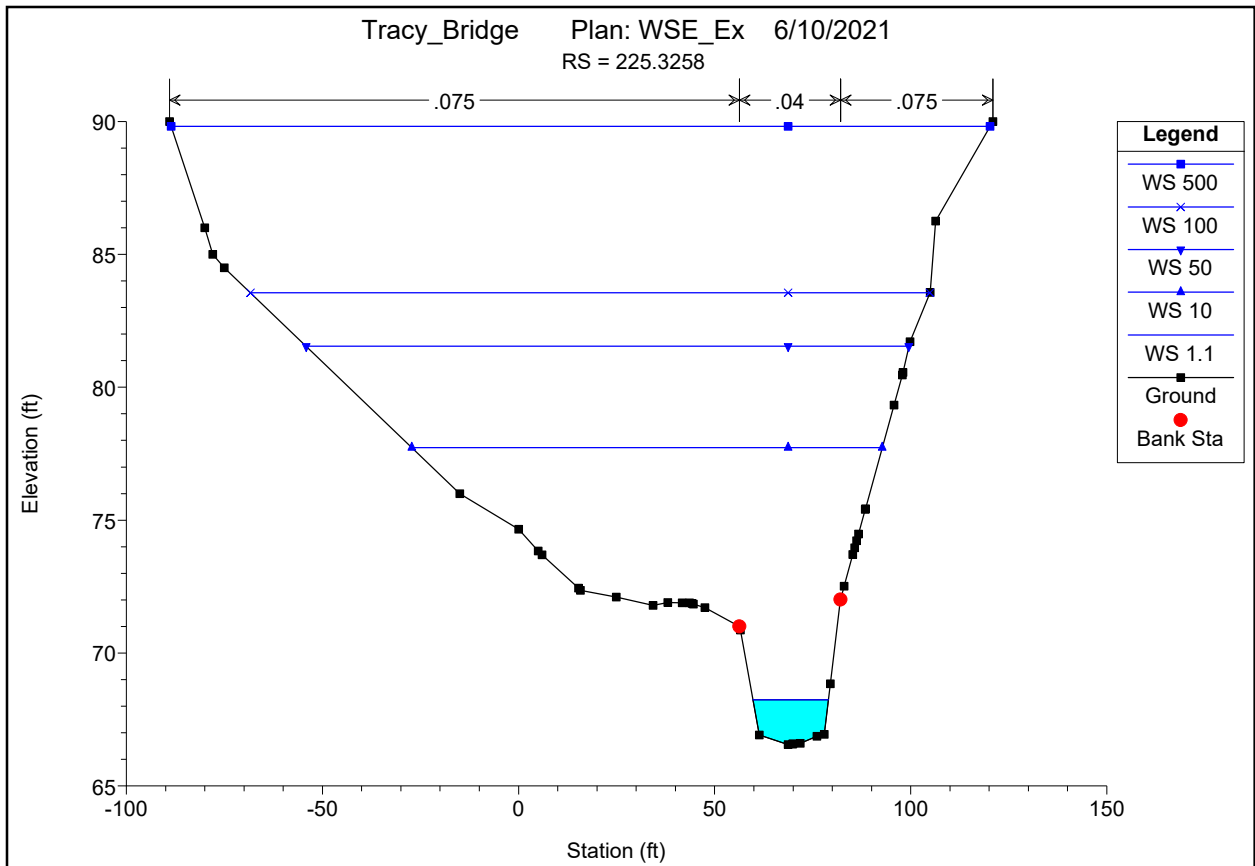
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Meadow_Brook	80.1124	100	383.60	66.13	83.53	69.49	83.56	0.000029	1.21	316.59	79.26	0.05
Meadow_Brook	80.1124	500	510.20	66.13	89.82	70.01	89.82	0.000006	0.69	1642.33	232.97	0.03
Meadow_Brook	68.2341	1.1	55.50	65.93	67.81	67.25	67.96	0.005000	3.07	18.08	15.61	0.48
Meadow_Brook	68.2341	10	216.90	65.93	77.70	68.50	77.72	0.000045	1.18	195.28	41.48	0.06
Meadow_Brook	68.2341	50	327.60	65.93	81.51	69.10	81.54	0.000038	1.31	267.36	50.31	0.06
Meadow_Brook	68.2341	100	383.60	65.93	83.53	69.38	83.55	0.000033	1.34	305.47	70.46	0.06
Meadow_Brook	68.2341	500	510.20	65.93	89.82	69.93	89.82	0.000006	0.69	1582.34	239.39	0.03
Meadow_Brook	35.30		Bridge									
Meadow_Brook	14.2816	1.1	55.50	64.89	66.41	66.26	66.73	0.016656	4.49	12.35	12.69	0.80
Meadow_Brook	14.2816	10	216.90	64.89	77.70	67.53	77.72	0.000075	1.19	191.26	23.98	0.06
Meadow_Brook	14.2816	50	327.60	64.89	81.50	68.14	81.52	0.000065	1.34	263.08	40.62	0.06
Meadow_Brook	14.2816	100	383.60	64.89	83.50	68.41	83.53	0.000059	1.38	300.88	64.93	0.06
Meadow_Brook	14.2816	500	510.20	64.89	89.81	69.00	89.82	0.000017	0.90	1038.57	104.26	0.03
Meadow_Brook	6.9889	1.1	55.50	64.55	66.40	66.06	66.57	0.008207	3.35	16.58	17.41	0.59
Meadow_Brook	6.9889	10	216.90	64.55	77.70	67.16	77.72	0.000029	1.03	209.90	27.63	0.05
Meadow_Brook	6.9889	50	327.60	64.55	81.50	67.71	81.52	0.000027	1.19	275.07	27.63	0.05
Meadow_Brook	6.9889	100	383.60	64.55	83.50	67.96	83.52	0.000025	1.24	309.37	27.63	0.05
Meadow_Brook	6.9889	500	510.20	64.55	89.80	68.48	89.82	0.000013	1.08	642.52	27.63	0.04

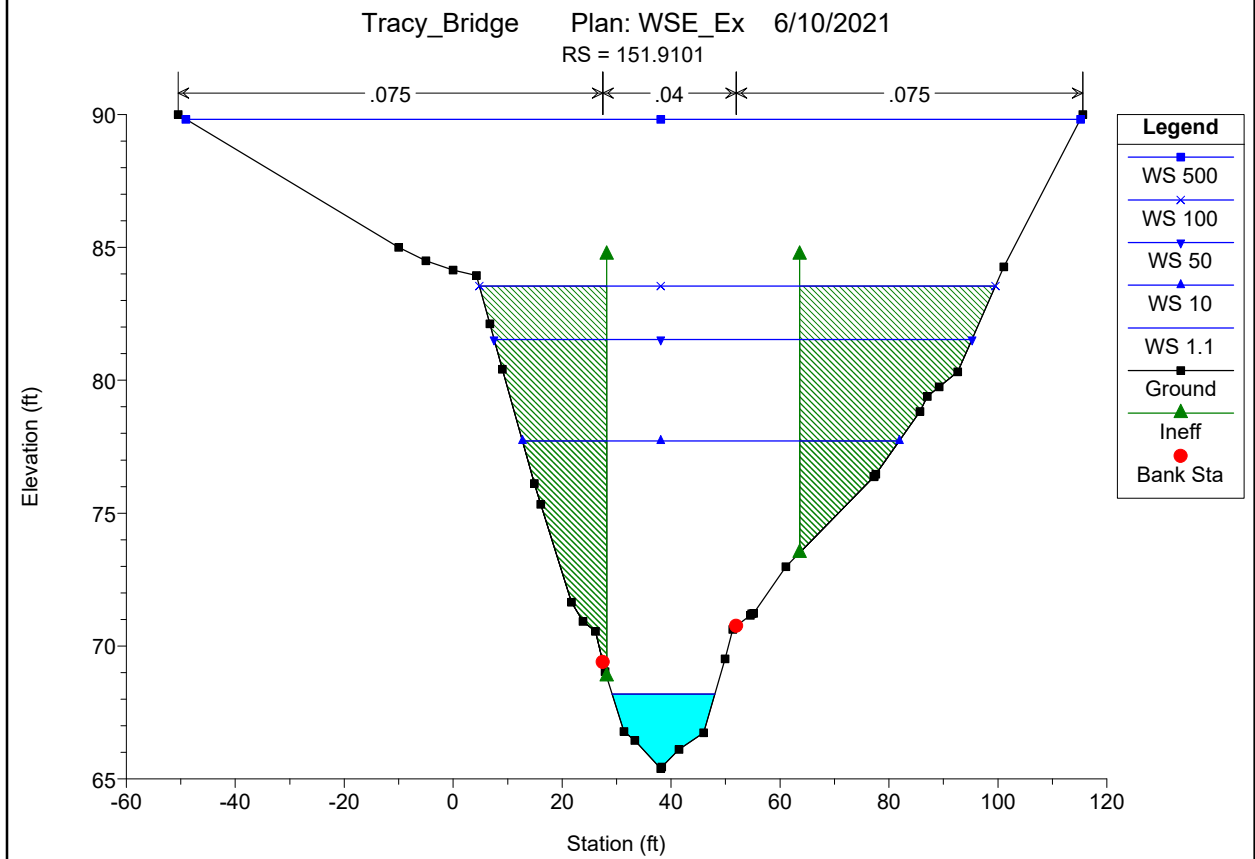
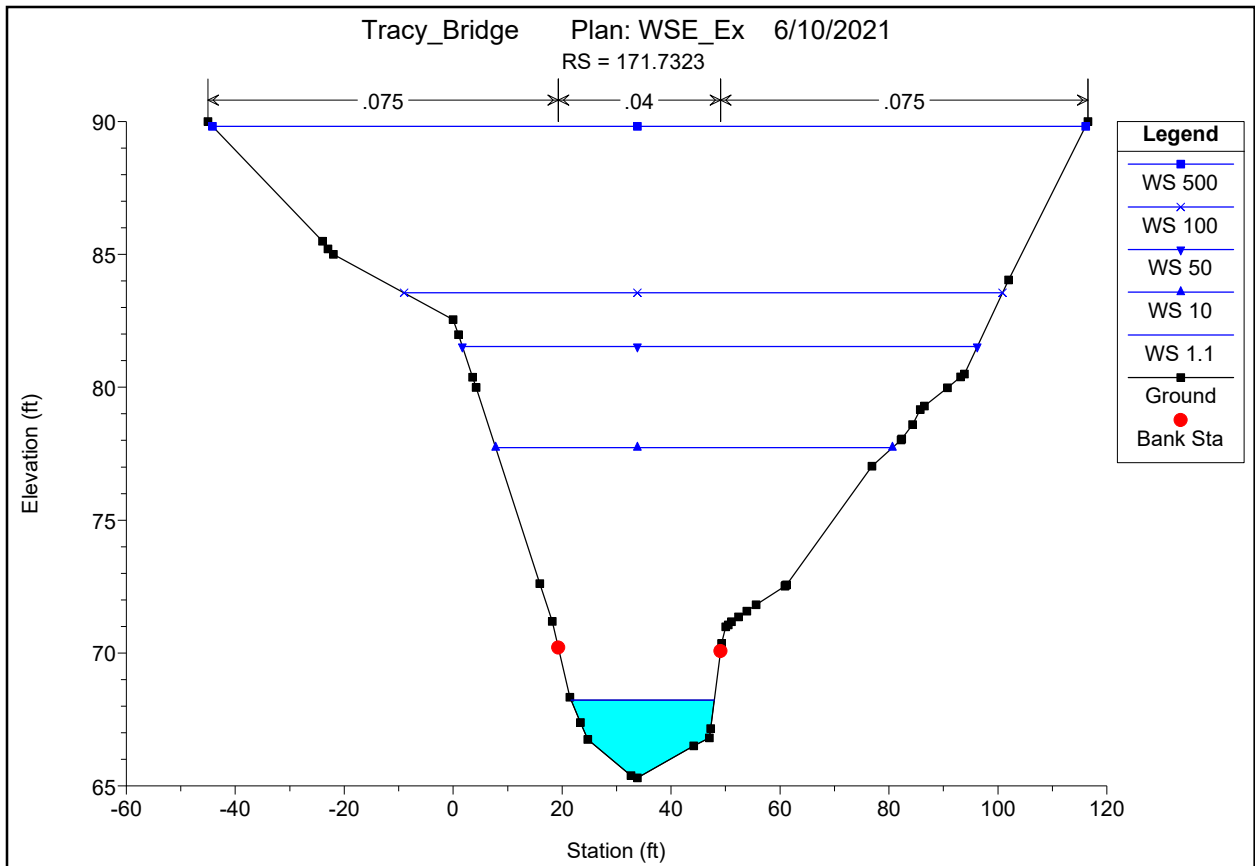


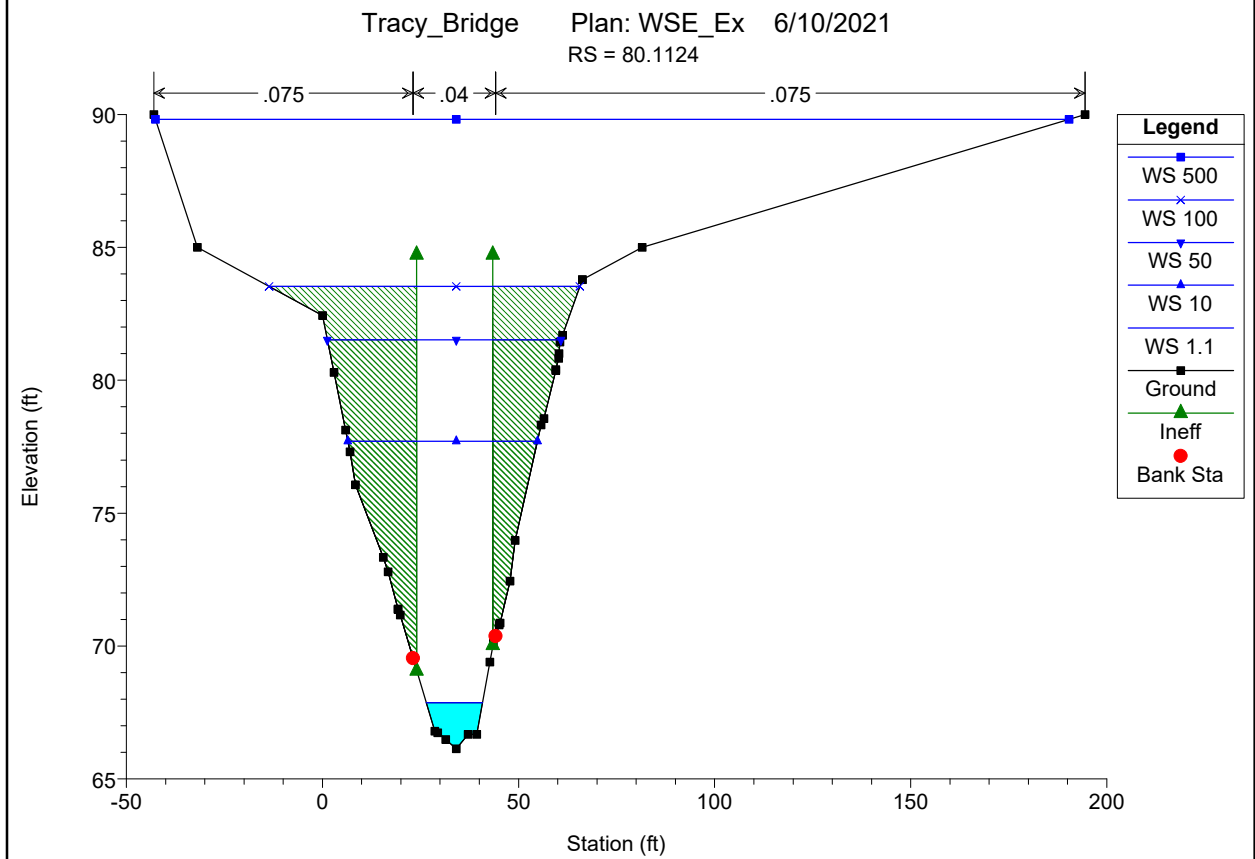
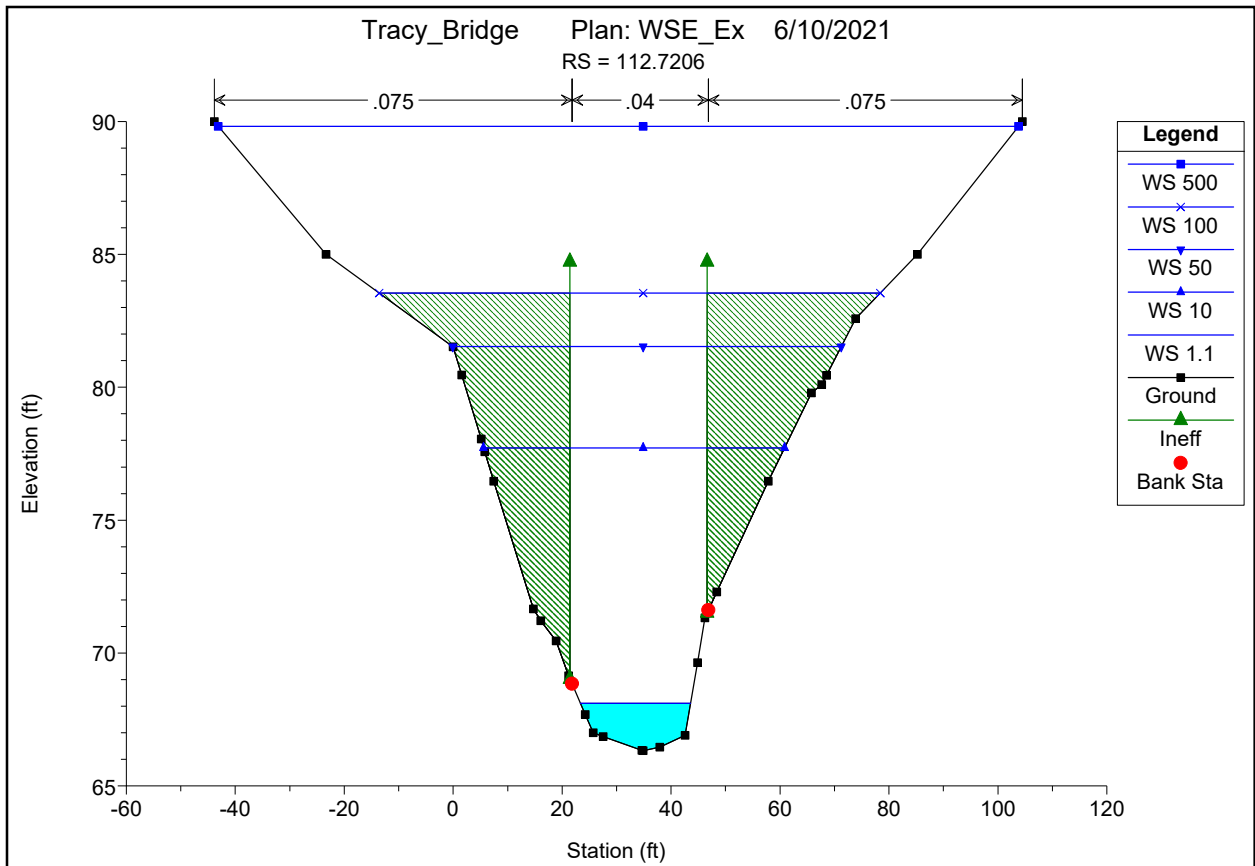


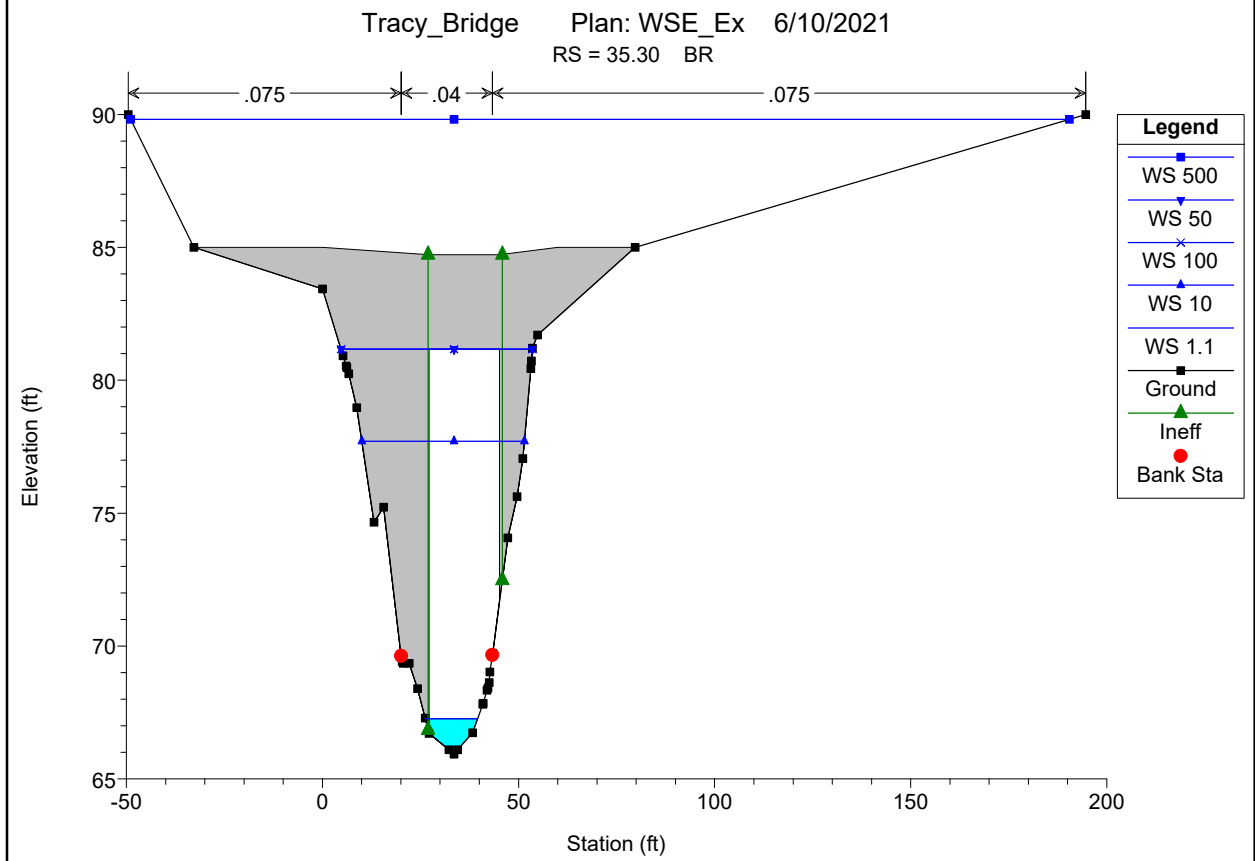
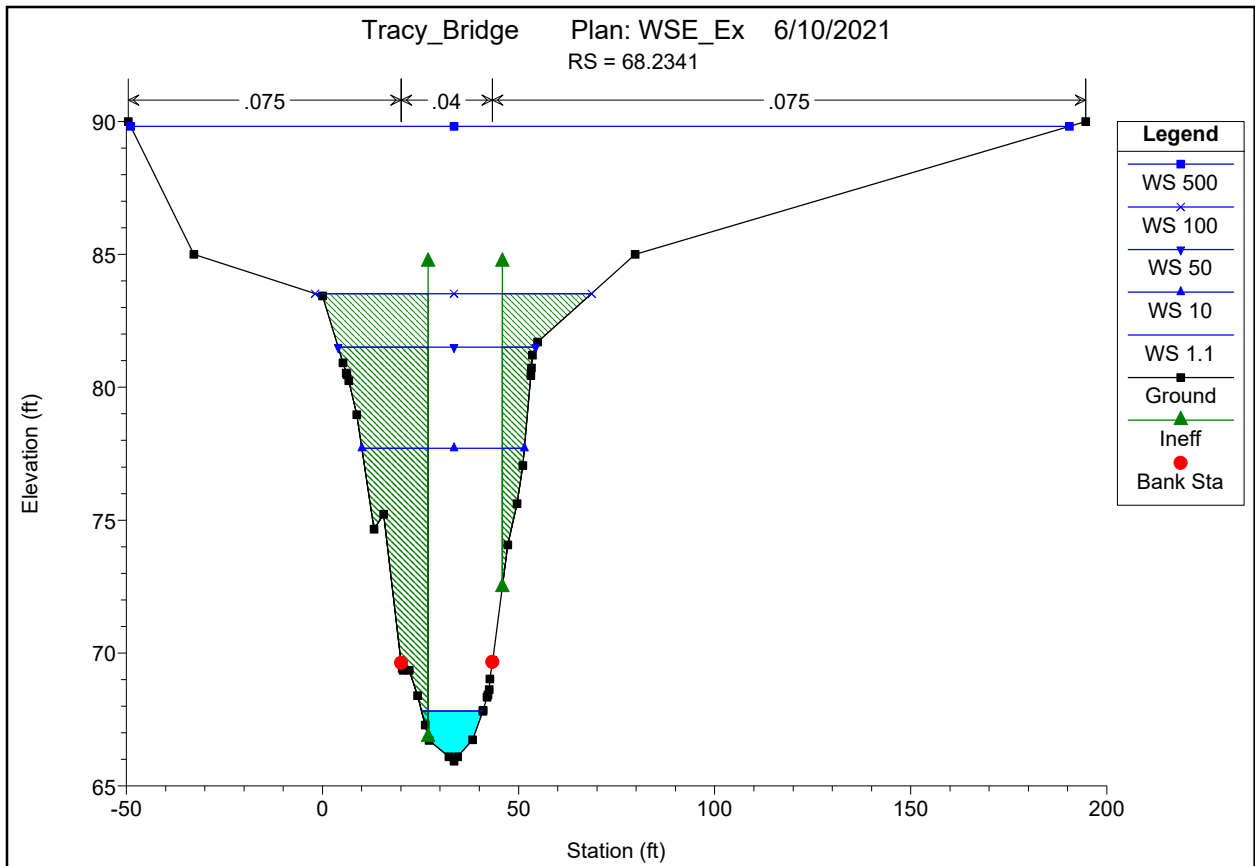


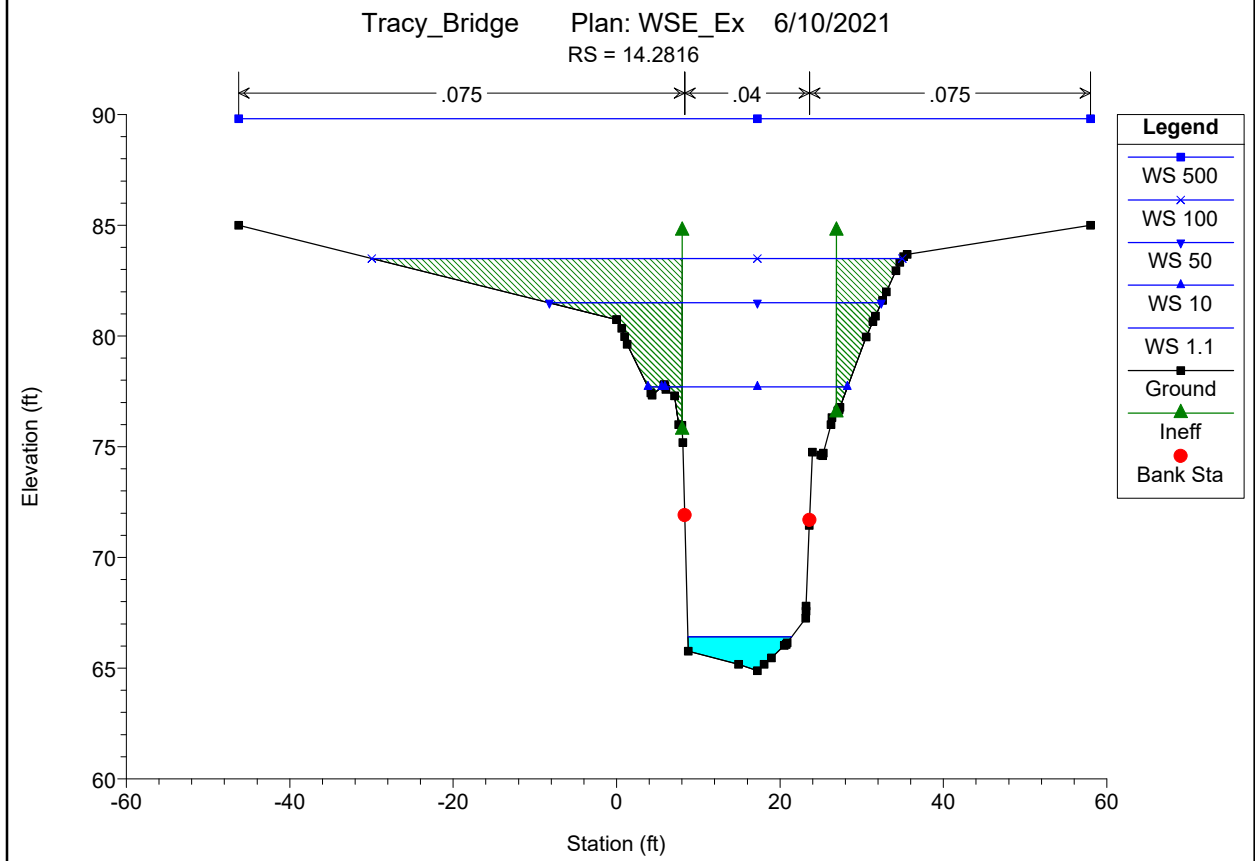
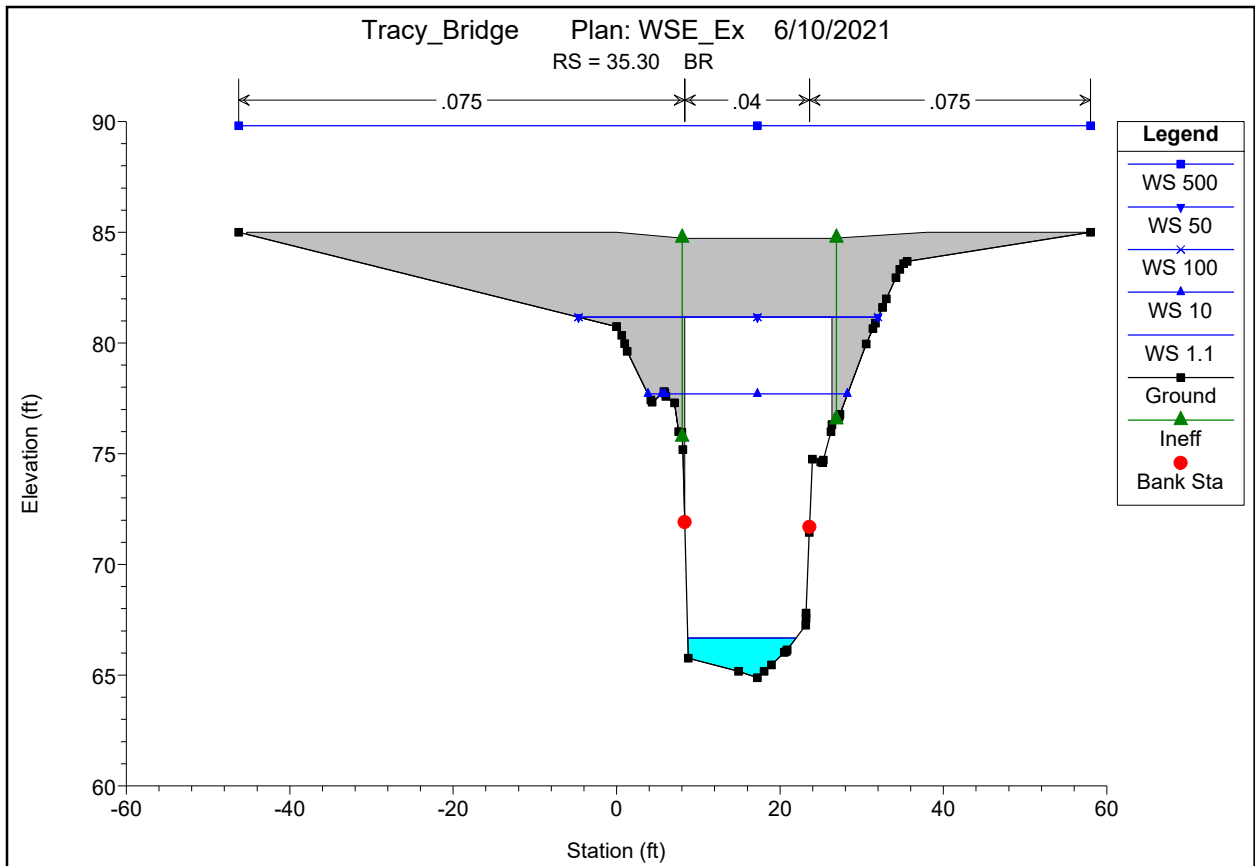




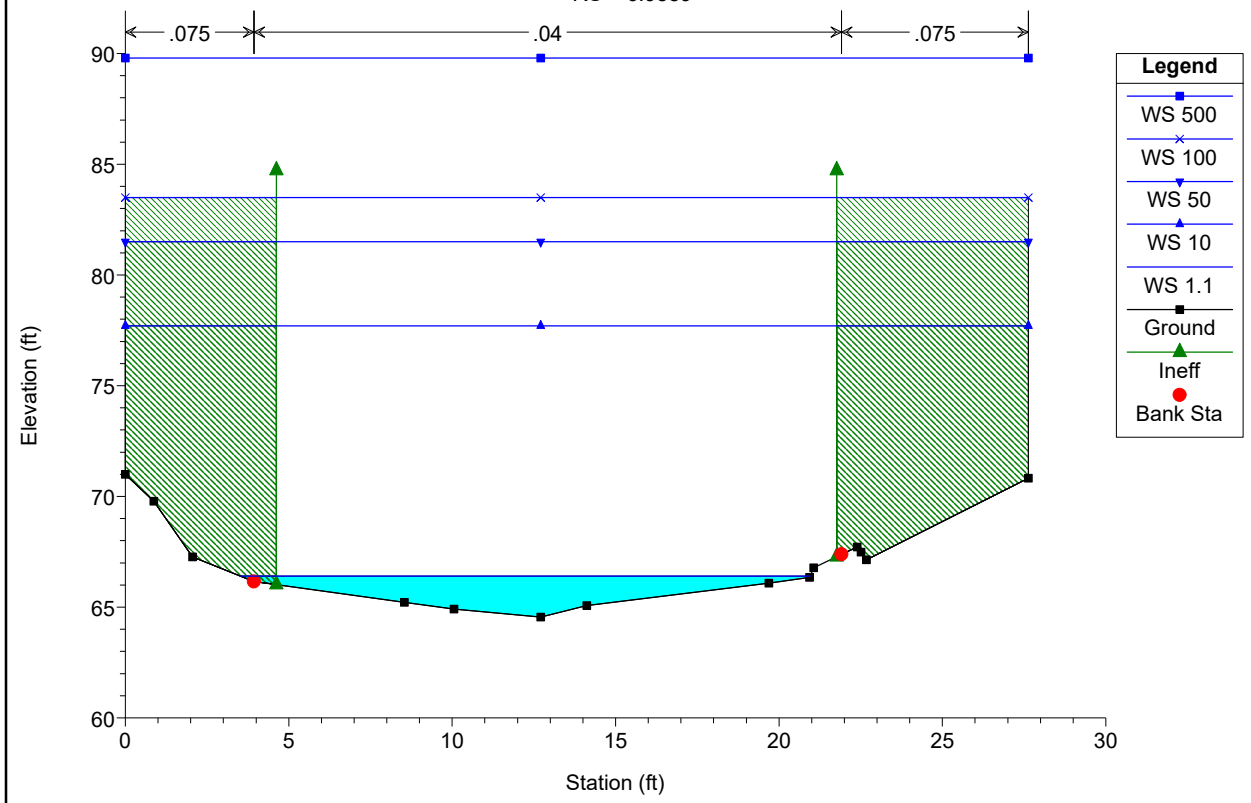




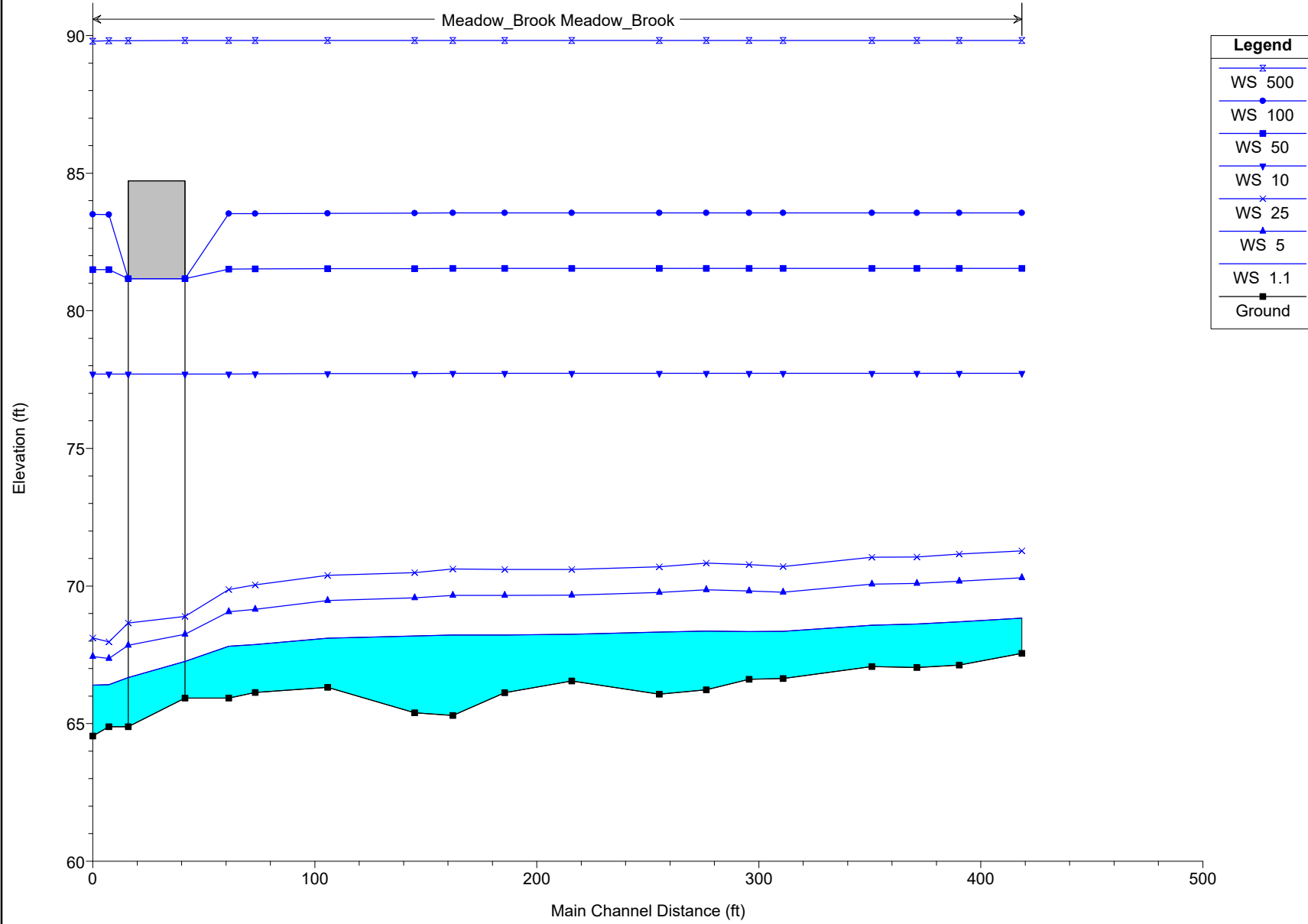




Tracy_Bridge Plan: WSE_Ex 6/10/2021
RS = 6.9889



Tracy_Bridge Plan: WSE_Ex 6/10/2021



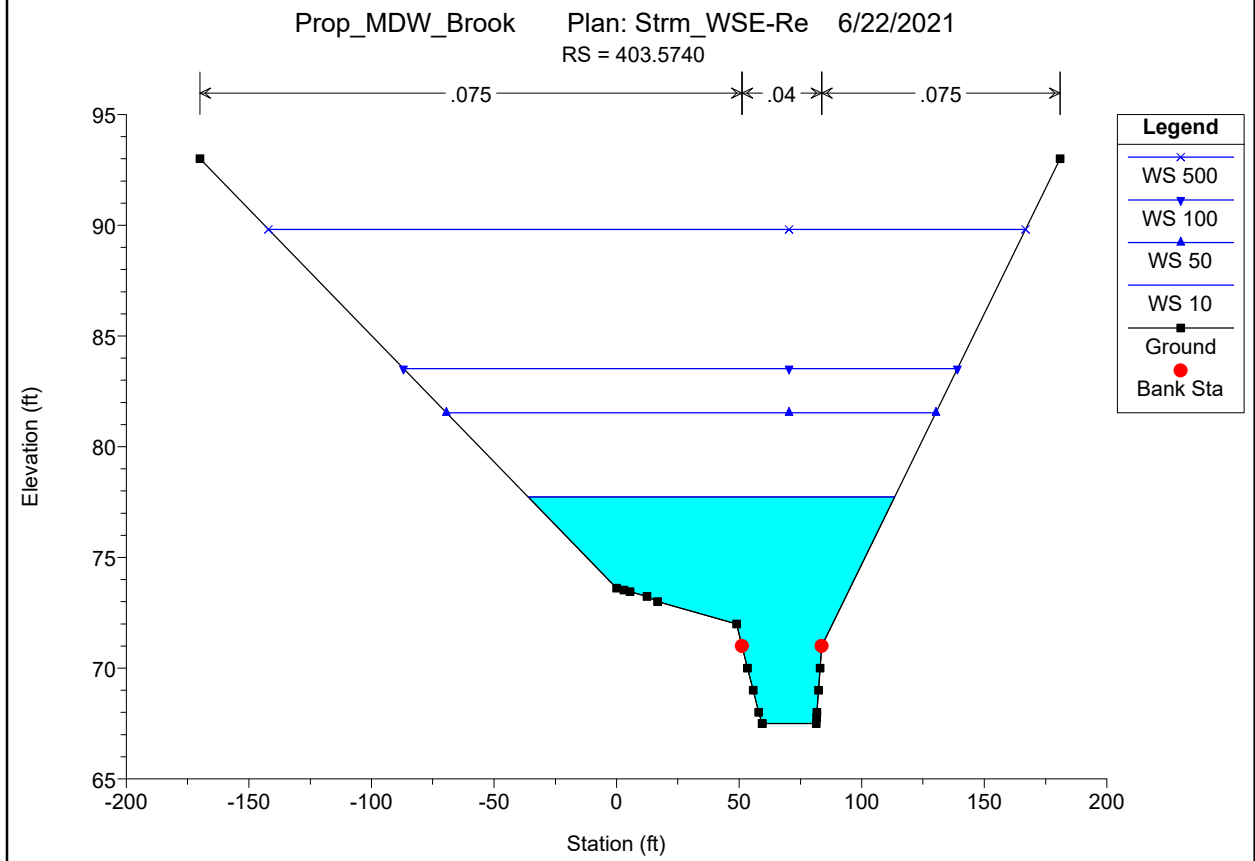
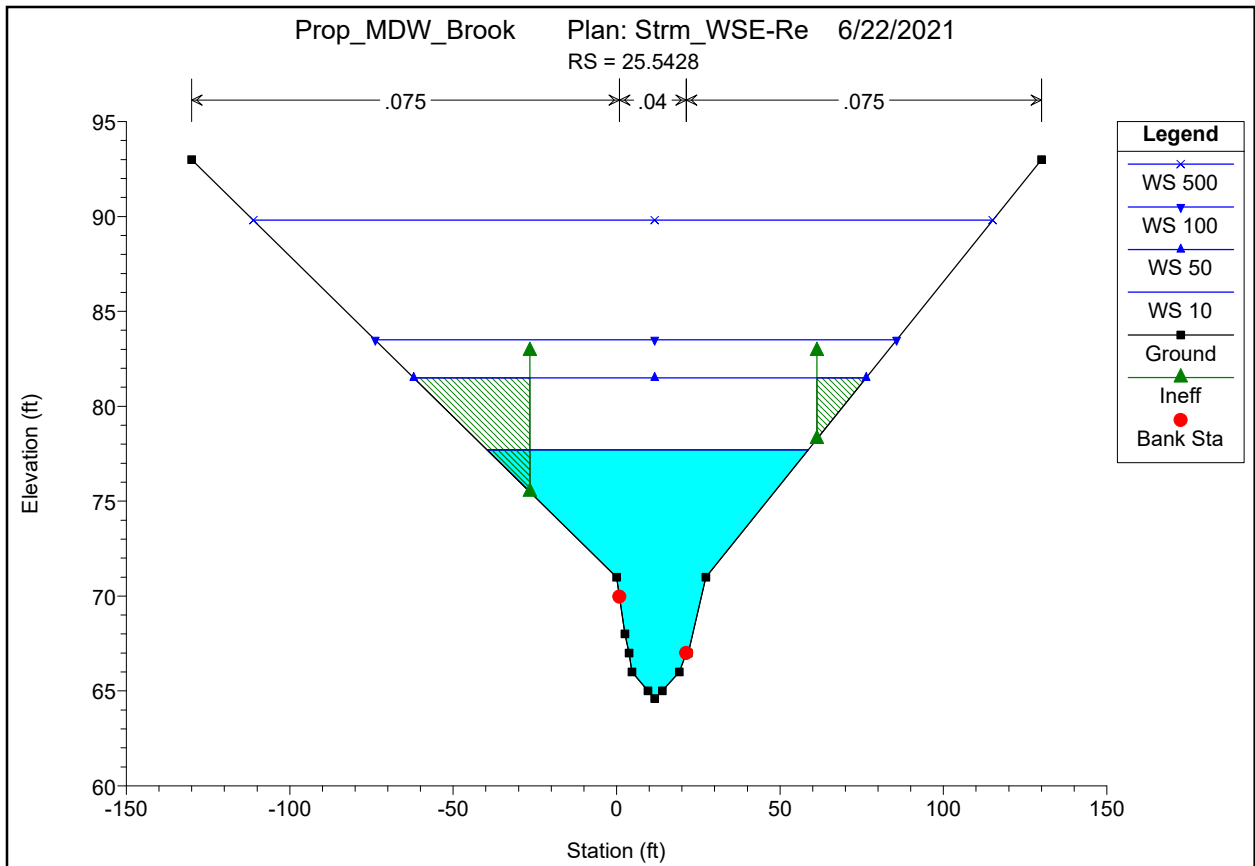
Proposed HEC-RAS

HEC-RAS Plan: Strm_WSE River: MeadowBrook Reach: Reach

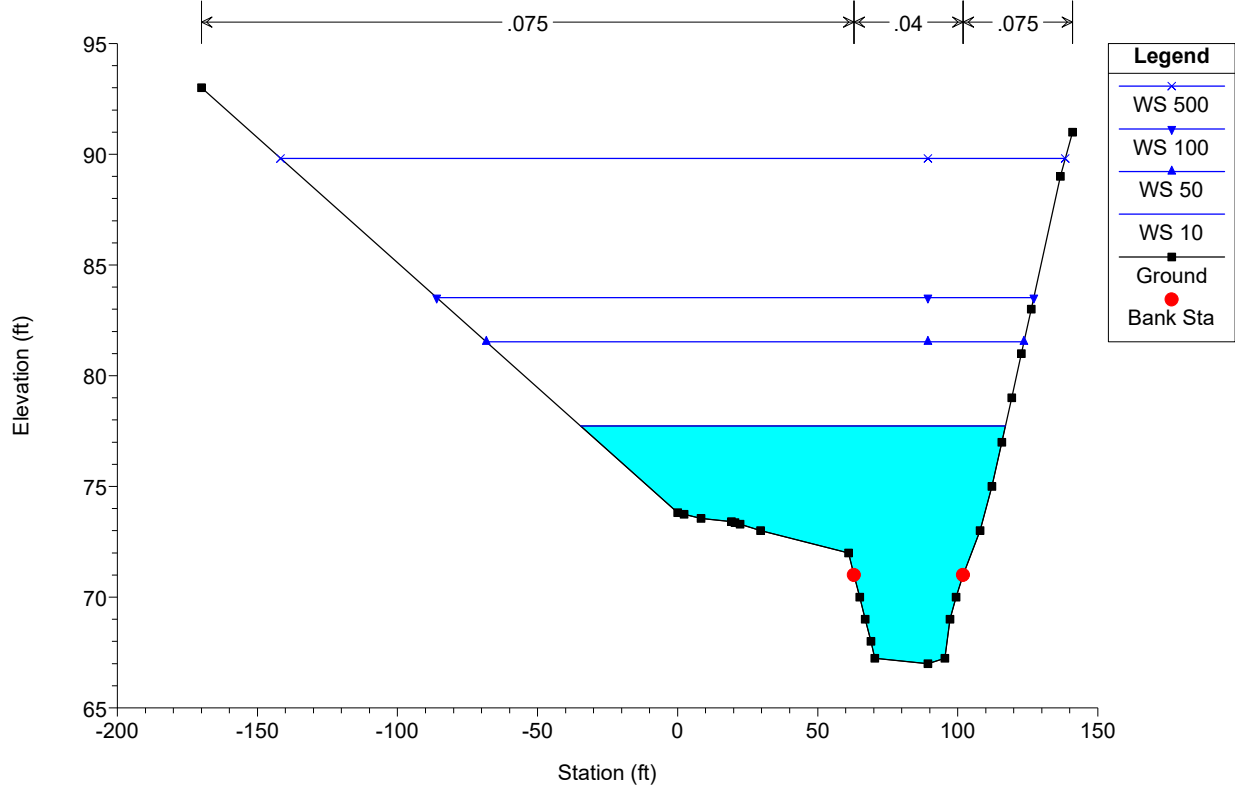
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach	403.5740	1.1	55.50	67.50	68.78		68.83	0.001914	1.91	30.81	26.01	0.30
Reach	403.5740	10	216.90	67.50	77.72		77.73	0.000011	0.58	743.94	149.50	0.03
Reach	403.5740	50	327.60	67.50	81.53		81.53	0.000006	0.51	1407.56	199.65	0.02
Reach	403.5740	100	383.60	67.50	83.52		83.53	0.000004	0.47	1832.96	226.01	0.02
Reach	403.5740	500	510.20	67.50	89.81		89.81	0.000001	0.35	3513.42	308.89	0.01
Reach	378.4437	1.1	55.50	67.00	68.77		68.80	0.000638	1.31	44.78	29.58	0.18
Reach	378.4437	10	216.90	67.00	77.72		77.73	0.000008	0.51	810.00	151.61	0.03
Reach	378.4437	50	327.60	67.00	81.53		81.53	0.000004	0.47	1463.01	191.95	0.02
Reach	378.4437	100	383.60	67.00	83.52		83.53	0.000003	0.44	1867.81	213.12	0.02
Reach	378.4437	500	510.20	67.00	89.81		89.81	0.000001	0.34	3416.34	280.10	0.01
Reach	345.6673	1.1	55.50	67.11	68.73		68.78	0.001442	1.68	33.12	25.00	0.26
Reach	345.6673	10	216.90	67.11	77.72		77.73	0.000009	0.52	780.99	148.70	0.03
Reach	345.6673	50	327.60	67.11	81.53		81.53	0.000005	0.47	1424.48	190.01	0.02
Reach	345.6673	100	383.60	67.11	83.52		83.53	0.000004	0.44	1826.17	211.92	0.02
Reach	345.6673	500	510.20	67.11	89.81		89.81	0.000001	0.34	3374.08	280.71	0.01
Reach	326.8960	1.1	55.50	66.99	68.66		68.73	0.002749	2.10	26.52	23.71	0.35
Reach	326.8960	10	216.90	66.99	77.72		77.73	0.000010	0.52	759.70	145.55	0.03
Reach	326.8960	50	327.60	66.99	81.53		81.53	0.000005	0.46	1390.58	186.35	0.02
Reach	326.8960	100	383.60	66.99	83.52		83.53	0.000004	0.44	1784.15	207.35	0.02
Reach	326.8960	500	510.20	66.99	89.81		89.81	0.000001	0.34	3264.76	263.20	0.01
Reach	308.9771	1.1	55.50	66.88	68.62		68.66	0.000944	1.54	36.92	25.43	0.21
Reach	308.9771	10	216.90	66.88	77.72		77.73	0.000011	0.58	698.15	129.20	0.03
Reach	308.9771	50	327.60	66.88	81.52		81.53	0.000006	0.53	1258.52	165.60	0.02
Reach	308.9771	100	383.60	66.88	83.52		83.53	0.000004	0.50	1608.68	184.74	0.02
Reach	308.9771	500	510.20	66.88	89.81		89.81	0.000002	0.39	2958.61	244.93	0.01
Reach	297.6656	1.1	55.50	66.82	68.60		68.64	0.000953	1.62	37.03	25.24	0.22
Reach	297.6656	10	216.90	66.82	77.72		77.73	0.000012	0.62	683.72	128.00	0.03
Reach	297.6656	50	327.60	66.82	81.52		81.53	0.000007	0.56	1240.47	164.87	0.03
Reach	297.6656	100	383.60	66.82	83.52		83.53	0.000005	0.53	1589.44	184.26	0.02
Reach	297.6656	500	510.20	66.82	89.81		89.81	0.000002	0.41	2939.54	246.09	0.02
Reach	287.5170	1.1	55.50	66.74	68.58		68.62	0.000941	1.60	36.78	24.66	0.22
Reach	287.5170	10	216.90	66.74	77.72		77.73	0.000012	0.64	660.16	126.71	0.03
Reach	287.5170	50	327.60	66.74	81.52		81.53	0.000007	0.57	1214.12	164.67	0.03
Reach	287.5170	100	383.60	66.74	83.52		83.53	0.000005	0.54	1563.27	184.63	0.02
Reach	287.5170	500	510.20	66.74	89.81		89.81	0.000002	0.41	2920.57	247.37	0.02
Reach	279.2907	1.1	55.50	66.70	68.57		68.60	0.000877	1.47	38.26	26.07	0.21
Reach	279.2907	10	216.90	66.70	77.72		77.73	0.000010	0.56	707.82	129.69	0.03
Reach	279.2907	50	327.60	66.70	81.52		81.53	0.000006	0.52	1269.26	165.93	0.02
Reach	279.2907	100	383.60	66.70	83.52		83.53	0.000004	0.49	1619.11	183.68	0.02
Reach	279.2907	500	510.20	66.70	89.81		89.81	0.000002	0.38	2970.93	247.29	0.01
Reach	241.5227	1.1	55.50	66.61	68.54		68.58	0.001069	1.57	35.25	22.77	0.22
Reach	241.5227	10	216.90	66.61	77.72		77.73	0.000011	0.55	658.14	116.39	0.03
Reach	241.5227	50	327.60	66.61	81.52		81.53	0.000006	0.52	1163.27	148.27	0.02
Reach	241.5227	100	383.60	66.61	83.52		83.53	0.000005	0.49	1474.77	163.38	0.02
Reach	241.5227	500	510.20	66.61	89.81		89.81	0.000002	0.39	2650.62	210.87	0.01
Reach	211.3500	1.1	55.50	66.21	68.51		68.55	0.000804	1.53	38.06	22.92	0.20
Reach	211.3500	10	216.90	66.21	77.72		77.72	0.000012	0.57	658.25	114.30	0.03
Reach	211.3500	50	327.60	66.21	81.52		81.53	0.000007	0.53	1166.55	152.56	0.02
Reach	211.3500	100	383.60	66.21	83.52		83.53	0.000005	0.51	1486.54	167.13	0.02
Reach	211.3500	500	510.20	66.21	89.81		89.81	0.000002	0.40	2676.08	211.47	0.01
Reach	189.8691	1.1	55.50	65.25	68.52		68.53	0.000229	0.98	59.15	26.76	0.11
Reach	189.8691	10	216.90	65.25	77.72		77.72	0.000012	0.60	509.02	75.54	0.03
Reach	189.8691	50	327.60	65.25	81.52		81.53	0.000008	0.60	830.75	97.38	0.03
Reach	189.8691	100	383.60	65.25	83.52		83.52	0.000007	0.61	1052.95	120.57	0.03
Reach	189.8691	500	510.20	65.25	89.81		89.81	0.000003	0.49	2059.56	199.75	0.02
Reach	170.8144	1.1	55.50	65.75	68.51		68.53	0.000333	1.09	51.10	24.84	0.13
Reach	170.8144	10	216.90	65.75	77.72		77.72	0.000014	0.66	393.13	77.56	0.04
Reach	170.8144	50	327.60	65.75	81.52		81.53	0.000011	0.71	571.71	105.34	0.03
Reach	170.8144	100	383.60	65.75	83.52		83.52	0.000007	0.60	1086.19	125.71	0.03
Reach	170.8144	500	510.20	65.75	89.81		89.81	0.000003	0.48	2054.01	182.26	0.02
Reach	131.6994	1.1	55.50	66.51	68.45		68.50	0.001321	1.72	32.24	21.34	0.25
Reach	131.6994	10	216.90	66.51	77.72		77.72	0.000017	0.69	399.64	58.61	0.04
Reach	131.6994	50	327.60	66.51	81.52		81.52	0.000012	0.70	648.89	157.66	0.03
Reach	131.6994	100	383.60	66.51	83.52		83.52	0.000009	0.65	1132.44	235.58	0.03

HEC-RAS Plan: Strm_WSE River: MeadowBrook Reach: Reach (Continued)

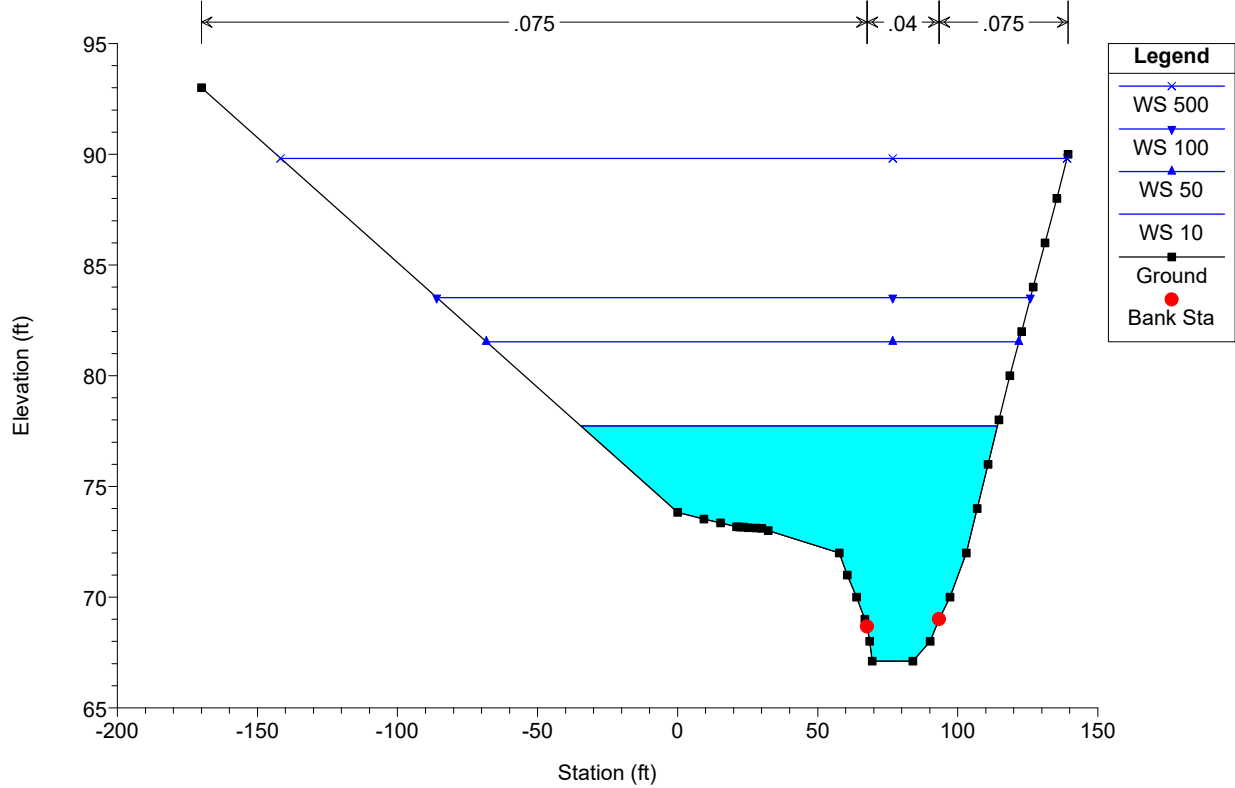
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach	131.6994	500	510.20	66.51	89.81	69.59	89.81	0.000002	0.42	3057.99	461.54	0.02
Reach	60.71		Bridge									
Reach	32.5927	1.1	55.50	64.80	67.36	67.36	68.09	0.031757	6.85	8.11	5.54	1.00
Reach	32.5927	10	216.90	64.80	77.69	69.56	77.72	0.000094	1.34	210.51	39.32	0.07
Reach	32.5927	50	327.60	64.80	81.49	70.42	81.52	0.000067	1.40	304.98	207.09	0.07
Reach	32.5927	100	383.60	64.80	83.50	70.75	83.51	0.000032	1.05	1119.15	367.87	0.05
Reach	32.5927	500	510.20	64.80	89.80	71.39	89.80	0.000002	0.35	4041.59	555.76	0.01
Reach	25.5428	1.1	55.50	64.60	66.06	66.06	66.44	0.026335	4.96	11.19	14.70	1.00
Reach	25.5428	10	216.90	64.60	77.70	67.19	77.71	0.000021	0.82	297.26	98.21	0.04
Reach	25.5428	50	327.60	64.60	81.50	67.75	81.51	0.000018	0.93	399.45	138.39	0.04
Reach	25.5428	100	383.60	64.60	83.50	67.99	83.50	0.000007	0.61	1286.19	159.54	0.03
Reach	25.5428	500	510.20	64.60	89.80	68.49	89.80	0.000003	0.46	2501.16	226.16	0.02

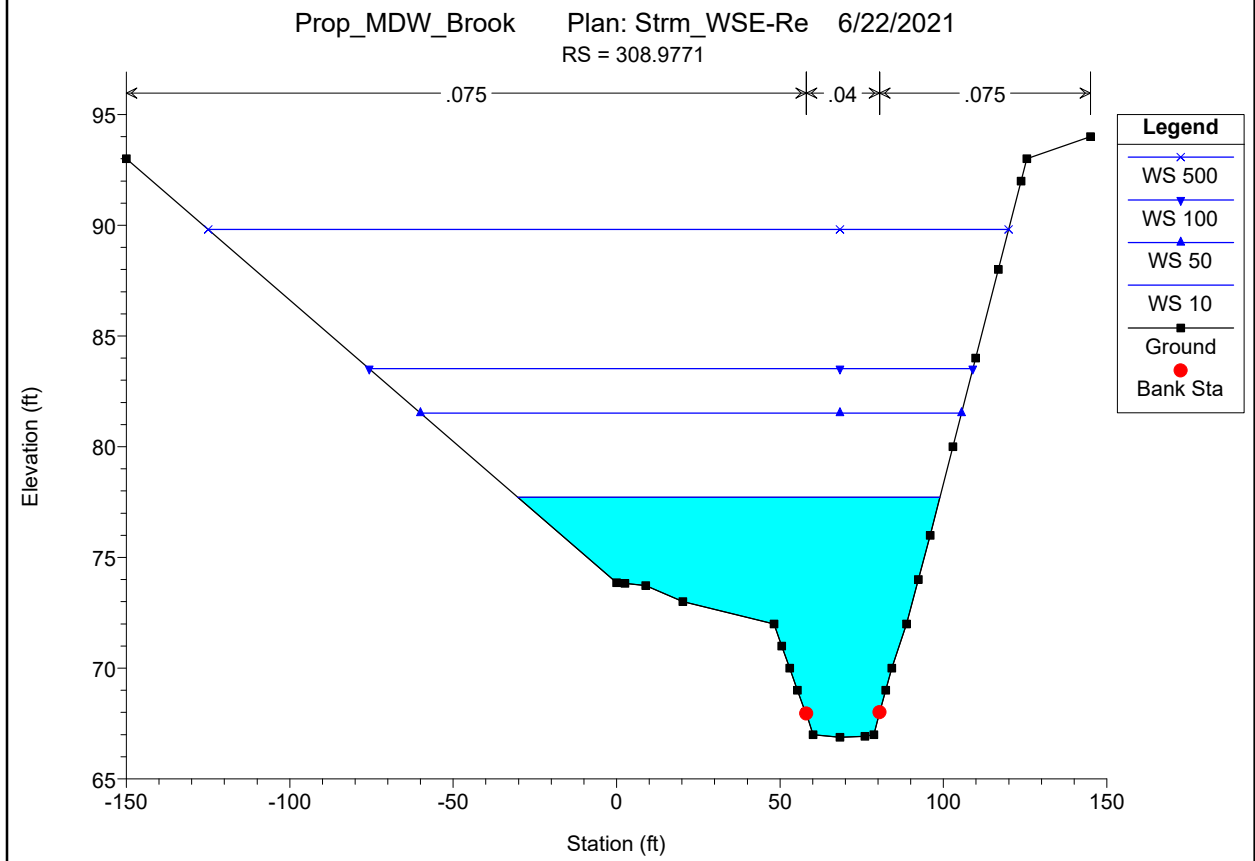
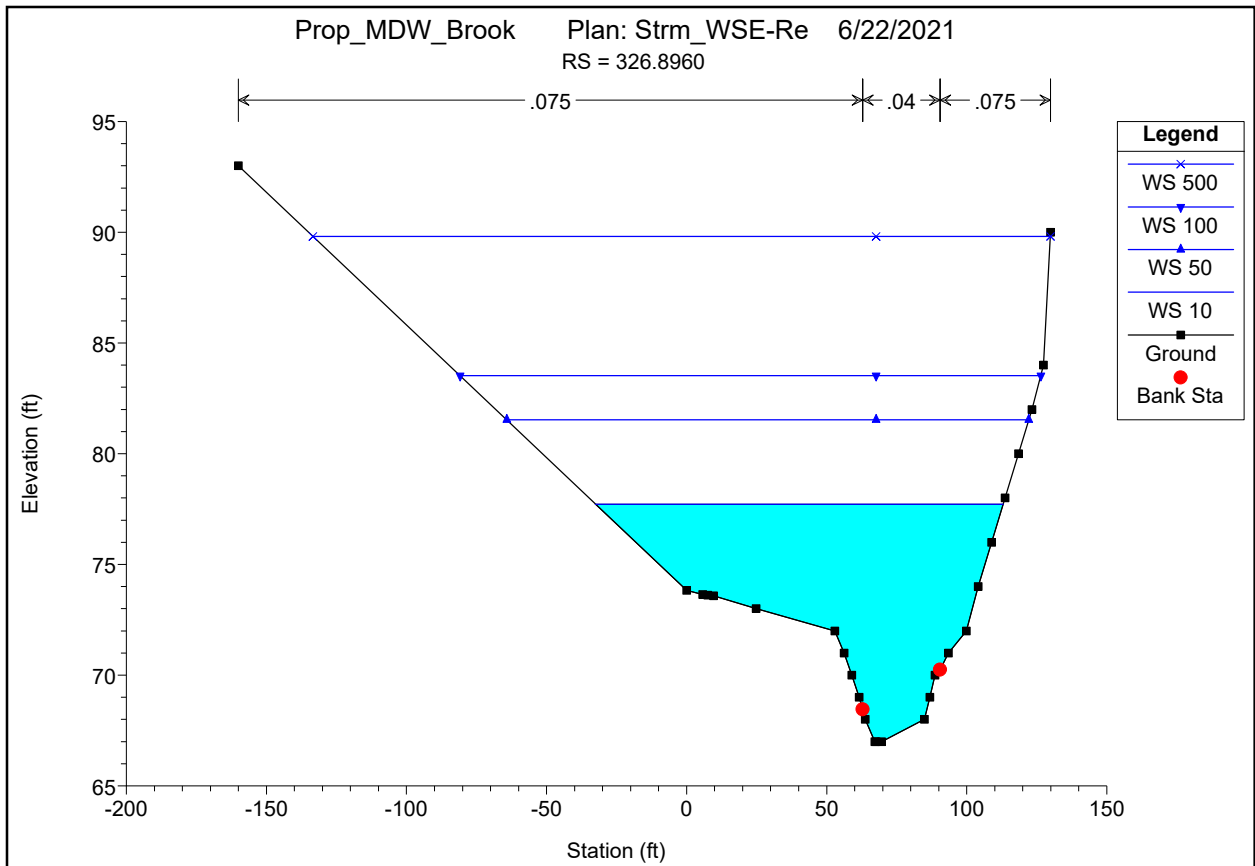


Prop_MDW_Brook Plan: Strm_WSE-Re 6/22/2021
RS = 378.4437

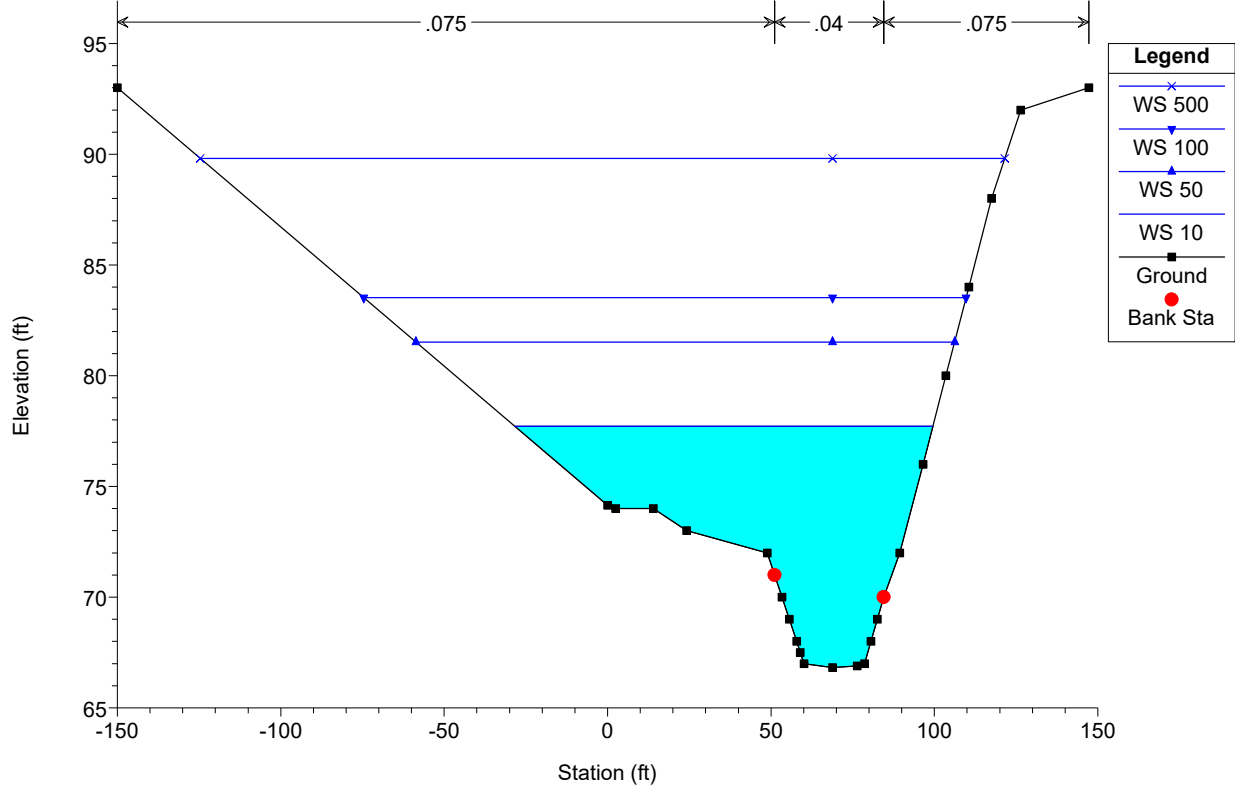


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RS = 345.6673

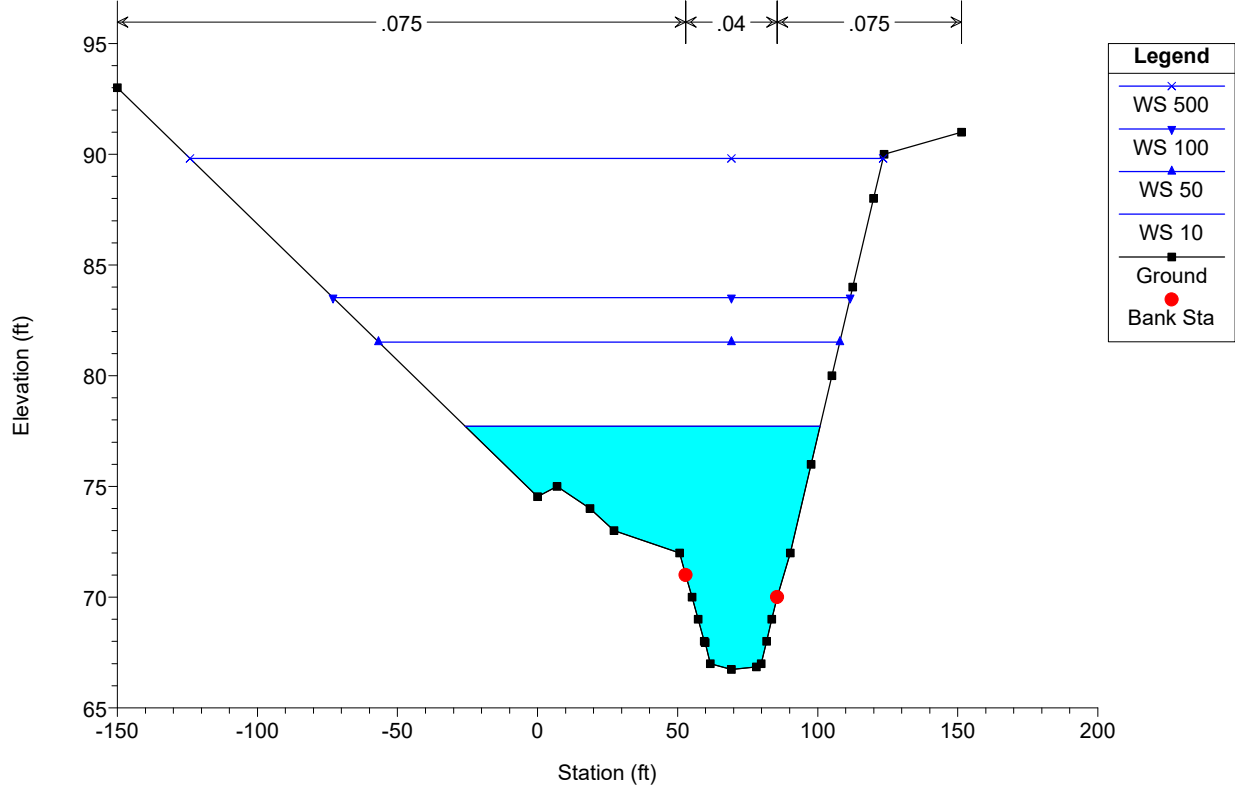


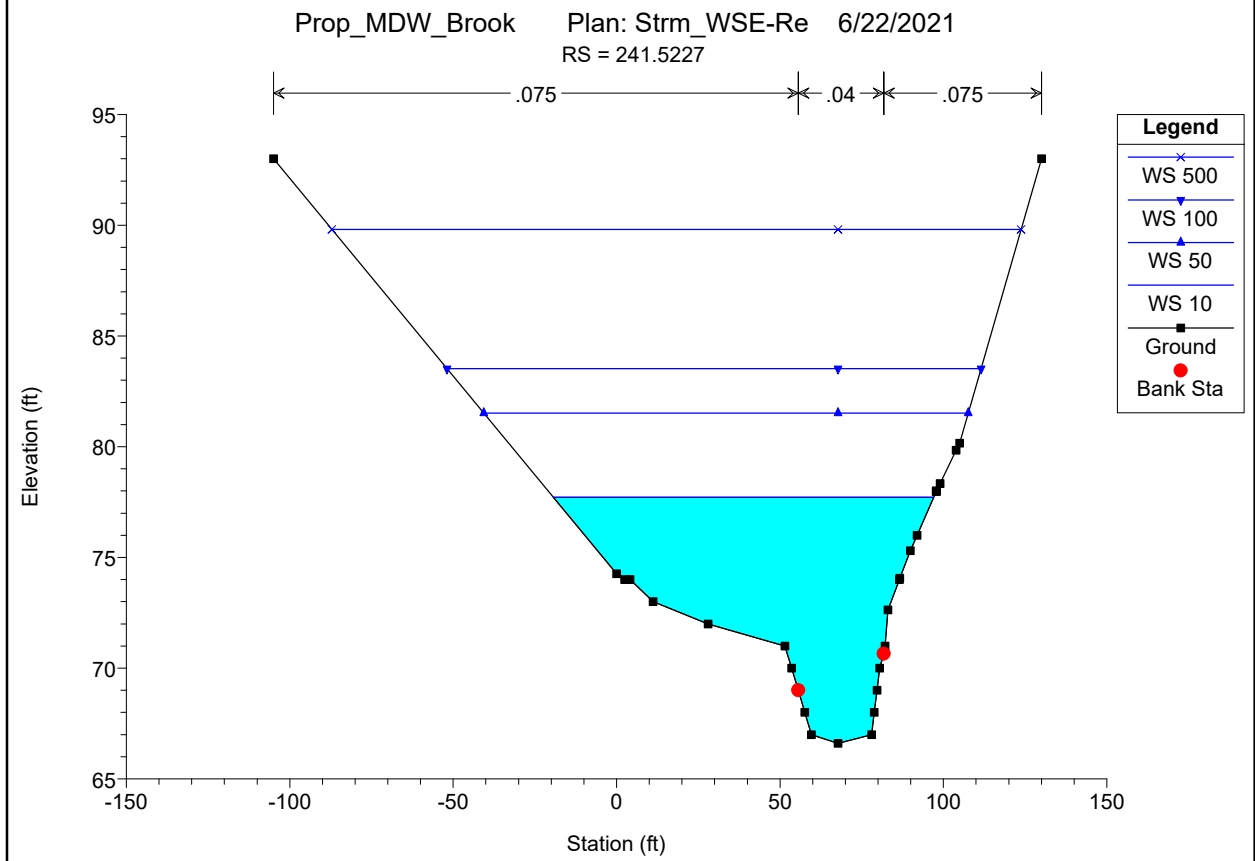
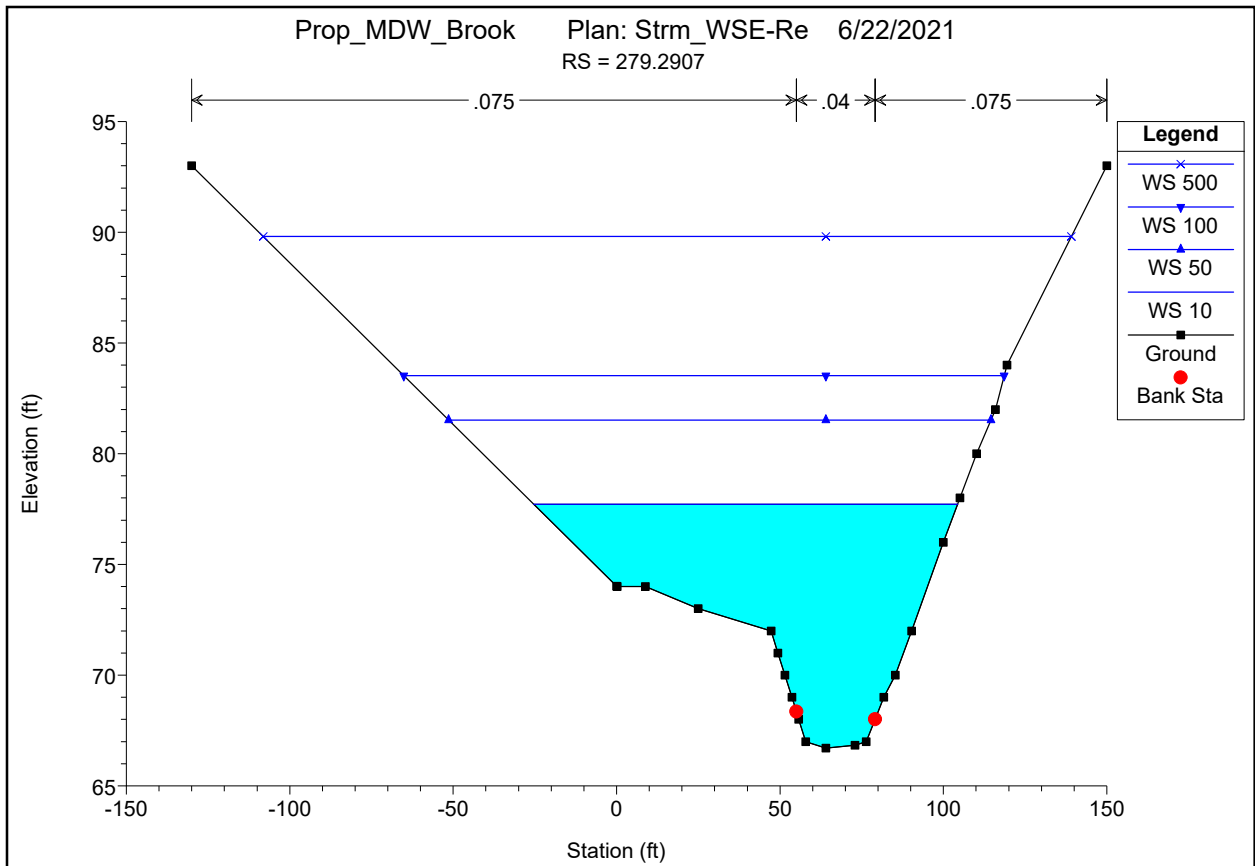


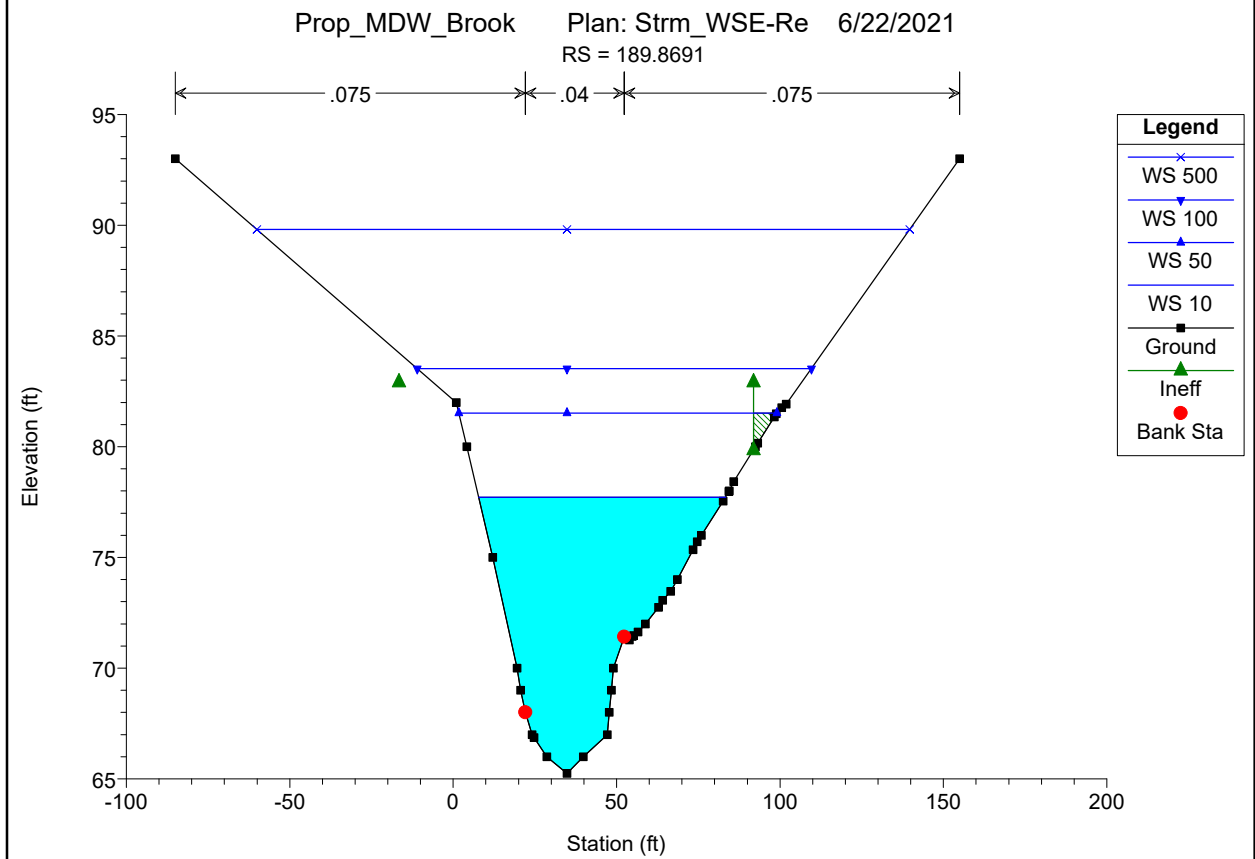
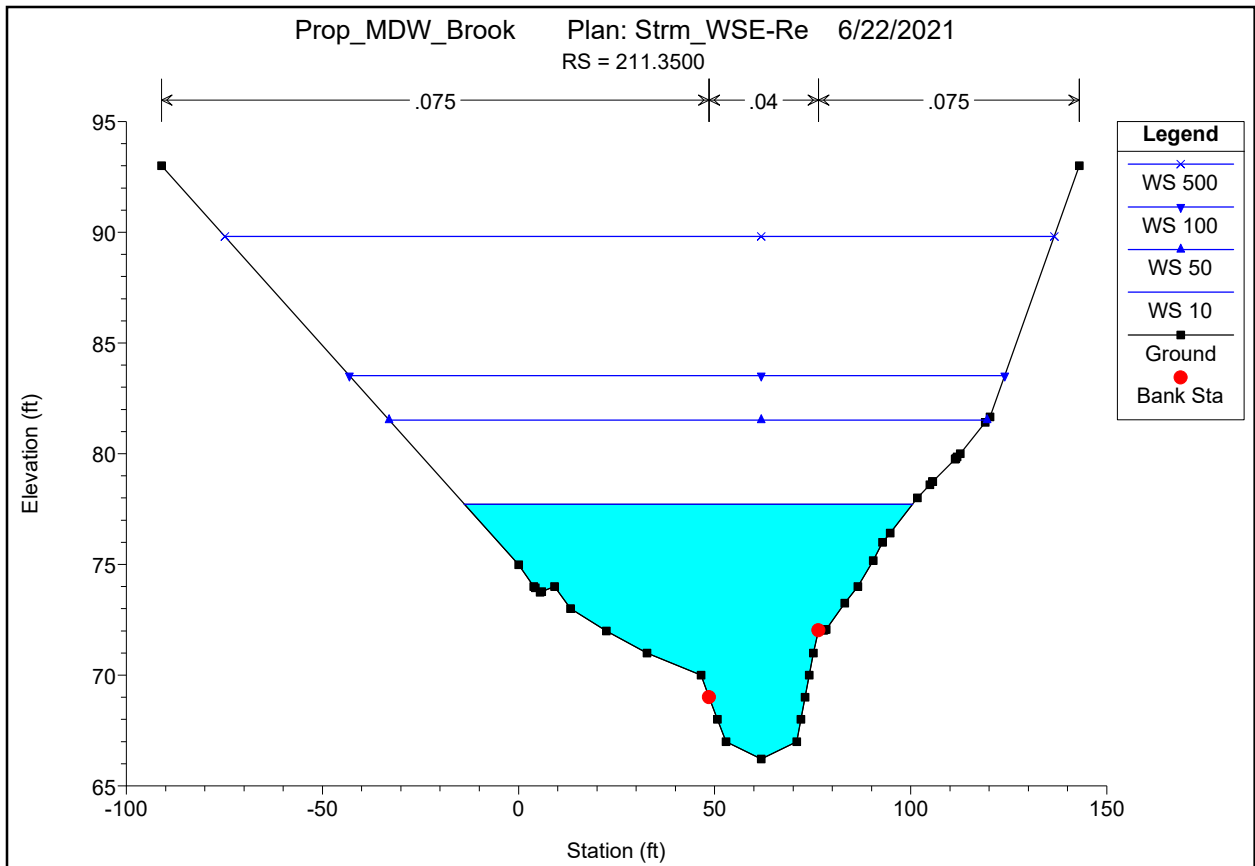
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RS = 297.6656

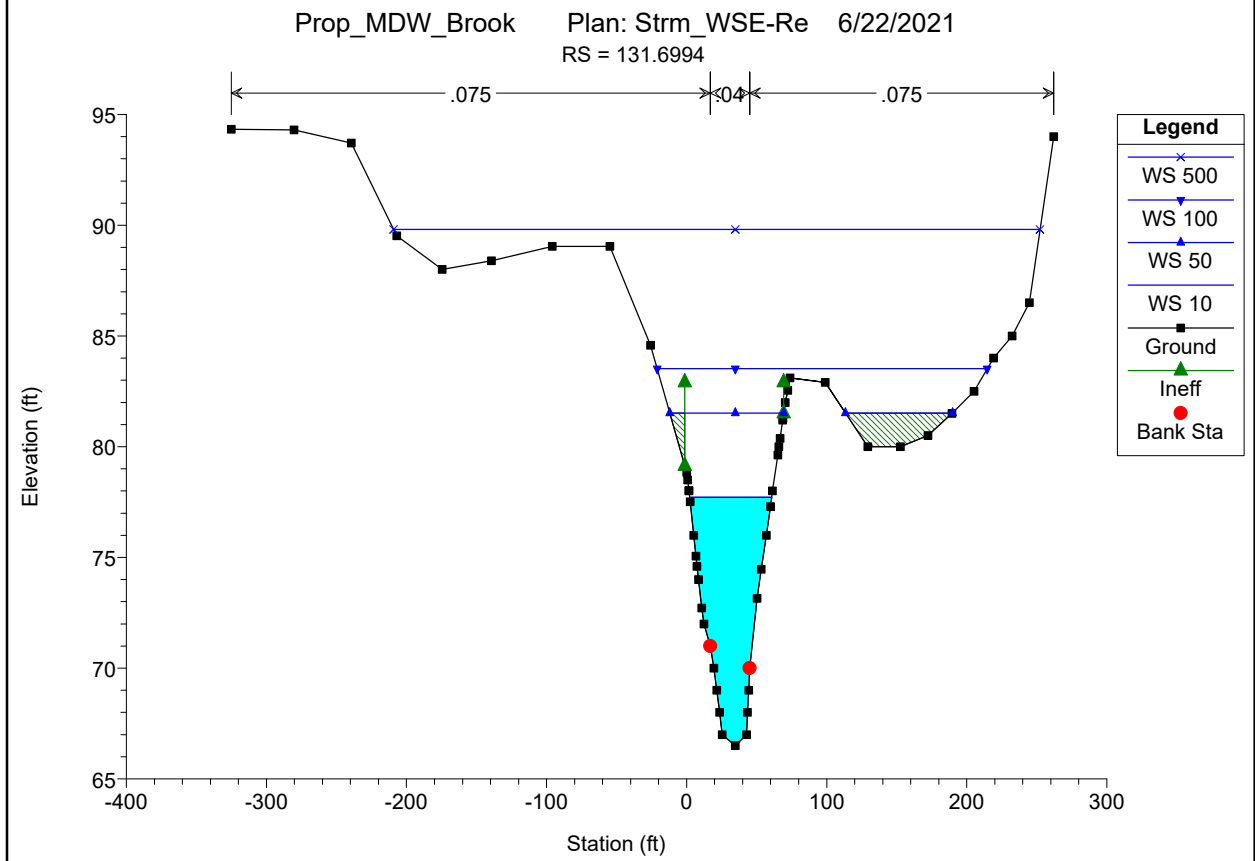
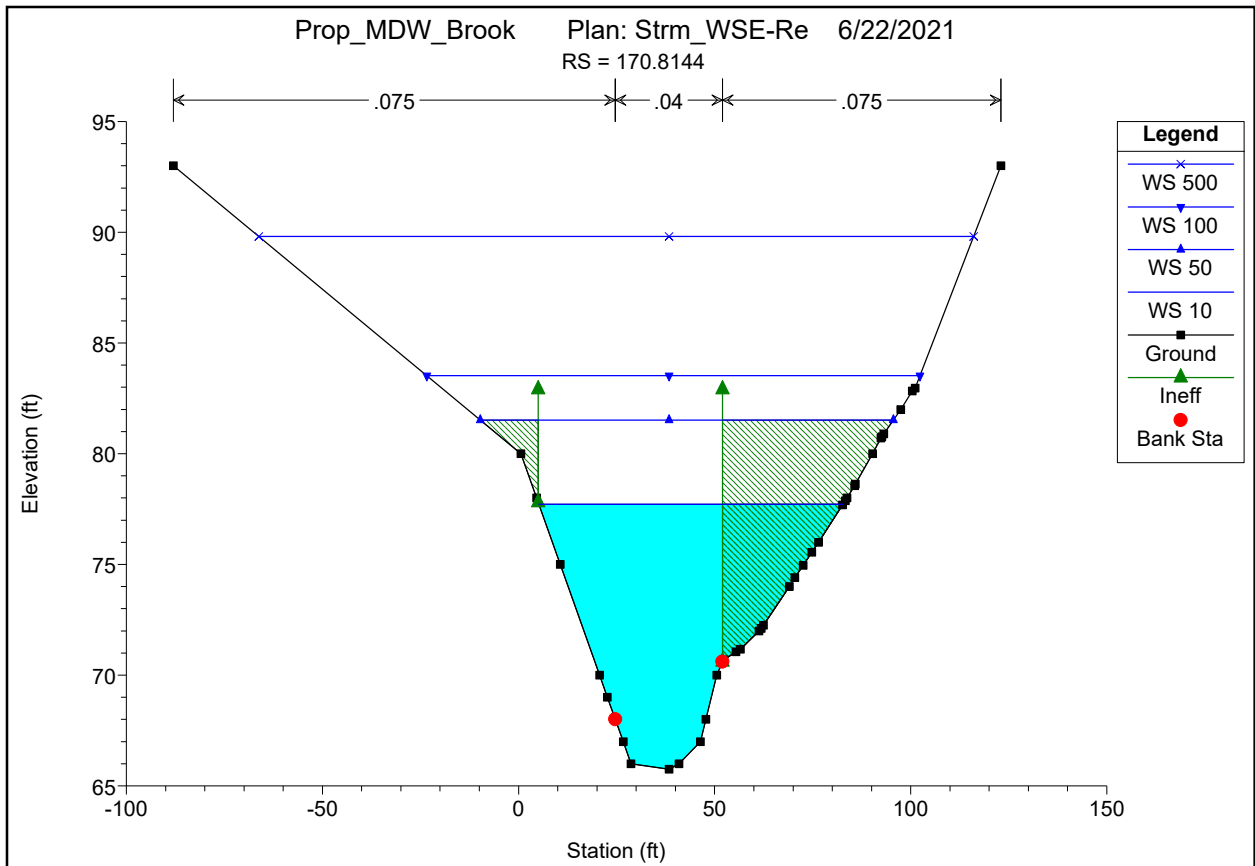


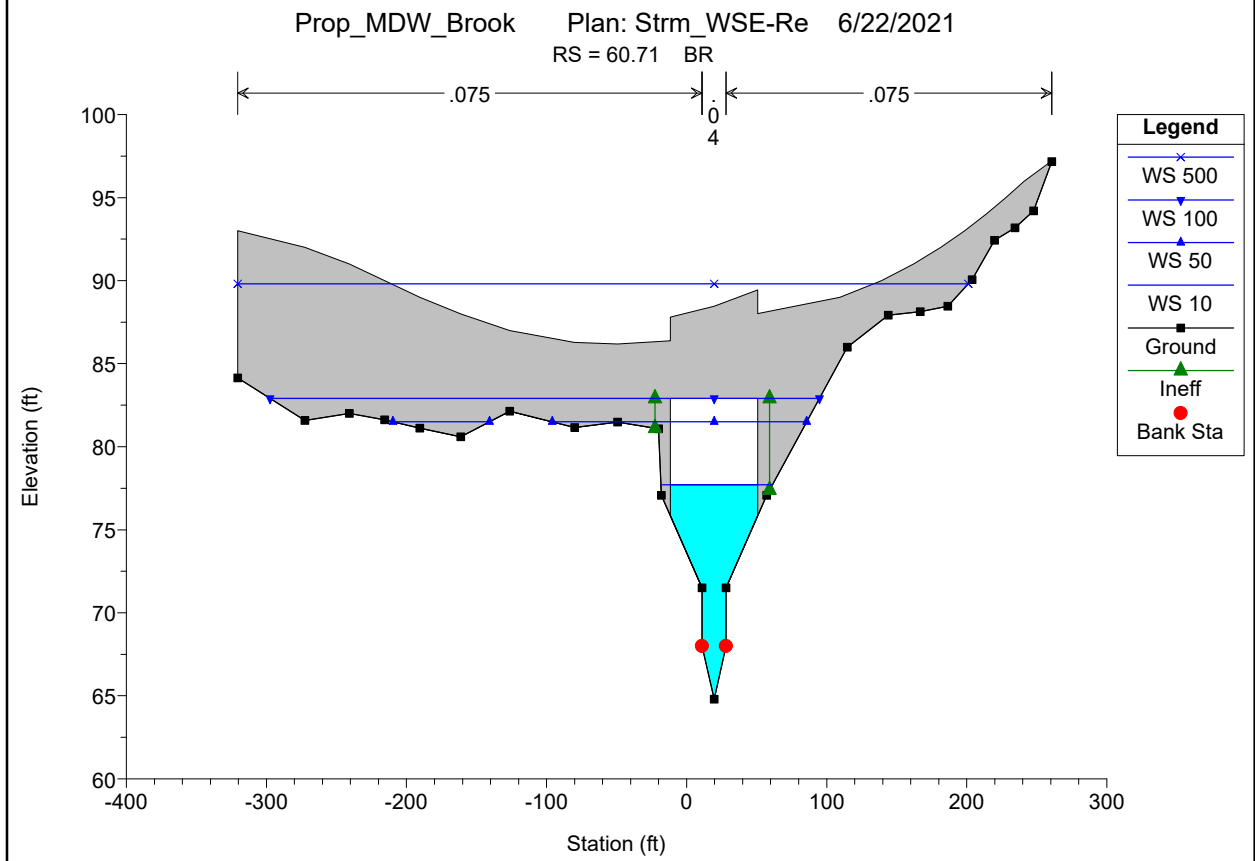
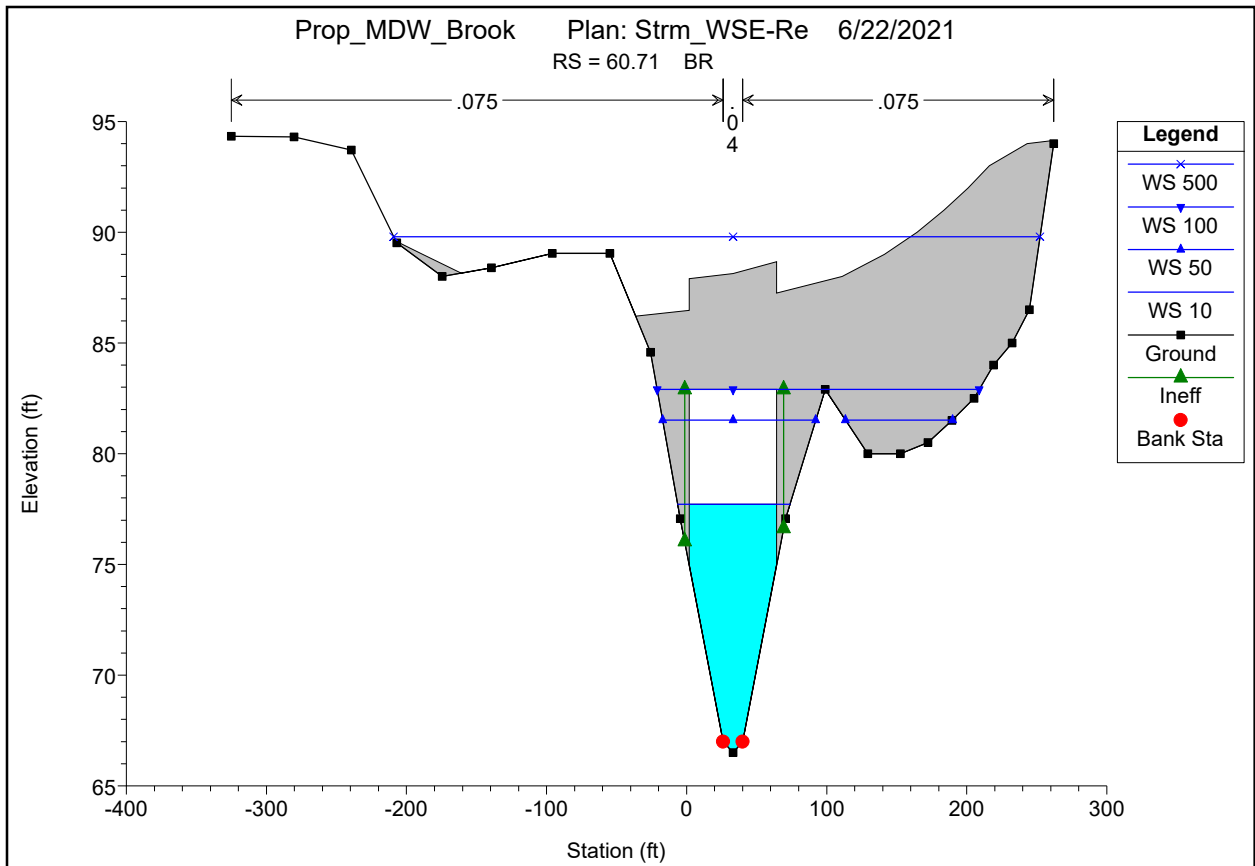
Prop_MDW_Brook Plan: Strm_WSE-Re 6/22/2021
RS = 287.5170



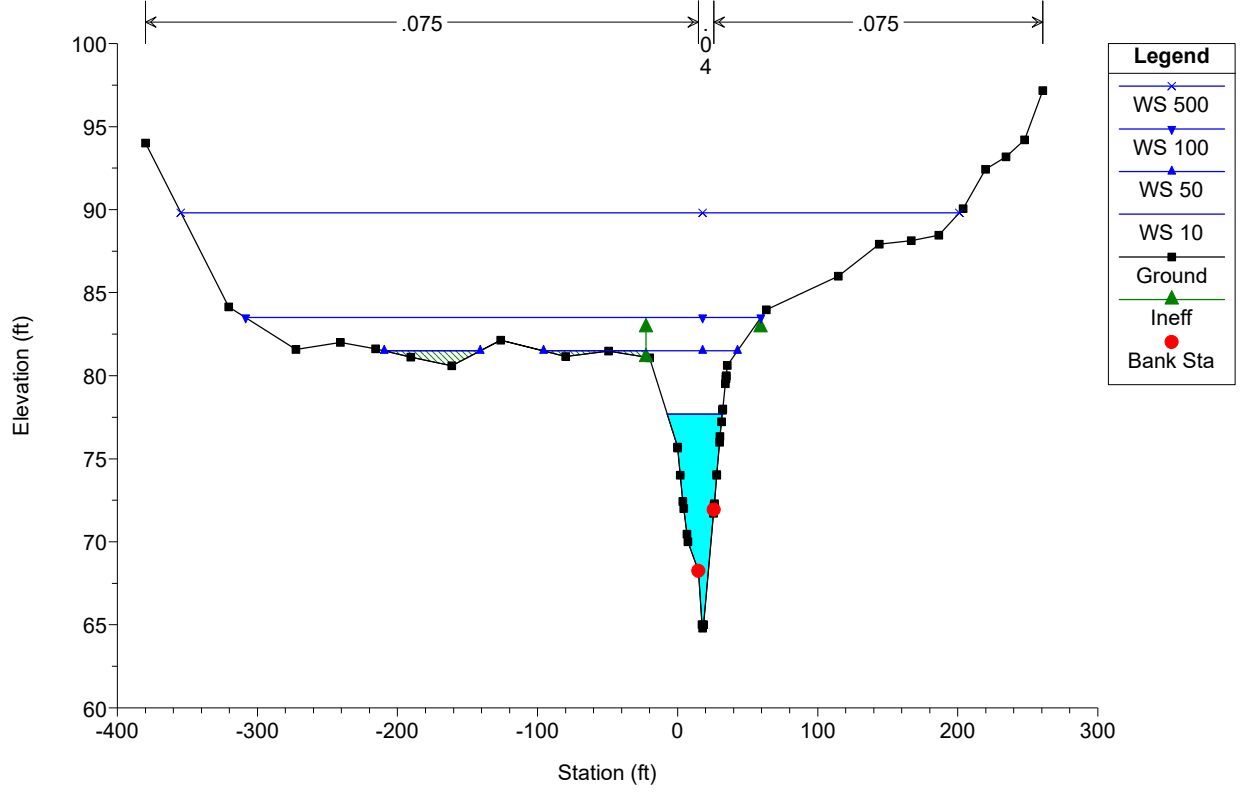






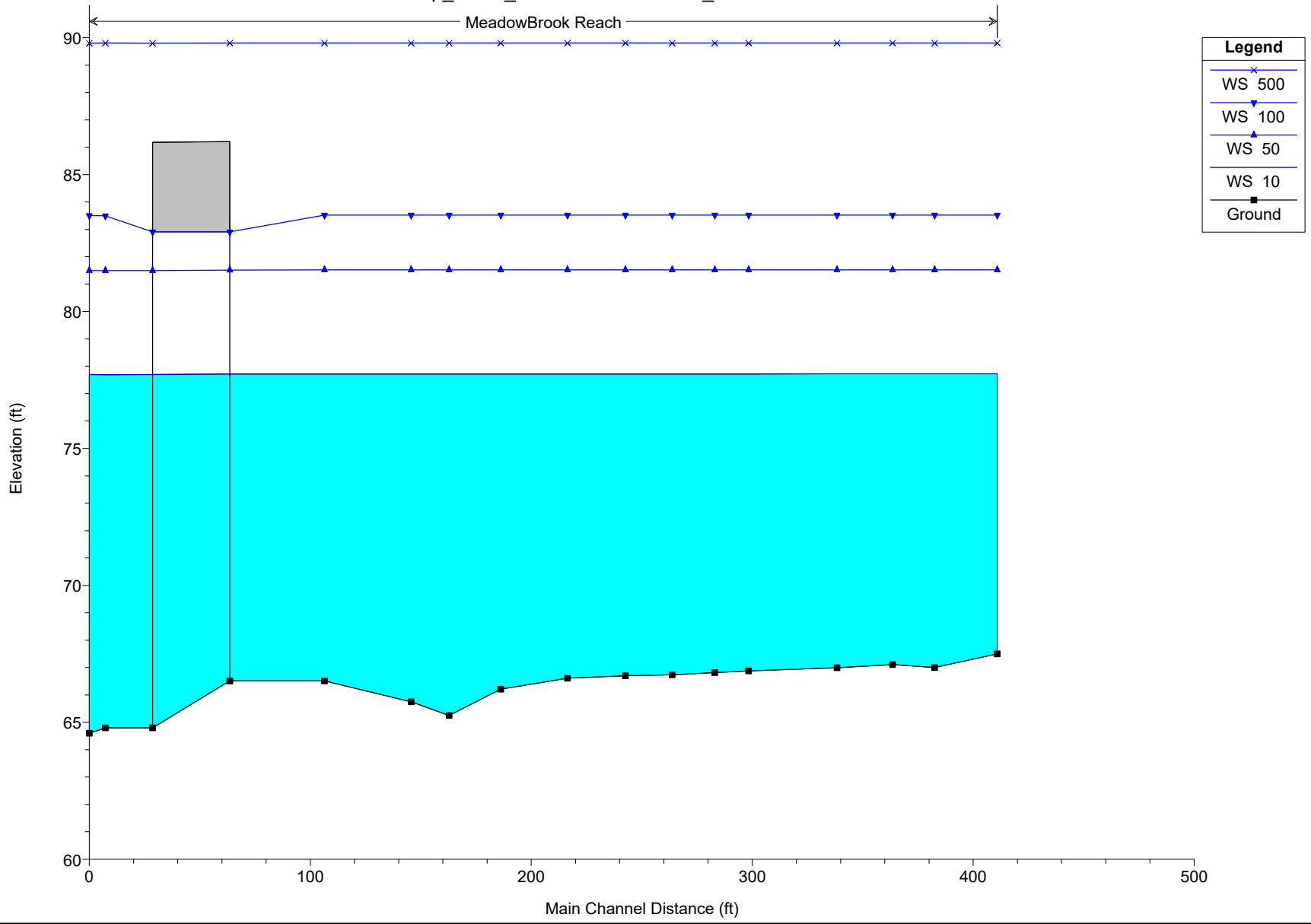


Prop_MDW_Brook Plan: Strm_WSE-Re 6/22/2021
RS = 32.5927



Prop_MDW_Brook Plan: Strm_WSE-Re 6/22/2021

MeadowBrook Reach



Appendix B

Scour Analysis

Proj. Tracy Bridge	Job No. 63738	Sheet No. 1 OF 7
Made by HHG	Checked by SPA	Backchecked by SPA
Date 6-09-2021	Date 6-10-2021	Date 6-10-2021



Scour Analysis: 100-year storm U/S face of Tracy Bridge over Meadow Brook

Aggradation/Degradation **ft**

Live Bed Vs. Clear Water

Depth of flow, y1	16.72	ft
Particle size in a mix of which 50% are smaller, D50 (m)	0.01200	m
Particle size in a mix of which 50% are smaller, D50 (ft)	0.03936	ft
Velocity of main Channel, V	0.6	ft/s
Critical Velocity, Vc	6.08	ft/s

$$V_c = K_u y^{1/6} D^{1/3}$$

(HEC-18, 5th Edition,
April 2012, Equation
6.1)

$$K_u = 11.17$$

Live Bed vs. Clear Water

Clear Water ←Type of Contraction Scour Analysis to be completed

Clear Water Scour

Discharge through the bridge, Q	383.60	ft ³ /s
Median diameter of bed material, D50	0.03936	ft
Diameter of smallest nontransportable particle, Dm	0.04920	ft
Bottom width of the contracted section, W	28.27	ft
Existing depth in the contracted Section, y0	15.90	ft

Avg depth in contracted section, y2

***Scour depth, ys**

2.75
-13.15

ft

$$y_2 = \left[\frac{K_u Q^2}{D_m^{2/3} W^2} \right]^{3/7}$$

(HEC-18, 5th Edition,
April 2012, Equation
6.4)

$$y_s = y_2 - y_0$$

*** If calculated y_s returns negative answer, the scour depth equals zero**

Proj. Tracy Bridge	Job No. 63738	Sheet No. 2 OF 7
Made by HHG	Checked by SPA	Backchecked by SPA
Date 6-09-2021	Date 6-10-2021	Date 6-10-2021



Scour Analysis: 100-year storm U/S face of Tracy Bridge over Meadow Brook

Pressure Flow Scour

Vertical size of the bridge opening prior to scour, hb	15.61 ft
Distance from the water surface to the lower face of bridge, ht	0.61 ft
Weir flow height, hw= ht - T for ht>T, hw = 0 for ht<T	0.00 ft
Upstream Channel flow depth, hu	15.9 ft
Separation Zone Thickness, t	4.05 ft

$$t = 0.5 \left(\frac{n_b \cdot n_t}{h_u^2} \right) \left(1 - \frac{h_w}{h_t} \right)$$

(HEC-18, 5th Edition, April 2012, Eqn. 6.16)

Y1 = hb + T (T = Obstruction height = 3.49 ft)	19.10 ft
Q1	383.60 ft ³ /s
Q2 = Flow through bridge opening	383.60 ft ³ /s
Top width of U/S main channel, W1	27.33 ft
Top width of the main channel in the contracted section, W2	28.27 ft
Exponent, k1	0.59
Avg depth in contracted section, y2	18.72 ft
Separation Zone Thickness, t	4.05 ft
Vertical size of the bridge prior to scour, hb	15.61 ft

$$\frac{y_2}{y_1} = \left(\frac{Q_2}{Q_1} \right)^{\frac{6}{7}} \left(\frac{W_1}{W_2} \right)^{k_1}$$

(HEC-18, April 2012, Eqn. 6.14)

Pressure Flow Scour Depth, ys	7.16 ft
--------------------------------------	----------------

$$y_s = y_2 + t - h_b$$

Proj. Tracy Bridge	Job No. 63738	Sheet No. 3 OF 7
Made by HHG	Checked by SPA	Backchecked by SPA
Date 6-09-2021	Date 6-10-2021	Date 6-10-2021



Scour Analysis: 100-year storm U/S face of Tracy Bridge over Meadow Brook

**Local Scour at Abutments
Near Abutment**

Coefficient for Abutment Shape, K1	0.55
Coefficient for angle of embankment to flow, K2	1.00
Length of active flow obstructed by embankment, L'	3.16 ft
Average depth of flow on embankment, ya	5.40 ft
Velocity on embankment, Ve	0.17 ft/s
Froude Number of approach flow = $V_e/(gy_a)^{1/2}$	0.013
Length of embankment projected to normal flow, L	ft

$$\frac{y_s}{y_a} = 2.27K_1K_2 \left(\frac{L'}{y_a}\right)^{0.43} (Fr)^{0.61} + 1$$

Near Abutment Scour Depth, ys

5.78 ft

(HEC-18, 5th Edition, April 2012, Equation 8.1)

Far Abutment

Coefficient for Abutment Shape, K1	0.55
Coefficient for angle of embankment to flow, K2	1.00
Length of active flow obstructed by embankment, L'	5.13 ft
Average depth of flow on embankment, ya	2.83 ft
Velocity on embankment, Ve	0.11 ft/s
Froude Number of approach flow = $V_e/(gy_a)^{1/2}$	0.012
Length of embankment projected to normal flow, L	ft

$$\frac{y_s}{y_a} = 2.27K_1K_2 \left(\frac{L'}{y_a}\right)^{0.43} (Fr)^{0.61} + 1$$

Far Abutment Scour Depth, ys

3.13 ft

(HEC-18, 5th Edition, April 2012, Equation 8.1)

Proj. Tracy Bridge	Job No. 63738	Sheet No. 4 OF 7
Made by HHG	Checked by SPA	Backchecked by SPA
Date 6-09-2021	Date 6-10-2021	Date 6-10-2021



Scour Analysis: 500-year storm U/S face of Tracy Bridge over Meadow Brook

Aggradation/Degradation ft

Live Bed Vs. Clear Water

Depth of flow, y1	23.01	ft
Particle size in a mix of which 50% are smaller, D50 (m)	0.01200	m
Particle size in a mix of which 50% are smaller, D50 (ft)	0.03936	ft
Velocity of main Channel, V	0.48	ft/s
Critical Velocity, Vc	6.41	ft/s

(HEC-18, 5th Edition, April 2012, Equation 6.1)

Live Bed vs. Clear Water

Clear Water ←Type of Contraction Scour Analysis to be completed

Clear Water Scour

Discharge through the bridge, Q	413.26	ft ³ /s
Median diameter of bed material, D50	0.03936	ft
Diameter of smallest nontransportable particle, Dm	0.04920	ft
Bottom width of the contracted section, W	28.27	ft
Existing depth in the contracted Section, y0	15.90	ft

Avg depth in contracted section, y2

***Scour depth, ys**

2.93
-12.97

$$y_2 = \left[\frac{K_u Q^2}{D_m^{2/3} W^2} \right]^{3/7}$$

$$y_s = y_2 - y_0$$

(HEC-18, 5th Edition, April 2012, Equations 6.2 and 6.3)

*** If calculated ys returns negative answer, the scour depth equals zero**

Proj. Tracy Bridge	Job No. 63738	Sheet No. 5 OF 7
Made by HHG	Checked by SPA	Backchecked by SPA
Date 6-09-2021	Date 6-10-2021	Date 6-10-2021



Scour Analysis: 500-year storm U/S face of Tracy Bridge over Meadow Brook

Pressure Flow Scour

Vertical size of the bridge opening prior to scour, hb
Distance from the water surface to the lower face of bridge, ht
Weir flow height, hw= ht - T for ht>T, hw = 0 for ht<T
Upstream Channel flow depth, hu
Separation Zone Thickness, t

15.61	ft
6.9	ft
3.41	ft
22.19	ft
6.17	ft

$$t = 0.5 \left(\frac{h_b \cdot h_t}{h_u^2} \right) \left(1 - \frac{h_w}{h_t} \right)$$

(HEC-18, 5th Edition, April 2012, Eqn. 6.16)

Y1 = hb + T (T = Obstruction height = 3.49 ft)

Q1

Q2 = Flow through bridge opening

Top width of U/S main channel, W1

Top width of the main channel in the contracted section, W2

Exponent, k1

Avg depth in contracted section, y2

Separation Zone Thickness, t

Vertical size of the bridge prior to scour, hb

19.10	ft
510.20	ft ³ /s
413.26	ft ³ /s
27.33	ft
28.27	ft
0.59	
15.63	ft
6.17	ft
15.61	ft

$$\frac{y_2}{y_1} = \left(\frac{Q_2}{Q_1} \right)^{\frac{6}{7}} \left(\frac{W_1}{W_2} \right)^{k_1}$$

(HEC-18, April 2012, Eqn. 6.14)

Pressure Flow Scour Depth, ys

6.18	ft
------	----

$$y_s = y_2 + t - h_b$$

Proj. Tracy Bridge	Job No. 63738	Sheet No. 6 OF 7
Made by HHG	Checked by SPA	Backchecked by SPA
Date 6-09-2021	Date 6-10-2021	Date 6-10-2021



Scour Analysis: 500-year storm U/S face of Tracy Bridge over Meadow Brook

Local Scour at Abutments Near Abutment

Coefficient for Abutment Shape, K1
 Coefficient for angle of embankment to flow, K2
 Length of active flow obstructed by embankment, L'
 Average depth of flow on embankment, ya
 Velocity on embankment, Ve
 Froude Number of approach flow = $V_e/(gy_a)^{1/2}$
 Length of embankment projected to normal flow, L

0.55
1.00
3.16 ft
3.21 ft
0.06 ft/s
0.006
ft

$$\frac{y_s}{y_a} = 2.27K_1K_2 \left(\frac{L'}{y_a}\right)^{0.43} (Fr)^{0.61} + 1$$

(HEC-18, 5th Edition,
April 2012, Equation
8.1)

Near Abutment Scour Depth, ys

3.38 ft

Far Abutment

Coefficient for Abutment Shape, K1
 Coefficient for angle of embankment to flow, K2
 Length of active flow obstructed by embankment, L'
 Average depth of flow on embankment, ya
 Velocity on embankment, Ve
 Froude Number of approach flow = $V_e/(gy_a)^{1/2}$
 Length of embankment projected to normal flow, L

0.55
1.00
5.13 ft
8.23 ft
0.12 ft/s
0.007
ft

$$\frac{y_s}{y_a} = 2.27K_1K_2 \left(\frac{L'}{y_a}\right)^{0.43} (Fr)^{0.61} + 1$$

(HEC-18, 5th Edition,
April 2012, Equation
8.1)

Far Abutment Scour Depth, ys

8.65 ft

Proj. Tracy Bridge	Job No. 63738	Sheet No. 7 OF 7
Made by HHG	Checked by SPA	Backchecked by SPA
Date 6-9-2021	Date 6-10-2021	Date 6-10-2021



Scour Summary

	100 - year storm	
	Near Abutment	Far Abutment
Aggradation/ Degradation (ft)	0.00	0.00
Contraction/Expansion Scour (ft)	0.00	0.00
Local Scour (ft)	5.78	3.13
Pressure Flow Scour (ft)	7.16	7.16
TOTAL SCOUR (ft)	<u>12.94</u>	<u>10.29</u>

	500-year storm	
	Near Abutment	Far Abutment
Aggradation/ Degradation (ft)	0.00	0.00
Contraction/Expansion Scour (ft)	0.00	0.00
Local Scour (ft)	3.38	8.65
Pressure Flow Scour (ft)	6.18	6.18
TOTAL SCOUR (ft)	<u>9.56</u>	<u>14.83</u>

Plan: Strm_WSE MeadowBrook Reach RS: 170.8144 Profile: 100

E.G. Elev (ft)	83.52	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.00	Wt. n-Val.	0.075	0.040	0.075
W.S. Elev (ft)	83.52	Reach Len. (ft)	43.00	39.24	17.77
Crit W.S. (ft)	68.38	Flow Area (sq ft)	271.46	456.97	357.76
E.G. Slope (ft/ft)	0.000007	Area (sq ft)	271.46	456.97	357.76
Q Total (cfs)	383.60	Flow (cfs)	42.21	275.28	66.10
Top Width (ft)	125.71	Top Width (ft)	48.06	27.33	50.32
Vel Total (ft/s)	0.35	Avg. Vel. (ft/s)	0.16	0.60	0.18
Max Chl Dpth (ft)	17.77	Hydr. Depth (ft)	5.65	16.72	7.11
Conv. Total (cfs)	148700.5	Conv. (cfs)	16364.3	106712.7	25623.6
Length Wtd. (ft)	36.23	Wetted Per. (ft)	51.14	28.99	52.05
Min Ch El (ft)	65.75	Shear (lb/sq ft)	0.00	0.01	0.00
Alpha	2.16	Stream Power (lb/ft s)	0.00	0.00	0.00
Frctn Loss (ft)	0.00	Cum Volume (acre-ft)	1.05	1.10	0.88
C & E Loss (ft)	0.00	Cum SA (acres)	0.19	0.06	0.16

UNCONTRACTED SECTION

Plan: Strm_WSE MeadowBrook Reach RS: 170.8144 Profile: 500

E.G. Elev (ft)	89.81	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.00	Wt. n-Val.	0.075	0.040	0.075
W.S. Elev (ft)	89.81	Reach Len. (ft)	43.00	39.24	17.77
Crit W.S. (ft)	68.82	Flow Area (sq ft)	708.14	628.75	717.12
E.G. Slope (ft/ft)	0.000003	Area (sq ft)	708.14	628.75	717.12
Q Total (cfs)	510.20	Flow (cfs)	90.12	304.53	115.55
Top Width (ft)	182.26	Top Width (ft)	90.90	27.33	64.03
Vel Total (ft/s)	0.25	Avg. Vel. (ft/s)	0.13	0.48	0.16
Max Chl Dpth (ft)	24.06	Hydr. Depth (ft)	7.79	23.01	11.20
Conv. Total (cfs)	304292.6	Conv. (cfs)	53748.8	181630.0	68913.8
Length Wtd. (ft)	33.04	Wetted Per. (ft)	94.44	28.99	67.13
Min Ch El (ft)	65.75	Shear (lb/sq ft)	0.00	0.00	0.00
Alpha	2.41	Stream Power (lb/ft s)	0.00	0.00	0.00
Frctn Loss (ft)	0.00	Cum Volume (acre-ft)	3.27	1.42	2.30
C & E Loss (ft)	0.00	Cum SA (acres)	0.76	0.07	0.40

UNCONTRACTED SECTION

Plan: Strm_WSE MeadowBrook Reach RS: 131.6994 Profile: 100

E.G. Elev (ft)	83.52	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.01	Wt. n-Val.	0.075	0.040	0.075
W.S. Elev (ft)	83.52	Reach Len. (ft)	42.77	42.77	42.77
Crit W.S. (ft)	69.13	Flow Area (sq ft)	204.04	449.52	478.87
E.G. Slope (ft/ft)	0.000009	Area (sq ft)	204.04	449.52	478.87
Q Total (cfs)	383.60	Flow (cfs)	35.00	293.70	54.91
Top Width (ft)	235.58	Top Width (ft)	37.81	28.27	169.50
Vel Total (ft/s)	0.34	Avg. Vel. (ft/s)	0.17	0.65	0.11
Max Chl Dpth (ft)	17.01	Hydr. Depth (ft)	5.40	15.90	2.83
Conv. Total (cfs)	130833.7	Conv. (cfs)	11935.8	100170.7	18727.3
Length Wtd. (ft)	42.77	Wetted Per. (ft)	40.22	30.60	172.68
Min Ch El (ft)	66.51	Shear (lb/sq ft)	0.00	0.01	0.00
Alpha	2.89	Stream Power (lb/ft s)	0.00	0.01	0.00
Frctn Loss (ft)		Cum Volume (acre-ft)	0.82	0.69	0.71
C & E Loss (ft)		Cum SA (acres)	0.15	0.03	0.12

CONTRACTED SECTION

Plan: Strm_WSE MeadowBrook Reach RS: 131.6994 Profile: 500

E.G. Elev (ft)	89.81	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.00	Wt. n-Val.	0.075	0.040	0.075
W.S. Elev (ft)	89.81	Reach Len. (ft)	42.77	42.77	42.77
Crit W.S. (ft)	69.59	Flow Area (sq ft)	726.13	627.25	1704.61
E.G. Slope (ft/ft)	0.000002	Area (sq ft)	726.13	627.25	1704.61
Q Total (cfs)	510.20	Flow (cfs)	46.39	260.79	203.02
Top Width (ft)	461.54	Top Width (ft)	226.07	28.27	207.20
Vel Total (ft/s)	0.17	Avg. Vel. (ft/s)	0.06	0.42	0.12
Max Chl Dpth (ft)	23.30	Hydr. Depth (ft)	3.21	22.19	8.23
Conv. Total (cfs)	341467.9	Conv. (cfs)	31051.1	174539.3	135877.5
Length Wtd. (ft)	42.77	Wetted Per. (ft)	228.99	30.60	211.22
Min Ch El (ft)	66.51	Shear (lb/sq ft)	0.00	0.00	0.00
Alpha	3.39	Stream Power (lb/ft s)	0.00	0.00	0.00
Frctn Loss (ft)	0.00	Cum Volume (acre-ft)	2.56	0.85	1.80
C & E Loss (ft)	0.00	Cum SA (acres)	0.60	0.04	0.35

**CONTRACTED
SECTION**

Plan: Strm_WSE MeadowBrook Reach RS: 60.71 Profile: 100

E.G. US. (ft)	83.52	Element	Inside BR US	Inside BR DS
W.S. US. (ft)	83.52	E.G. Elev (ft)	83.52	83.51
Q Total (cfs)	383.60	W.S. Elev (ft)	82.91	82.91
Q Bridge (cfs)	383.60	Crit W.S. (ft)	69.44	68.89
Q Weir (cfs)		Max Chl Dpth (ft)	16.40	18.11
Weir Sta Lft (ft)		Vel Total (ft/s)	0.48	1.05
Weir Sta Rgt (ft)		Flow Area (sq ft)	803.59	366.15
Weir Submerg		Froude # Chl	0.03	0.05
Weir Max Depth (ft)		Specif Force (cu ft)	5402.12	2807.35
Min El Weir Flow (ft)	86.22	Hydr Depth (ft)		14.73
Min El Prs (ft)	82.91	W.P. Total (ft)	151.30	33.15
Delta EG (ft)	0.02	Conv. Total (cfs)	65898.2	71410.0
Delta WS (ft)	0.02	Top Width (ft)		62.48
BR Open Area (sq ft)	366.15	Frctn Loss (ft)		
BR Open Vel (ft/s)	1.05	C & E Loss (ft)		
BR Sluice Coef		Shear Total (lb/sq ft)	0.01	0.02
BR Sel Method	Press Only	Power Total (lb/ft s)	0.01	0.02

Plan: Strm_WSE MeadowBrook Reach RS: 60.71 Profile: 500

E.G. US. (ft)	89.81	Element	Inside BR US	Inside BR DS
W.S. US. (ft)	89.81	E.G. Elev (ft)	89.81	89.80
Q Total (cfs)	510.20	W.S. Elev (ft)	89.80	89.80
Q Bridge (cfs)	413.26	Crit W.S. (ft)	69.94	69.41
Q Weir (cfs)		Max Chl Dpth (ft)	23.29	25.00
Weir Sta Lft (ft)		Vel Total (ft/s)	0.37	0.37
Weir Sta Rgt (ft)		Flow Area (sq ft)	1378.41	1381.05
Weir Submerg		Froude # Chl	0.02	0.02
Weir Max Depth (ft)		Specif Force (cu ft)	11527.19	10055.29
Min El Weir Flow (ft)	86.22	Hydr Depth (ft)	3.74	4.02
Min El Prs (ft)	82.91	W.P. Total (ft)	515.52	494.81
Delta EG (ft)	0.01	Conv. Total (cfs)	75446.6	77297.2
Delta WS (ft)	0.00	Top Width (ft)	368.99	343.83
BR Open Area (sq ft)	366.15	Frctn Loss (ft)	0.00	0.00
BR Open Vel (ft/s)	0.59	C & E Loss (ft)	0.00	0.00
BR Sluice Coef		Shear Total (lb/sq ft)	0.01	0.01
BR Sel Method	Energy only	Power Total (lb/ft s)	0.00	0.00

D50 Fall Velocity

correctly account for the increase in transport that will occur as the result of the bed planing out (which decreases resistance to flow, increases the velocity and the transport of bed material at the bridge). That is, Laursen's equation indicates a decrease in scour for this case, whereas in reality, there would be an increase in scour depth. In addition, at flood flows, a plane bedform will usually exist upstream and through the bridge waterway, and the values of Manning n will be equal. Consequently, the n value ratio is not recommended or presented in Equation 6.2.

4. W_1 and W_2 are not always easily defined. In some cases, it is acceptable to use the topwidth of the main channel to define these widths. Whether topwidth or bottom width is used, it is important to be consistent so that W_1 and W_2 refer to either bottom widths or top widths.

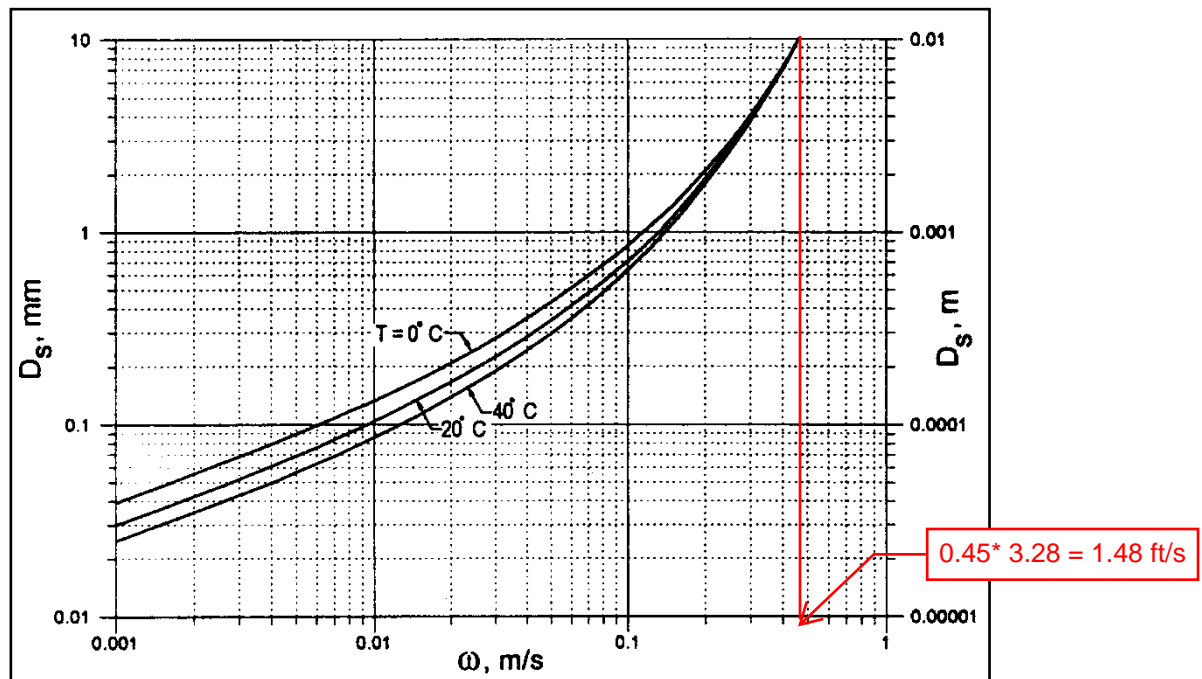


Figure 6.8. Fall velocity of sand-sized particles with specific gravity of 2.65 in metric units.

5. The average width of the bridge opening (W_2) is normally taken as the bottom width, with the width of the piers subtracted.
6. Laursen's equation will overestimate the depth of scour at the bridge if the bridge is located at the upstream end of a natural contraction or if the contraction is the result of the bridge abutments and piers. At this time, however, it is the best equation available.
7. In sand channel streams where the contraction scour hole is filled in on the falling stage, the y_0 depth may be approximated by y_1 . Sketches or surveys through the bridge can help in determining the existing bed elevation.
8. **Scour depths with live-bed contraction scour may be limited by coarse sediments in the bed material armoring the bed. Where coarse sediments are present, it is recommended that scour depths be calculated for live-bed scour conditions using the clear-water scour equation (given in the next section) in addition to the live-bed equation, and that the smaller calculated scour depth be used.**

Appendix C

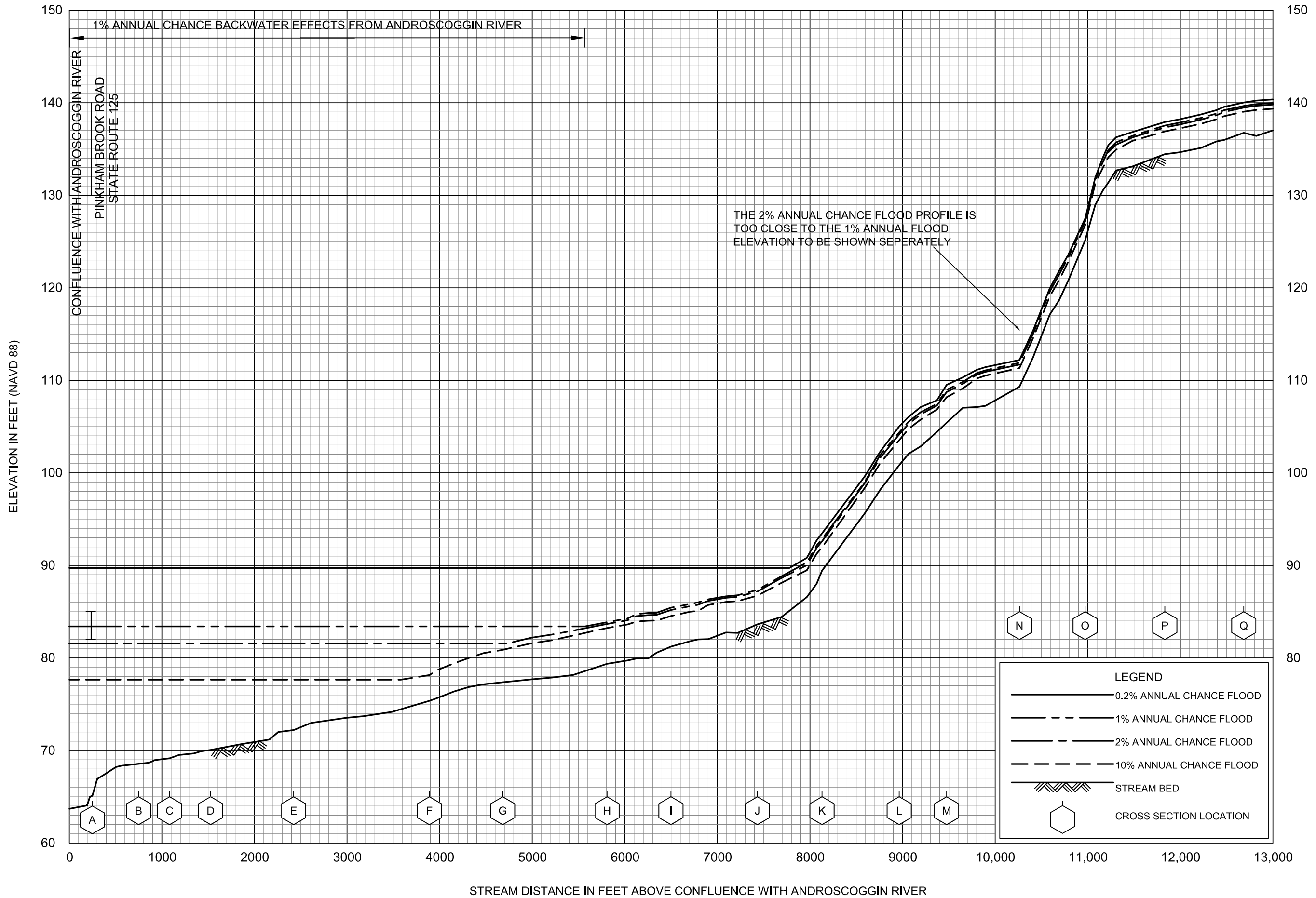
FEMA Flood Insurance Study

Table 7 – Manning’s “n” Values (Continued)

<u>Stream</u>	<u>Channel</u>	<u>Overbank</u>
Hart Brook	0.045-0.060	0.040-0.120
Hooper Brook	0.040-0.065	0.078-0.095
Hooper Brook Tributary 1	0.065-0.067	0.078-0.095
House Brook	0.045	0.045-0.150
Jepson Brook	0.013-0.050	0.032-0.120
Lake Auburn	0.032-0.040	0.032-0.120
Lapham Brook	0.035-0.045	0.035-0.090
Little Androscoggin River	0.050	0.035-0.110
Lively Brook	*	*
Lower Range Pond	0.032	0.032-0.090
Martin Stream	*	*
Maxwell Brook	0.060-0.075	0.080-0.085
Meadow Brook	0.045	0.035-0.150
Moody Brook	0.035-0.045	0.050-0.080
Newell Brook	0.050	0.032-0.130
Nezinscot River	0.040-0.052	0.060-0.080
No Name Brook	0.040-0.045	0.030-0.150
No Name Brook Tributary A	0.045-0.050	0.040-0.120
Potash Brook	0.032-0.055	0.032-0.150
Range Brook	0.032-0.060	0.035-0.090
Redwater Brook	0.032-0.055	0.032-0.150
Sabattus River	0.015-0.062	0.017-0.092
Salmon Brook	0.040	0.040-0.150
Soper Mill Brook	0.045	0.040-0.090
Stetson Brook	0.065	0.035-0.150
Taylor Brook	0.035-0.070	0.030-0.120
Taylor Brook Tributary 1	0.050	0.100
The Basin	0.032	0.032-0.120
Tributary A to Little Androscoggin River	0.045	0.032-0.090
Tributary A to Soper Mill Brook	0.045	0.032-0.150
Waterhouse Brook	0.060	0.040-0.090
Winter Brook	0.035-0.060	0.060-0.105
Worthley Brook	0.055	0.035-0.150
Worthley Pond	0.032-0.055	0.055-0.120

*Data not available

All qualifying benchmarks within a given jurisdiction that are catalogued by the National Geodetic Survey (NGS) and entered into the National Spatial Reference System (NSRS) as First or Second Order Vertical and have a vertical stability classification of A, B or C are shown and labeled on the FIRM with their 6-character NSRS Permanent Identifier.



FLOOD PROFILES

MEADOW BROOK

FEDERAL EMERGENCY MANAGEMENT AGENCY

ANDROSCOGGIN COUNTY, ME

(ALL JURISDICTIONS)

Appendix D

Peak Flow Calculations

WIN:	23657.00		
Town:	Durham		
Route No.:	125		
Asset ID:	2852		
Lat:	43.99095	Long:	-70.0542

Project Name:	Durham
Stream Name:	Meadow Brook
Bridge Name:	Tracy Brook Bridge
Analysis by:	AWM
Date:	4/30/2018

Peak Flow Calculations by USGS Regression Equations (Hodgkins, 1999 & Lombard/Hodgkins, 2015)

Enter data in blue cells only!

	km ²	mi ²	ac
A	9.84	3.80	2432.0
W	1.47	0.6	362.1
P _c	413448	4868495	
County	Androscoggin		
pptA	45.3		
SG	0.00		

Enter data in [mi²]

Watershed Area *DRNAREA*
Wetlands area (by NWI)

watershed centroid (E, N; UTM 19N; meters)
choose county from drop-down menu
mean annual precipitation (inches; by look-up)
sand & gravel aquifer as decimal fraction of watershed A

Worksheet prepared by:

Charles S. Hebson, PE
Environmental Office
Maine Dept. Transportation
Augusta, ME 04333-0016
207-557-1052
Charles.Hebson@maine.gov
ver. 2017 Jun. 09

A (km ²)	9.84	Conf Lvl	0.67
W (%)	14.89		

NWI Wetlands % *STORNWI*

References:

Hodgkins, G.A., 1999.
Estimating the magnitude of peak flows for streams
in Maine for selected recurrence intervals
WRIR 99-4008, USGS Augusta, ME

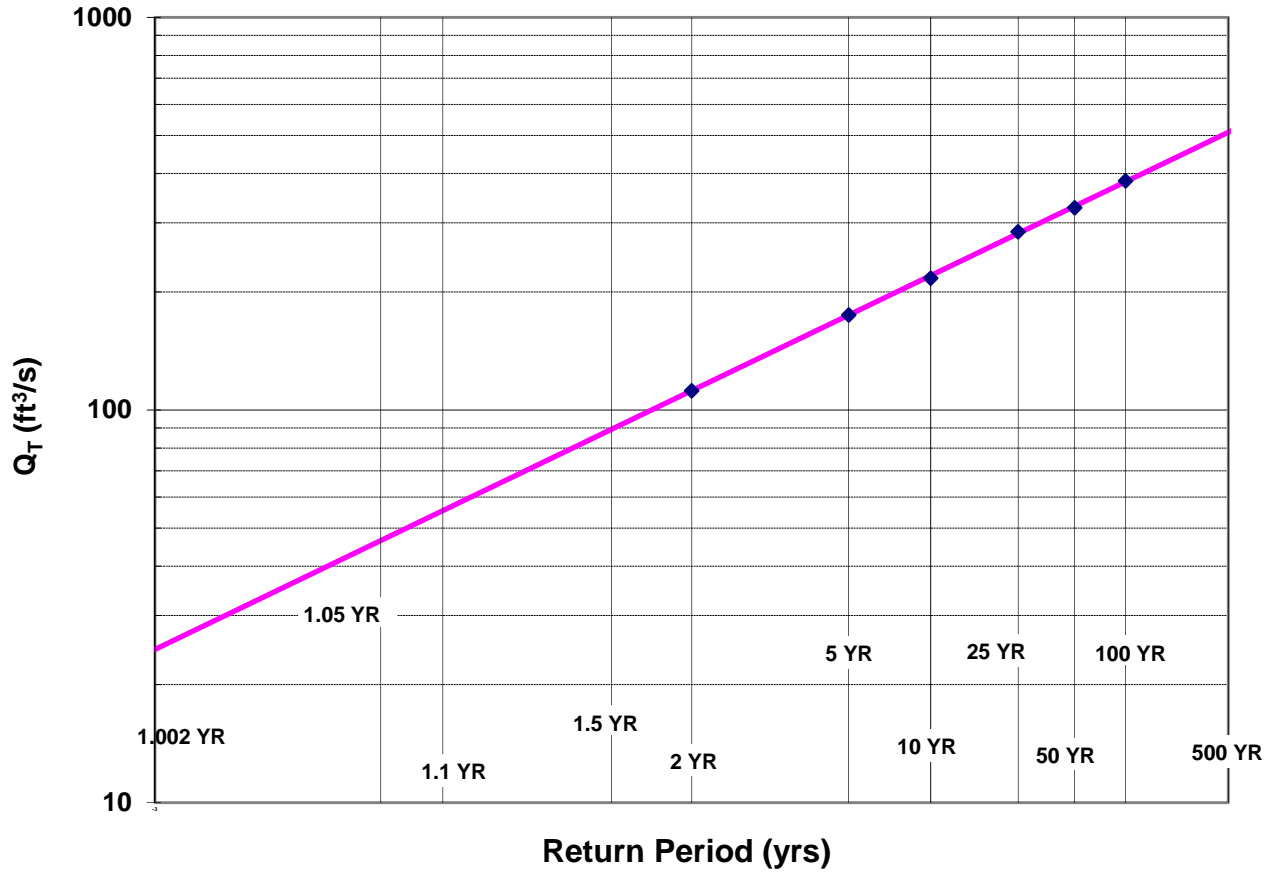
Lombard, P.J. & G.A. Hodgkins, 2015.
Peak flow regression equations for small, ungaged streams in
Maine - Comparing map-based to field-based variables
SIR 2015-4059, USGS, Augusta, ME

$$Q_T = b \times A^a \times 10^{-ww}$$

Ret Pd	Peak Flow Estimate		
T (yr)	Lower	Q _T (m ³ /s)	Upper
1.1		1.57	
2		3.17	
5		4.95	
10		6.14	
25		8.06	
50		9.28	
100		10.87	
500		14.45	

Q _T (ft ³ /s)
55.5
112.1
174.6
216.9
284.5
327.6
383.6
510.2

Log-Normal Probability Plot



WIN:	23657.00
Town:	Durham
Route No.:	125
Asset ID:	2852
Lat:	43.99095
Long:	-70.05417

Project Name:	Durham
Stream Name:	Meadow Brook
Bridge Name:	Tracy Brook Bridge
Analysis by:	AWM
Date:	4/30/2018

DO NOT ENTER ANY DATA ON THIS PAGE; EVERYTHING IS CALCULATED

MAINE MONTHLY MEDIAN FLOWS and HYDRAULIC GEOMETRY BY USGS REGRESSION EQUATIONS (2004, 2013)

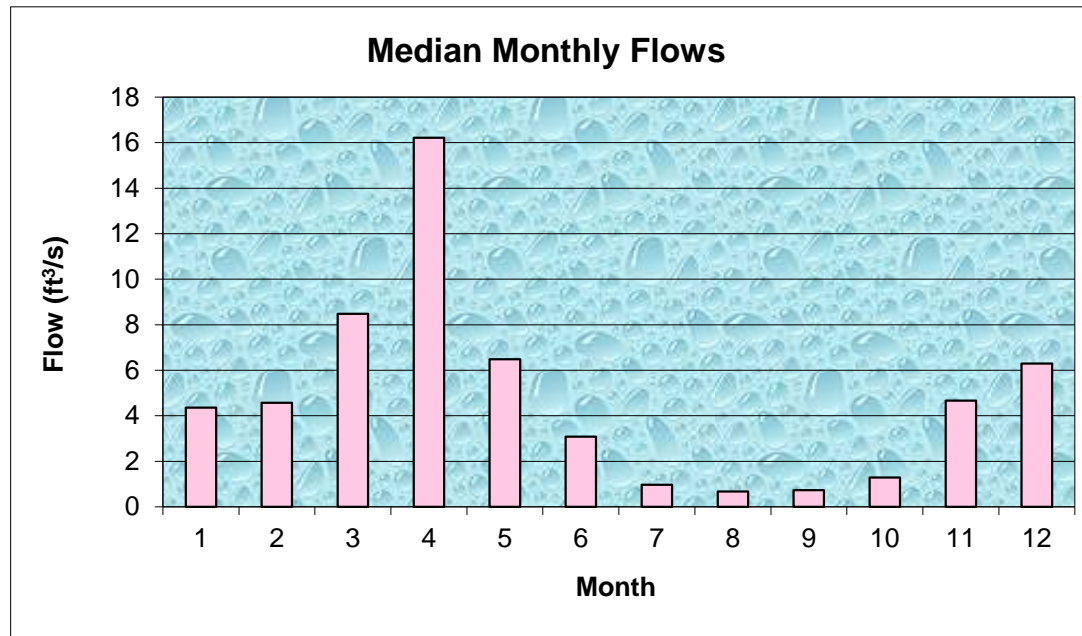
Value	Variable	Explanation
3.80	A	Area (mi ²)
413448	P _c	Watershed centroid (E,N; UTM; Zone 19; meters)
47.54	DIST	Distance from Coastal reference line (mi)
45.3	pptA	Mean Annual Precipitation (inches)
0.00	SG	Sand & Gravel Aquifer (decimal fraction of watershed area)

Month	Q _{median} (ft ³ /s)	(m ³ /s)
Jan	4.35	0.1234
Feb	4.57	0.1295
Mar	8.47	0.2401
Apr	16.22	0.4596
May	6.48	0.1837
Jun	3.09	0.0875
Jul	0.97	0.0276
Aug	0.68	0.0192
Sep	0.73	0.0206
Oct	1.29	0.0365
Nov	4.67	0.1323
Dec	6.29	0.1782

Q _{bf}	21.1
ann avg	7.9
ann med	4.0
Q _{1.002}	24.6
Q _{1.01}	32.8
Q _{1.05}	46.5
Q _{bf}	70.3

assume v = 4ft/s

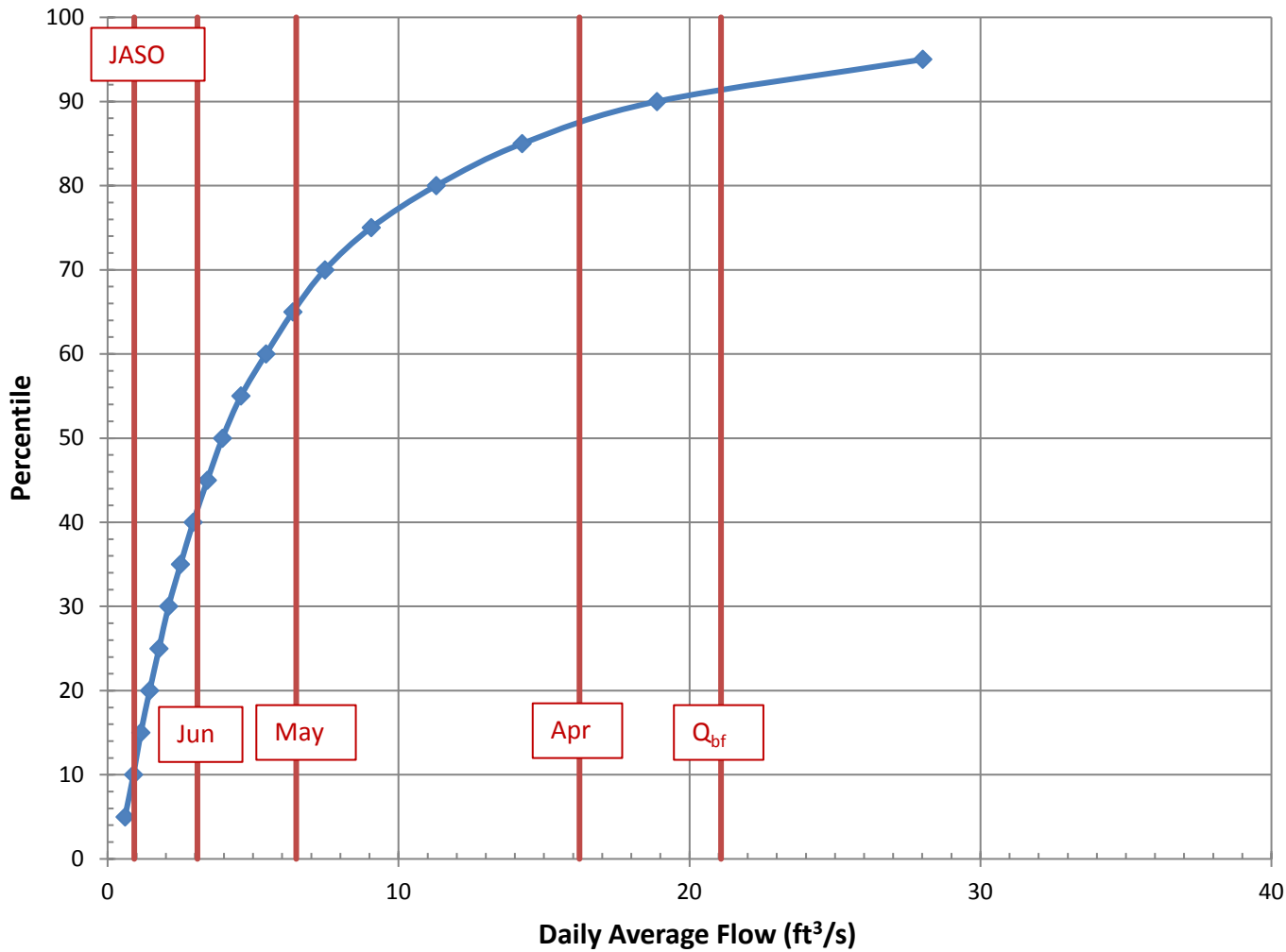
W _{bf}	18.8	estimated bankfull width (ft)
d _{bf}	0.9	estimated bankfull depth (ft)
A _{bf}	14.3	estimated bankfull flow area (ft ²)



References

- Dudley, R.W., 2013. FY2013 Progress Report - Phase 1 ..., USFWS QRP Project
- Dudley, R.W., 2004. Estimating Monthly Streamflows ... , SIR 2004-5026

Daily Average Flow Distribution



Daily Avg Flow Dist

$A_{ws} = (mi^2)$ 3.8

Q (ft³/s)

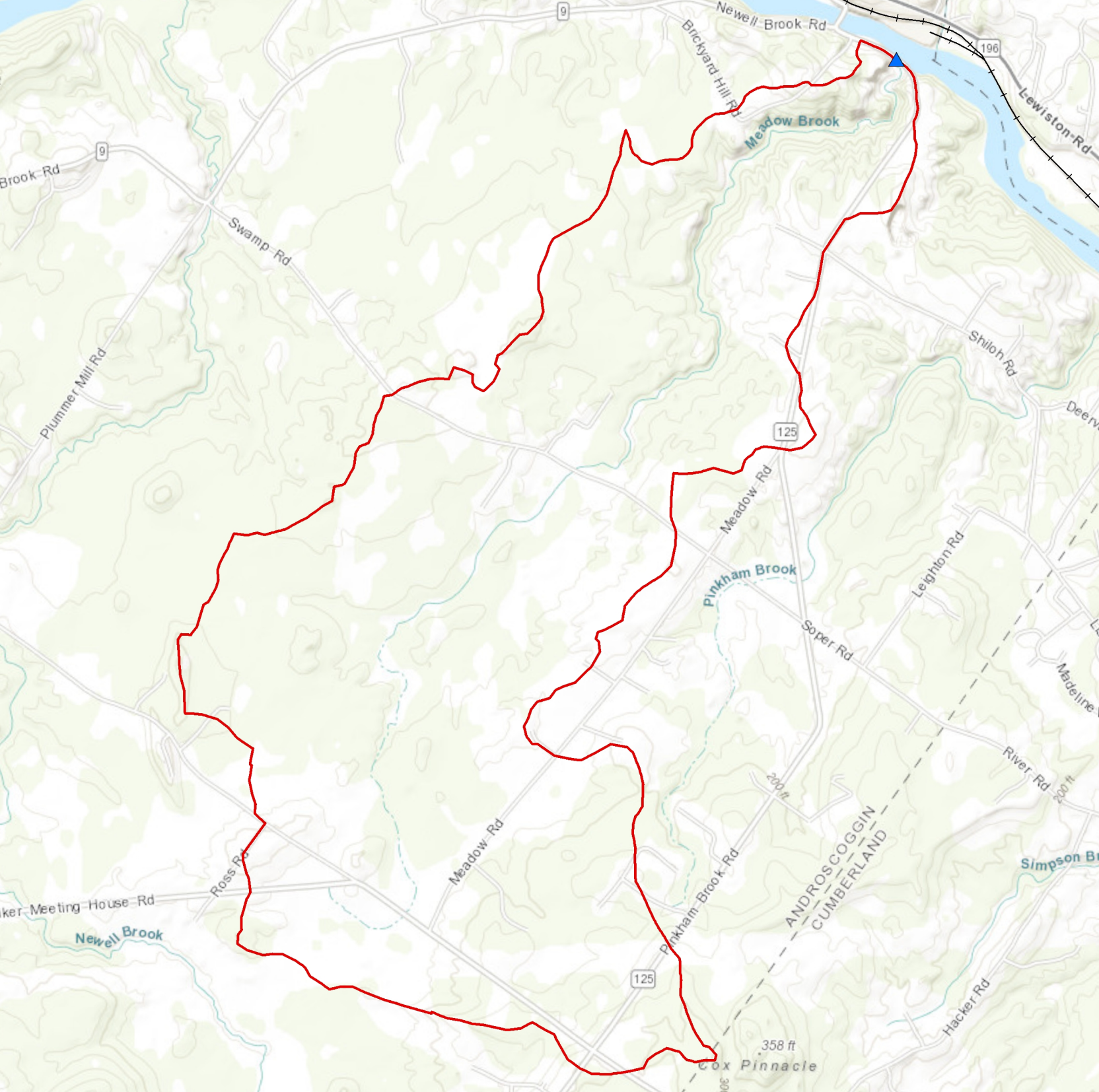
Pctl	Median	84 th pctl
5	0.60	0.96
10	0.89	1.34
15	1.14	1.67
20	1.45	2.02
25	1.77	2.37
30	2.09	2.70
35	2.51	3.09
40	2.94	3.55
45	3.43	4.01
50	3.95	4.74
55	4.58	5.51
60	5.44	6.47
65	6.37	7.54
70	7.47	8.80
75	9.06	10.58
80	11.29	12.63
85	14.25	16.19
90	18.88	21.74
95	28.02	33.81

Q_{bf} 21.1

Q_{1.002} 24.6

Q_{1.1} 55.5

Q₂ 112.1



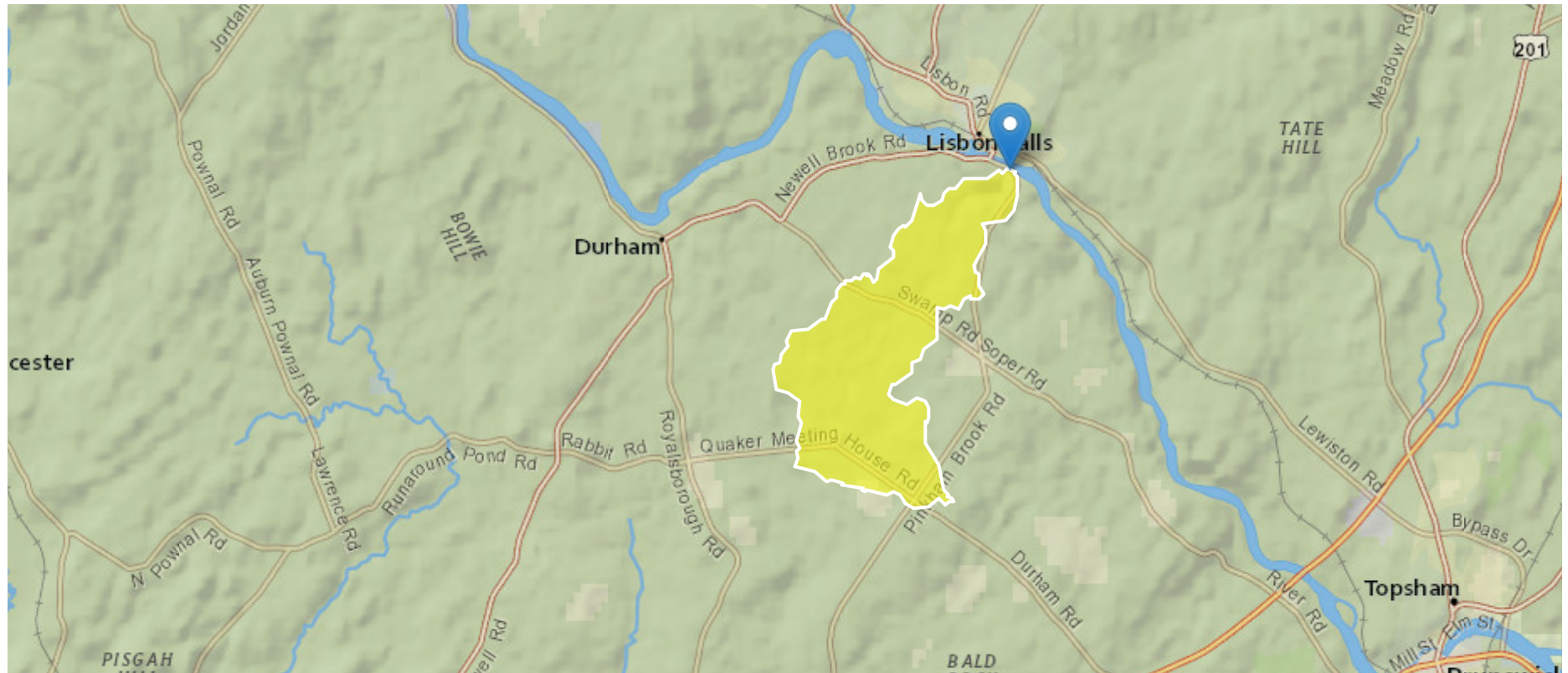
Durham 23657 Tracy Brook (Meadow Brk) at ME125 (Pinkham Brk Rd)

Region ID: ME

Workspace ID: ME20190620175639805000

Clicked Point (Latitude, Longitude): 43.99095, -70.05417

Time: 2019-06-20 13:57:00 -0400



Basin Characteristics

Parameter Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	3.8	square miles
STORNWI	Percentage of storage (combined water bodies and wetlands) from the National Wetlands Inventory	14.94	percent
SANDGRAVAF	Fraction of land surface underlain by sand and gravel aquifers	0	dimensionless
ELEV	Mean Basin Elevation	175.7	feet
BSLDEM10M	Mean basin slope computed from 10 m DEM	4.36	percent
CENTROIDX	Basin centroid horizontal (x) location in state plane coordinates	413448.4	feet
CENTROIDY	Basin centroid vertical (y) location in state plane units	4868494.8	feet
COASTDIST	Shortest distance from the coastline to the basin centroid	48	miles
ELEVMAX	Maximum basin elevation	322.9	feet
LC06WATER	Percent of open water, class 11, from NLCD 2006	0	percent
LC11DEV	Percentage of developed (urban) land from NLCD 2011 classes 21-24	5.42	percent
LC11IMP	Average percentage of impervious area determined from NLCD 2011 impervious dataset	0.94	percent
PRECIP	Mean Annual Precipitation	45.5	inches
SANDGRAVAP	Percentage of land surface underlain by sand and gravel aquifers	0	percent
STATSGOA	Percentage of area of Hydrologic Soil Type A from STATSGO	10	percent

Bankfull Statistics Parameters[Central and Coastal Bankfull 2004 5042]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	3.8	square miles	2.92	298

Bankfull Statistics Flow Report[Central and Coastal Bankfull 2004 5042]

Statistic	Value	Unit
Bankfull Streamflow	21.1	ft ³ /s
Bankfull Width	15.4	ft
Bankfull Depth	0.935	ft
Bankfull Area	14.3	ft ²

Bankfull Statistics Citations

Dudley, R.W.,2004, Hydraulic-Geometry Relations for Rivers in Coastal and Central Maine: U.S. Geological Survey Scientific Investigations Report 2004-5042, 30 p (<http://pubs.usgs.gov/sir/2004/5042/pdf/sir2004-5042.pdf>)

Peak-Flow Statistics Parameters[Statewide Peak Flow DA LT 12sqmi 2015 5049]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	3.8	square miles	0.31	12
STORNWI	Percentage of Storage from NWI	14.94	percent	0	22.2

Peak-Flow Statistics Flow Report[Statewide Peak Flow DA LT 12sqmi 2015 5049]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, SEp: Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	SEp
1.01 Year Peak Flood	33.7	ft ³ /s	38
2 Year Peak Flood	112	ft ³ /s	34
5 Year Peak Flood	174	ft ³ /s	35
10 Year Peak Flood	216	ft ³ /s	37
25 Year Peak Flood	284	ft ³ /s	39

Statistic	Value	Unit	SEp
50 Year Peak Flood	327	ft ³ /s	41
100 Year Peak Flood	383	ft ³ /s	42
250 Year Peak Flood	427	ft ³ /s	44
500 Year Peak Flood	509	ft ³ /s	47

Peak-Flow Statistics Citations

Lombard, P.J., and Hodgkins, G.A., 2015, Peak flow regression equations for small, ungaged streams in Maine— Comparing map-based to field-based variables: U.S. Geological Survey Scientific Investigations Report 2015–5049, 12 p. (<http://dx.doi.org/10.3133/sir20155049>)

Flow-Duration Statistics Parameters^[Statewide Annual SIR 2015 5151]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	3.8	square miles	14.9	1419
SANDGRAVAF	Fraction of Sand and Gravel Aquifers	0	dimensionless	0	0.212
ELEV	Mean Basin Elevation	175.7	feet	239	2120

Flow-Duration Statistics Disclaimers^[Statewide Annual SIR 2015 5151]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Flow-Duration Statistics Flow Report^[Statewide Annual SIR 2015 5151]

Statistic	Value	Unit
1 Percent Duration	0.0085	ft ³ /s
5 Percent Duration	0.0573	ft ³ /s

Statistic	Value	Unit
10 Percent Duration	0.167	ft ³ /s
25 Percent Duration	0.927	ft ³ /s
50 Percent Duration	3.34	ft ³ /s
75 Percent Duration	8.82	ft ³ /s
90 Percent Duration	20	ft ³ /s
95 Percent Duration	31.6	ft ³ /s
99 Percent Duration	79	ft ³ /s

Flow-Duration Statistics Citations

Dudley, R.W.,2015, Regression equations for monthly and annual mean and selected percentile streamflows for ungaged rivers in Maine: U.S. Geological Survey Scientific Investigations Report 2015–5151, 35 p. (<http://dx.doi.org/10.3133/sir20155151>)

Annual Flow Statistics Parameters^[Statewide Annual SIR 2015 5151]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	3.8	square miles	14.9	1419
SANDGRAVAF	Fraction of Sand and Gravel Aquifers	0	dimensionless	0	0.212
ELEV	Mean Basin Elevation	175.7	feet	239	2120

Annual Flow Statistics Disclaimers^[Statewide Annual SIR 2015 5151]

One or more of the parameters is outside the suggested range. Estimates were extrapolated with unknown errors

Annual Flow Statistics Flow Report^[Statewide Annual SIR 2015 5151]

Statistic	Value	Unit
Mean Annual Flow	8.14	ft ³ /s

Annual Flow Statistics Citations

Dudley, R.W.,2015, Regression equations for monthly and annual mean and selected percentile streamflows for ungaged rivers in Maine: U.S. Geological Survey Scientific Investigations Report 2015–5151, 35 p. (<http://dx.doi.org/10.3133/sir20155151>)

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Application Version: 4.3.8