## STATE OF MAINE DEPARTMENT OF TRANSPORTATION

#### **SPECIFICATIONS**

Design: Load and Resistance Factor Design per AASHTO LRFD Bridge Design Specifications, Ninth Edition 2020.

#### **DESIGN LOADING**

ive Load \_\_\_\_\_ Maine Legal Loads

#### **MATERIALS**

	Class "LP"
er	Class "A"
ng:	
einforcing Steel	ASTM A615, Grade 60
l Steel:	
erial (except as noted)	ASTM A709, Grade 50
	ASTM F3125, Grade A325, Type 1
	er ng: einforcing Steel I Steel: erial (except as noted) trength Bolts

#### BASIC DESIGN STRESSES

f 'c = $4,000 \text{ ps}$
f 'c = $5,000 \text{ ps}$
·
$f y = 60,000 ps$
-
$F y = 50,000 ps$
F $\mu = 120,000 \text{ ps}$
-



# BENEDICTA AROOSTOOK COUNTY CASEY ROAD / I-95 BRIDGE

OVER I-95

FEDERAL AID PROJECT NO. 2623800 BRIDGE NO. 6165

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#### **UTILITIES**

None

#### TRAFFIC DATA

Current (2022) AADT	160
Future (2042) AADT	180
DHV - % of AADT	12%
Design Hour Volume	22
Heavy Trucks (% of AADT)	16%
Heavy Trucks (% of DHV)	29%
Directional Distribution (% of DHV)	76%
18 kip Equivalent P 2.0	20
18 kip Equivalent P 2.5	19
Design Speed (mph)	45 MPH

#### MAINTENANCE OF TRAFFIC

Casey Road - Staged construction with alternating one-way traffic using temporary traffic signals.

I-95 - Single lane closures allowed for specific work activities.

Lat. 45°48'00.5" N Long. 68°25'37.7" W

PROJECT LOCATION	CASEY ROAD / I-95 BRIDGE (\$6165) OVER INTERSTATE 95 LOCATED 0.7 MILES WEST OF AROOSTOOK ROAD	
OUTLINE OF WORK	BRIDGE DECK REPLACEMENT	

WIN 26238.00

CASEY ROAD / I-95 BRIDGE

PROJECT MAN

EDICTA

AROOSTOOK COUNTY

CONSULTANT

PROJECT RESI

CONTRACTOR

SHEET NUMBER

**I**OF 29

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	<u>ESTIMATED QUANTITIES</u>		
ITEM NO.	ITEM DESCRIPTION	QUANTITY	UNIT
202.10	REMOVING EXISTING SUPERSTRUCTURE PROPERTY OF CONTRACTOR (430 CY)	1	LS
202.121	REMOVING EXISTING CONCRETE (54 CY)	1	LS
202.13	REMOVING EXISTING RAILINGS (RETAINED BY DEPARTMENT)	910	LF
202.202	REMOVING PAVEMENT SURFACE	1100	SY
203.20	COMMON EXCAVATION	250	CY
203.25	GRANULAR BORROW	55	CY
206.082	STRUCTURAL EARTH EXCAVATION - MAJOR STRUCTURES, PLAN QUANTITY	73	CY
304.10	AGGREGATE SUBBASE COURSE - GRAVEL	130	CY
403.208	HOT MIX ASPHALT 12.5 MM HMA SURFACE HOT MIX ASPHALT 12.5 MM BASE	110 23	T T
403.213	BITUMINOUS TACK COAT, APPLIED	71	G
502.219	STRUCTURAL CONCRETE, ABUTMENTS AND RETAINING WALLS (25 CY)	1	LS
502.26	STRUCTURAL CONCRETE ROADWAY AND SIDEWALK SLAB ON STEEL BRIDGES (360 CY)	1	LS
502.291	SAW CUT GROOVING (10500 SF)	1	LS
502.31	STRUCTURAL CONCRETE APPROACH SLAB (18 CY)	1	LS
502.49	STRUCTURAL CONCRETE CURBS AND SIDEWALKS (43 CY)	1	LS
502.77	FRP BRIDGE DRAIN, TYPE D	4	EA
503.12	REINFORCING STEEL, FABRICATED AND DELIVERED	142100	LB
503.13	REINFORCING STEEL, PLACING	142100	LB
503.17	MECHANICAL/WELDED SPLICE	1850	ΕA
504.70	STRUCTURAL STEEL FABRICATED AND DELIVERED (430 LB)	1	LS
504.71	STRUCTURAL STEEL ERECTION (430 LB)	1	LS
505.08	SHEAR CONNECTORS (6280 EA)	1	LS
506.1775	FIELD PAINTING, NEW AND EXISTING STEEL WITH ZINC RICH PAINT (20 SF)	1	LS
507.0821	STEEL BRIDGE RAILING, 3 BAR (920 LF)	1	LS
507.0822	STEEL APPROACH RAILING: 3-BAR	4	EA
507.131	TEMPORARY BRIDGE RAIL (900 LF)	1	LS
508.13	SHEET WATERPROOFING MEMBRANE (50 SY)	1	LS
515.21	PROTECTIVE COATING FOR CONCRETE SURFACES (2200 SY)	1	LS
520.21	EXPANSION DEVICE - GLAND SEAL	2	EA
523.52	BEARING INSTALLATION	8	EA
523.5402	LAMINATED ELASTOMERIC BEARINGS, EXPANSION	8	EA
524.301	TEMPORARY STRUCTURAL SUPPORT (BEARING REPLACEMENT)	1	LS
524.301	TEMPORARY STRUCTURAL SUPPORT, TEMPORARY SPACING	1	LS
524.301	TEMPORARY STRUCTURAL SUPPORT - TEMPORARY BRACING	1	LS
524.40 526.301	PROTECTIVE SHIELD PORTABLE CONCRETE BARRIER, TYPE I (230 LF)	1	LS LS
526.305	TEMPORARY CONCRETE BARRIER, BRACED TYPE I (100 LF)	1	LS
527.33	TRUCK MOUNTED ATTENUATOR	2	EA
527.34	WORK ZONE CRASH CUSHIONS	2	UN
606.1301	31" W-BEAM GUARDRAIL, MID-WAY SPLICE-SINGLE FACED	370	LF
	31" W-BEAM GUARDRAIL, MID-WAY SPLCE FLARED TERMINAL	3	EA
606.1721	BRIDGE TRANSITION - TYPE 1	4	EA
606.353	REFLECTORIZED FLEXIBLE GUARDRAIL MARKER	20	EΑ
607.183	CHAIN LINK SNOW FENCE 33 INCH (260 LF)	1	LS
610.08	PLAIN RIPRAP	120	CY
613.319	EROSION CONTROL BLANKET	85	SY
615.07	LOAM	5	CY
618.14	SEEDING METHOD NUMBER 2	1	UN
619.12	MULCH	1	UN
619.14	EROSION CONTROL MIX	10	CY
620.58	EROSION CONTROL GEOTEXTILE	180	SY
627.733	4" WHITE OR YELLOW PAINTED PAVEMENT MARKING LINE	3150	LF
627.75	WHITE OR YELLOW PAVEMENT & CURB MARKING	44	SF
627.77	REMOVING EXISTING PAVEMENT MARKING	440	SF
627.78	TEMPORARY 4" PAINTED PAVEMENT MARKING LINE, WHITE OR YELLOW	1150	LF
629.05	HAND LABOR, STRAIGHT TIME	25	HR
631.10	AIR COMPRESSOR (INCLUDING OPERATOR)	5	HR
631.11	AIR TOOL (INCLUDING OPERATOR)	5	HR
631.12	ALL PURPOSE EXCAVATOR (INCLUDING OPERATOR)  TRUCK - LARGE (INCLUDING OPERATOR)	25 25	HR
631.172	TRUCK - LARGE (INCLUDING OPERATOR)	25	HR

	<u>ESTIMATED QUANTITIES</u>							
ITEM NO.	I NO. ITEM DESCRIPTION							
631.22	FRONT END LOADER (INCLUDING OPERATOR)	25	HR					
639.19	FIELD OFFICE TYPE B	1	EA					
643.72	TEMPORARY TRAFFIC SIGNAL	1	LS					
652.30	FLASHING ARROW	2	EA					
652.312	TYPE III BARRICADE	4	EA					
652.33	DRUM	70	EA					
652.34	CONE	10	EA					
652.35	CONSTRUCTION SIGNS	690	SF					
652.361	MAINTENANCE OF TRAFFIC CONTROL DEVICES	1	LS					
652.38	FLAGGERS	320	HR					
652.41	PORTABLE-CHANGEABLE MESSAGE SIGN	2	EA					
656.75	TEMPORARY SOIL EROSION & WATER POLLUTION CONTROL	1	LS					
659.10	MOBILIZATION	1	LS					

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
FEDERAL AID PROJECT NO. 2623800 WIN 26238,00 BRIDGE NO. 6165 P.E. NUMBER AROOSTOOK COUNTY CASEY ROAD / I-95 BRIDGE ESTIMATED QUANTITIES BENEDICTA SHEET NUMBER 2

- 1. Approximate Right-Of-Way lines were developed from record plans and are shown on the Plans. All work shall be performed within the existing Right-Of-Way.
- All clearing shall be considered incidental to the Contract and no separate payment will be made. Clear trees and shrubs under the bridge and to a 15' offset on both sides of the bridge, as directed by the Resident.
- All utility facilities shall be adjusted by the respective utilities unless otherwise noted.
- Existing signs within the Project limits shall be removed and reset as directed by the Resident. Payment for removal and reinstallation of existing signs will be considered incidental to the Contract. No separate payment will be made.
- 5. Do not excavate for Aggregate Subbase Course where existing material is suitable as determined by the Resident.
- In areas where the Resident directs the Contractor not to excavate to the subgrade line shown on the Plans, payment for removing existing pavement, grubbing, shaping, ditching, and compacting the existing subbase and layers of new subbase 6 inches or less thick will be made under appropriate equipment rental items.
- Place loam 2 inches deep on all new or reconstructed sideslopes or as directed by Resident.
- Erosion Control Mix may be substituted in those areas normally receiving loam and seed as directed by the Resident. Placement shall be in accordance with Standard Specifications Section 619, Mulch. Payment will be made under Pay Item 619.14. Erosion Control Mix.
- 9. A MASH compliant guardrail end treatment shall be installed concurrently with the placement of each leading end section of beam guardrail.
- 10. Where it is apparent that runoff will cause continual erosion, Erosion Control Blanket, seeded gutters, or riprap downspouts shall be constructed after paving and shoulder work is completed. Payment will be made under the appropriate Contract Items.
- 11. Protective Coating for Concrete Surfaces shall be applied to the following areas:

All exposed surfaces of concrete curbs, Fascias down to the drip notch, Concrete wearing surfaces, Top of abutment backwalls and wingwalls, To one foot below the top of backwalls and wingwalls on the back side, All exposed horizontal faces of abutments and piers, and All exposed vertical faces of abutments and piers.

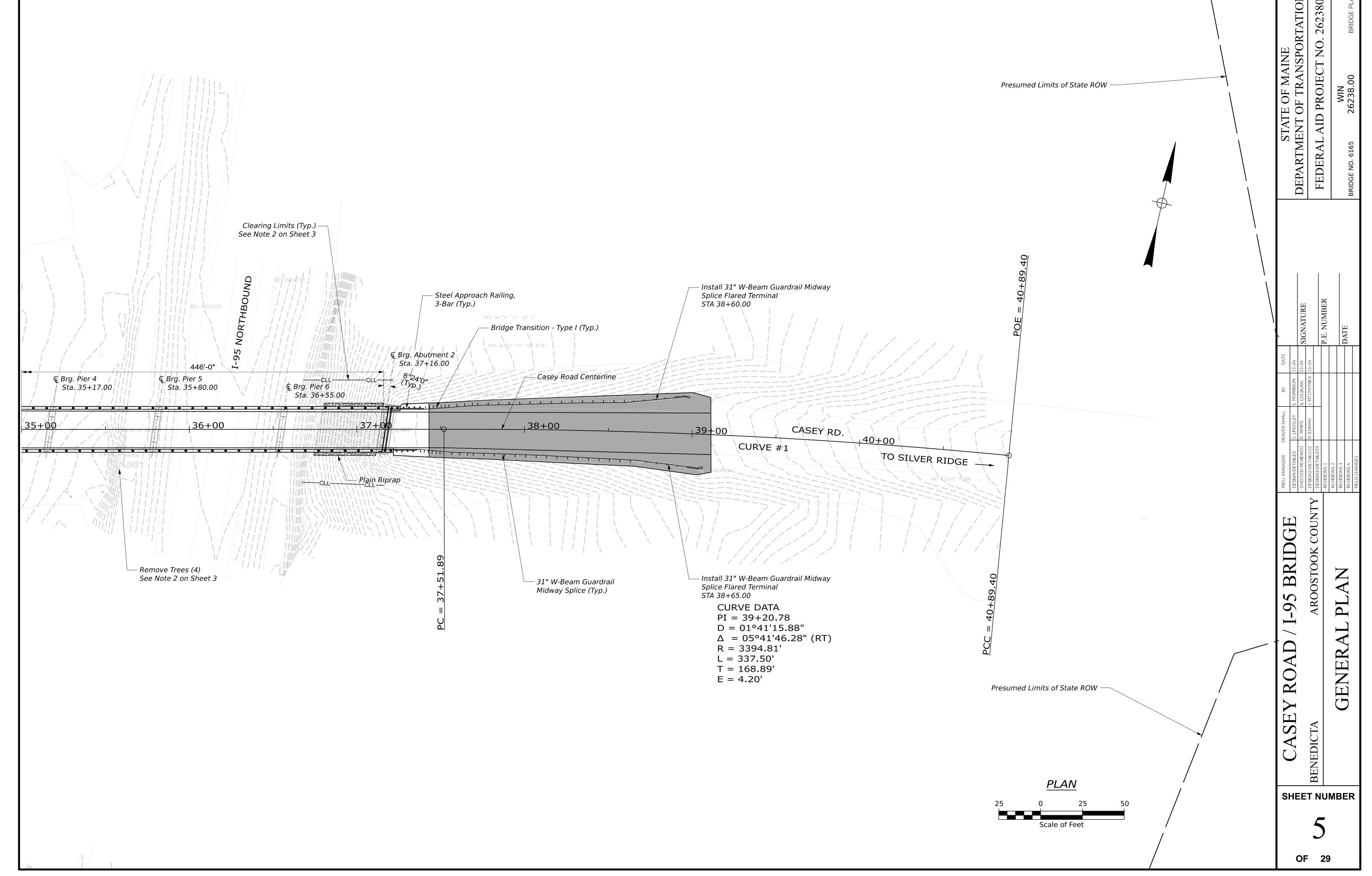
- 12. Project information referred to below may be accessed at the following MaineDOT web address: http://www.maine.gov/mdot/contractors/
- 13. The existing bridge plans may be accessed at the MaineDOT web address. The Plans are reproductions of the original drawings as prepared for the construction of the bridge. It is very unlikely that the plans will show any construction field changes or any alterations which may have been made to the bridge during its life span.
- 14. Quantities included for pay items measured and paid for by Lump Sum are estimated quantities and are provided by MaineDOT for informational purposes only. Lump Sum pay items will be paid for at the Contract Bid amount, with no addition or reduction in payment to the Contractor if the actual final quantities are different from the MaineDOT provided estimated quantities, except as follows:
  - a. If a Lump Sum pay item is eliminated, the requirements of Standard Specification Section 109.2, Elimination of Items, will take precedence.
  - b. If other Contract Documents specifically allow a change in payment for a Lump Sum pay item, those requirements will be followed.
  - c. If a design change results in changes to estimated quantities for Lump Sum pay items, price adjustments will be made in accordance with Standard Specifications Section 109.7, Equitable Adjustments to Compensation and
- 15. The Contractor shall submit Bridge Demolition Plans to the Resident at least 10 business days prior to the start of demolition work. The plan shall outline the methods and equipment to be used to remove and dispose of all materials included in the existing bridge deck, portions of abutments and wingwalls, and identified diaphragms and bearings. No work related to the removal of the bridge deck shall be undertaken by the Contractor until MaineDOT has reviewed the Bridge Demolition Plans for appropriateness and completeness. Payment for all work necessary for developing, submitting and finalizing the Bridge Demolition Plans will be considered incidental to the appropriate bridge removal pay items.

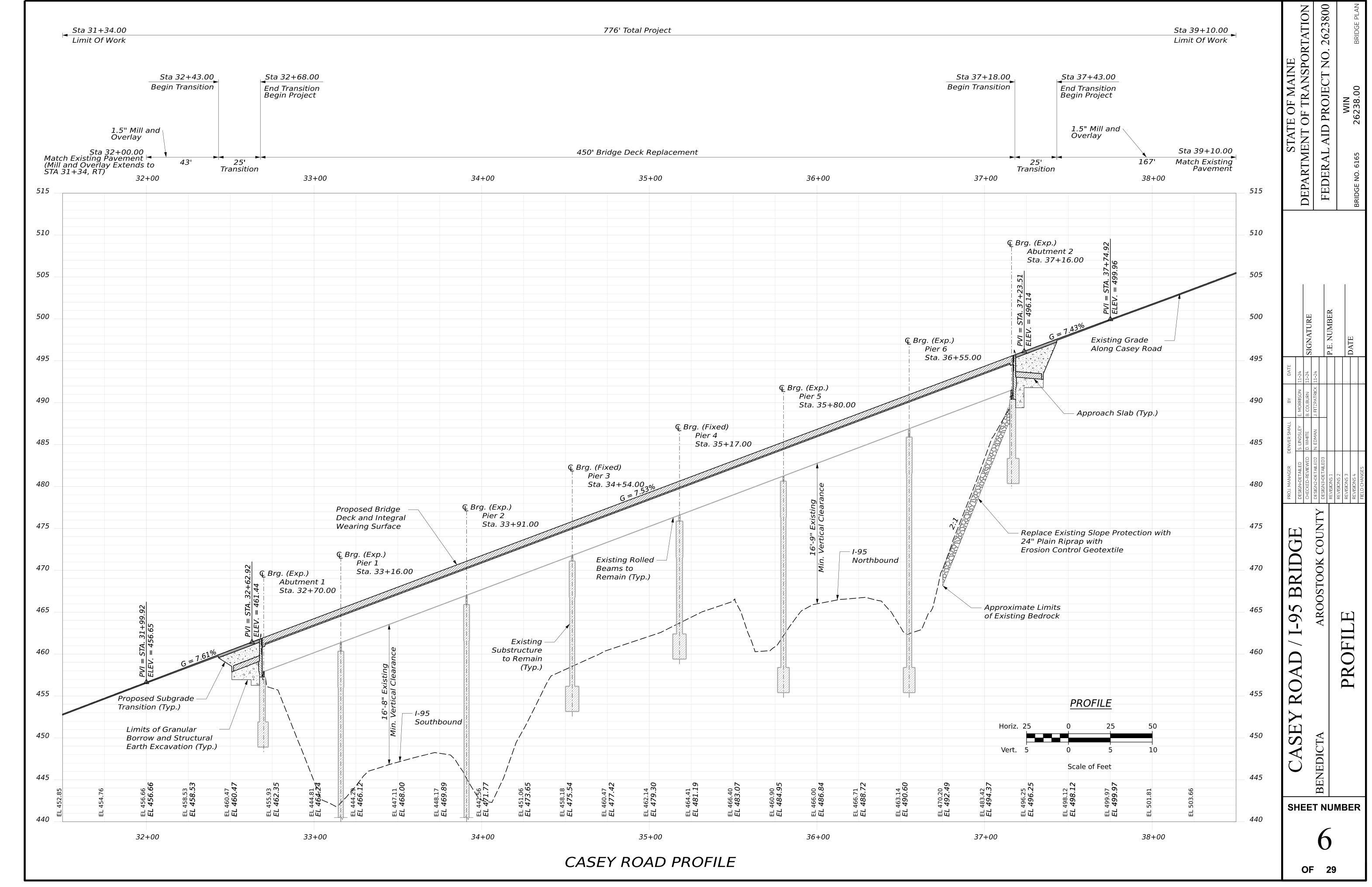
- 16. The existing bridge components to be removed shall be removed by and become the property of the Contractor. The steel portions of the existing bridges may be coated with a lead-based paint system. The Contractor is responsible for the containment, proper management and disposal of hazardous waste generated by the process of demolishing the existing decks, modifications of structural steel, and installation of shear studs. The Contractor is responsible for implementing appropriate OSHA mandated personal protection standards related to these processes. Once the existing bridge components are removed, the Contractor is solely responsible for the care, custody, and control of the components of the existing bridges and any hazardous water generated as a result of the storage, recycling, or disposal of the bridge components, including lead-coated steel. The Contractor shall recycle or reuse the steel in accordance with the Maine Department of Environmental Protection's "Maine Hazardous Waste Management Regulations," Chapter 850. A copy of this regulation is available at MaineDOT's offices on Child Street in Augusta. Payment for all labor, materials, equipment, and other costs required to remove and dispose of the existing bridge components will be considered incidental to related Contract Items.
- 17. Where a joint between new pavement and existing pavement is called for on the Plans, the existing pavement shall be sawcut along a smooth line to a neat, even, vertical joint as directed by the Resident. Broken or raveled edges will not be permitted. All work necessary for the preparation of this joint will be considered incidental to the related Contract Items.
- 18. All existing delineators and mile marker posts on Interstate 95 which are impacted shall be removed and reset. Payment for removing and resetting delineators and mile marker posts shall be incidental to the Contract.
- 19. Relocating existing ground mounted signs during traffic control phasing will not be directly measured for payment; payment shall be considered incidental to related various Contract Items. Signs temporarily relocated within 30' of an active travel way shall have breakaway devices or be protected with portable concrete barrier. Temporary sign supports shall meet the requirements of Specification Section 652.
- 20. Payment for connections of proposed guardrail to existing guardrail will be considered incidental to related Contract
- 21. The Resident shall be informed a minimum of two weeks prior to the removal of any survey monuments from the Project. Monuments shall be carefully salvaged by the Contractor and will remain the property of the Department.

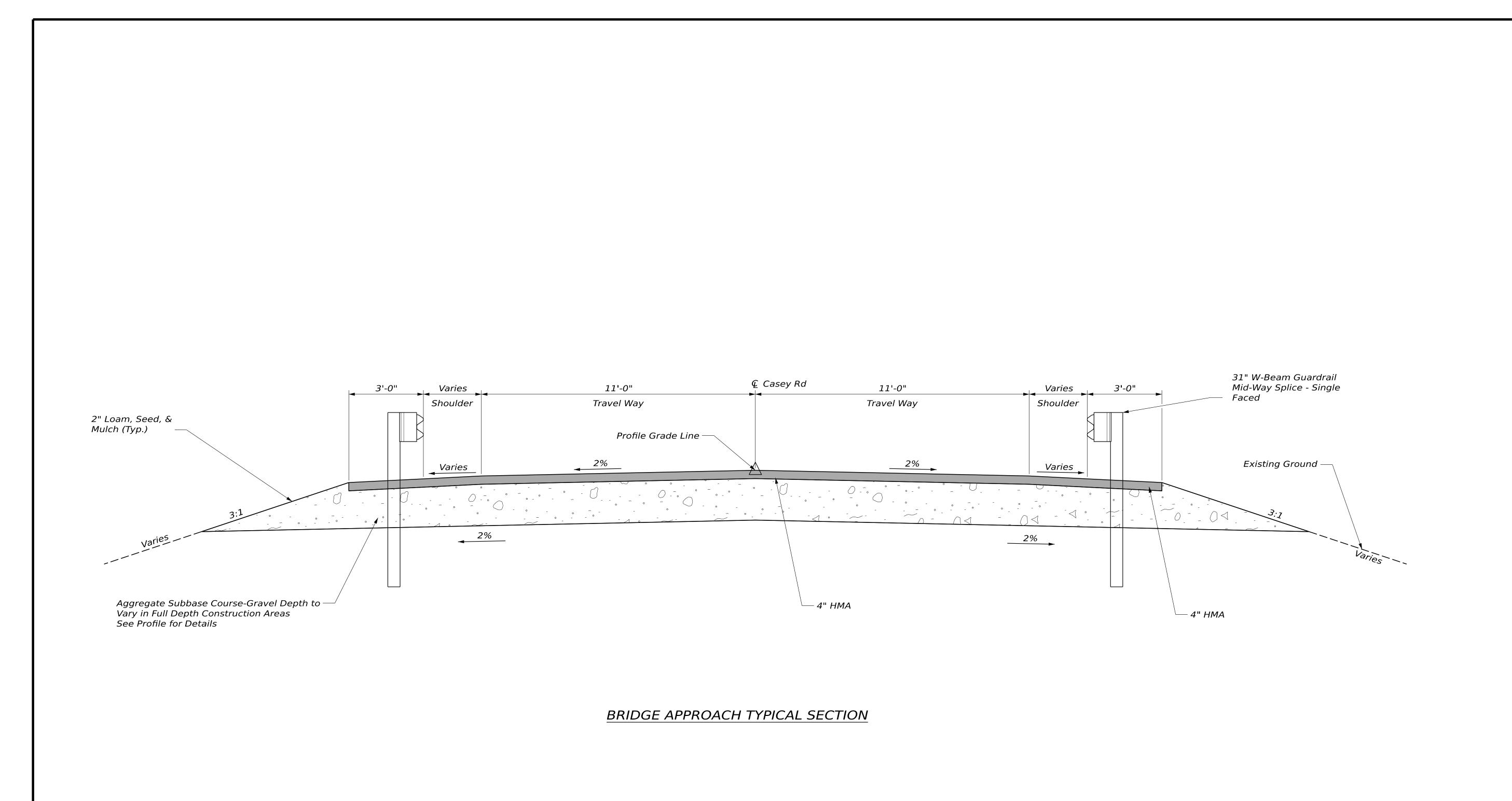
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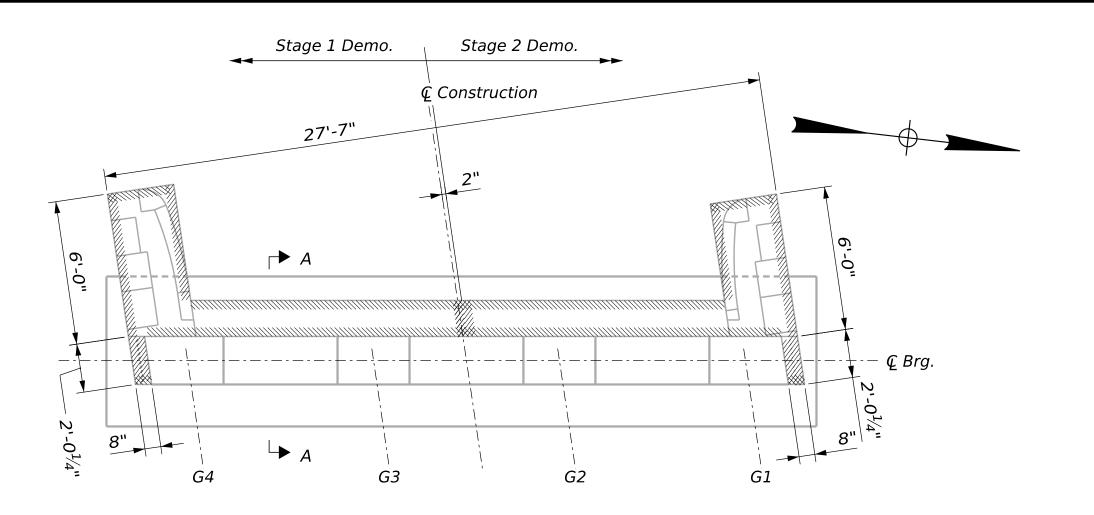
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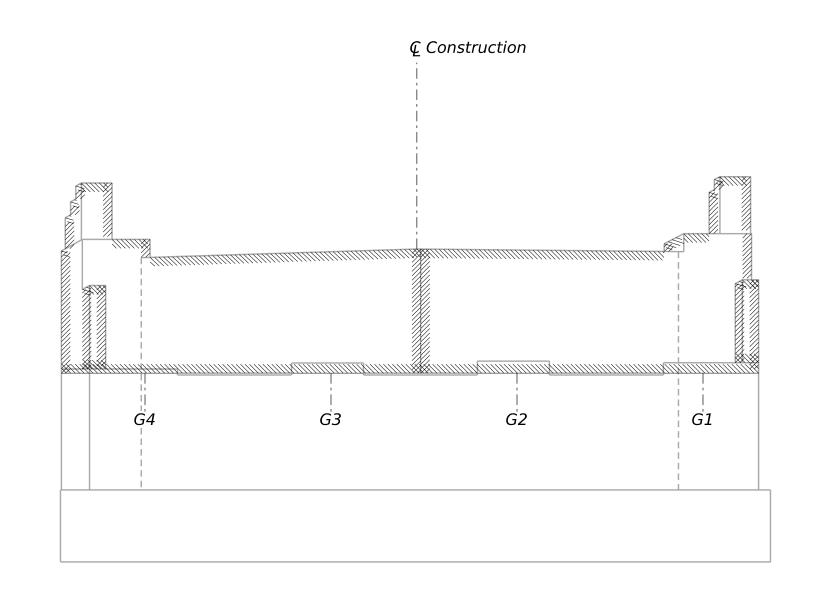
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SECTIONS

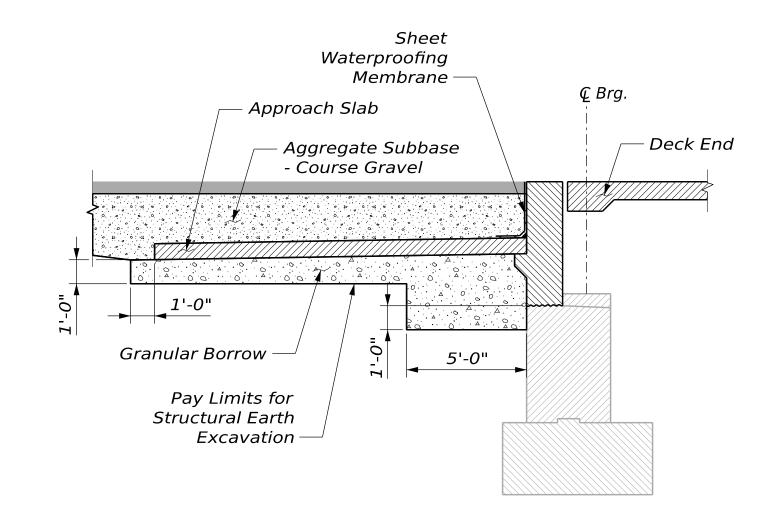


#### ABUTMENT 1 DEMOLITION PLAN

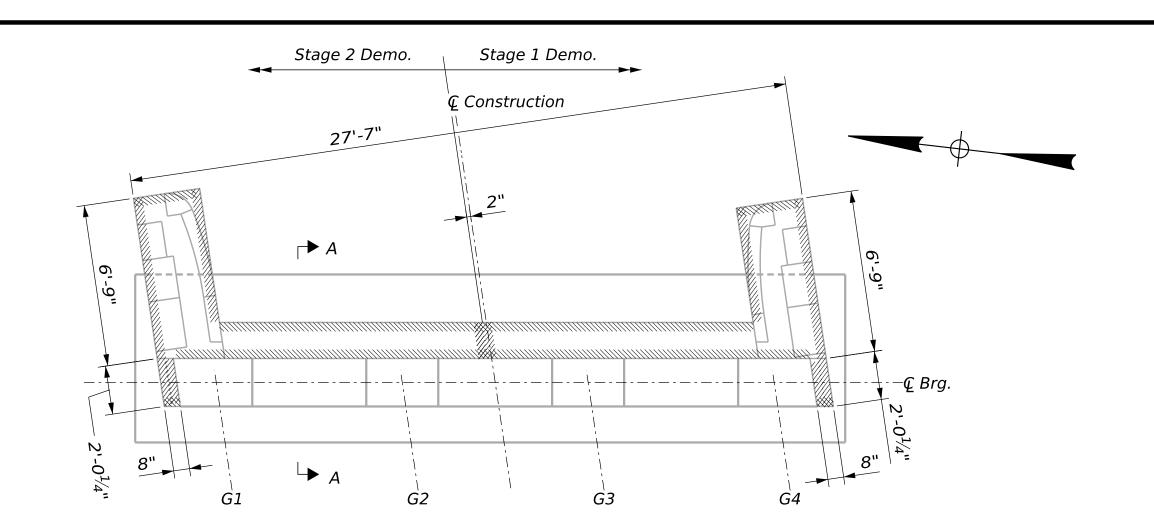
See Sheet 11 for Abutment Demolition Section



#### ABUTMENT 1 DEMOLITION ELEVATION

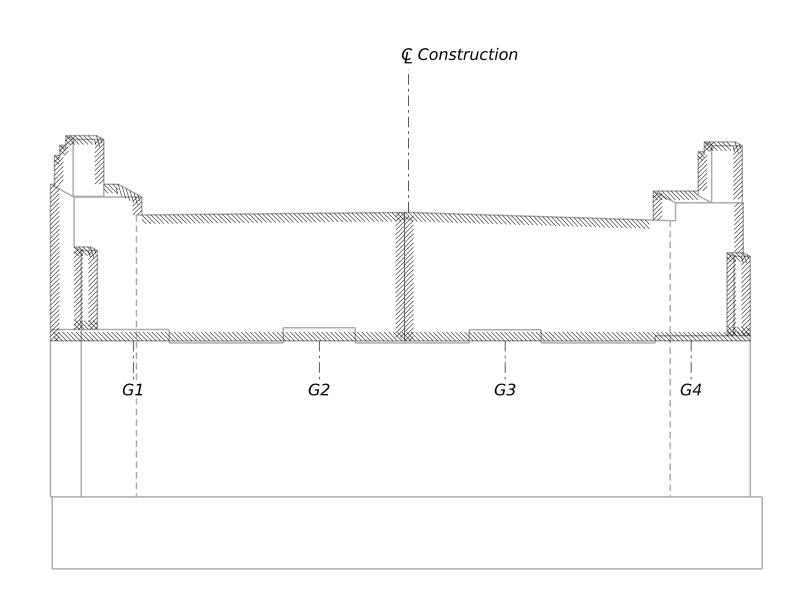


ABUTMENT 1 BACKFILL DETAIL

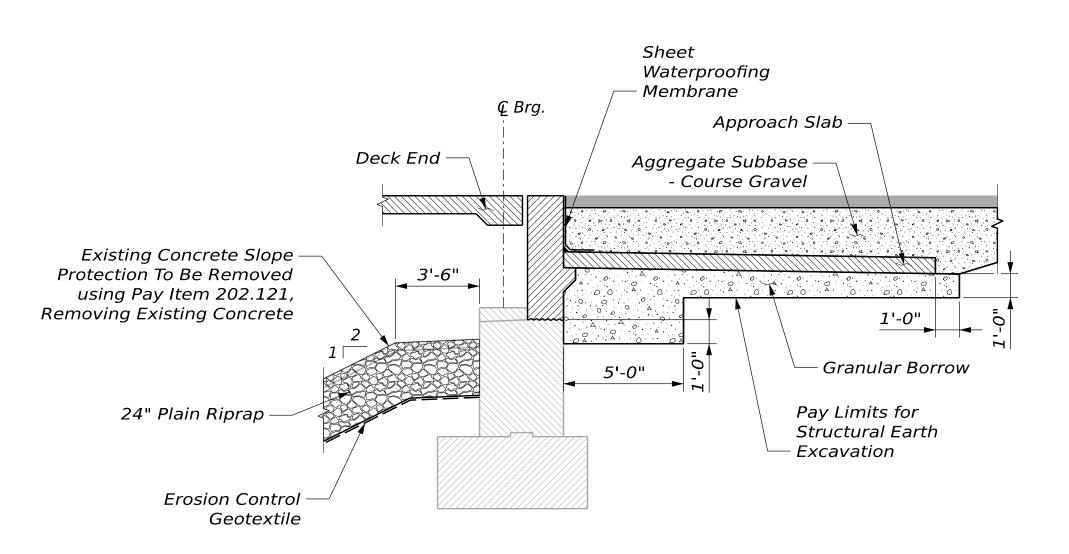


#### ABUTMENT 2 DEMOLITION PLAN

See Sheet 11 for Abutment Demolition Section



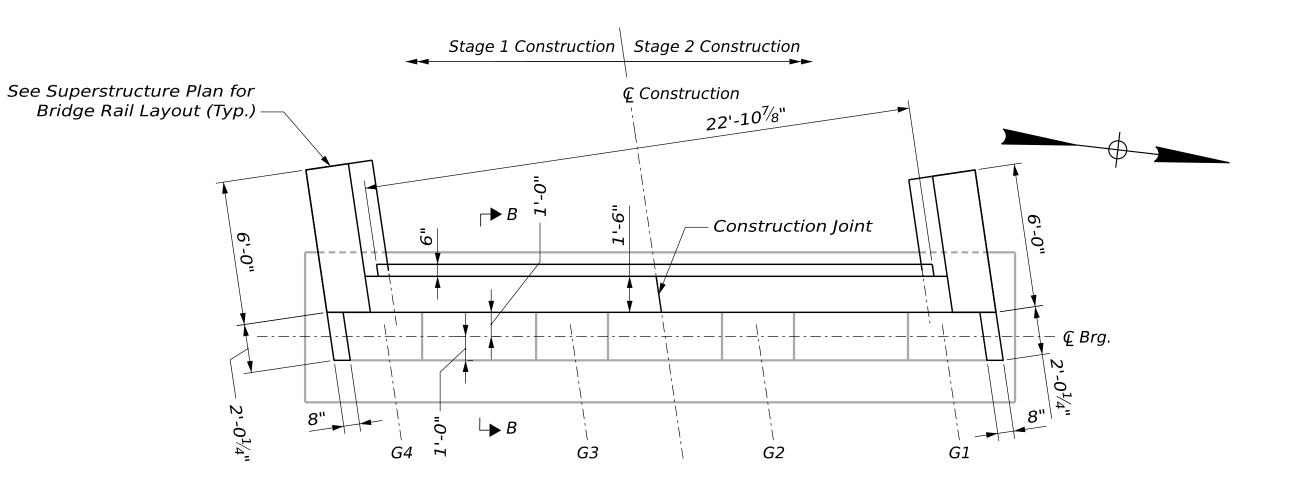
#### ABUTMENT 2 DEMOLITION ELEVATION



ABUTMENT 2 BACKFILL DETAIL

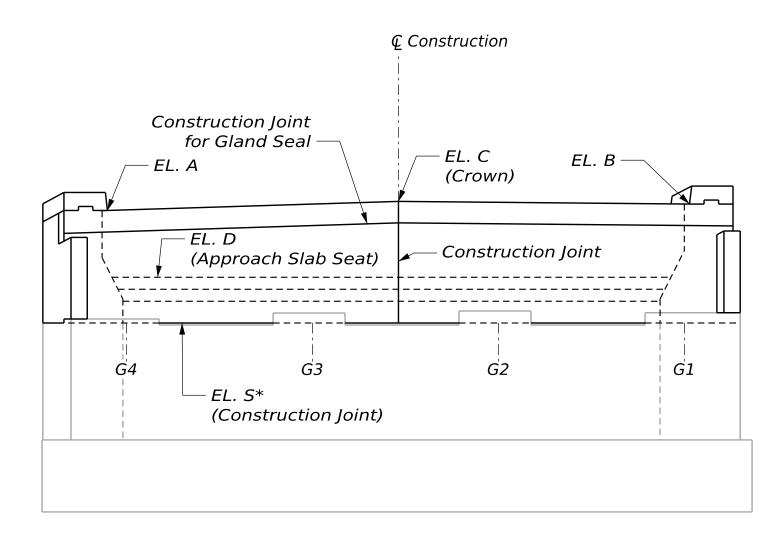
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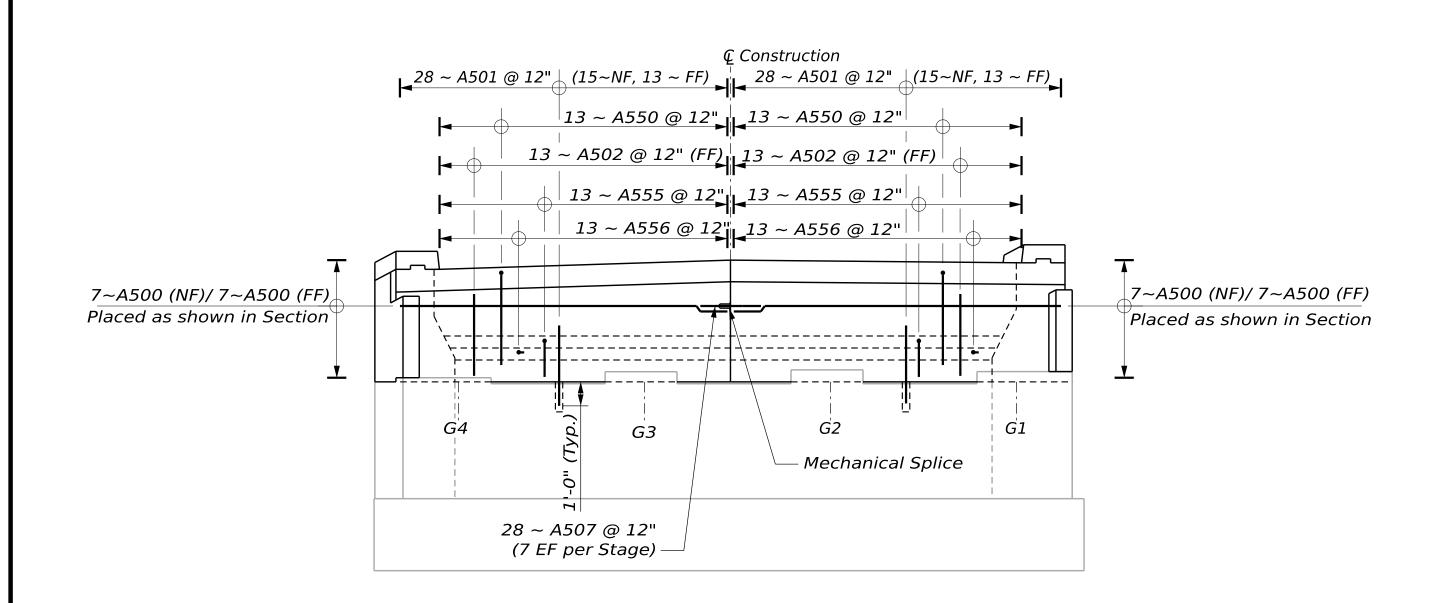


#### ABUTMENT 1 CONSTRUCTION PLAN

See Sheet 11 for Abutment Construction Section. Approach Slab not shown for Clarity.



ABUTMENT 1 CONSTRUCTION ELEVATION



ABUTMENT 1 REINFORCING ELEVATION

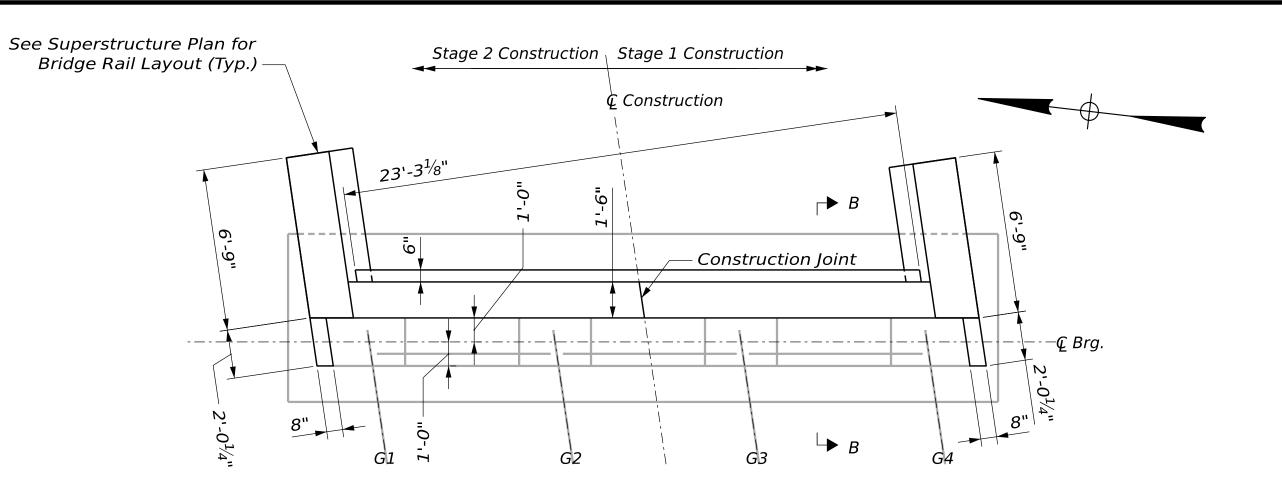
	Abutment 1 Elevations										
EL	. A	EL. B EL. C				EL. D	EL. S*				
NF	FF	NF	FF	NF FF		EL. D	EL. S				
461.53	461.41	461.79	461.68	461.90	461.79	458.75	456.78				
		NF	= Near Face	FF= Far Face							

\*Elevation to coincide with the existing construction joint between the backwall and abutment seat. Elevations provided are approximated from the existing bridge plans. To be field verified.

#### ABUTMENT AND WINGWALL MODIFICATION NOTES

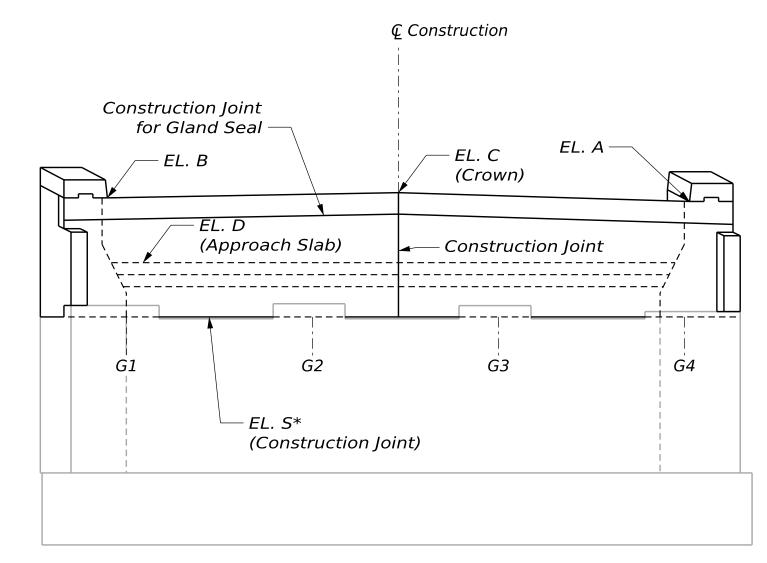
- 1. The Contractor shall use care not to damage any existing reinforcing steel which is to remain. All existing reinforcing steel which is to remain shall be cleaned of all loose rust by sandblasting, wire brushing, machine wire brushing, or other methods approved by the Resident. Any damaged reinforcing steel shall be replaced as directed by the Resident at no expense to the Department.
- 2. Before drilling and grouting new reinforcing steel, the Contractor shall locate reinforcing steel in existing concrete by non-destructive methods to avoid conflicts. All costs associated with this work shall be incidental to related Contract Items.
- 3. Reinforcing steel shall have 2 inches cover unless otherwise noted.
- 4. Existing abutment and wingwall concrete to be removed as shown on the Plans shall be sawcut 1 inch deep prior to removal of existing concrete. All costs associated with this work shall be incidental to related Contract Items.
- 5. Where drilling and anchoring of reinforcement is required, the Contractor shall use a material listed on the Maine Department of Transportation Qualified Products List of Concrete Adhesive Anchor Systems. The depth of embedment shall be sufficient to develop 125% of the yield strength of the bar per the manufacturer's recommendations or 12 inches, whichever is greater. Proposed anchoring material and embedment depth shall be submitted for approval. Payment for drilling and anchoring will be incidental to related Contract Items.
- 6. Dimensions and layout shown are based on available record plans. Contractor shall field verify all dimensions prior to any related work.
- 7. All surfaces to be rehabilitated shall be clean of all debris and foreign material and shall be roughened to a  $\frac{1}{2}$ " amplitude prior to placement of the new concrete. Payment shall be incidental to related concrete items.
- 8. Projecting reinforcing that can be maintained, in addition to that shown in the details, may be left in place at Contractor's discretion with approval of the Resident.
- 9. Cover joints where waterstops are not required in accordance with Standard Details Section 502.
- 10. The approach slab construction limits shall follow the same transverse offsets as the bridge deck as shown in the Staged Construction details. The approach slab shall be constructed in each stage per the standard details, with the two stages having a butted joint. There will be no reinforcing lapping/splicing between stages.

	SIGNATURE P.E. NUMBER						H * C	DAIE	
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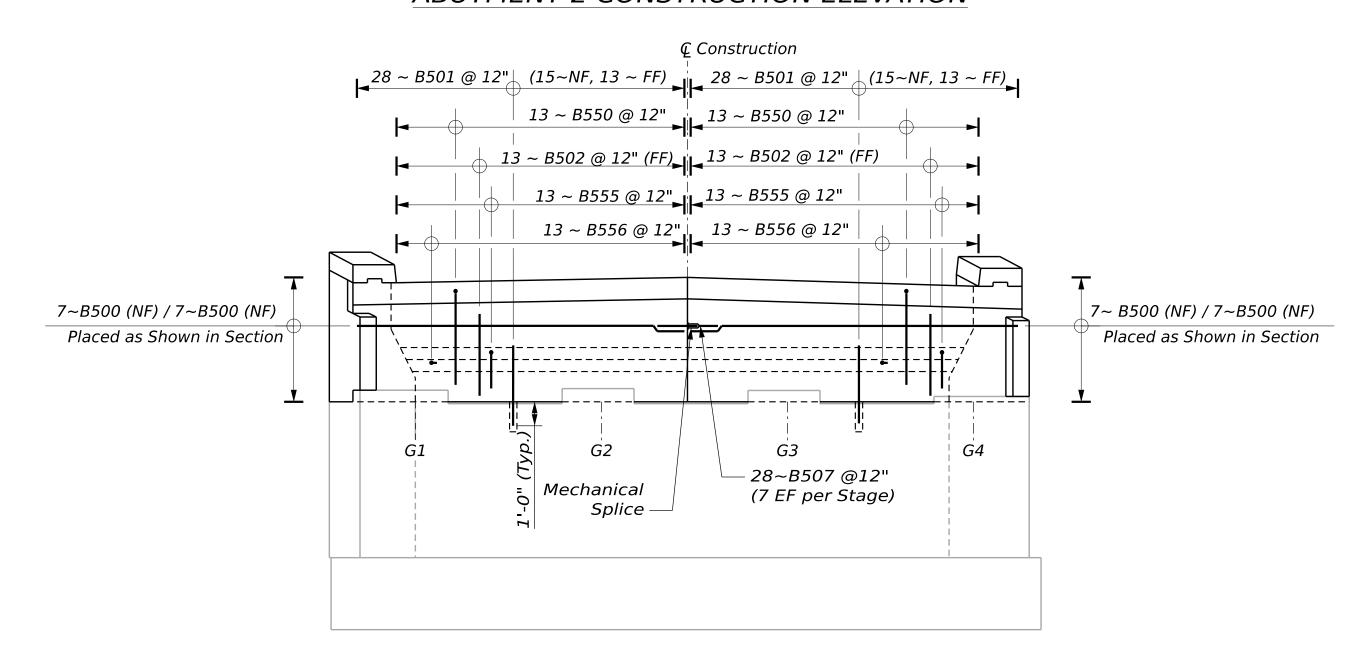


#### ABUTMENT 2 CONSTRUCTION PLAN

See Sheet 11 for Abutment Construction Section. Approach Slab not shown for Clarity.



#### ABUTMENT 2 CONSTRUCTION ELEVATION



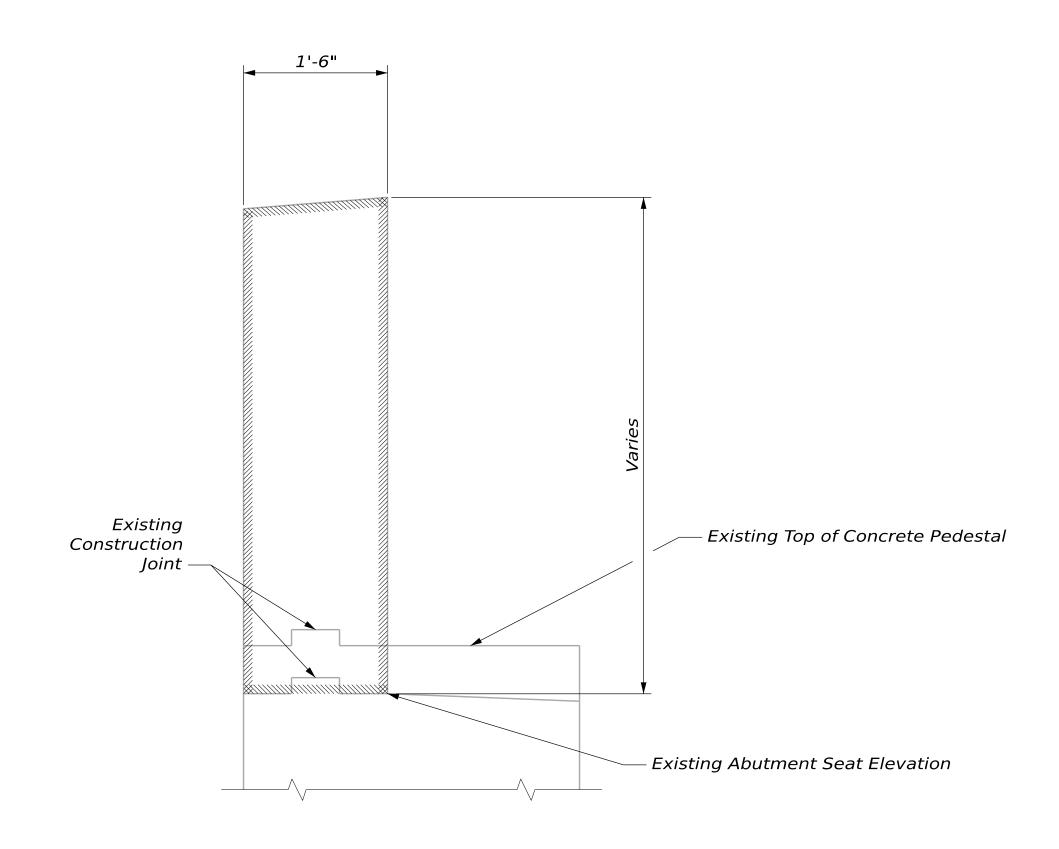
ABUTMENT 2 REINFORCING ELEVATION	
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			Al	outment 2 i	Elevations			
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495	5.28	495.39	495.54	495.66	495.65	495.76	492.72	490.41
			NF	= Near Face	FF= Far Face	9		

\*Elevation to coincide with the existing construction joint between the backwall and abutment seat.

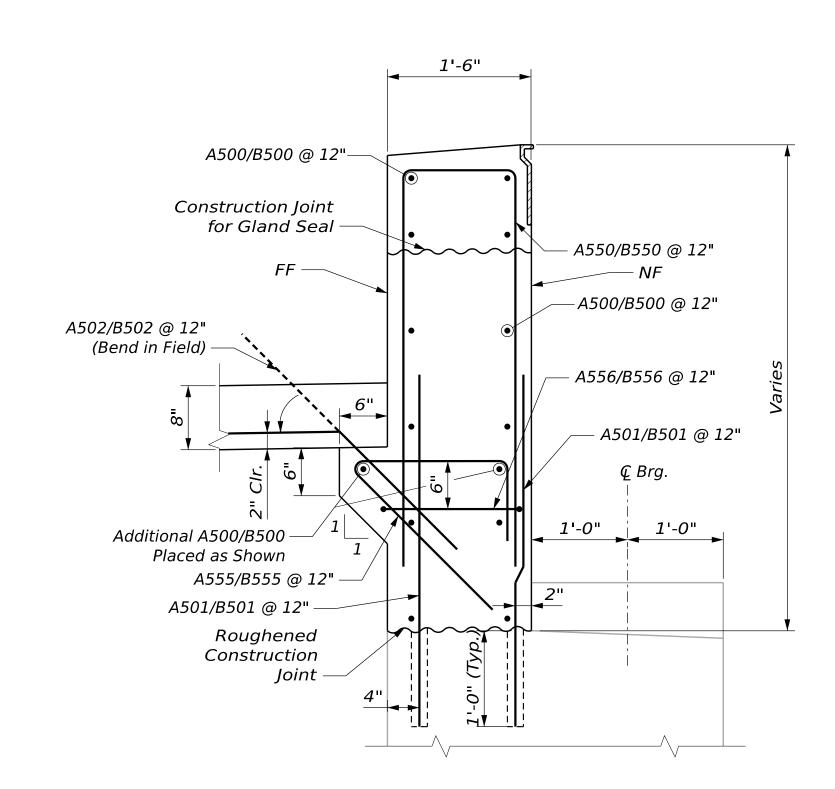
Elevations provided are approximated from the existing bridge plans. To be field verified.

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### ABUTMENT DEMOLITION SECTION (SECTION A-A)

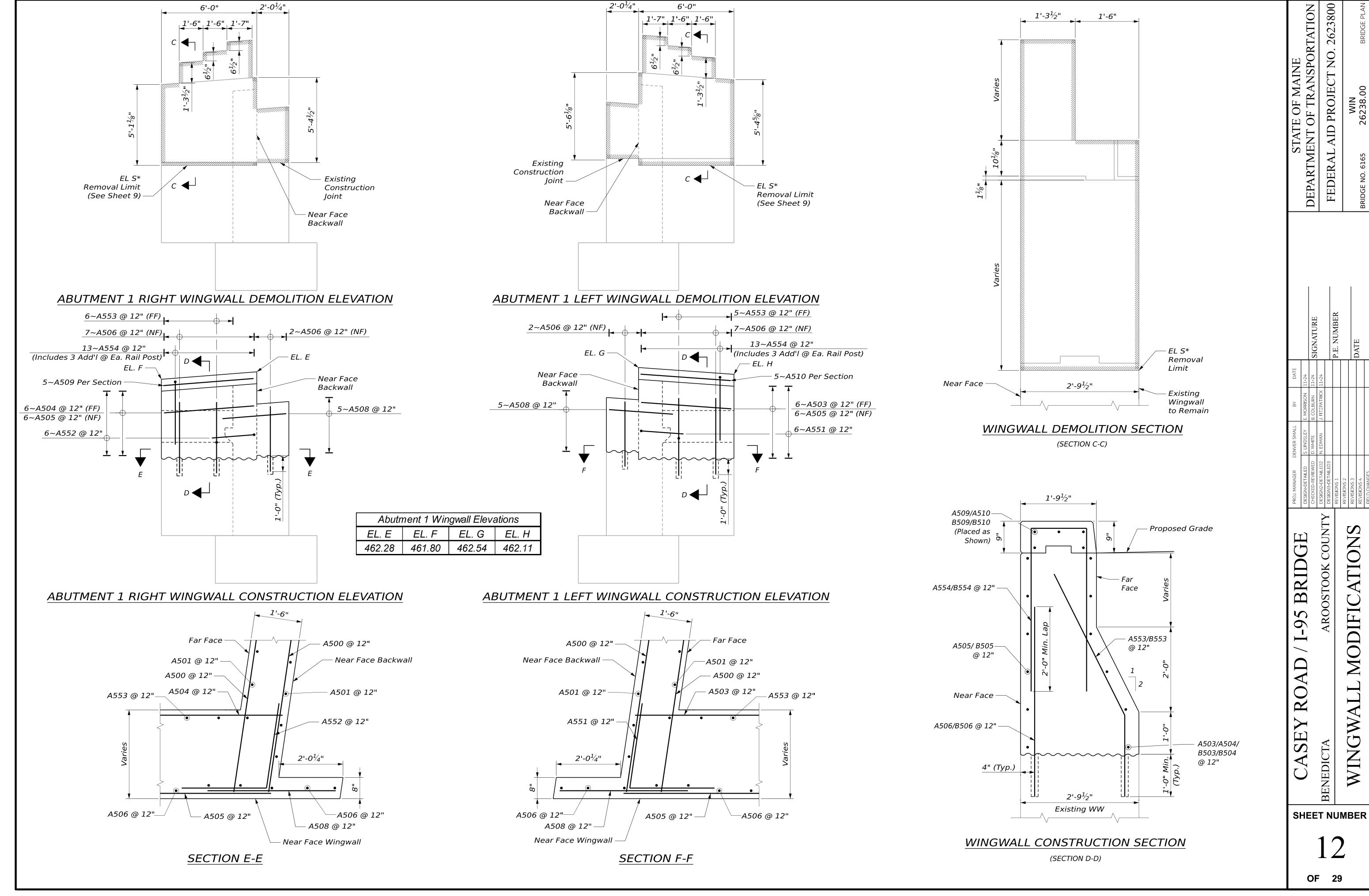
Abutment 1 Shown, Abutment 2 Similar

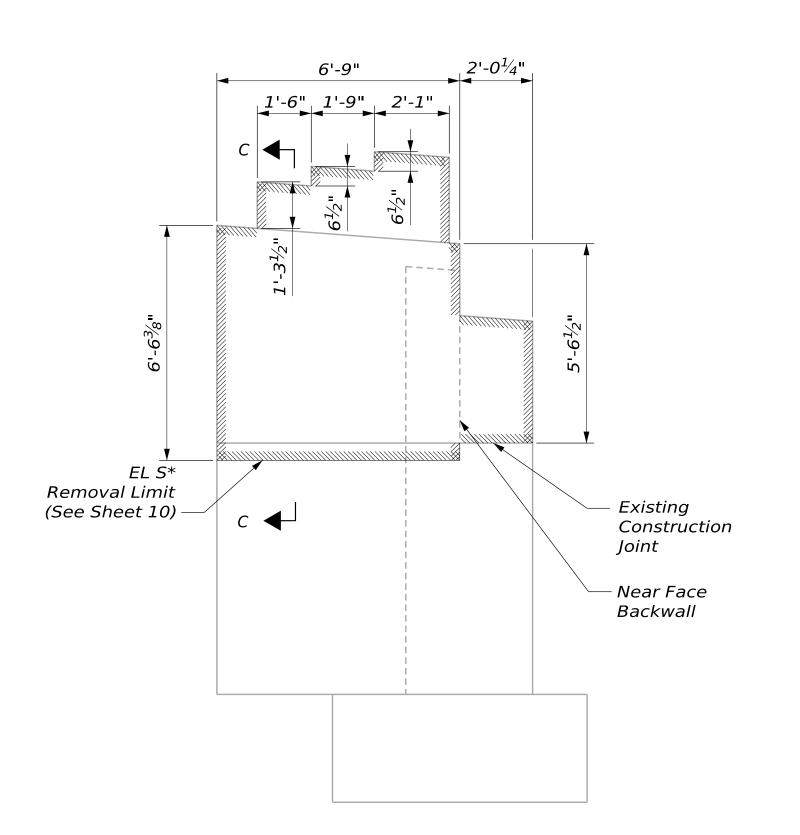


#### ABUTMENT CONSTRUCTION SECTION

(SECTION B-B)
Abutment 1 Shown, Abutment 2 Similar

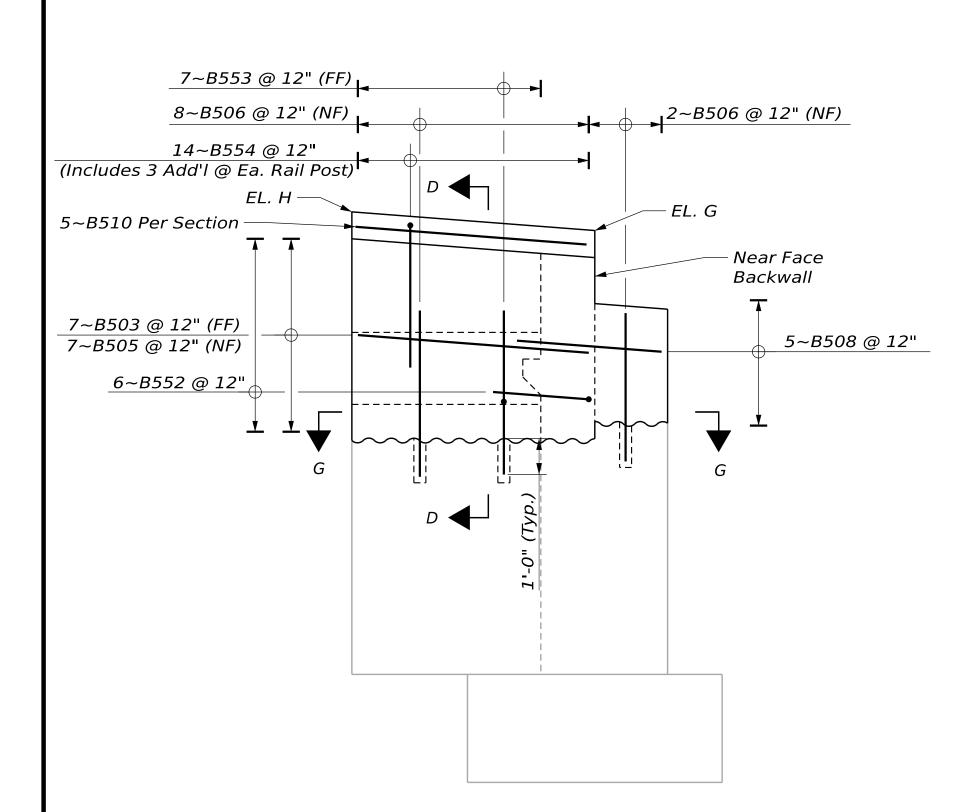
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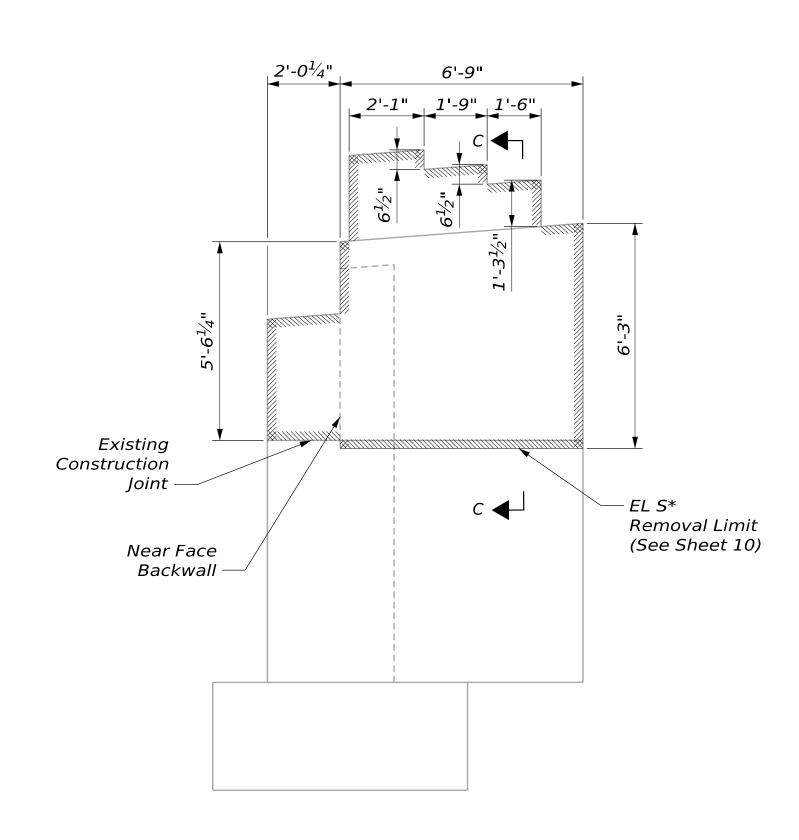
#### ABUTMENT 2 LEFT WINGWALL DEMOLITION ELEVATION

See Sheet 12 for Wingwall Demolition Section.



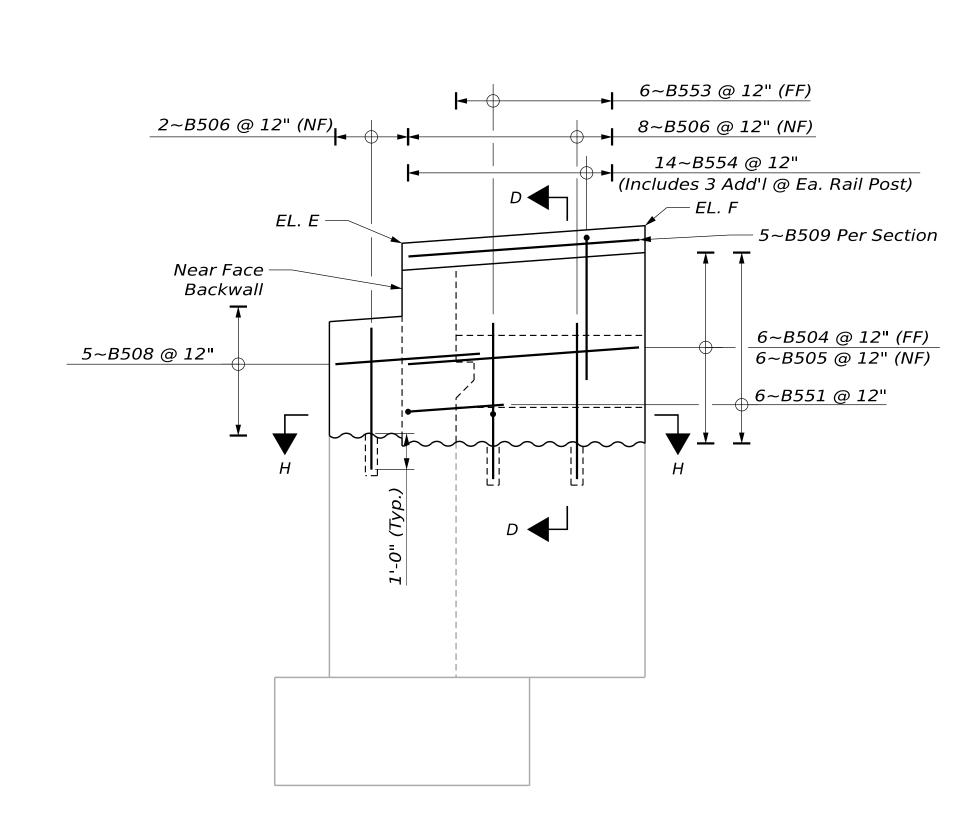
#### ABUTMENT 2 LEFT WINGWALL CONSTRUCTION ELEVATION

See Sheet 12 for Wingwall Construction Section.



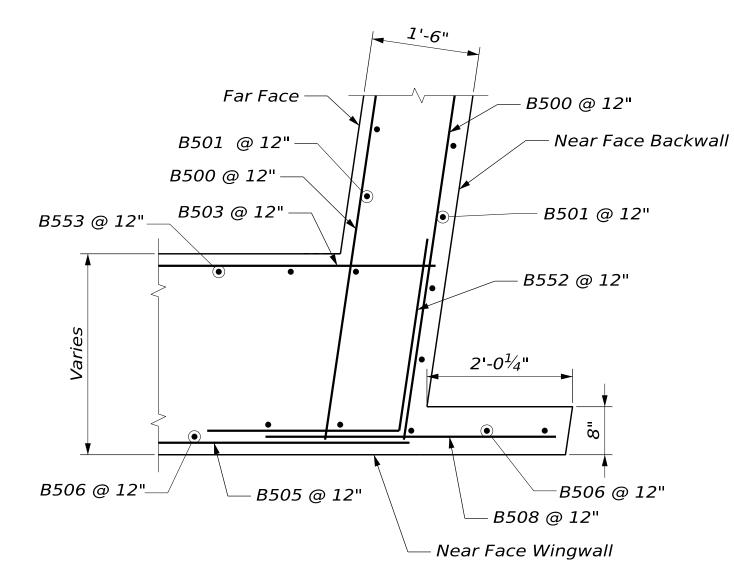
#### ABUTMENT 2 RIGHT WINGWALL DEMOLITION ELEVATION

See Sheet 12 for Wingwall Demolition Section.



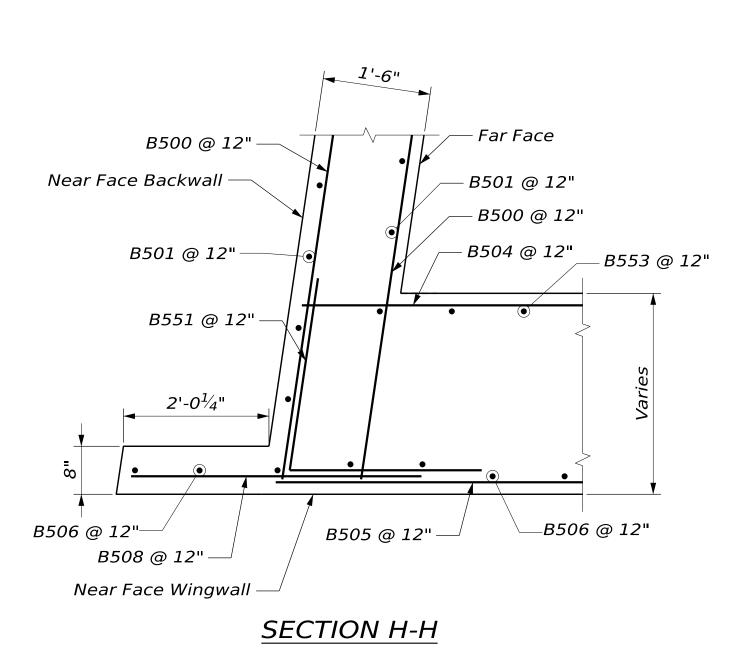
#### ABUTMENT 2 RIGHT WINGWALL CONSTRUCTION ELEVATION

See Sheet 12 for Wingwall Construction Section.



SECTION G-G

Abutn	nent 2 Win	gwall Eleva	ations
EL. E	EL. F	EL. G	EL. H
496.03	496.52	496.29	496.82



SHEET NUMBER

BENEDICTA

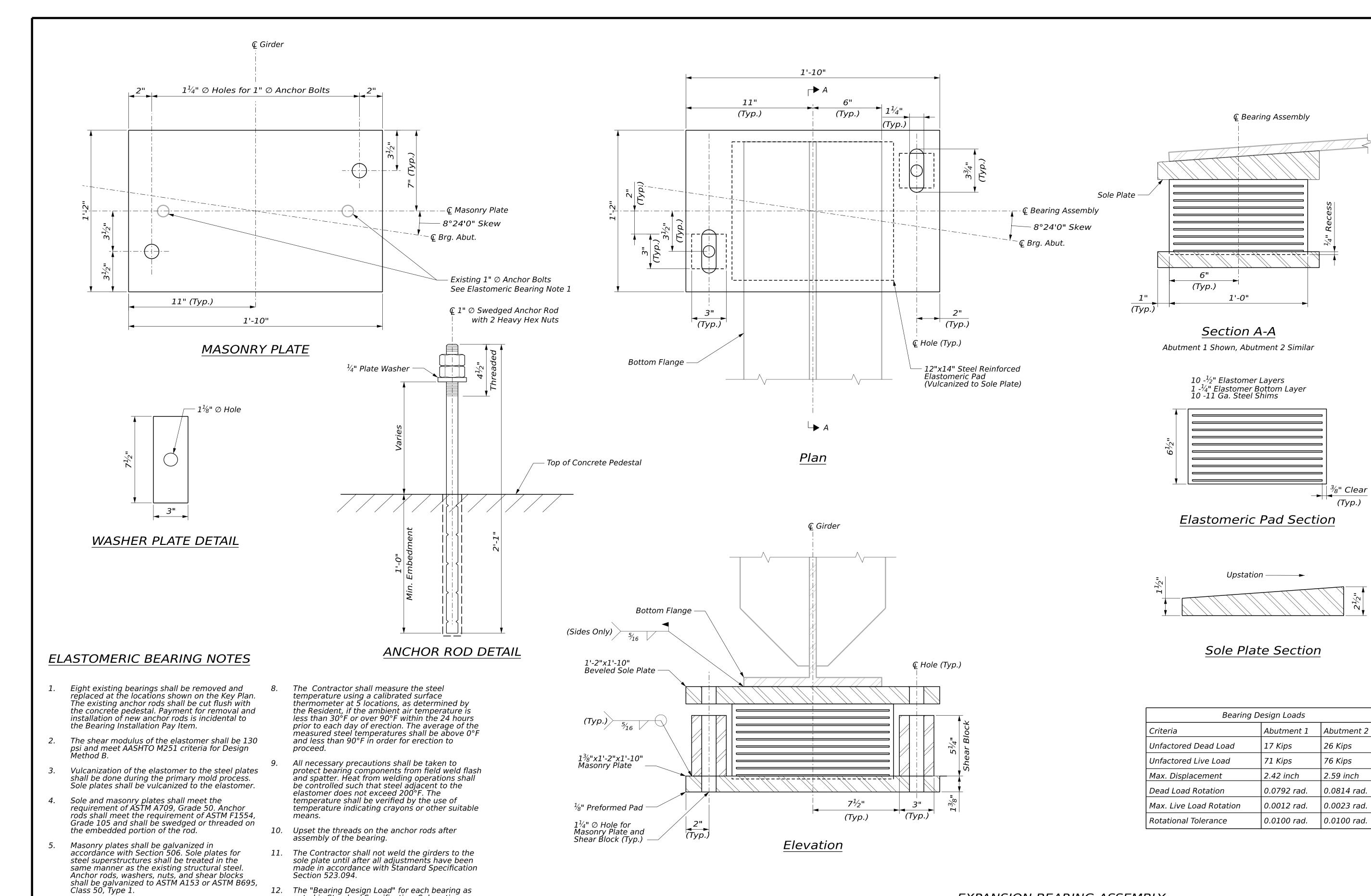
WINGWALL

AROOSTOOK COUNTY

BRIDGE

6

P.E. NUMBER



EXPANSION BEARING ASSEMBLY

SHEET NUMBER

P.E. NUMBER

COUNTY

AROOSTOOK

**DETAIL** 

RIN

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OF 29

All bearings shall be marked prior to shipping. The marks shall include the bearing location on

Bearings shall be covered during shipping and at any time prior to installation that the bearings

the bridge and a direction arrow that points upstation. All marks shall be permanent and shall be visible after the bearing is installed.

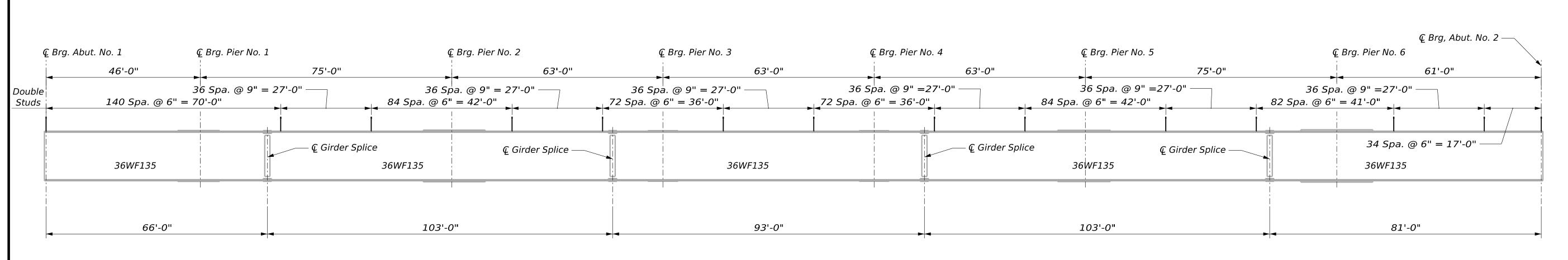
may be exposed to sunlight.

The "Bearing Design Load" for each bearing as noted in Standard Specification, Subsection 523.23.4 is given in the table. This is the total load for the Service I load combination without

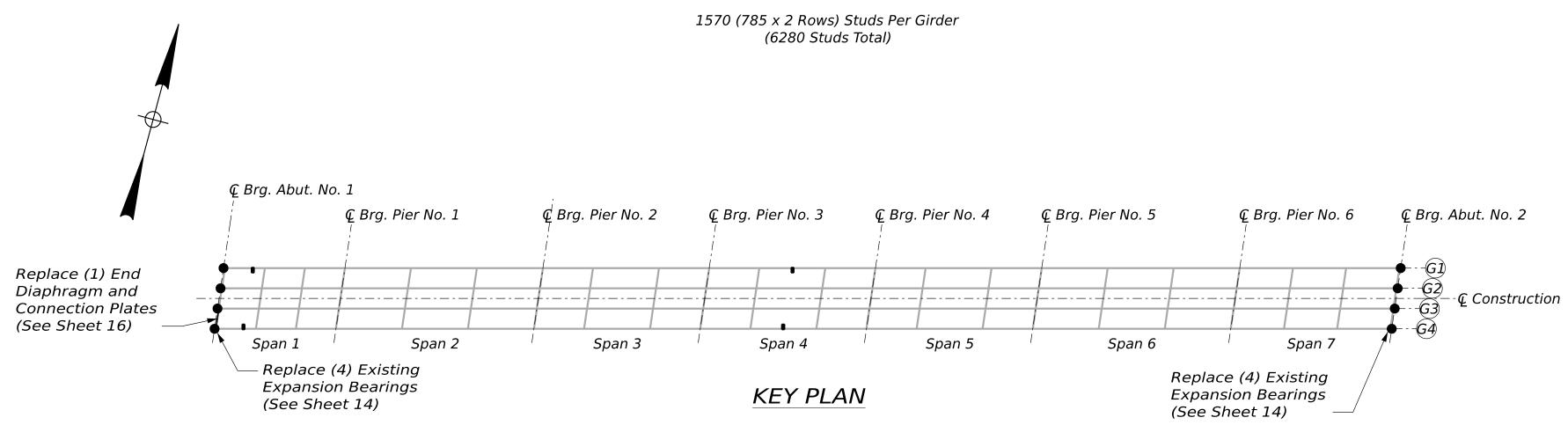
13. Anchor rods shall be drilled and grouted in place using a material listed on the Maine Department of Transportation Qualified Products List of

Concrete Adhesive Anchoring Systems.

impact.

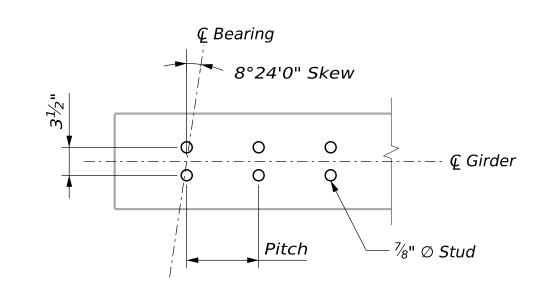


#### SHEAR CONNECTOR LAYOUT



#### LEGEND

Proposed Expansion Bearing Replacement



SHEAR CONNECTOR DETAIL

#### SHEAR CONNECTOR NOTES

- 1. Dimensions are measured along centerline of the girder.
- 2. Prior to installing the proposed shear studs, the Contractor shall clean the top flange so that it is free of debris, rust, scale, oil and other contaminates that would adversely affect the welding operation. Payment for cleaning the top flange for installation of proposed shear studs shall be incidental to Item 505.08, Shear Connectors. Existing steel may be coated with a lead-based paint system. See General Notes for more information.
- 3. The proposed shear studs shall be  $\frac{7}{8}$ "  $\emptyset$ . Studs shall penetrate into the deck a minimum of 2" and maintain a clear cover of 2" to the top of the studs.
- 4. If existing shear connectors are present, they shall be removed such that they project 1" maximum above the top of the existing top flange unless they conflict with the installation of the new shear connectors or any other work. If the existing shear connectors interfere with installation of the new shear connectors or any other work, they shall be removed completely and ground flush with the top flange. All costs associated with this work shall be incidental to related Contract Items.
- 5. Pitch spacing may be adjusted at bolted splices to avoid splice bolts.

CASEY ROAD / I-95 BRIDGE

BENEDICTA

AROOSTOOK COUNTY

KEY PLAN &

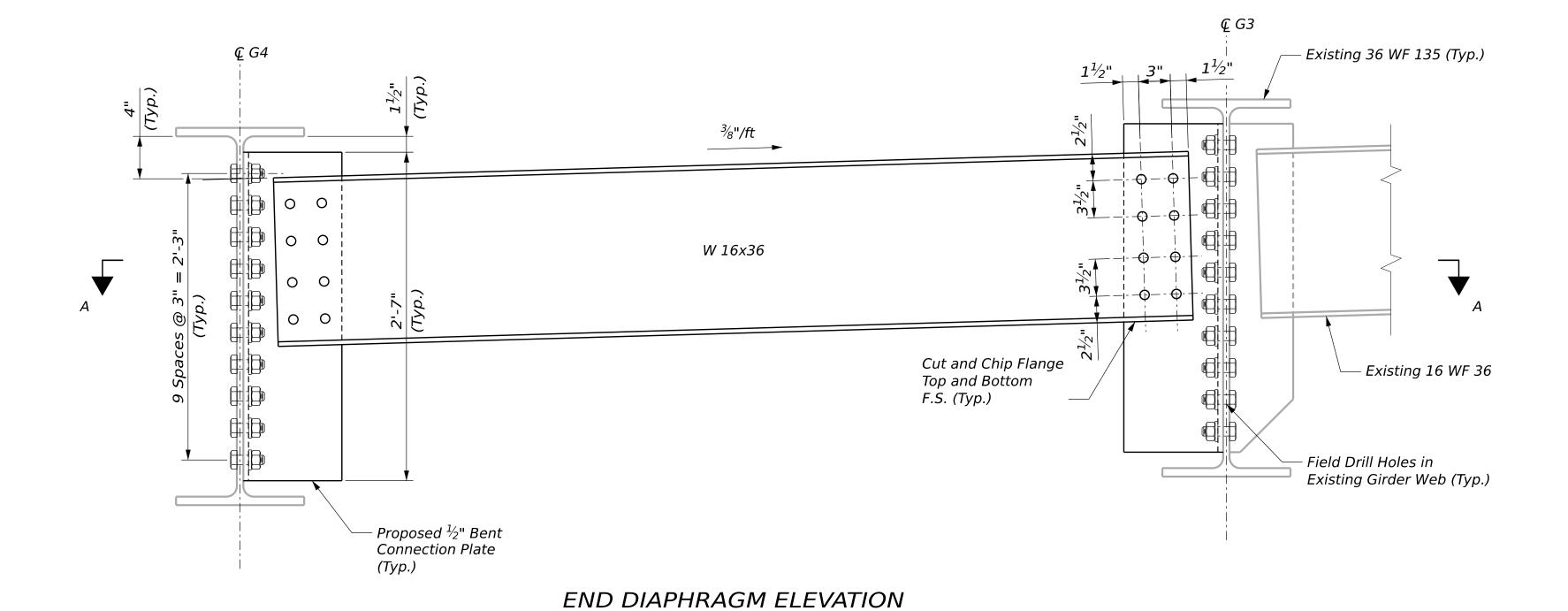
SHEAR CONNECTOR LAYOUT

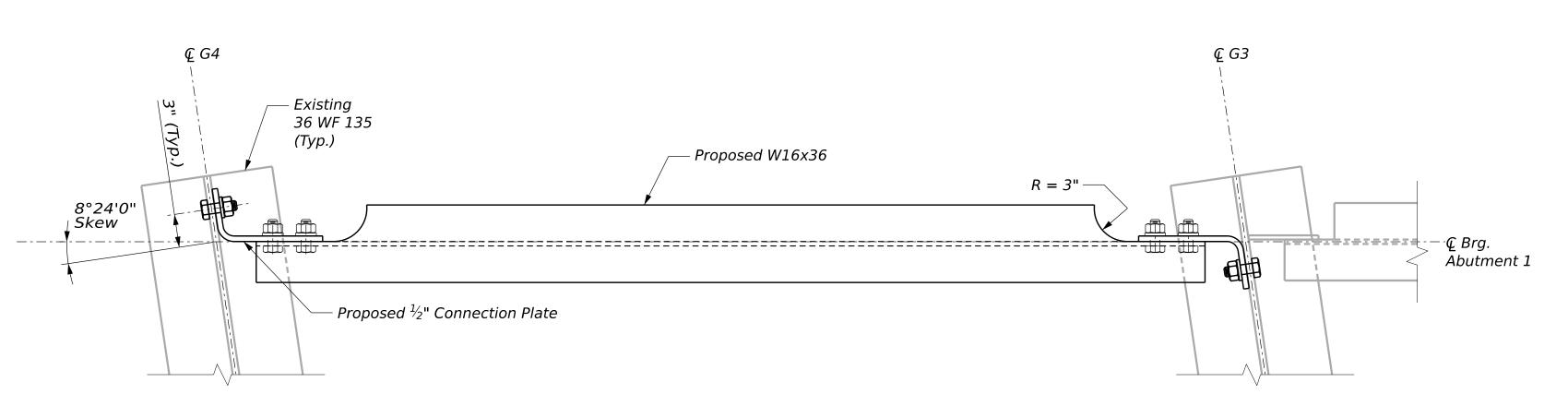
OF 29

2623800

P.E. NUMBER

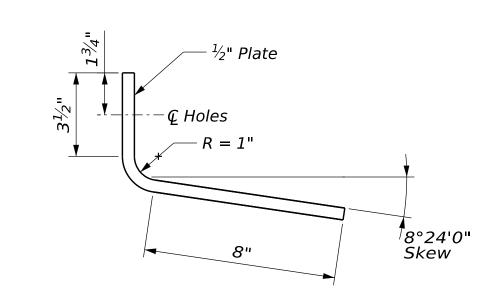
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SECTION A-A

(Abutment 1)

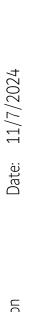


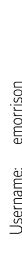
CONNECTION PLATE DETAIL

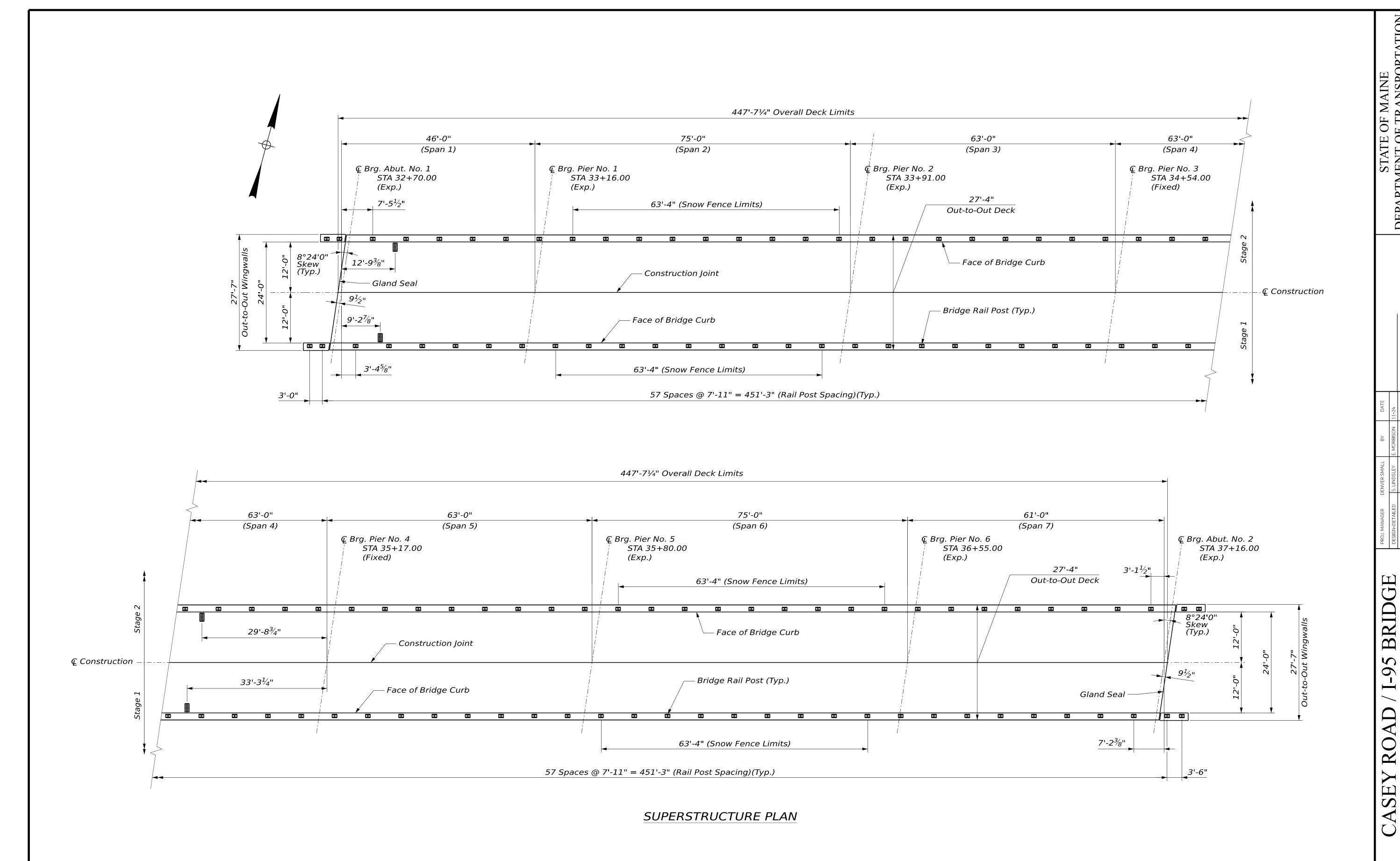
#### STRUCTURAL STEEL NOTES

- 1. Replace (1) Diaphragm and associated connection plates as shown on Key Plan.
- 2. The reuse of existing bolts will not be permitted.
- 3. All bolts shall be  $\frac{7}{8}$ " Ø, ASTM F3125 Grade A325, Type I, High Strength Bolts.
- 4. Proposed Structural Steel shall be coated in accordance with Standard Specifications Section 506, Shop Applied Protective Coating Steel (Zinc Rich Coating System), except NEPCOAT Qualified Product List C may be used. Payment for the coating will be incidental to Pay Item 504.70.
- 5. Shop drill  $^{15}\!\!/_{16}$ "  $\oslash$  holes in diaphragms and connection plates for diaphragm connection. Shop drill  $^{11}\!\!/_{16}$ "  $\oslash$  holes in the connection plate for girder connection. Field drill  $^{15}\!\!/_{16}$ "  $\oslash$  holes in existing girders.
- 6. Proposed steel contact surfaces shall be cleaned in accordance with Special Provision Section 506, Shop Applied Protective Coating Steel (Field Painting, New and Existing Steel with Zinc Rich Coating System).
- 7. Payment for field measuring and field drilling shall be incidental to Pay Item 504.71, "Structural Steel Erection".
- 8. Existing connection plates that are being replaced shall be removed without damage to the girder. Payment will be incidental to Pay Item 504.71.

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1			DESIGN2-DETAILED2 N. EDMAN		J. FITZPATRICK 11-24			
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	11		_				מות	
6	∟ U∣		REVISIONS 1			1	P.E. NUMBEK	
)	M		REVISIONS 2					
	B		REVISIONS 3			-		
	ΞF		REVISIONS 4				DALE	BRIDGE NO 6165
	₹		FIELD CHANGES					







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AROOSTOOK COUNTY

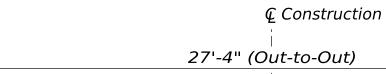
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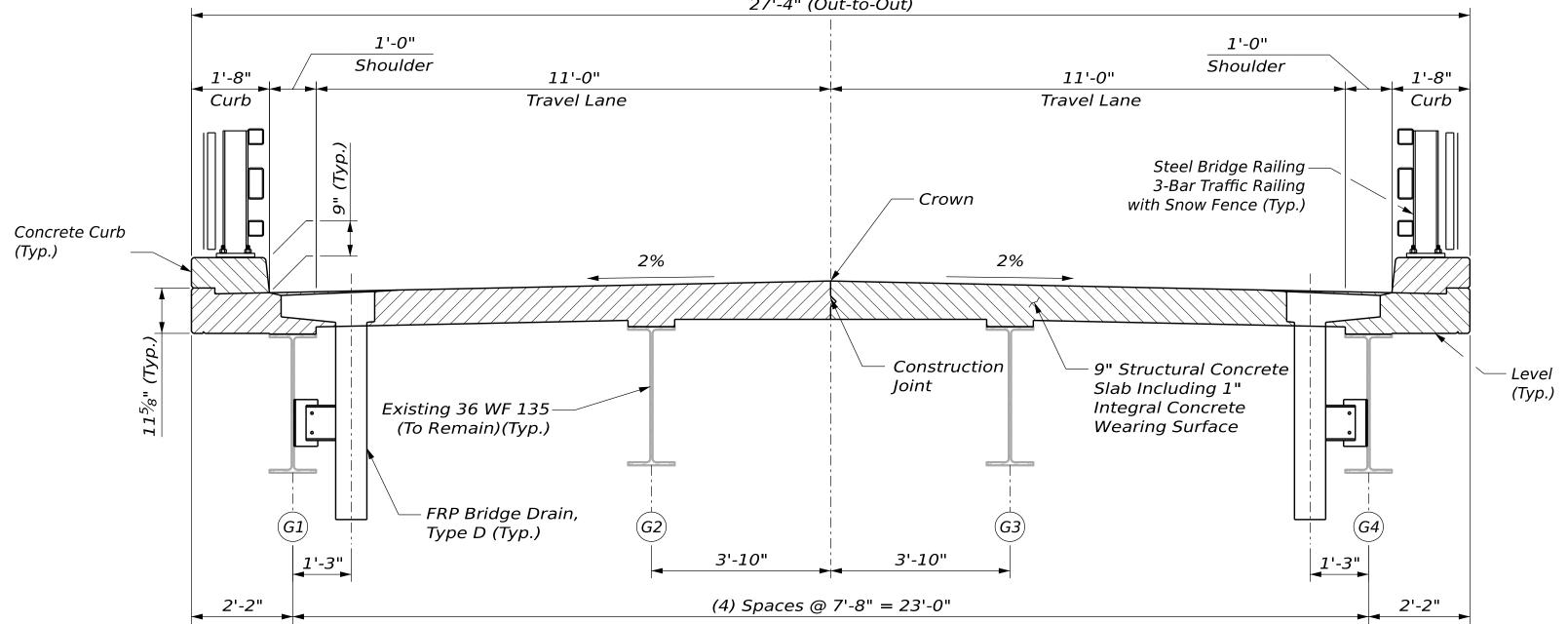
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SHEET NUMBER





#### TRANSVERSE SECTION

#### SUPERSTRUCTURE NOTES

- 1. The theoretical blocking used for design of the structure is 1.75 inches at the centerlines of bearing of the abutments and piers as measured from the top of the rolled beam flange. Refer to Standard Detail 502(03) for blocking details.
- 2. Reinforcing steel shall have a minimum concrete cover of 2 inches unless otherwise noted.
- 3. Form a one inch V-groove on the fascias of the horizontal joint between the curb and slab.
- 4. The superstructure slab concrete shall be placed continuously, per Stage, and shall be kept plastic until the entire placement has been made.
- 5. Precast Concrete Deck Panels are not allowed on this project.
- 6. The Saw Cut Grooving shall be in the longitudinal direction.
- 7. Contractor shall stagger the splice locations of the longitudinal bars.
- 8. Location of the bridge drains shall be finalized in the field. Placement may shift up to 1.5 feet upstation or downstation, if needed, to avoid conflicts for attachment of bridge drain support

																				ВОТТО	M OF SLAB ELE	EVATIONS																			
GIRDEI	CL BRG. ABUT. NO.	0.1 x L	0.2 x L	0.3 x L	0.4 x L	0.5 x L	0.6 x L	0.7 x L	. 0.8 x	L 0.9 x L	CL BRG. PIER NO. 1	0.1 x L	0.2 x L	0.3 x L	0.4 x L	0.5 x L	0.6 x L	0.7 x L	0.8 x L	0.9 x L	CL BRG. PIER NO. 2	0.1 x L	0.2 x L	0.3 x L	0.4 x L	0.5 x L	0.6 x L	0.7 x L	0.8 x L	0.9 x L	CL BRG. PIER NO. 3	0.1 x L	0.2 x L	0.3 x L	0.4 x L	0.5 x L	0.6 x L	0.7 x L	0.8 x L	0.9 x L	CL BRG. PIER NO. 4
G1	461.13	461.47	461.82	462.17	462.52	462.86	463.20	463.55	463.8	9 464.24	464.59	465.16	465.75	466.32	466.90	467.47	468.03	468.58	469.13	469.68	470.24	470.71	471.19	471.67	472.14	472.62	473.10	473.57	474.04	474.51	474.99	475.46	475.95	476.43	476.91	477.38	477.86	478.33	478.79	479.26	479.73
G2	461.19	461.54	461.89	462.24	462.58	462.93	463.27	463.62	2 463.9	6 464.31	464.66	465.24	465.82	466.40	466.98	467.55	468.11	468.66	469.21	469.75	470.31	470.78	471.26	471.73	472.21	472.69	473.17	473.64	474.11	474.58	475.05	475.53	476.02	476.50	476.98	477.46	477.93	478.40	478.86	479.33	479.80
G3	461.11	461.46	461.81	462.15	462.50	462.84	463.19	463.53	3 463.8	8 464.22	464.57	465.15	465.73	466.32	466.89	467.46	468.02	468.57	469.12	469.67	470.22	470.69	471.17	471.65	472.13	472.60	473.08	473.55	474.02	474.49	474.97	475.45	475.93	476.41	476.89	477.37	477.84	478.31	478.78	479.24	479.71
G4	460.87	461.22	461.57	461.91	462.26	462.60	462.95	463.29	463.6	4 463.99	464.33	464.91	465.49	466.07	466.64	467.21	467.77	468.33	468.88	469.43	469.98	470.46	470.93	471.41	471.89	472.36	472.84	473.31	473.78	474.26	474.73	475.21	475.69	476.17	476.65	477.13	477.60	478.07	478.54	479.00	479.48
																																_									

0/5555															воттом с	OF SLAB E	LEVATION	S													
GIRDER	CL BRG. PIER NO. 4	0.1 x L	0.2 x L	0.3 x L	0.4 x L	0.5 x L	0.6 x L	0.7 x L	0.8 x L	0.9 x L	CL BRG. PIER NO. 5	0.1 x L	0.2 x L	0.3 x L	0.4 x L	0.5 x L	0.6 x L	0.7 x L	0.8 x L	0.9 x L	CL BRG. PIER NO. 6	0.1 x L	0.2 x L	0.3 x L	0.4 x L	0.5 x L	0.6 x L	0.7 x L	0.8 x L	0.9 x L	CL BRG. ABUT. NO. 2
G1	479.73	480.21	480.68	481.16	481.64	482.12	482.59	483.06	483.53	484.00	484.48	485.05	485.63	486.20	486.77	487.34	487.90	488.46	489.01	489.57	490.13	490.59	491.05	491.52	491.99	492.45	492.91	493.37	493.82	494.27	494.72
G2	479.80	480.28	480.75	481.23	481.71	482.19	482.66	483.13	483.60	484.07	484.55	485.12	485.70	486.28	486.85	487.42	487.98	488.53	489.08	489.64	490.20	490.66	491.12	491.59	492.06	492.53	492.99	493.45	493.90	494.35	494.79
G3	479.71	480.19	480.67	481.15	481.63	482.10	482.57	483.04	483.51	483.98	484.46	485.03	485.61	486.19	486.76	487.33	487.89	488.45	489.00	489.55	490.11	490.57	491.04	491.51	491.97	492.44	492.90	493.36	493.81	494.26	494.70
G4	479.48	479.95	480.43	480.91	481.38	481.86	482.33	482.80	483.27	483.75	484.22	484.79	485.37	485.95	486.52	487.08	487.65	488.20	488.75	489.31	489.87	490.33	490.80	491.26	491.73	492.20	492.66	493.11	493.57	494.02	494.47

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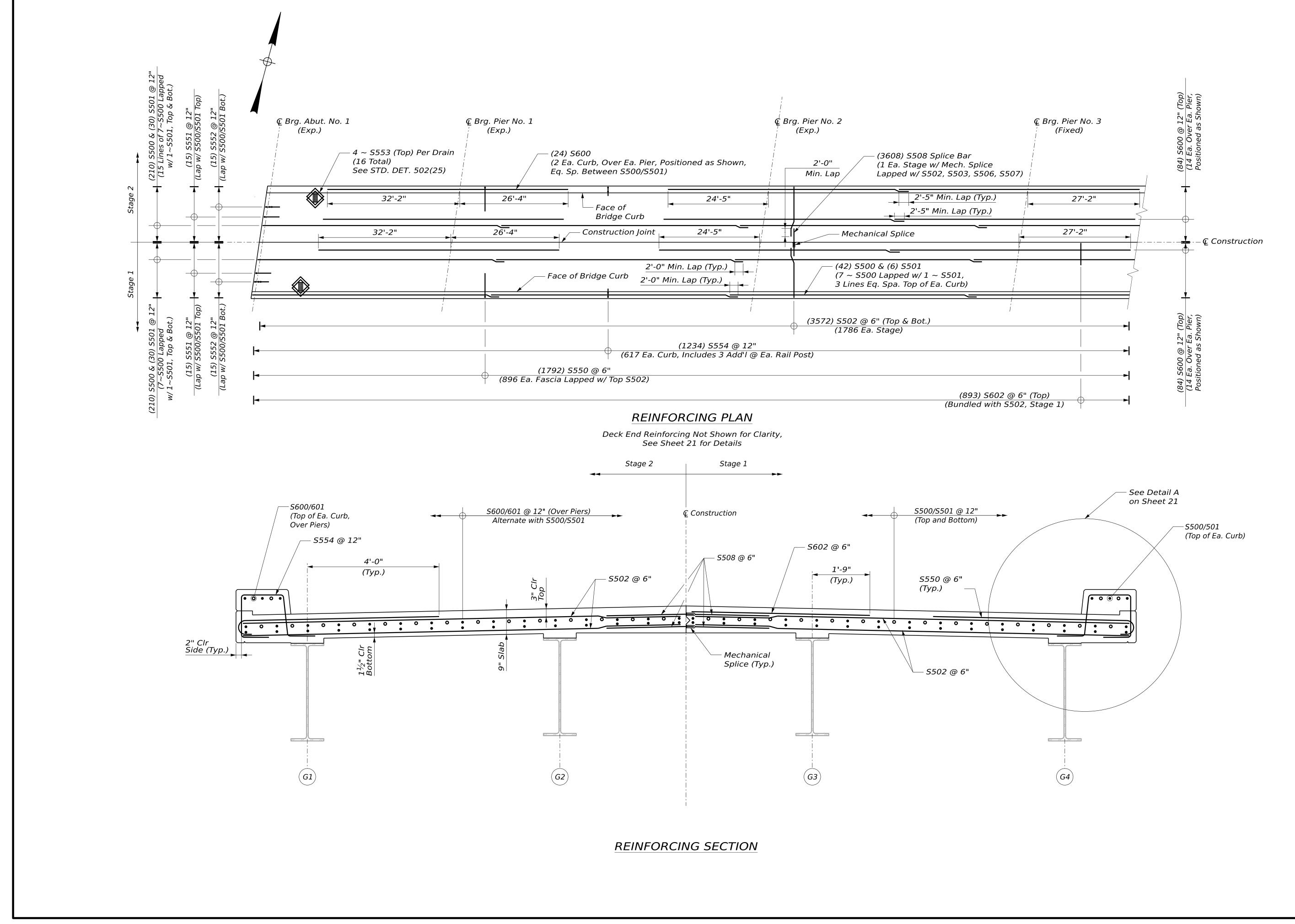
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BRIDGE

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BRIDGE

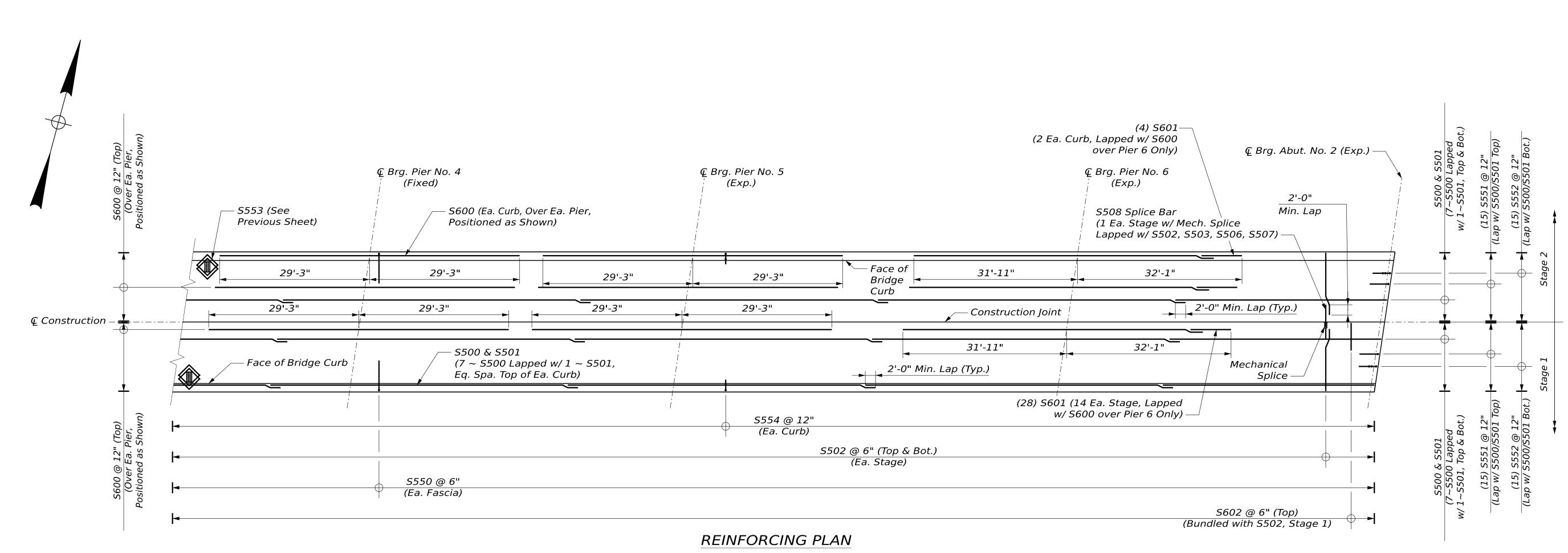
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REINFORCING

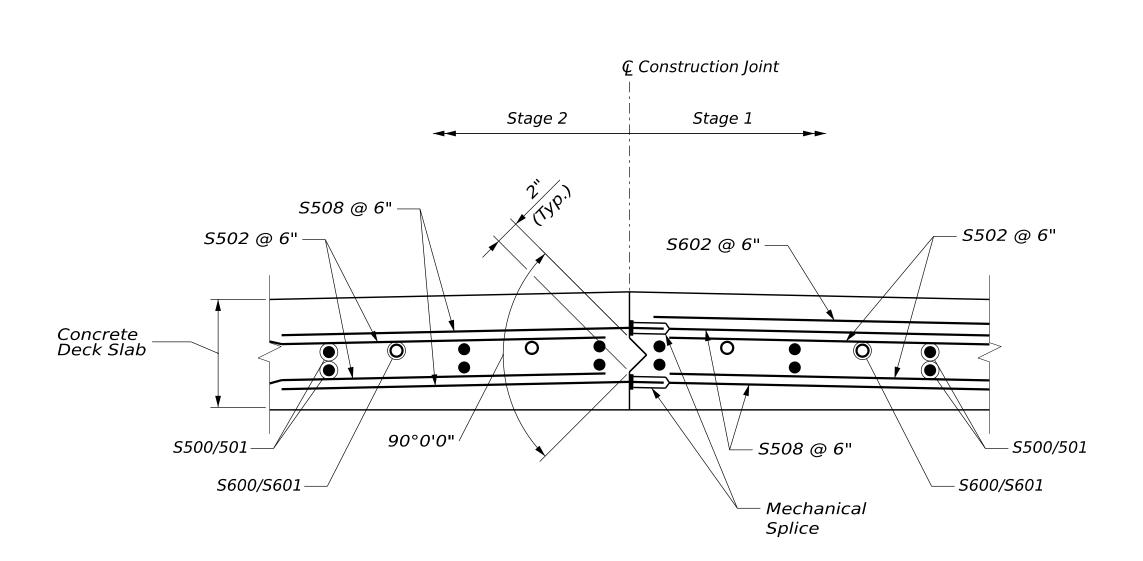
SHEET NUMBER

OF 29

WIN 26238.00



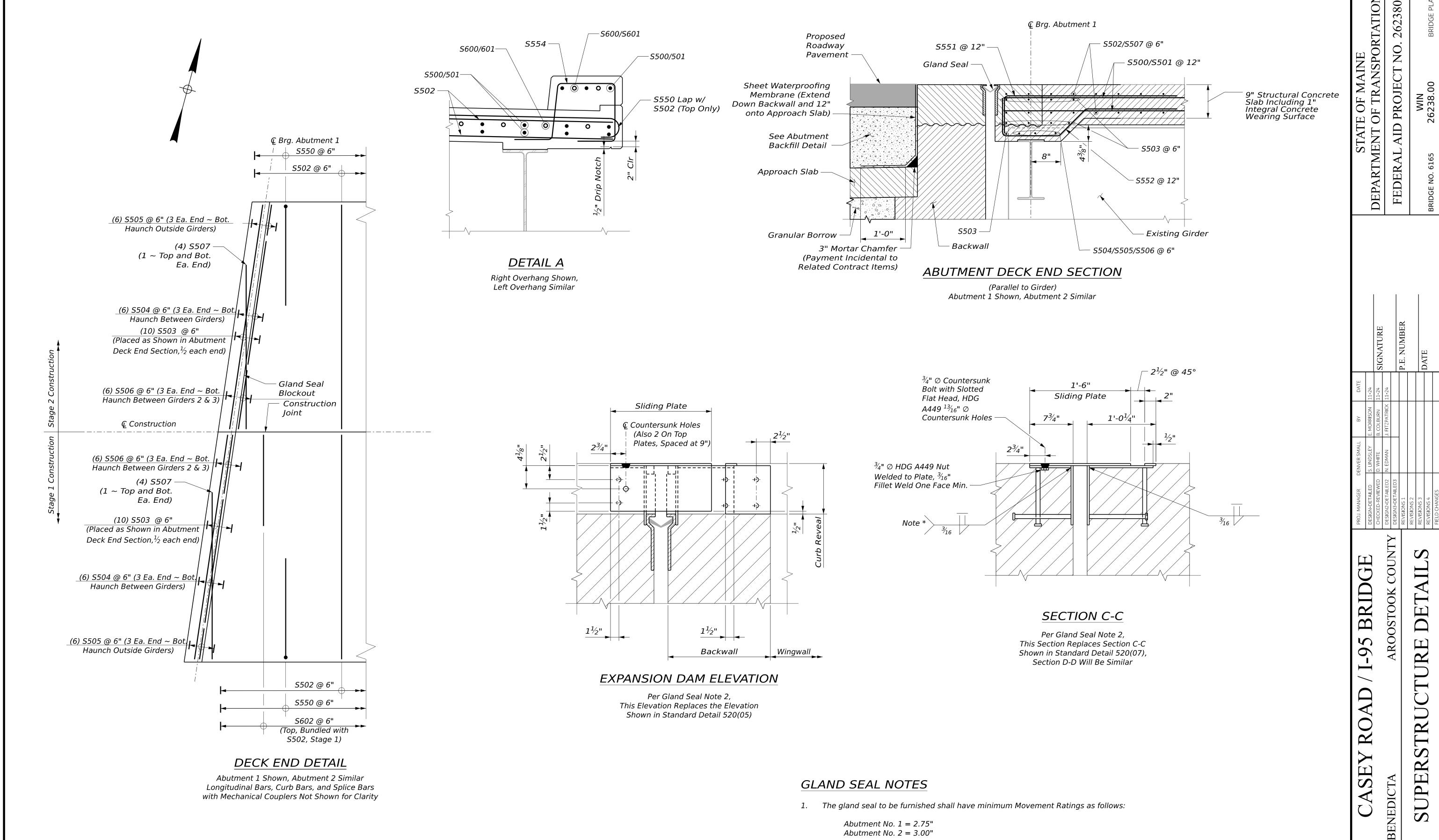
Deck End Reinforcing Not Shown for Clarity, See Sheet 21 for Details



LONGITUDINAL CONSTRUCTION JOINT DETAIL

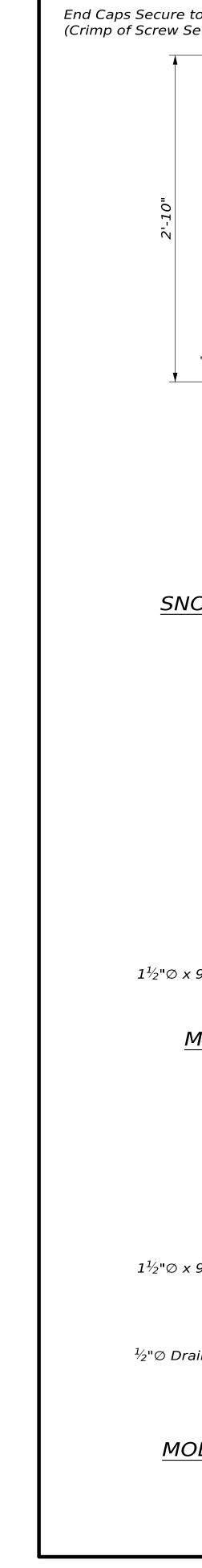
STATE OF MAINE DEPARTMENT OF TRANSPORTATION AID PROJECT NO. WIN 26238.00 FEDERAL P.E. NUMBER BRIDGE AROOSTOOK REINFORCING -95 SHEET NUMBER OF 29

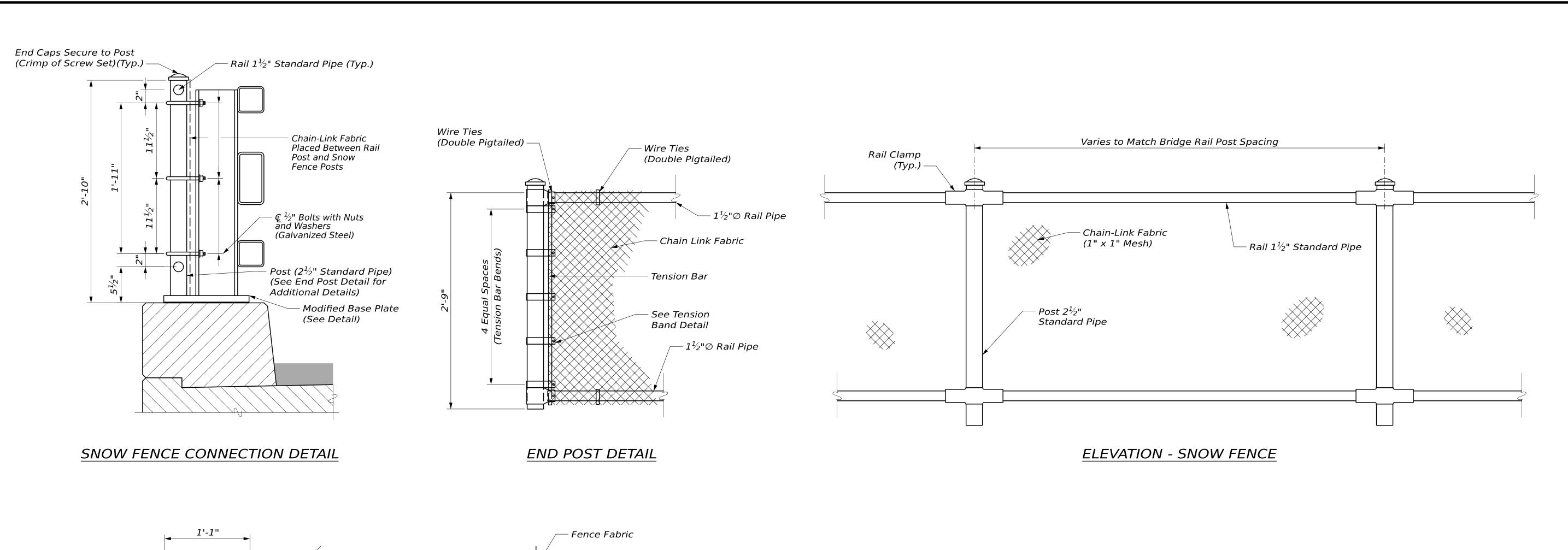
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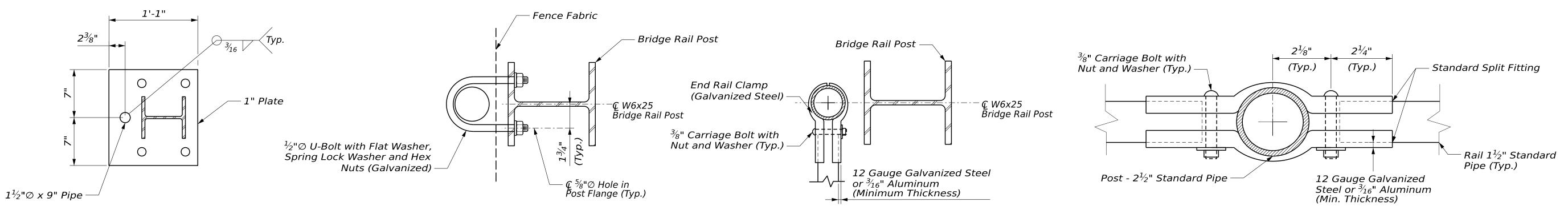


2. The Standard sliding plate detail for the curbs shall be modified to use countersunk bolts.

SHEET NUMBER







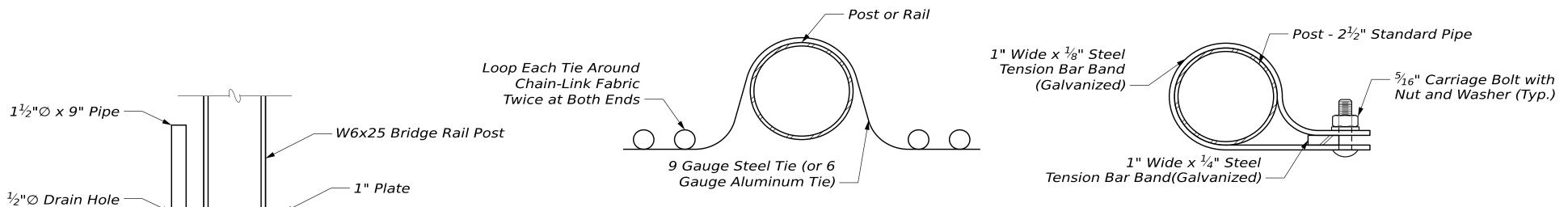
MODIFIED BASE PLATE DETAIL

U-BOLT CONNECTION DETAIL

END POST SECTION

(U-Bolt Not Shown)

RAIL CLAMP DETAIL



MODIFIED BASE PLATE ELEVATION DOUBLE PIGTAILED TIE

TENSION BAND DETAIL

#### SNOW FENCE NOTES

- 1. Chain-Link fence shall conform to Section 710.03 and Special Provision Section 607. The size of wire mesh (fabric) shall be 1".
- 2. Post and rail pipe shall be hot-dip galvanized. All pipe shall be schedule 40, standard weight. Nominal pipe sizes are shown.
- 3. Tension bars, bar bands, boulevard and end rail clamps shall be steel or aluminum alloy conforming to AASHTO M181 (ASTM F626). Steel components shall be hot-dip galvanized in accordance with AASHTO M111 (ASTM A123) or AASHTO M232 (ASTM A153) as applicable.
- 4. All bolts and nuts shall be steel conforming to ASTM A307 and ASTM A563 grade A respectively. Washers shall be hardened steel commercial type A plain and shall meet the dimensional requirements of ANSI B18.22. All bolts, nuts, and washers shall be hot-dip galvanized in accordance with AASHTO M111 (ASTM A123) or AASHTO M232 (ASTM A153) as applicable.
- 5. Rail may be field cut (sawn) to fit post spacing. Repair galvanizing on cut edges in accordance with ASTM A780.
- 6. Payment for modified base plate will be considered incidental to related Contract Items.

CASEYROAD / I-95 BRIDGE

BENEDICTA

BENEDICTA

AROOSTOOK COUNTY

BENSIONS 1

SNOW FENCE DETAILS

REVISIONS 2

REVISIONS 4

REVISIONS 4

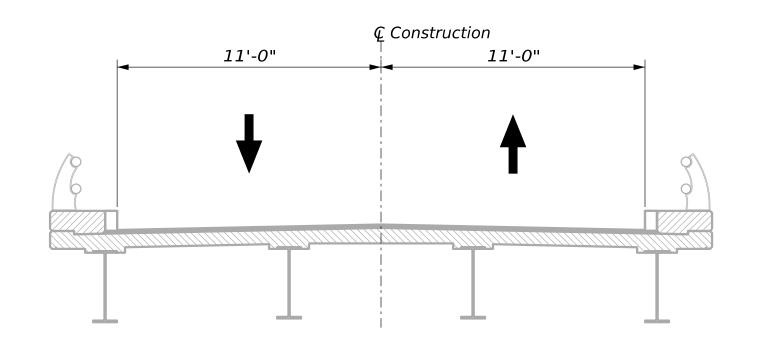
REVISIONS 4

REVISIONS 4

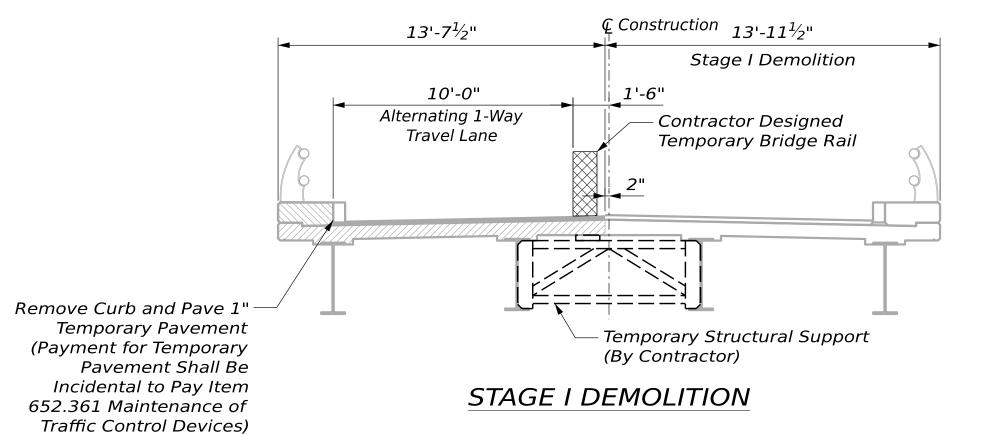
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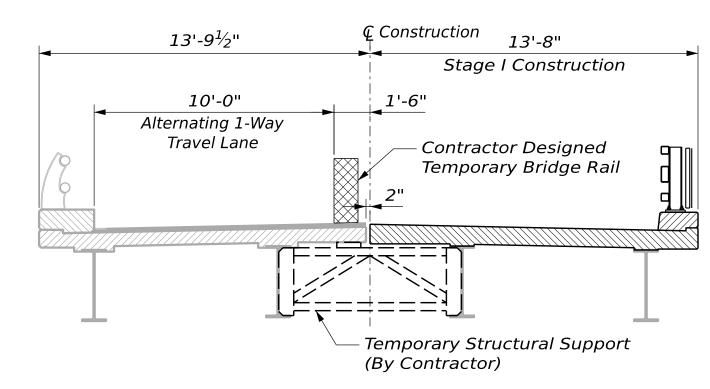
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	(	STRAIGHT BARS					·					BENT BARS					1		70	z
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	Approach Slabs		butment 1	S550 S551	1792	6'-7" 4'-8"	J	6'-0" 3'-10"	7" 10"			Superstructure		7"		Deck Overhang  Deck End	$B \bigcap_{D} D$	<b>-</b>	RTATI ), 2623	BRIDGE
AS601 92 15'-0	-0" Longitudinal	A502 26 3'-2"	Approach Slab Seat	S552	60	4'-10"	SJ	2'-0"	11"	1'-1"	10"	0"				Deck End	A G	$A \longrightarrow A$		
	O ve a ratio vati va	A503 6 5'-3"	Left Wingwall	S553	16	6'-0"		3'-0"	3'-0"	41.0"	41.0"		40"	41	<b></b>	Bridge Drain	<u>O</u>	$\overline{B}$	AINE NSPC	1
S500 462 60'-0	Superstructure -0" Longitduinal	A504 6 5'-11" A505 12 5'-8"	Right Wingwall  Left & Right WW	S554	1234	5'-5"	SC	10"	1'-3"	1'-3"	1'-3"		10"	1'	5"	Curb Stirrups	<u>SC</u>	J		00
S501 66 41'-4	-4" Longitduinal	A506 18 4'-1"	Left & Right WW Vert. (D. & G.)	+								Abutment 1					1	<del>-</del>	I	WIN 238.0
S502 3572 13'-4		A507 28 3'-0"	Mechanical Splice	A550	26	9'-6"	U		4'-2"	1'-2"	4'-2"			01.7.4/01		Backwall			E OF TO PRO	M W
S503         20         13'-5           S504         12         7'-5		A508 10 3'-11" A509 5 5'-8"	Curtainwall Right Wingwall Curb	A551 A552	6 6	5'-4" 5'-4"	A V				2'-8" 2'-8"	2'-8" 2'-8"		2'-7 1/2" 2'-7 1/2"		Backwall Corner Backwall Corner	_		TATI NT (	,   '`
S505 12 1'-10		A510 5 5'-4"	Left Wingwall Curb	A553	11	5'-9"	V				2'-0"	3'-9"		1'-8"		Left and Right Wingwall	$\frac{E}{1}$	A	ST/ IEN LA	1
S506 12 3'-6				A554		10'-7 1/2"	U			1'-5 1/2"	4'-7"					Left and Right Wingwall	A B 1			165
S507         8         9'-1'           S508         3608         2'-4			butment 2  Backwall	A555 A556	26 26	4'-5" 1'-10 1/2"		2'-0" 5 1/2"	1'-7" 1'-5"				10" 10"			Corbel Ties	С	В	RT RE	0 0
3000 2-4	4 Weenaniea Opiice	B500 26 13-7 B501 56 3'-8"	Backwall Vertical (Drill & Grout)	7,000	20	1-10-1/2	1 0	3 1/2	1-5				10			OOIDOI 1103	<u>SJ</u>	<u>L</u>	PA G	
S600 192 58'-0			Approach Slab Seat						T			Abutment 2							DE	3RID
S601         32         7'-1'           S602         893         5'-5			Left Wingwall  Right Wingwall	B550 B551	26	10'-0" 5'-4"	U A		4'-5"	1'-2"	4'-5" 2'-8"	2'-8"		2'-7 1/2"		Backwall Backwall Corner	D	C		
	7 Maillonal Overnain	B505 13 6'-5"	Left & Right WW	B552	6	5'-4"	V				2'-8"	2'-8"		2'-7 1/2"		Backwall Corner		В		
		B506 20 4'-1"	Left & Right WW Vert. (D. & G.)	B553	13	5'-9"	V		<b>-</b> 1 411	41 5 4/00	2'-0"	3'-9"		1'-8"		Left and Right Wingwall	)	1 1		
		B507 28 3'-0" B508 10 3'-11"	Mechanical Splice  Curtainwall	B554 B555	28 7	11'-7 1/2" 4'-5"	P3	2'-0"	5'-1" 1'-7"	1'-5 1/2"	5'-1"		10"			Left and Right Wingwall Corbel	<u>P3</u>	U		
		B509 5 6'-1"	Right Wingwall Curb	B556	+	1'-10 1/2"		5 1/2"	1'-5"				10"			Corbel Ties	1	<del></del>		
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																	Bending details and hoo	ks shall conform to	500	S H
																	the recommendations of of ACI Standard 315 and	the current revision	-9 AR	D H
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																	Plain Reinforcing Steel: A Stainless Steel Reinforci Glass Fiber Reinforced P	אונא א אווי A אווי א טווי A אווי אווי אווי אווי אווי אווי אווי		THE COLUMN
																	Glass Fiber Reinforced P Low-Carbon Chromium S	olymer: ASTM D7957 teel: ASTM A1035		RC HE
				1													Type CS, Grade 100			OF CF
																	CENTER	N NOTEC		F S
				<u> </u>														AL NOTES		REINF(S)
				<u> </u>													1. The first digit(s) follow mark indicate the size of	ving tne letter(s) of the the bar:		
																	Mark "A502" = bar size		SE	
				<u> </u>													Mark "P805" = bar size	#8		
				<del> </del>													Mark "S650" = bar size Mark "P1404" = bar siz			
																	1	following the bar number	ENE	
																	indicates the material of	the bar.	BE	
																	"A500b", b = (Black) Pl	ain Steel	SHEET NU	JMBER
																	"A550s", $s = Stainless$ "S500p", $p = Glass Fibe$	Steel		<b>   \</b>
																	"P510c", $c = Low$ -Carbo	on Chromium Steel	1 7	2
																	3. All reinforcement bar	s shall be Plain	23	)
																	Reinforcing Steel.			
																			OF 2	<i>2</i> 9

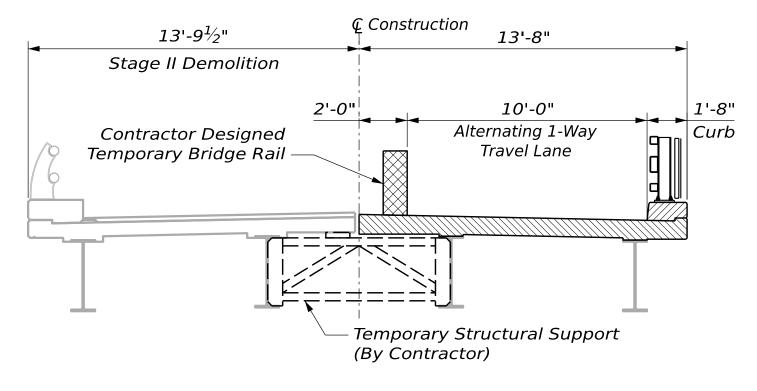


EXISTING TRANSVERSE SECTION

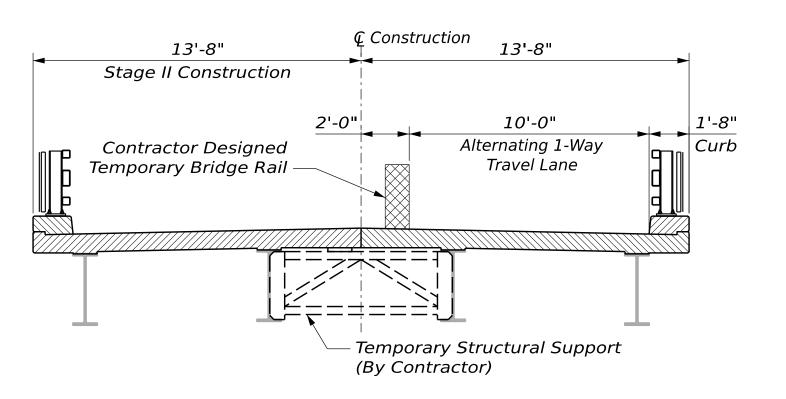




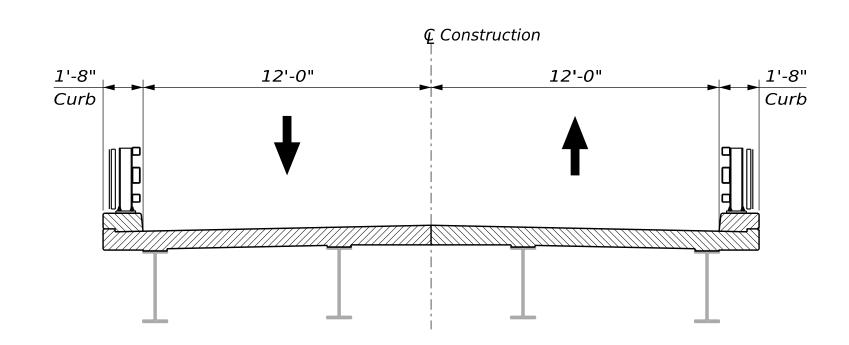
STAGE I CONSTRUCTION



STAGE II DEMOLITION



STAGE II CONSTRUCTION



PROPOSED TRANSVERSE SECTION

SHEET NUMBER

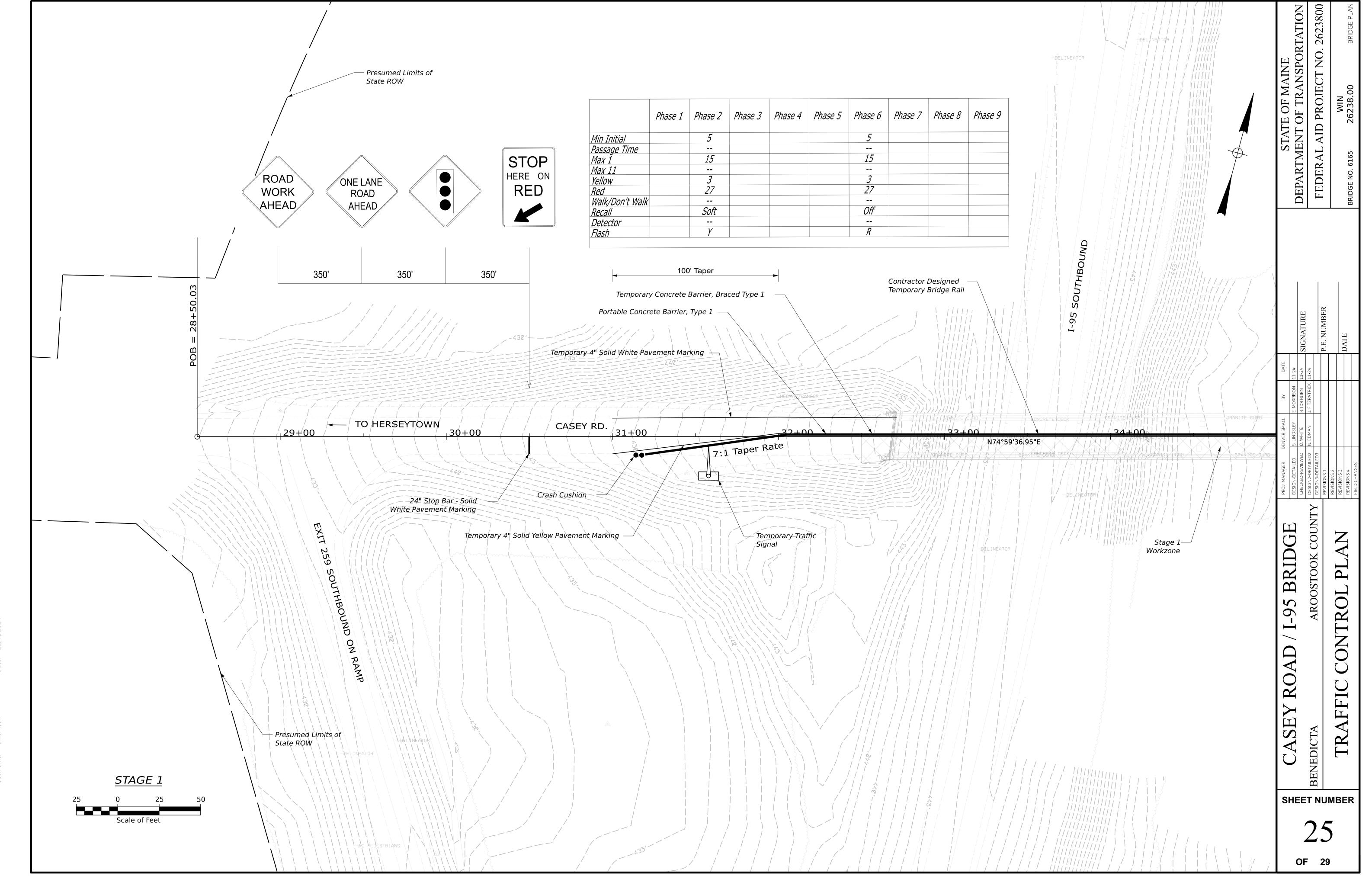
AROOSTOOK COUNTY

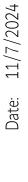
CONSTRUCTION

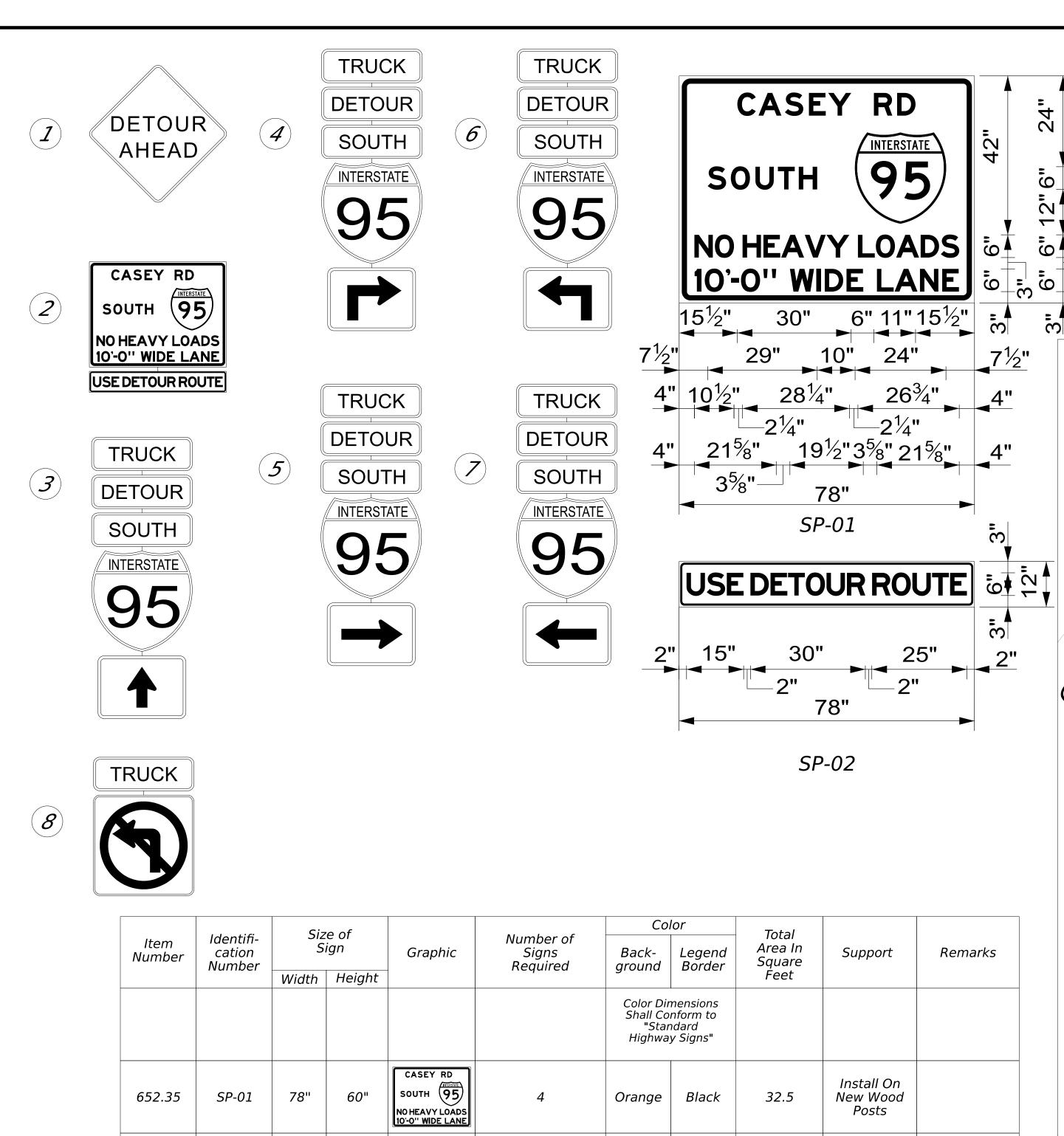
BRIDGE

-95

P.E. NUMBER







		Siz	re of		N	Со	lor	Total		
Item Number	Identifi- cation Number		ign	Graphic	Number of Signs Required	Back- ground	Legend Border	Area In Square	Support	Remarks
	Number	Width	Height		Neganea	ground	Border	Feet		
						Shall Co "Star	mensions Inform to Indard Indard Indars			
652.35	SP-01	78"	60"	CASEY RD SOUTH 95 NO HEAVY LOADS 10'-0" WIDE LANE	4	Orange	Black	32.5	Install On New Wood Posts	
652.35	SP-02	78"	12"	USE DETOUR ROUTE	4	Orange	Black	6.5	Install Beneath SP-01	

#### **DETOUR NOTES**

- 1. All sign locations are approximate, actual locations shall be determined in the field by the Resident.
- 2. All modifications to traffic control devices shall be in accordance with the Manual On Uniform Traffic Control Devices. Changes and adjustments to traffic control devices shall be approved by the Resident prior to implementation.
- 3. Any and all changes and adjustments to traffic control devices shall be included in the Traffic Control Plan submitted. The Traffic Control Plan shall be submitted, designed, and stamped by a Professional Engineer licensed in State of Maine, and shall be approved by the Resident prior to implementation.

- 4. The Contractor shall cover all existing signs that conflict with work zone signs. Payment will be incidental to Pay Item 652.35.
- 5. In addition to signs shown, the Contractor shall place portable changeable message signs prior to closure. Locations and messages to be determined by the Contractor and approved by the Resident.
- 6. Additional signs may be needed as directed by the Resident.

