

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION



ALBION
KENNEBEC COUNTY
PUDDLE DOCK BRIDGE
OVER
FIFTEENMILE STREAM
SOUTH FREEDOM ROAD
FEDERAL AID PROJECT NO. 2529900
PROJECT LENGTH 0.110 mi.
BRIDGE NO. 3107

SPECIFICATIONS

Design: Load and Resistance Factor Design per AASHTO LRFD Bridge Design Specifications, Ninth Edition 2020.

DESIGN LOADING

Live Load HL - 93 Modified for Strength I

HYDROLOGIC DATA

Drainage Area	16.91 sq mi
Design Discharge (Q50)	1,362.81 cfs
Check Discharge (Q100)	1,541.55 cfs
Headwater Elevation (Q1.1)	219.48 ft
Headwater Elevation (Q10)	221.61 ft
Headwater Elevation (Q25)	222.22 ft
Headwater Elevation (Q50)	222.68 ft
Headwater Elevation (Q100)	223.13 fps
Discharge Velocity (Q1.1)	5.20 fps
Discharge Velocity (Q10)	8.18 fps
Discharge Velocity (Q25)	8.78 fps
Discharge Velocity (Q50)	9.18 fps
Discharge Velocity (Q100)	9.50 fps

MATERIALS

Concrete:	
Curbs	Class "LP"
All Other	Class "A"

Reinforcing:	
Plain Reinforcing Steel	ASTM A615, Grade 60
Glass Fiber Reinforcing Polymer (GFRP)	ASTM D7957
Low-Carbon Chromium Steel:	ASTM A1035, Type CS, Grade 100

Structural Steel:	
All Material (except as noted)	ASTM A709, Grade 50W (unpainted)
High Strength Bolts	ASTM F3125, Grade A325, Type 3
H-Piles	ASTM A572, Grade 50

BASIC DESIGN STRESSES

Concrete:	
Class "A"	f'c = 4,000 psi
Class "LP"	f'c = 5,000 psi

Reinforcing:	
Plain Reinforcing Steel	f y = 60,000 psi
Low-Carbon Chromium Steel	f y = 100,000 psi

Glass Fiber Reinforced Polymer	f fu = 100,000
Minimum Elastic Modulus	E f = 8,700,000 psi
Minimum Nominal Design Tensile Strain	e fu = 1.1%

Structural Steel:	
ASTM A709, Grade 50W	F y = 50,000 psi
ASTM F3125, Grade A325	F μ = 120,000 psi

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UTILITIES

Central Maine Power Company
Charter Communications, Inc.
Unitel, Inc.

TRAFFIC DATA

Current (2021) AADT	1070
Future (2041) AADT	1230
DHV - % of AADT	10%
Design Hour Volume	123
Heavy Trucks (% of AADT)	11
Heavy Trucks (% of DHV)	14
Directional Distribution (% of DHV)	51
18 kip Equivalent P 2.0	52
18 kip Equivalent P 2.5	49
Design Speed (mph)	25

MAINTENANCE OF TRAFFIC

Existing bridge is currently closed to traffic and shall remain closed during construction.

PROJECT LOCATION	On the South Freedom Rd. 0.11 of a Mile North of Knights Rd. Over Fifteenmile Stream. Lat./Long. 44°30'48.22" N 69°25'08.39" W
PROGRAM AREA	Highway Bridges - Traditional
OUTLINE OF WORK	Bridge Replacement

STATE OF MAINE DEPARTMENT OF TRANSPORTATION	APPROVED	DATE
COMMISSIONER: <i>[Signature]</i>		11-25-22
CHIEF ENGINEER: <i>[Signature]</i>		11-20-2024

STATE OF MAINE ENGINEER Roger M. Neale 9918 LICENSED PROFESSIONAL ENGINEER	SIGNATURE <i>[Signature]</i>	P.E. NUMBER 9918	DATE NOV 18 2024
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PROJECT INFORMATION	PROGRAM	PROJECT MANAGER	DESIGNER	CONSULTANT	PROJECT RESIDENT	CONTRACTOR	PROJECT COMPLETION DATE
	BRIDGE PROGRAM	BRIAN NICHOLS	ERIN BREWER				

ALBION PUDDLE DOCK BRIDGE	TITLE SHEET
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SHEET NUMBER
1
OF 31

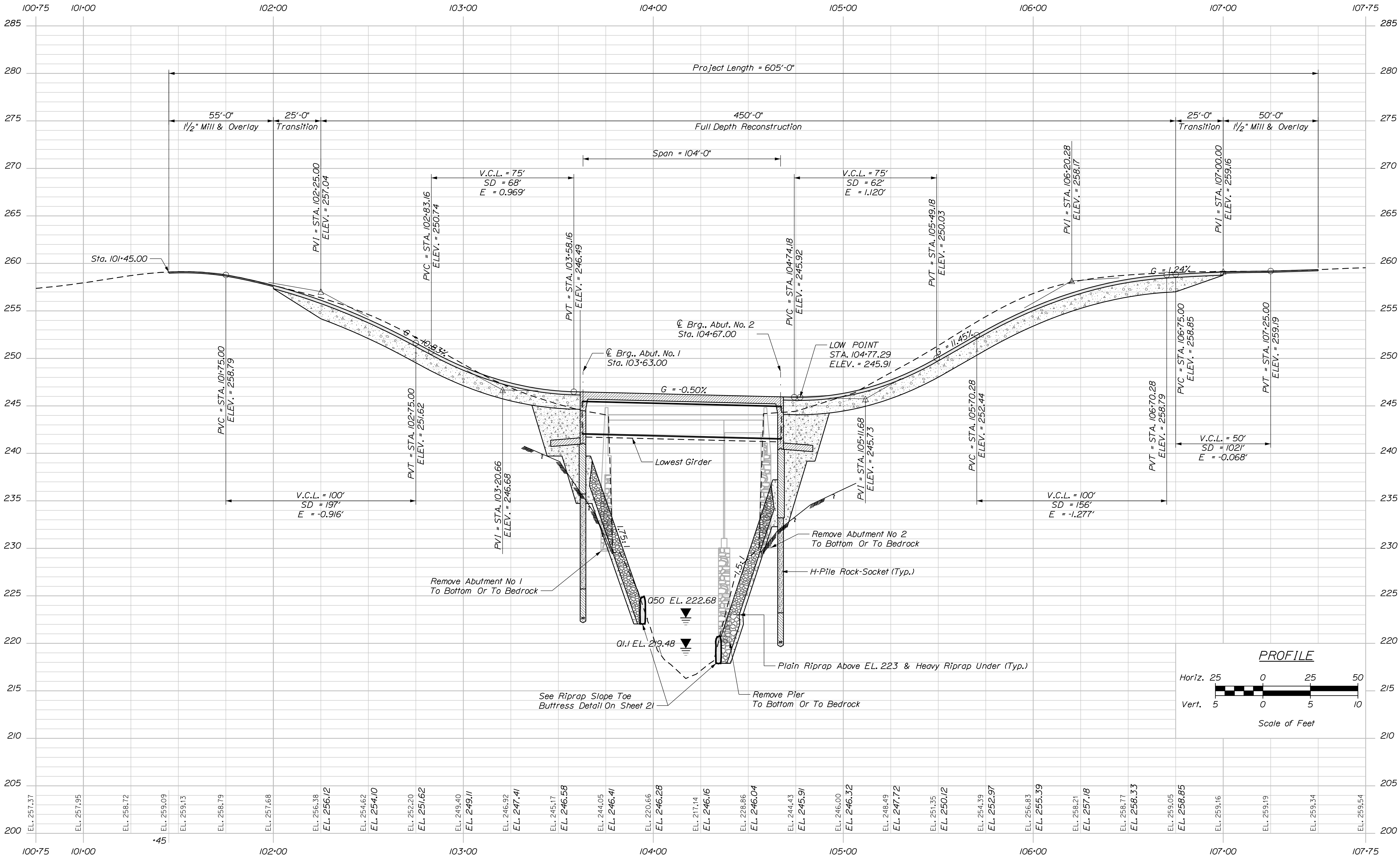
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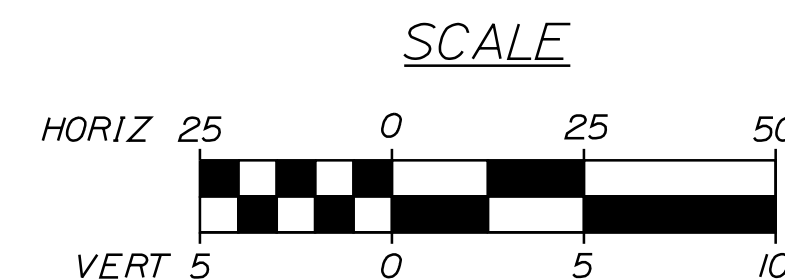
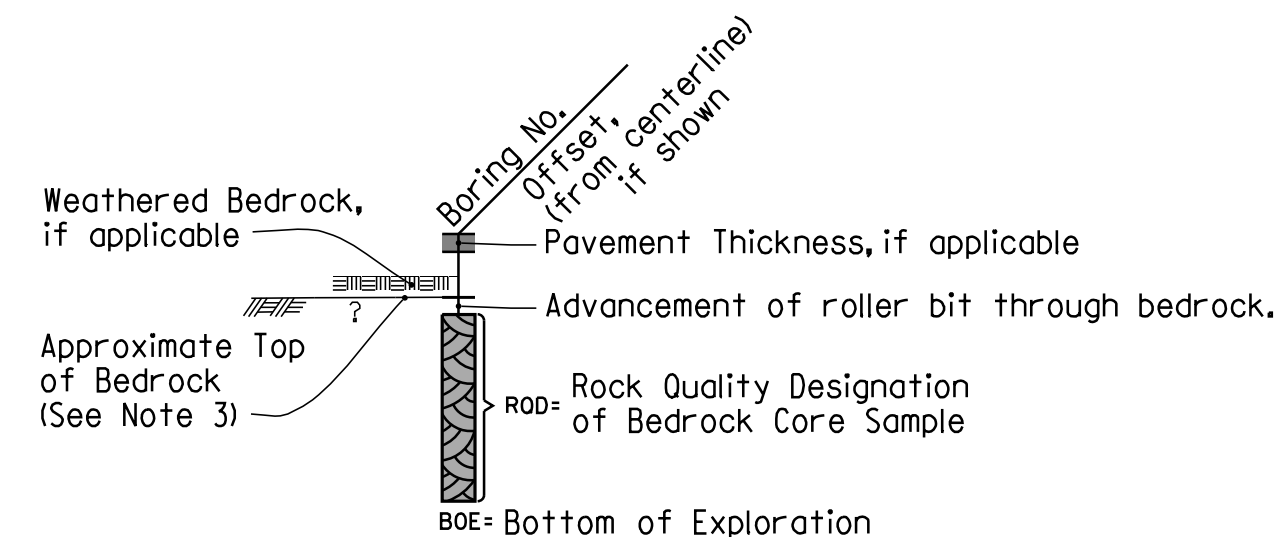
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SHEET NUMBER										STATE OF MAINE									
4										DEPARTMENT OF TRANSPORTATION									
										02529900									
ALBION										BRIDGE NO. 3107									
										WIN									
FIFTEENMILE STREAM										025299.00									
										BRIDGE PLANS									
KENNEBEC COUNTY										SIGNATURE									
										P.E. NUMBER									
PROFILE										DATE									
PROJECT MANAGER										BRIAN NICHOLS									
										BY									
DESIGN-DETAILED										MRP									
										E. BREWER									
CHECKED-REVIEWED										RWN									
DESIGN2-DETAILED2																			
DESIGN3-DETAILED3																			
REVISONS 1																			
REVISONS 2																			
REVISONS 3																			
REVISONS 4																			
FIELD CHANGES																			

OF 31



SHEET NUMBER 5 OF 31	PUDDLE DOCK BRIDGE FIFTEENMILE STREAM ALBION KENNEBEC				PROJ. MANAGER TONY BEAULIEU E. BREWER	BY MRP	DATE Sept 2024
	DESIGNED-REVIEWED				SIGNATURE		
	DESIGN2-DETAILED2 IN WILLIAMS				P.E. NUMBER		
	DESIGN3-DETAILED3				DATE		
	REVISIONS 1						
BORING LOCATION PLAN & INTERPRETIVE SUBSURFACE PROFILE				REVISIONS 2			
				REVISIONS 3			
				REVISIONS 4			
				FIELD CHANGES			
				BRIDGE NO. 3107			
				WIN			
				025299.00			
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				STATE OF MAINE DEPARTMENT OF TRANSPORTATION			

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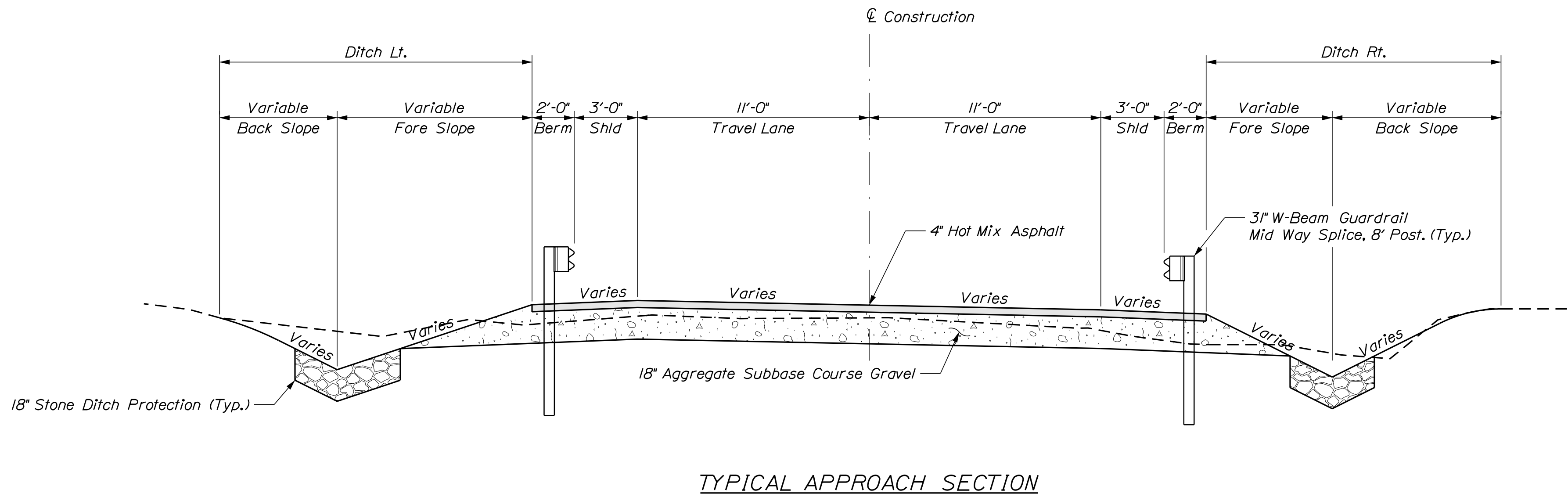
Maine Department of Transportation										Project: Puddle Back Bridge #1071 carries South Freedom Road over Location: Alden, Maine		Boring No.: BB-AFWS-102	
Soil/Bore Exploration Log US CUSTOMER UNITS										WIN: 25299.00			
Drillers: S.W. Cole		Elevation (ft.): 244.5		Auger 10/00:		5" Solid Stem							
Operator: Kevin/Kyle		Datum: NW088				Sampler: Standard Split Spoon							
Logged By: B. Wilder		Rig type: DIXIECH 0-50		Hammer Str./ft.: 140W/30"									
Date Start/Finish: 12/23/2021 07:30-09:30		Drilling Method: Coated Wash Boring		Core Barrel: NW-2"									
Boring Location: 103469.7, 3.6 ft Li.		Casing 10/00: NW-3"		Water Level#: None Observed									
Hammer Efficiency Factor: 0.92				Hammer Type: Automatic ☐ Hydraulic ☐ Rope & Cathead ☐									
P = Rock Core Sample S = Split Stem Sample B = Unconsolidated Split Stem Sample Attached W = Thin Wall Tube Sample H = Hydraulic Thin Wall Tube Sample F = Field Test (See Log) R = Rock Core Sample SS = Solid Stem Auger SA = Solid Stem Auger NC = Roller Core NW = Weight of Rock Hammer NWG = Weight of Rock of Casing B = Unconsolidated Split Stem Sample Attached S = Rock Core Sample S = Sub Area (arches shear strength test) B = Borehole (arches shear strength test) H = Horizontal N = Non-Recorded Np = Non-Recorded Np = Non-Recorded Np = Non-Recorded S = Rock Core Sample S = Sub Area (arches shear strength test) B = Borehole (arches shear strength test) H = Horizontal N = Non-Recorded Np = Non-Recorded Np = Non-Recorded Np = Non-Recorded													
Depth (ft.)		Sample No.		Sample Depth (ft.)		Sample Depth (ft.)		Sample Depth (ft.)		Laboratory Testing Results/ASTM and Unified Class			
Depth (ft.)		Sample No.		Sample Depth (ft.)		Sample Depth (ft.)		Sample Depth (ft.)		Laboratory Testing Results/ASTM and Unified Class			
0		1D		24/14		5.00 ± 7.00		8/9/14/2		13 20			
5		1D		24/14		5.00 ± 7.00		8/9/14/2		13 20			
10		2D		6/5		10.00 ± 10.50		57/6"		---			
15		R1		37/2		14.10 ± 15.00		R0D ± 0%		NO-2"			
20		R2		22/8		14.10 ± 15.00		R0D ± 0%		NO-2"			
25		R3		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
30		R4		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
35		R5		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
40		R6		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
45		R7		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
50		R8		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
55		R9		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
60		R10		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
65		R11		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
70		R12		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
75		R13		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
80		R14		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
85		R15		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
90		R16		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
95		R17		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
100		R18		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
105		R19		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
110		R20		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
115		R21		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
120		R22		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
125		R23		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
130		R24		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
135		R25		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
140		R26		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
145		R27		62/55		16.00 ± 21.00		R0D ± 0%		NO-2"			
150		R28		62/55		16.0							

Maine Department of Transportation										Project: Paddle Back Bridge #7031 carries South Freedom Road over Location: Allison, Maine		Boring No.: BB-AFMS-103	
Soil/Bore Exploration Log US CUSTOMARY UNITS										WIN:		25299.00	
Drillers: Wayne/DJ		Elevation (ft.): 244.4		Auger 10/ID: 5" Solid Stem									
Operator: Daggett/Brooks		Datum: NAVD83		Sampler: Standard Split Spoon									
Logged By: J. Monahan		Rig Type: CM 45C		Hammer Wt./Falliz 140W/30"									
Date Start/Finish: 12/21/2021 07:45-09:45		Drilling Method: Cased Wash Boring		Core Barrel: NQ-2"									
Boring Location: 10445.6, 2.4 ft Rt.		Logging 10/ID: NQ-3"		Water Level#: LOST H2O after coring									
Hammer Efficiency Factor: 0.906				Hammer Type: Automatic ☐ Hydraulic ☒		Rope & Cathed ☐							
R = Rock Core Sample SA = Solid Stem Auger NS = Unconsolidated Soil / Spoon Sample Arrest SS = Soil Stem Auger NC = Non-Cohesive RC = Roller Cone U = Unclassified, This Well Has Loose Material RW = Weight of Hole, Hammer J = Field Test Stage Test PR = Pocket Penetrometer MWG = Weight of Rods or Cores NS = Unconsolidated Field Test Stage Test MWG = Weight of Rods or Cores				Automatic ☐ Hydraulic ☒ N = Non-Cohesive T = Thin Section S = Shear Strength Test U = Lab test unclassified shear strength test G = Geotechnical Investigation C = Consolidation N = Unconsolidated - Raw Field SPT blow Hammer Efficiency Factor: 0.906 C = Consolidation N = Unconsolidated Correlated for Hammer Efficiency G = Grain Size Analysis N = Unconsolidated Factor 0.906 Unconsolidated C = Consolidation Test									
Depth (ft.)	Sonde No.	New/Roc. (in)	Sample Depth (ft.)	Blow/V.S. (ft/m)	Shear Strm (psi) or SDP (%)	Unconsolidated	Neg	Coring Status	Elevation (ft.)	Pneumatic Log	Visual Description and Remarks	Laboratory Testing Results/ASTM and Unified Class	
								SSA					
5	10	24	5.00 - 7.00	3/3/5/4	8	12					Brown, damp, medium dense, fine to medium SAND, some silt, trace gravel, fillit.	GR80783 4.2-4.5, SM WC13.9%	
											Boulder from 8.0-9.0 ft bgs.		
10											Brown, damp, Silty SAND,		
	SI		1,2/1-2	10.00 - 10.10	35(1,2")	----					Sample of weathered rock taken from Auger 11.0-12.0 ft bgs	17.0	
	R1	60/60	12.80 - 17.80	R0D = 48%							019 blows for 0.5 ft.	-12.5	
											Top of Bedrock at Elev. 231.9 ft.	-12.8	
15											R1: Bedrock Grey to blue-grey, very fine to medium-grained, GRANULOSITY moderately hard, distinct layering is steeply inclined, fairly strong foliation parallelizing layering; weak and shelled along foliation; near vertical fractured across foliation, annealed, well-sorted, close.		
	R2	60/60	17.80 - 22.80	R0D = 56%							(Vassalboro Formation) R1 Core Times (min/sec) 12-8-10.8 ft (1218) 13-8-10.8 ft (1247) 14-8-10.8 ft (1264) 15-8-10.8 ft (1295) 16-8-11.8 ft (1271) 100% Recovery		
20											R2: Bedrock similar to R1, except core run increases in competency with depth, and two joint sets steep and moderately dipping north, fresh and with rock rind 1/4 group.	17.8	
											Rock Quality = Fair R2 Core Times (min/sec) 17-8-10.8 ft (1218) 18-8-10.8 ft (1252) 19-8-20.8 ft (1149) 20-8-21.8 ft (1217) 21-8-22.8 ft (1428) 100% Recovery	20.71-21.09 q=15-1465 GST 22.33-22.71 q=15-1300 PSI	
25											Bottom of Exploration at 22.8 feet below ground surface.	-27.8	
30													
REMARKS:													

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.

* Asterisk level readings have been made at times and under conditions states. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.

Maine Department of Transportation										Project: Rustic Road Bridge #107 carries South Freedom Road over Project Allision, Maine		Boring No.: BB-AFMS-104	
Soil/Rock Exploration Log										WIN:		25299.00	
US CUSTOMER UNITS													
Driller:		McIntosh		Elevation (ft.):		244.9		Auger (ID/B):		5" Solid Stem			
Operator:		Daggett/Brooks		Datum:		NAV88		Sampler:		Standard Split Spoon			
Logger:		J. Wenhon		Rig Type:		CME 45C		Hammer Wt./Fall:		140w/30"			
Date Start/Finish:		12/21/2024 10:00-12:30		Drilling Method:		Closed Wash Boring		Core Barrel:		MD-2"			
Boring Location:		104#2.8, 5.1 ft. E.		Casing (ID/B):		NK-3"		Water Level:		None Observed			
Hammer Efficiency Factor: 0.906 Corrections:				Hammer Type:		☒ Automatic ☐ Hydraulic ☐ Rope & Cable ☐							
R = Rock Core Sample S = Split Stem Sample M = unconsolidated Split Spoon Sample Attempt T = Thin Wall Tube Sample H = unconsolidated Thin Wall Tube Sample Attempt F = Field Test Results U = unconsolidated Field Test Sample				R = Rock Core Sample SA = Solid Stem Auger HSA = Hollow Stem Auger M = Hammer Core RW = Weight of 140lb. Hammer W = Weight of Rods or Casing RSP = Radius of Job Range		N = Unconsolidated Field Test Spore String (SPST) SLP = Lab use unaltered shear strength test SQ = Unconsolidated Concrete Strength Test SPT = Standard Penetration Test SPT = Unconsolidated Field Test for Nuclear Efficiency Factor SPT = Unconsolidated Field Test for Nuclear Efficiency Factor SPT = Unconsolidated Field Test for Nuclear Efficiency Factor		R = Rock Core Sample SA = Solid Stem Auger HSA = Hollow Stem Auger M = Hammer Core RW = Weight of 140lb. Hammer W = Weight of Rods or Casing RSP = Radius of Job Range					
Sample Information										Visual Description and Remarks		Laboratory Testing Results/ASHTO and Unified Class	
Depth (ft.)	Sample No.	Rev./Rec. (ft.)	Depth (ft.)	Depth (ft.)	Depth (ft.)	Depth (ft.)	Depth (ft.)	Depth (ft.)	Depth (ft.)	Elevation (ft.)	Remarks	Remarks	Remarks
-5	1D	24/24	7.00	4.6/6.7	12	18				254.6	Drill grey, moist, medium Silt, little sand.		GW80784 A=4, CL MC-20.15
-10	2D	3.6/3	10.00	251.6-251.6	---					254.6	Brown, moist, fine to coarse sand, some gravel, little silt, fill.		
			10.30	251.6-251.6									
			15.30	250.0-250.0									
-15	R2	40/40	15.30	229.6	27%					229.6	Top of Bedrock at Elev. 224.6 ft. R1: Bedrock Gray to blue-gray, very fine to medium-grained, GRANULS, moderately hard, distinct layering is strongly inclined, weathered and stained. Rock Quality = Very Poor R1 Core Times (min/sec): 10.3-15.3 (12:00) 11.3-12.3 (12:15) 12.3-13.3 (12:30) 13.3-14.3 (12:45) 14.3-15.3 (12:55) 50% Recovery R2 Bedrock similar to R1, exact weakness along foliation not as prominent and competency of rock increases with depth. Near vertical joint set noted across layering, overlain with some weathering. Rock Quality = Poor R2 Core Times (min/sec): 10.3-14.3 (12:15) 14.3-17.3 (12:35) 17.3-18.3 (12:55) 18.3-19.3 (13:10) 19.3-20.3 (13:25) 100% Recovery		
-20										224.6	Bottom of Exploration at 20.3 feet below ground surface.		
(Remarks)													
Stratification lines represent approximate boundaries between soil types; transitions may be gradual. * Water level readings have been made at times under non-steady conditions. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.													



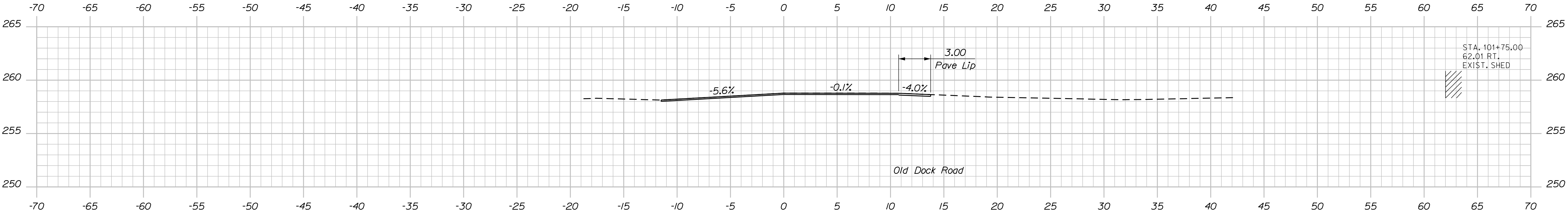
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PROJ. MANAGER BRIAN NICHOLS						
DESIGN-DETAILED		E. BREWER	MRP	Nov. 2024		
CHECKED-REVIEWED		RMM	E. BREWER	Nov. 2024	SIGNATURE	
DESIGNS-DETAILED					P.E. NUMBER	
REVISIONS 1						
REVISIONS 2						
REVISIONS 3					DATE	
REVISIONS 4						
FIELD CHANGES						
PUDDLE DOCK BRIDGE FIFTEENMILE STREAM ALBION KENNEBEC COUNTY						
TYPICAL SECTIONS						
SHEET NUMBER 2 OF 31						

Date:11/18/2024

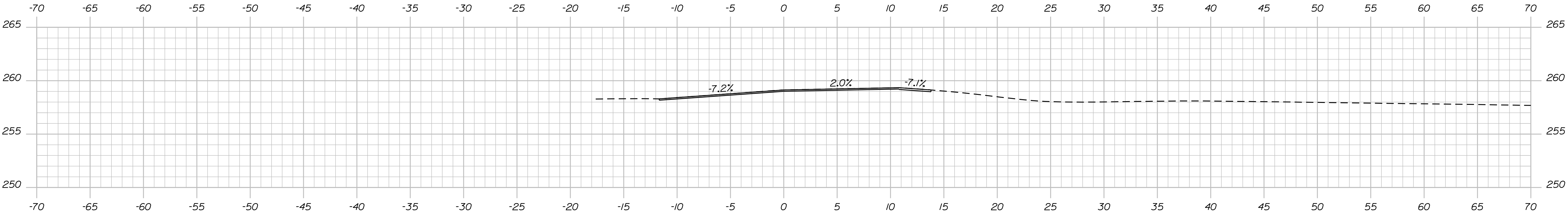
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Division: BRIDGE

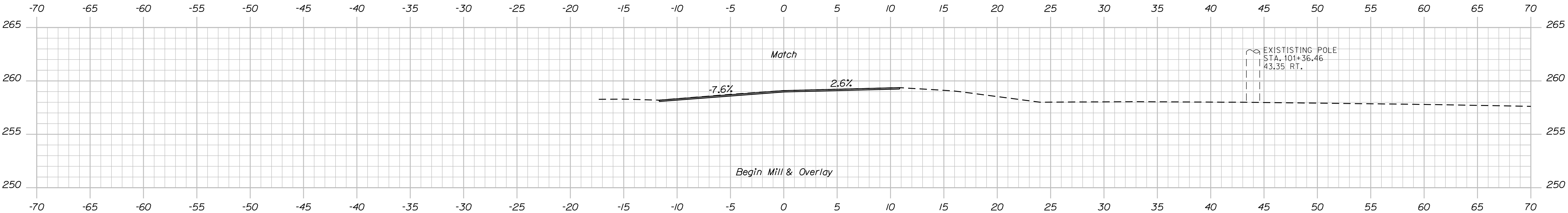
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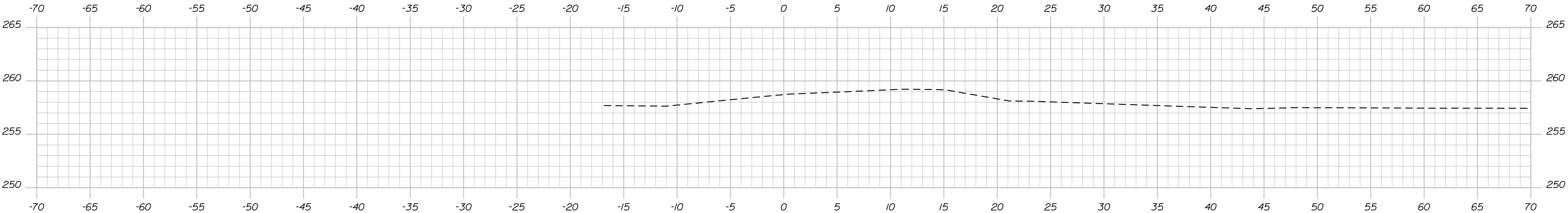
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101+50.00



101+45.00



101+25.00

STATE OF MAINE

DEPARTMENT OF TRANSPORTATION

02529900

WIN

025299.00

BRIDGE NO. 3107

BRIDGE PLANS

PROJ. MANAGER	BRIAN NICHOLS	BY	DATE
DESIGN-DETAILED	E. BREWER	MP	Nov. 2024
CHECKED-REVIEWED	RUN	E. BREWER	Nov. 2024
DESIGN-DETAILED			SIGNATURE
REVISIONS 1			P.E. NUMBER
REVISIONS 2			DATE
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			

PUDDLE DOCK BRIDGE

FIFTEENMILE STREAM

ALBION

KENNEBEC COUNTY

CROSS SECTIONS

SHEET NUMBER

8

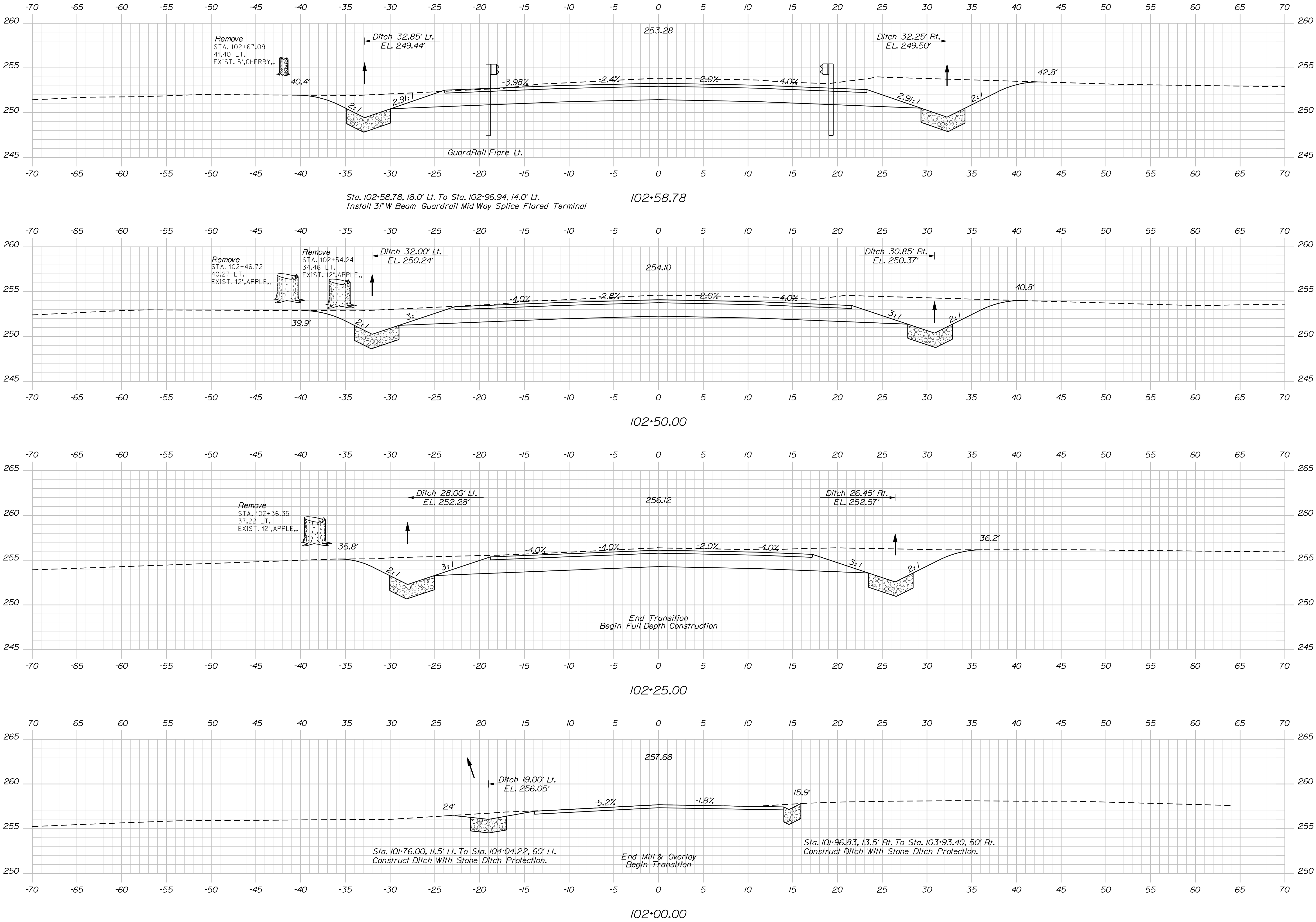
OF 31

Date:11/18/2024

Username: Brian.Nichols

Division: BRIDGE

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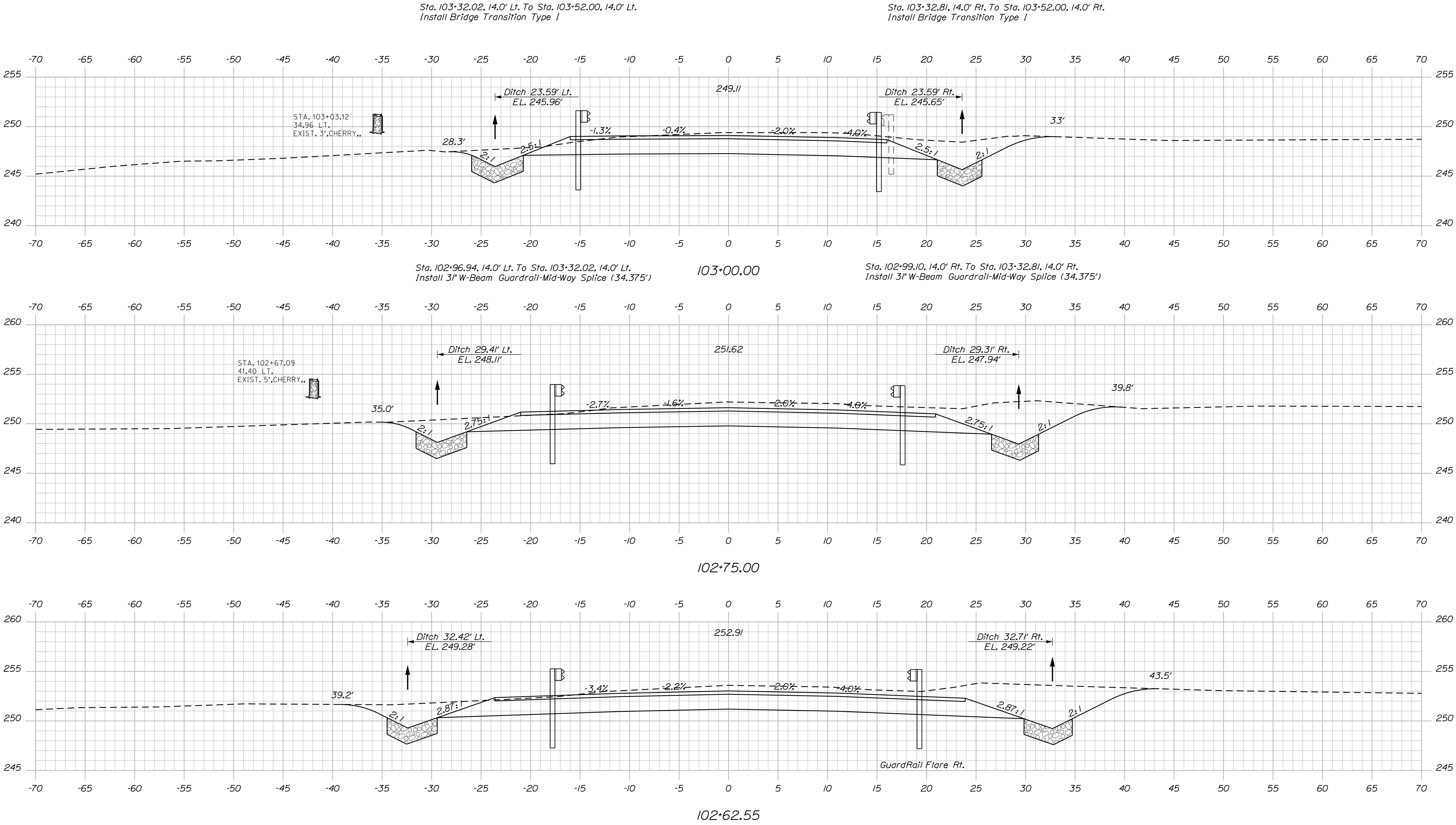
PROJ. MANAGER	BY	DATE	SIGNATURE	P.E. NUMBER	DATE
BRIAN NICHOLS	MRP	Nov. 2024			
DESIGN-DETAILED	E. BREWER	Nov. 2024			
CHECKED-REVIEWED	E. BREWER	Nov. 2024			
DESIGN-DETAILED					
REVISIONS 1					
REVISIONS 2					
REVISIONS 3					
REVISIONS 4					
FIELD CHANGES					

Date:11/18/2024

Username: Brian.Nichols

Division: BRIDGE

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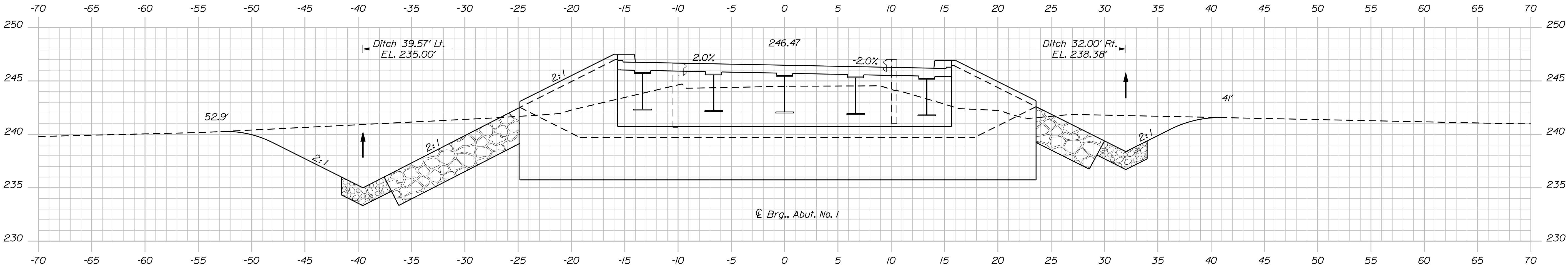
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PUDDLE DOCK BRIDGE		FIFTEENMILE STREAM		KENNEBEC COUNTY		ALBION		CROSS SECTIONS		SHEET NUMBER		10	
PROJ. MANAGER		BRIAN NICHOLS		DATE		BY		SIGNATURE		P.E. NUMBER		DATE	
DESIGNED-DETAILED		E. BREWER		Nov. 2024		MRP		Nov. 2024					
CHECKED-REVIEWED		E. BREWER				E. BREWER							
DESIGNS-DETAILED													
DESIGNS-DETAILED													
REVISIONS 1													
REVISIONS 2													
REVISIONS 3													
REVISIONS 4													
FIELD CHANGES													

Date:11/18/2024

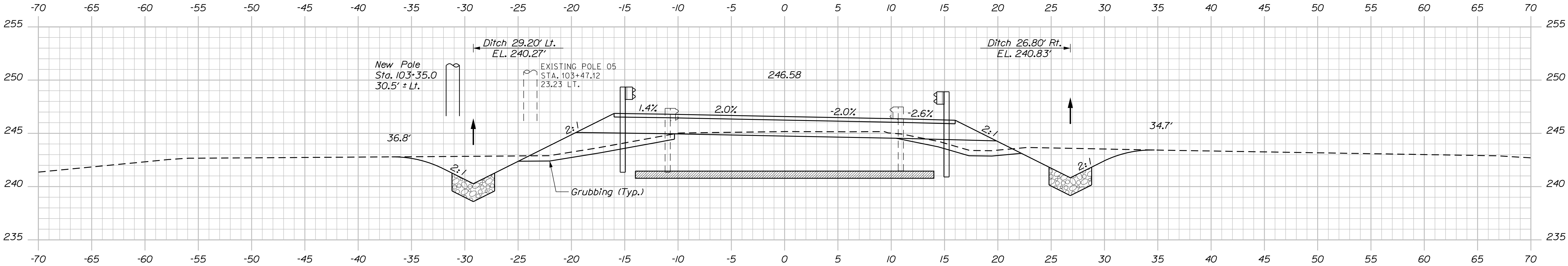
Username: Brian.Nichols

Division: BRIDGE

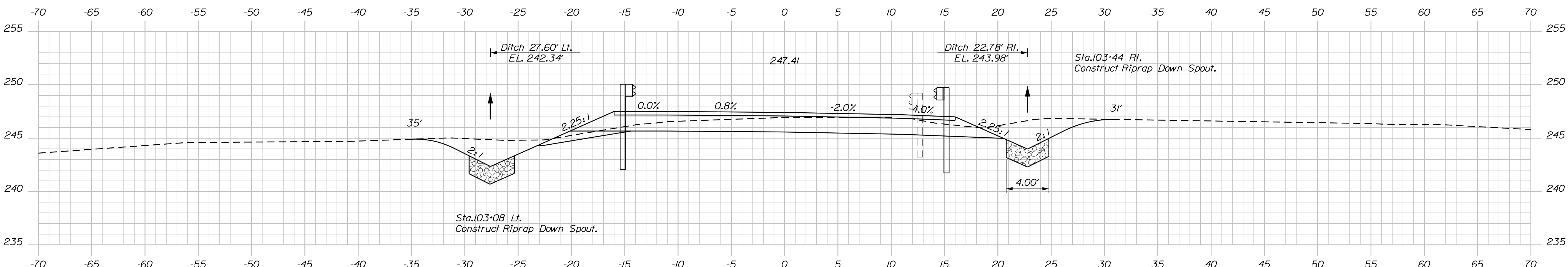
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103+63.00



103+50.00



103+25.00

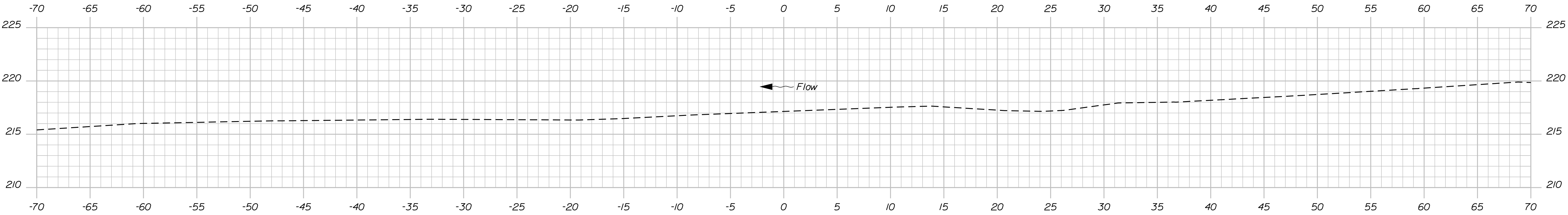
PROJ. MANAGER	DATE	BY	DATE	SIGNATURE	P.E. NUMBER	DATE
BRIAN NICHOLS	Nov. 2024	E. BREWER	Nov. 2024			
CHECKED-REVIEWED	MP	E. BREWER	Nov. 2024			
DESIGNED-DETAILED	DESIGNED-DETAILED					
REVISIONS 1						
REVISIONS 2						
REVISIONS 3						
REVISIONS 4						
FIELD CHANGES						

Date:11/18/2024

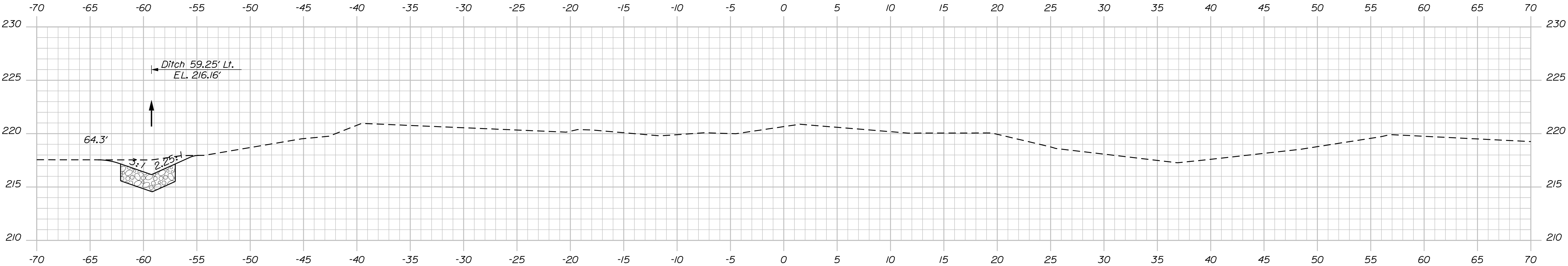
Username: Brian.Nichols

Division: BRIDGE

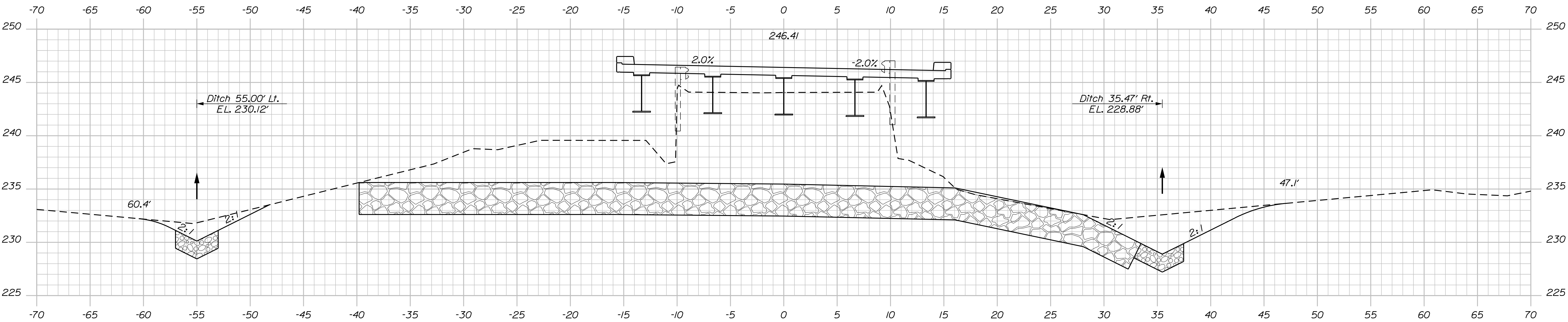
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104+25.00



104+00.00



103+75.00

STATE OF MAINE DEPARTMENT OF TRANSPORTATION		
	02529900	
	WIN	025299.00
BRIDGE NO. 3107		
BRIDGE PLANS		

PROJ. MANAGER	BY	DATE
BRIAN NICHOLS	MRP	Nov. 2024
E. BREWER	E. BREWER	Nov. 2024
DESIGNED-DETAILED		
CHECKED-REVIEWED		
DESIGN-DETAILED		
REVISIONS 1		
REVISIONS 2		
REVISIONS 3		
REVISIONS 4		
FIELD CHANGES		

PUDDLE DOCK BRIDGE FIFTEENMILE STREAM KENNEBEC COUNTY	
ALBION	

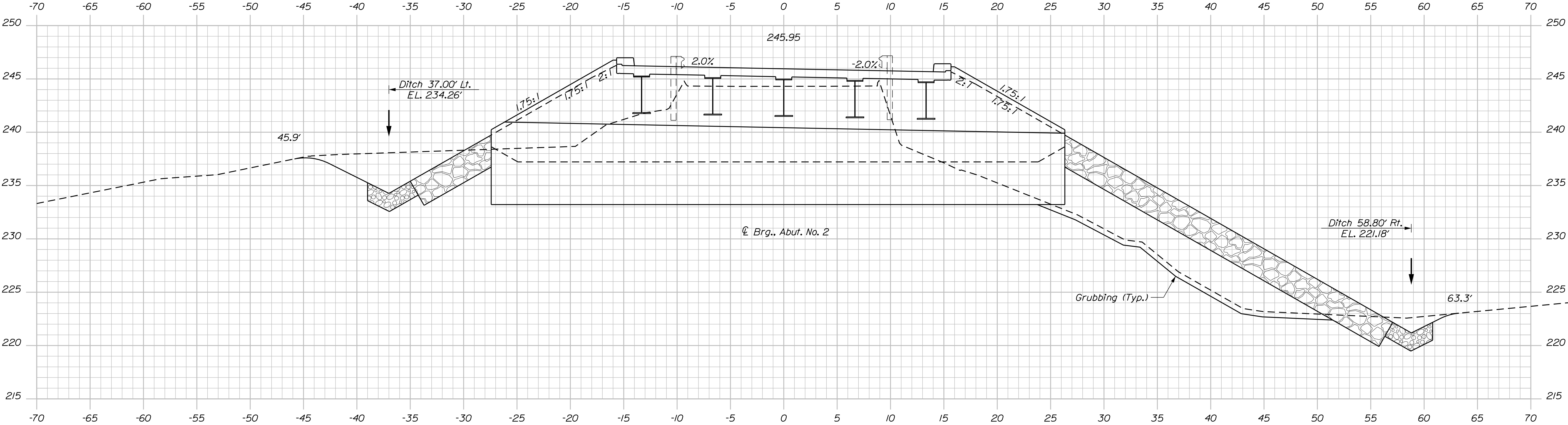
SHEET NUMBER
12
OF 31

Date:11/18/2024

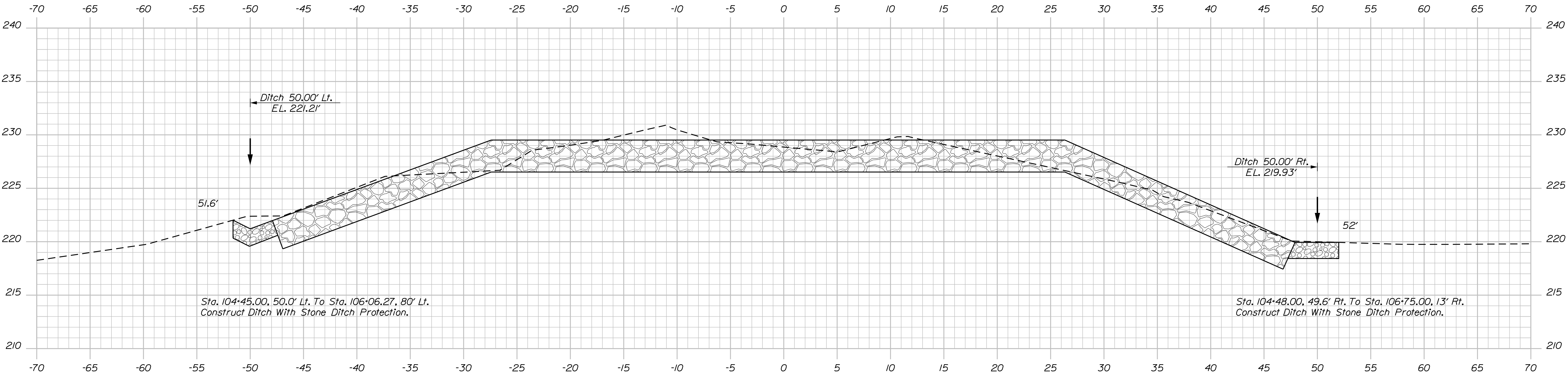
Username: Brian.Nichols

Division: BRIDGE

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104+67.00



104+50.00

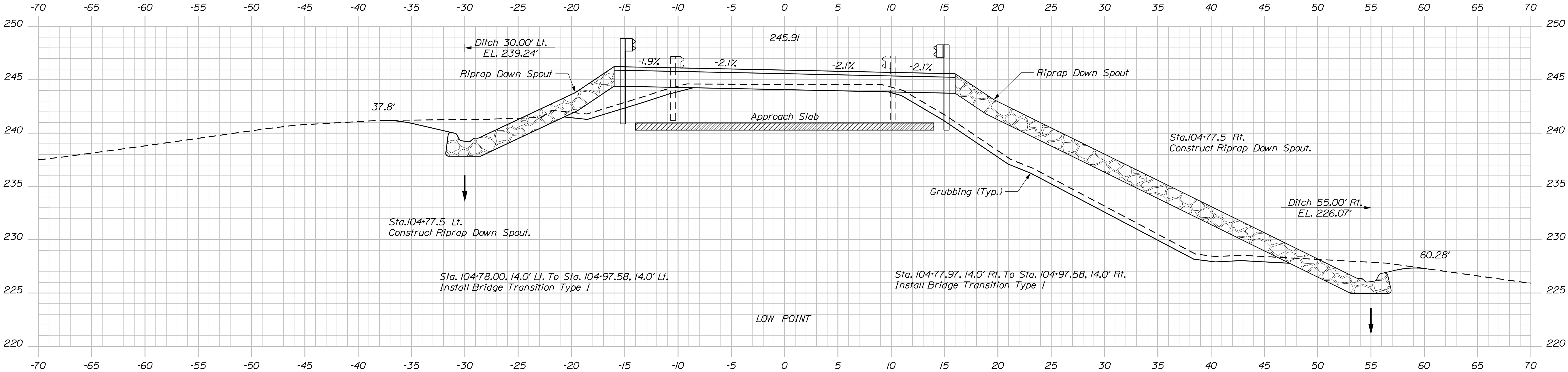
STATE OF MAINE		DEPARTMENT OF TRANSPORTATION		02529900		WIN		025299.00		BRIDGE NO. 3107		BRIDGE PLANS	
PUDDLE DOCK BRIDGE		FIFTEENMILE STREAM		KENNEBEC COUNTY		ALBION		CROSS SECTIONS		SHEET NUMBER		13	
PROJ. MANAGER		BRIAN NICHOLS		DATE		NOV. 2024		SIGNATURE		P.E. NUMBER		DATE	
DESIGNED-DETAILED		E. BREWER		NOV. 2024		NOV. 2024		SIGNATURE		P.E. NUMBER		DATE	
CHECKED-REVIEWED		E. BREWER		NOV. 2024		NOV. 2024		SIGNATURE		P.E. NUMBER		DATE	
DESIGNED-DETAILED		E. BREWER		NOV. 2024		NOV. 2024		SIGNATURE		P.E. NUMBER		DATE	
REVISIONS 1													
REVISIONS 2													
REVISIONS 3													
REVISIONS 4													
FIELD CHANGES													

Date:11/18/2024

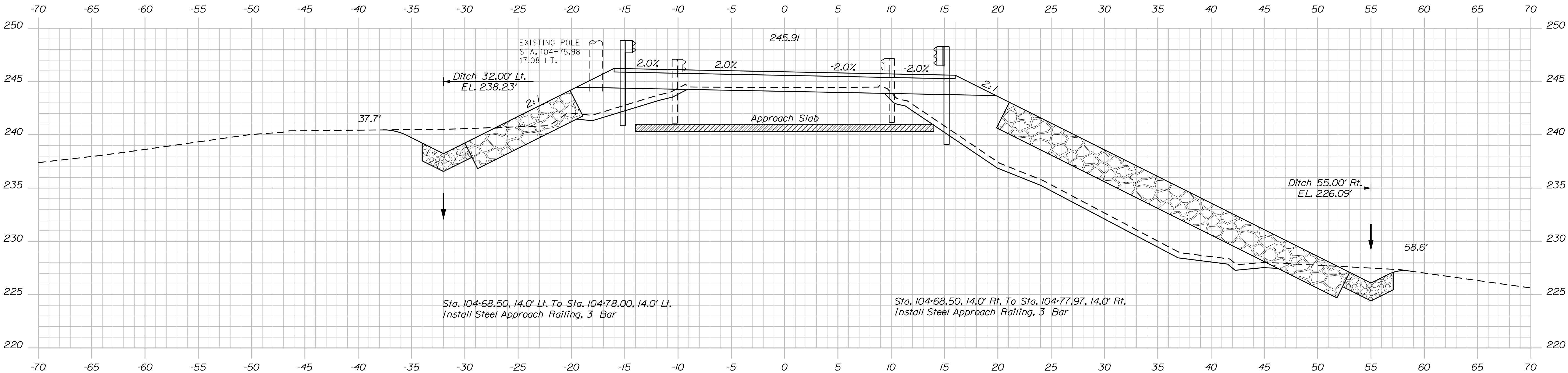
Username: Brian.Nichols

Division: BRIDGE

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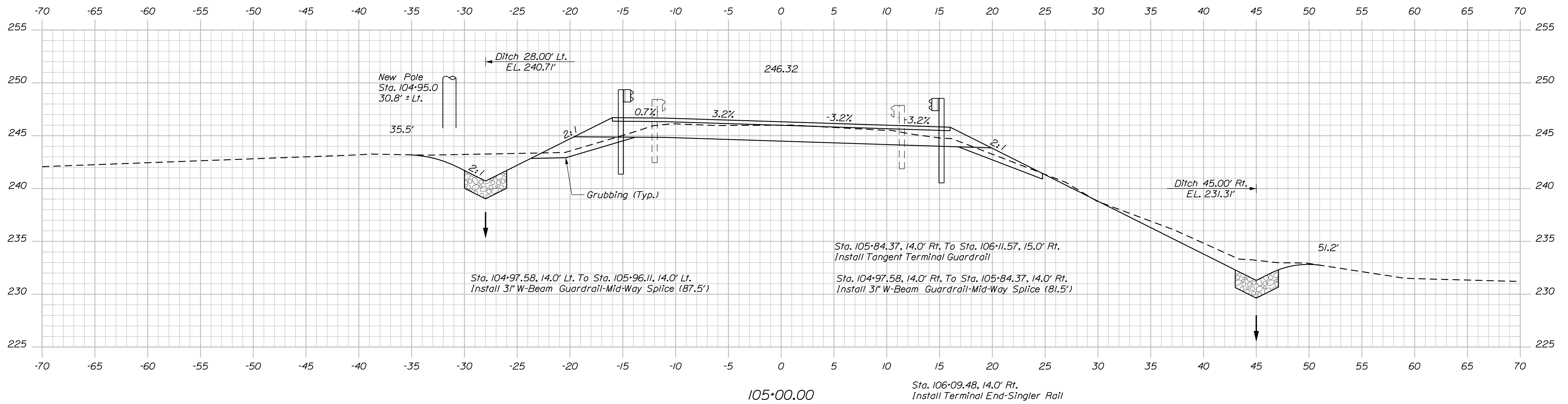
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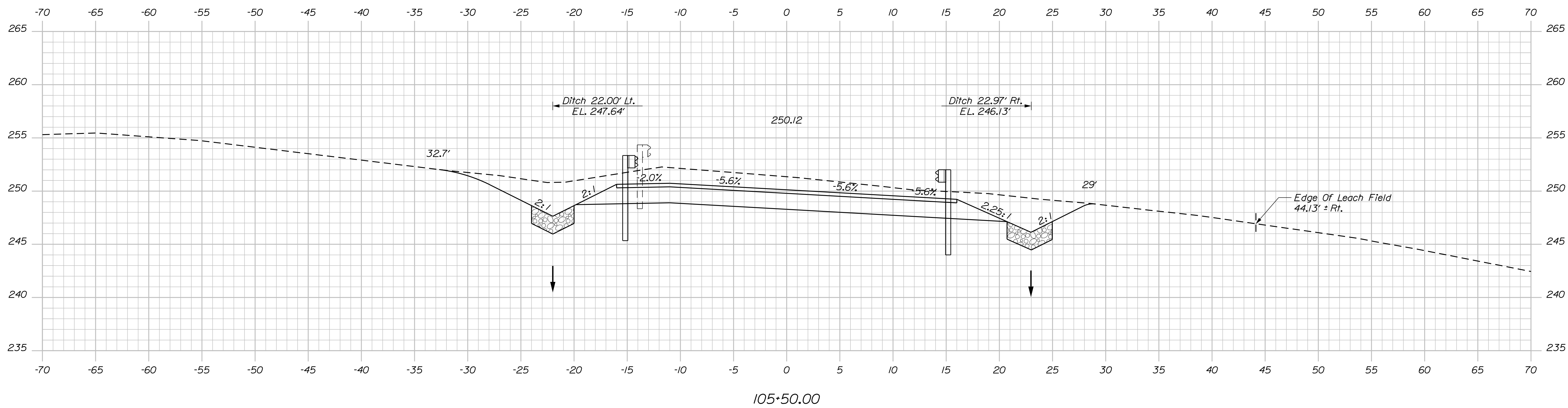
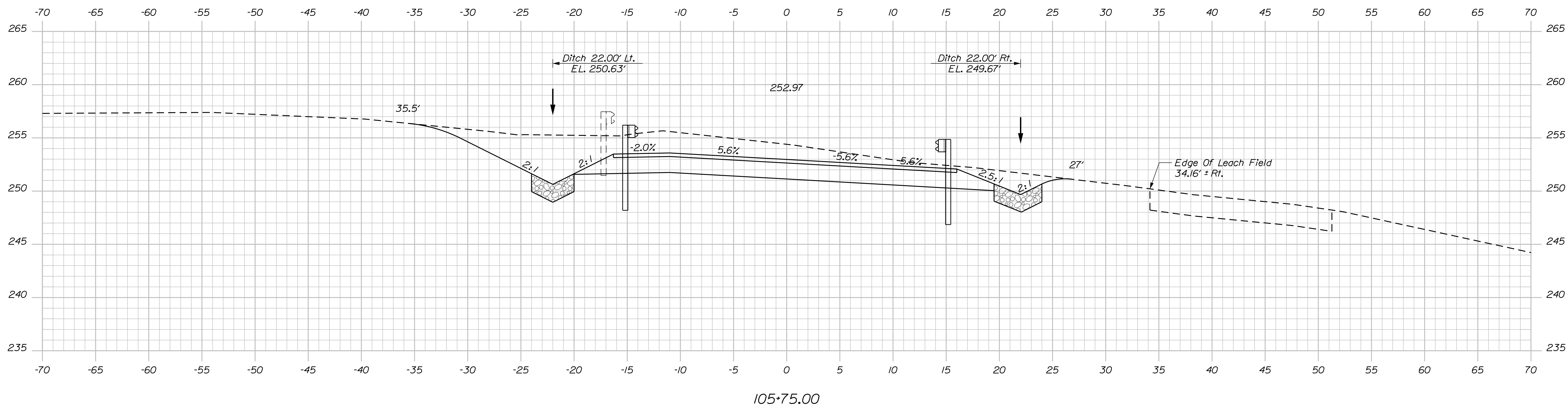
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STATE OF MAINE		DEPARTMENT OF TRANSPORTATION		02529900		WIN		025299.00		BRIDGE NO. 3107		BRIDGE PLANS	
PUDDLE DOCK BRIDGE		FIFTEENMILE STREAM		KENNEBEC COUNTY		ALBION		CROSS SECTIONS		SHEET NUMBER		14	
PROJ. MANAGER		BRIAN NICHOLS		BY		DATE		SIGNATURE		P.E. NUMBER		DATE	
DESIGNED-DETAILED		E. BREWER		MRP		Nov. 2024		Nov. 2024		Nov. 2024		Nov. 2024	
CHECKED-REVIEWED		E. BREWER		E. BREWER		Nov. 2024		Nov. 2024		Nov. 2024		Nov. 2024	
DESIGNS-DETAILED		E. BREWER		E. BREWER		Nov. 2024		Nov. 2024		Nov. 2024		Nov. 2024	
REVISIONS 1													
REVISIONS 2													
REVISIONS 3													
REVISIONS 4													
FIELD CHANGES													

Filename: ... \MSTA\015_XSECT_105+00_008.dgn Division: BRIDGE



Sta. 105+00.00 to Sta. 105+25.00

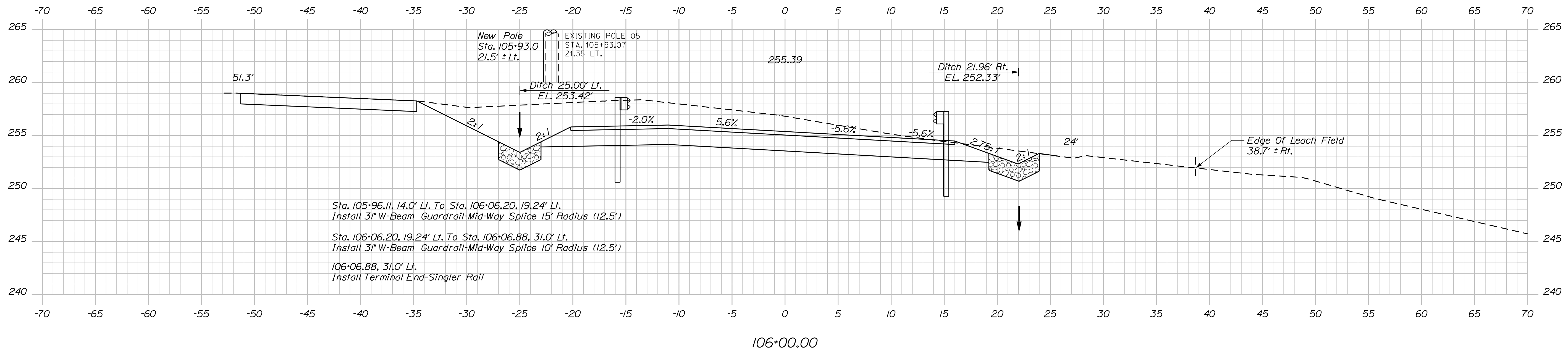


STATE OF MAINE	
DEPARTMENT OF TRANSPORTATION	
02529900	
BRIDGE NO. 3107	WIN 025299.00
BRIDGE PLANS	

PROJ. MANAGER		BRIAN NICHOLS	BY	DATE
DESIGN-DETAILED	E. BREWER	WRP		Nov. 2024
CHECKED-REVIEWED	RWN	E. BREWER		Nov. 2024
DESIGN2-DETAILED2				
DESIGN3-DETAILED3				
REVISIONS 1				P.E. NUMBER
REVISIONS 2				
REVISIONS 3				
REVISIONS 4				DATE
FIELD CHANGES				

ALBION	PUDDLE DOCK BRIDGE FIFTEENMILE STREAM KENNEBEC COUNTY
CROSS SECTIONS	

SHEET NUMBER
16
OF 31



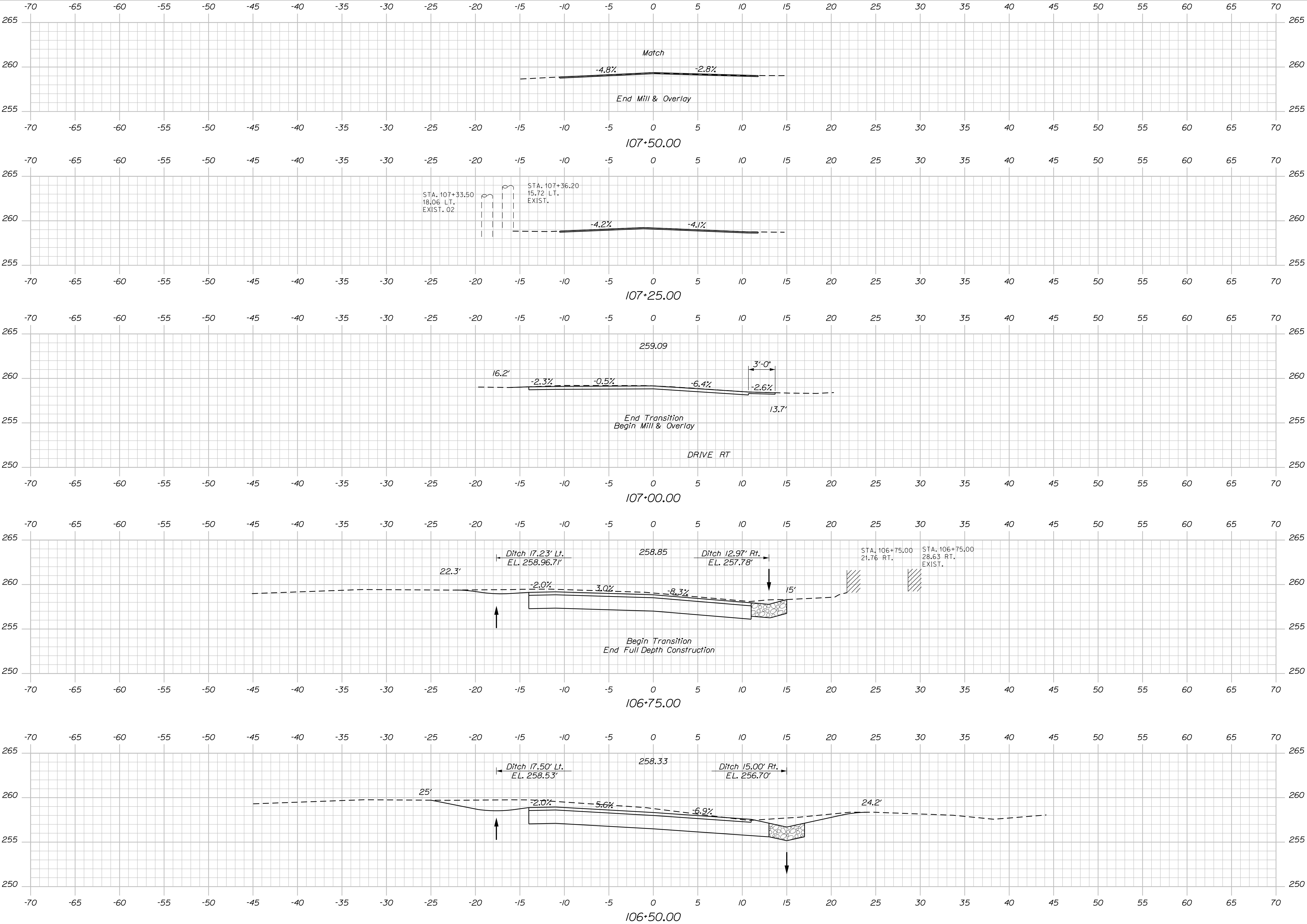
17 OF 31	SHEET NUMBER	ALBION Kennebec County CROSS SECTIONS	PUDDLE DOCK BRIDGE	PROJ. MANAGER	BRIAN NICHOLS	BY	DATE
			FIFTEENMILE STREAM	DESIGN-DETAILED	E. BREWER	MRP	Nov. 2024
				CHECKED-REVIEWED	RWN	E. BREWER	Nov. 2024
				DESIGN-DETAILED2			
				DESIGN-DETAILED3			
			REVISIONS 1				
			REVISIONS 2				
			REVISIONS 3				
			REVISIONS 4				
			FIELD CHANGES				
			STATE OF MAINE DEPARTMENT OF TRANSPORTATION				
			02529900				
			BRIDGE NO. 3107 WIN 025299.00 BRIDGE PLANS				

Date:11/18/2024

Username: Brian.Nichols

Division: BRIDGE

Filename: ... \MSTA018_XSECT_106+50_011.dgn



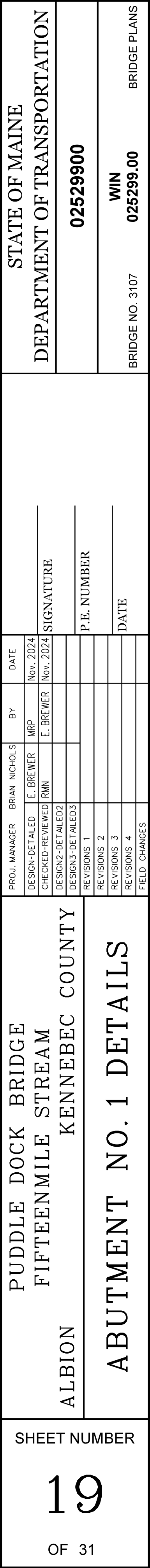
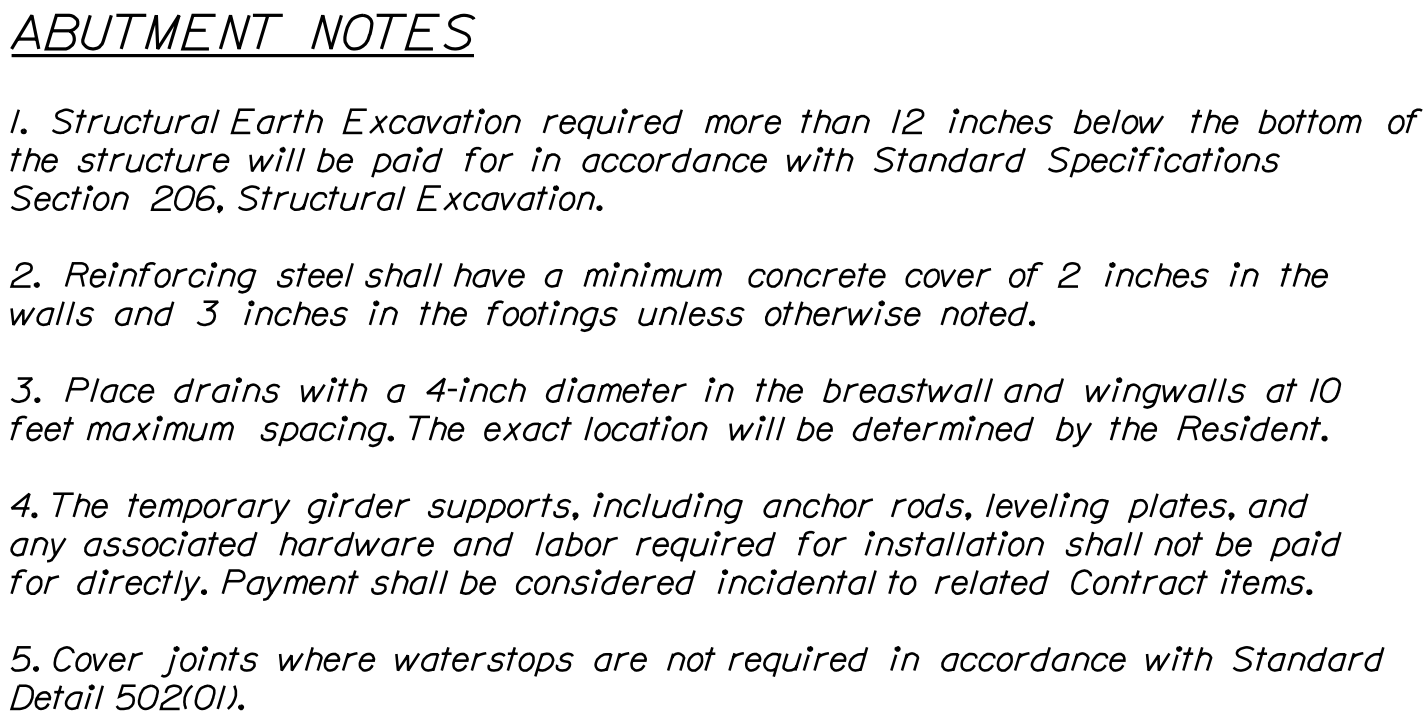
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
02529900
WIN
025299.00
BRIDGE NO. 3107
BRIDGE PLANS

PUDDLE DOCK BRIDGE
FIFTEENMILE STREAM
ALBION
KENNEBEC COUNTY
CROSS SECTIONS

SHEET NUMBER
18
OF 31

PROJ. MANAGER	BRIAN NICHOLS	BY	DATE
DESIGN-DETAILED	E. BREWER	MRP	Nov. 2024
CHECKED-REVIEWED	RUN	E. BREWER	Nov. 2024
DESIGN-DETAILED			
REVISIONS 1			
REVISIONS 2			
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			

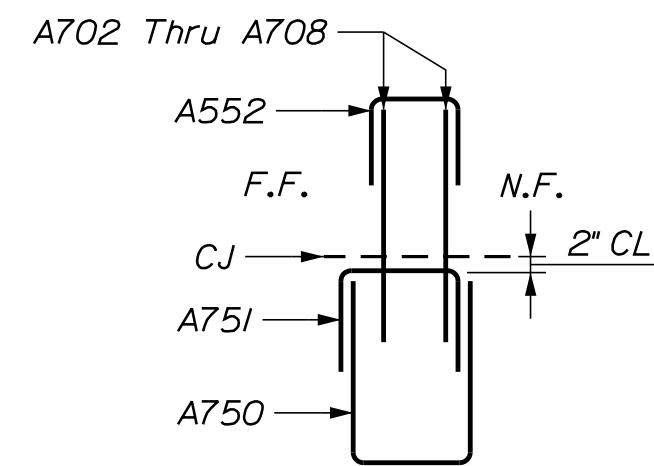
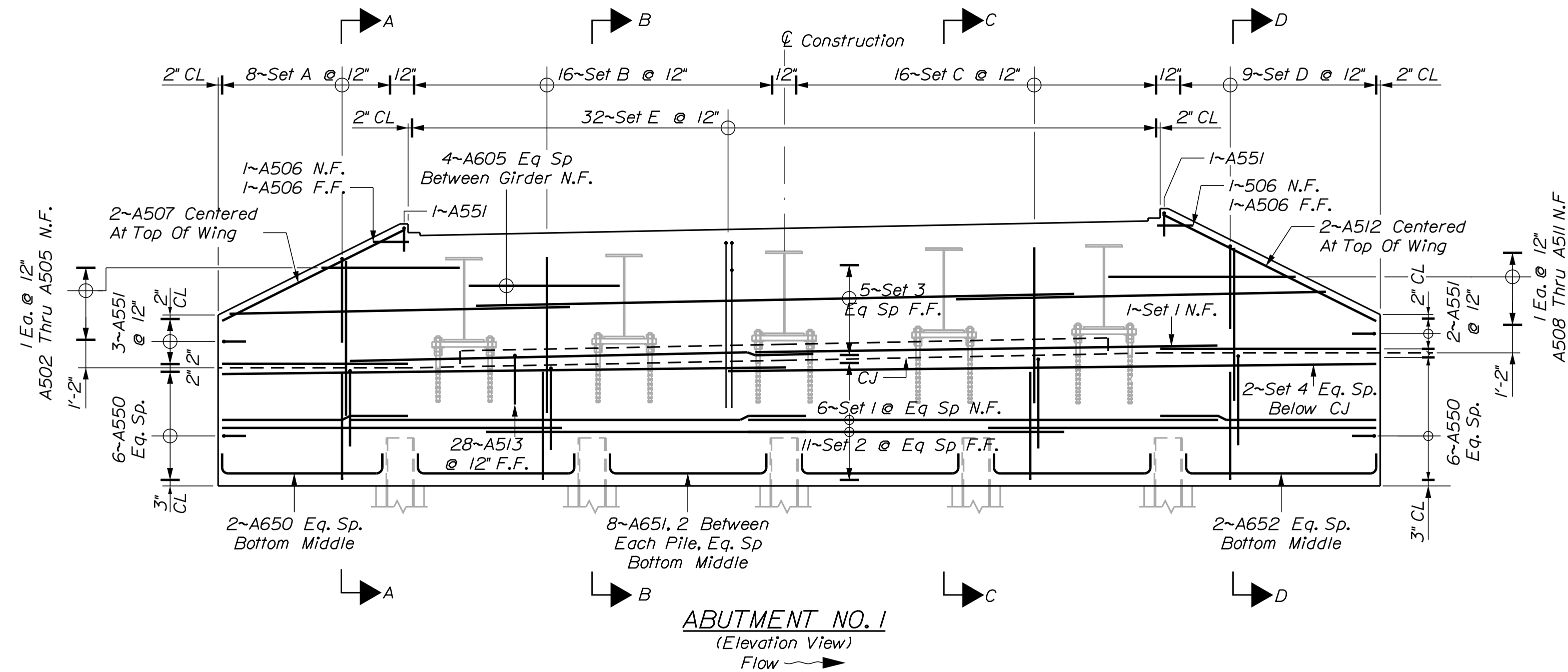
SIGNATURE
P.E. NUMBER
DATE



N.F. = Near Face
F.F. = Far Face
E.F. = Each Face
CL = Clear
CJ = Construction Joint
Eq. Sp. = Equally Space

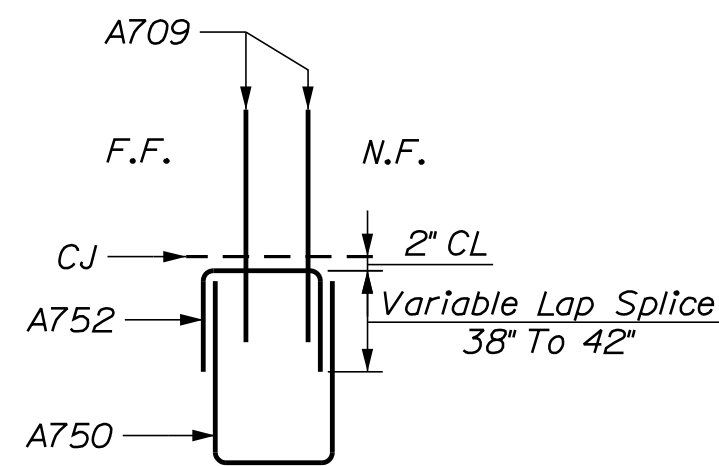
*When Two Different Size Rebar Lap Each Other
Use The Bigger Bar Size For Lap Splice.*

Rebar = (Black) Plain Steel, Other wise Mark.



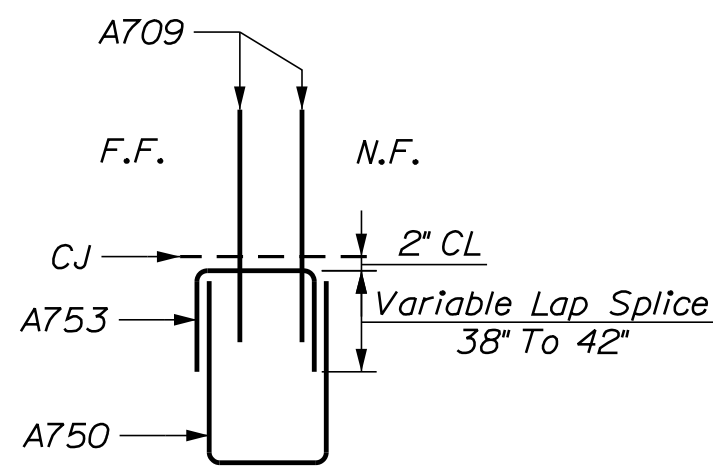
*Trim A552 If Needed
To Construction Joint*

When A750 Interferes With Pile
Replace It With 2~A719
1 N.F. & 1 F.F.



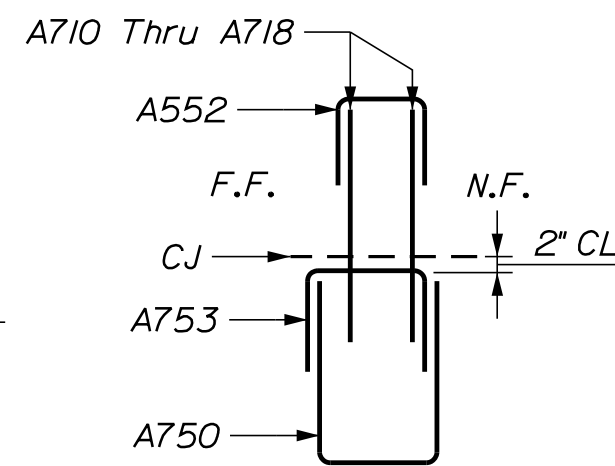
*Do Not Install A709 N.F. When It
Interferes With Bottom Of Girder.*

When A750 Interferes With Pile
Replace It With 2~A719
1 N.F. & 1 F.F.



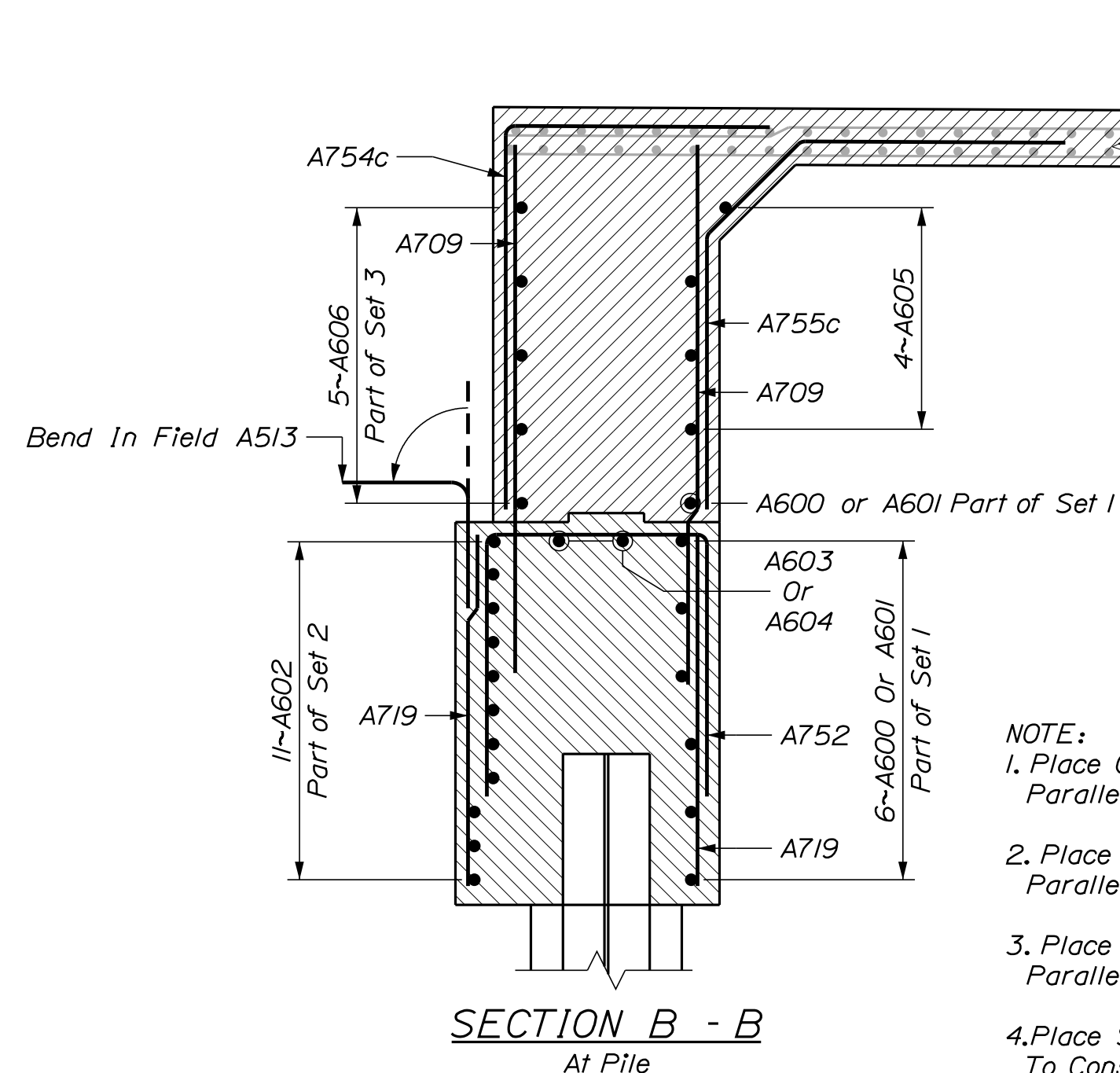
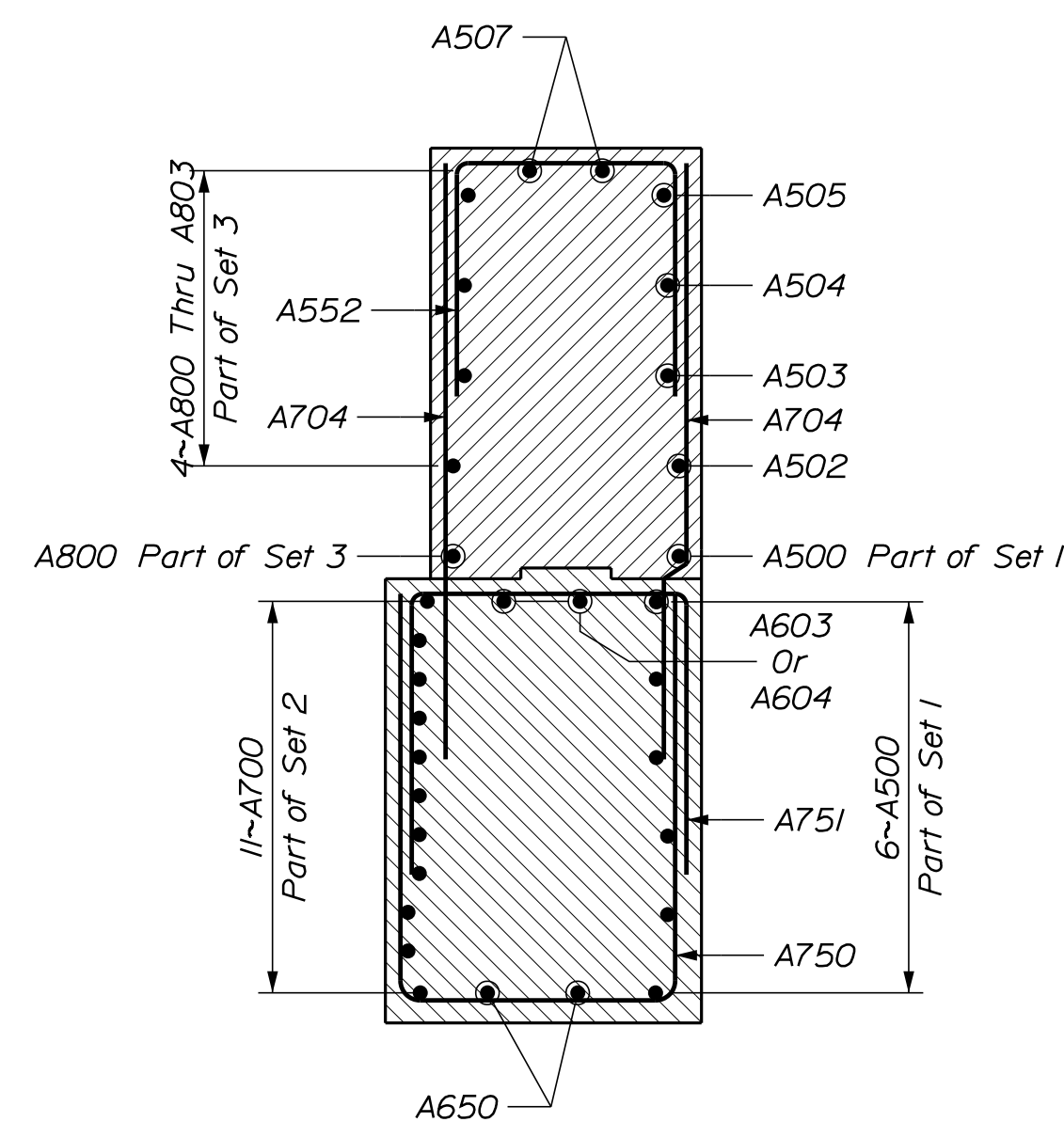
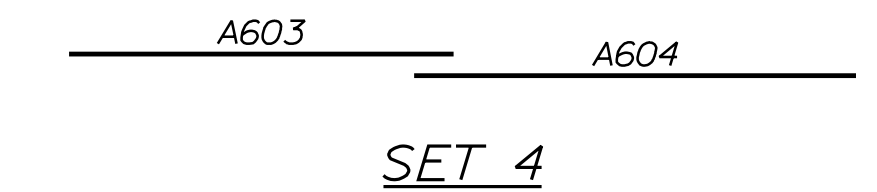
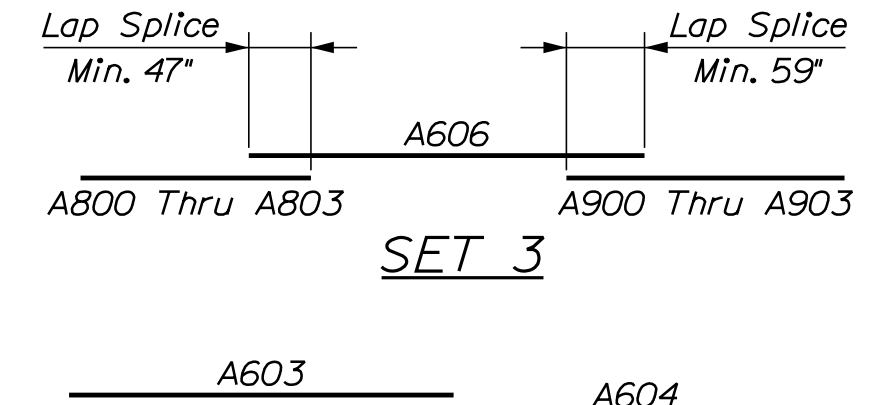
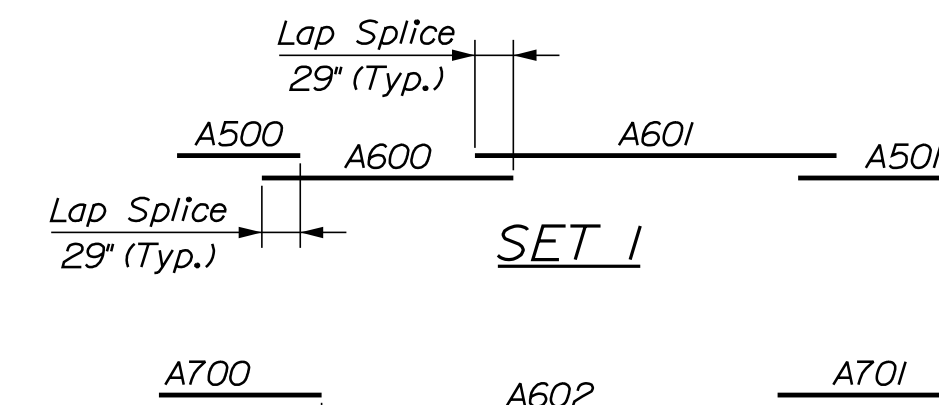
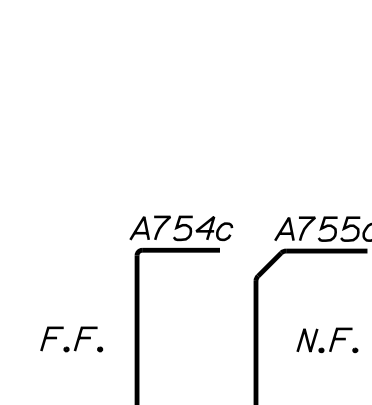
*Do Not Install A709 N.F. When It
Interferes With Bottom Of Girder.*

When A750 Interferes With Pile
Replace It With 2~A719
1 N.F. & 1 F.F.



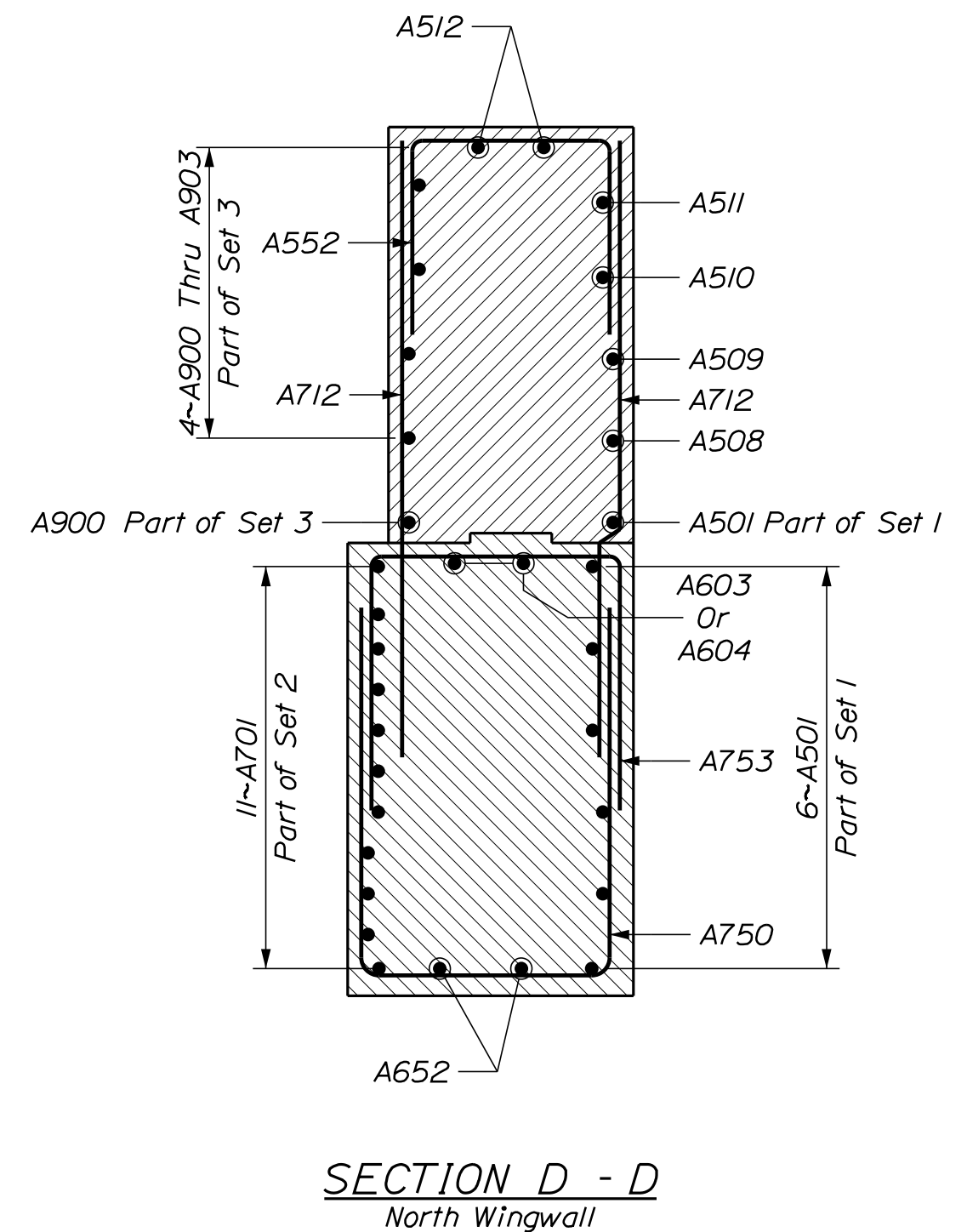
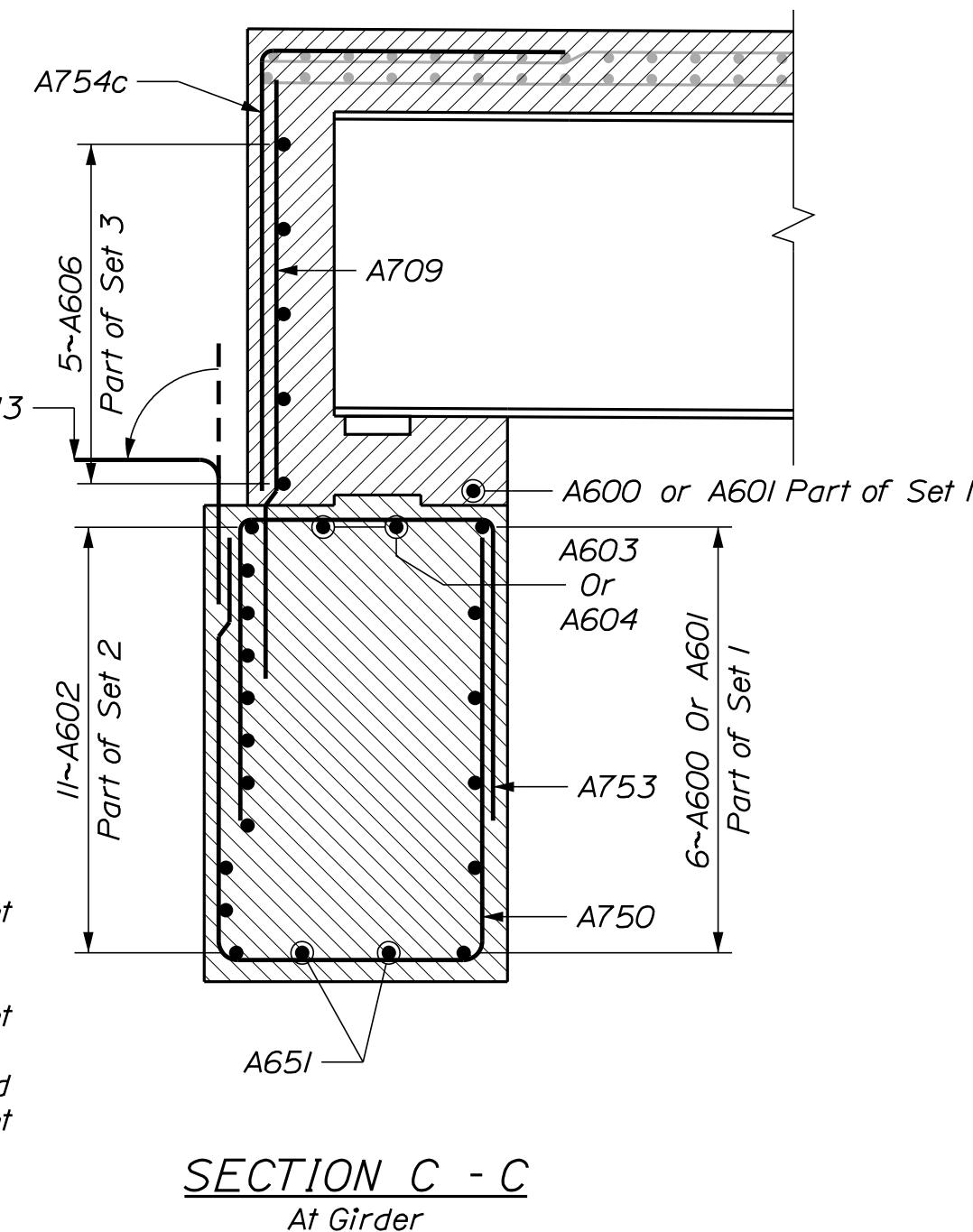
Trim A552 If Needed
To Construction Joint

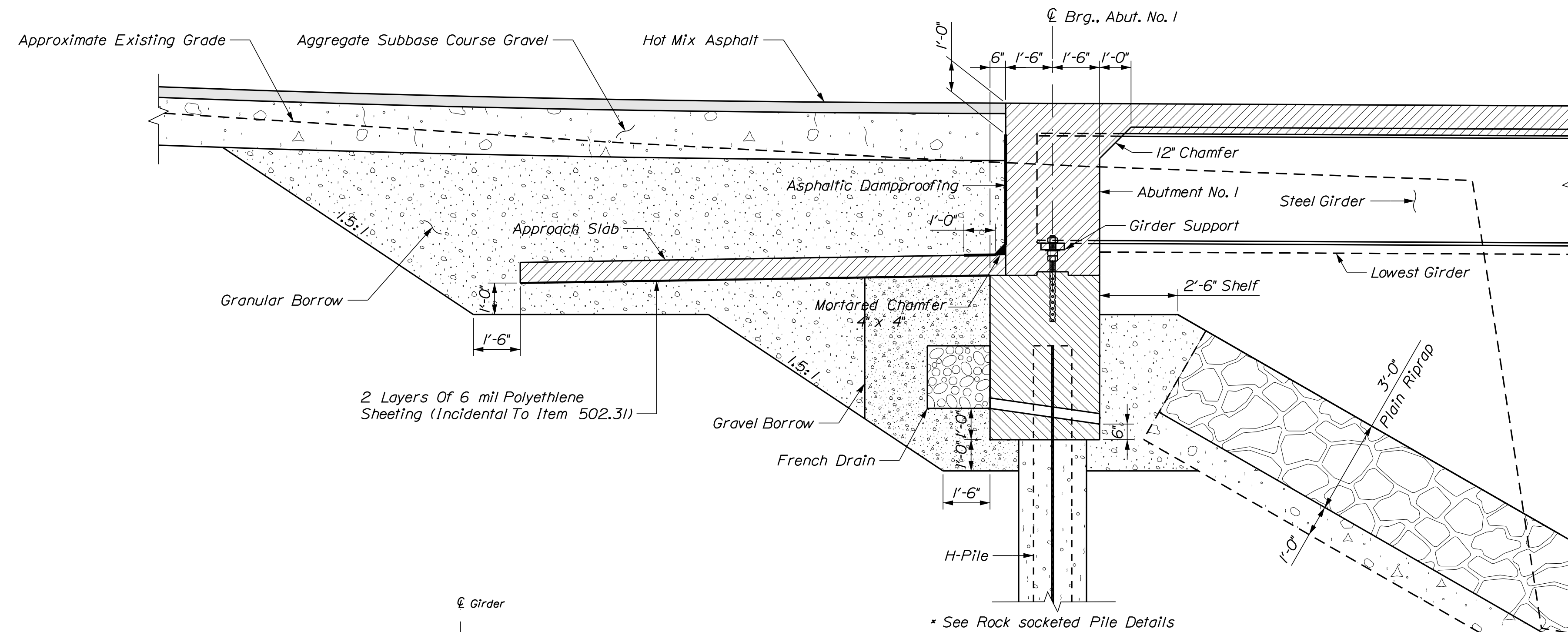
When A750 Interferes With Pile
Replace It With 2~A719
1 N.F. & 1 F.F.



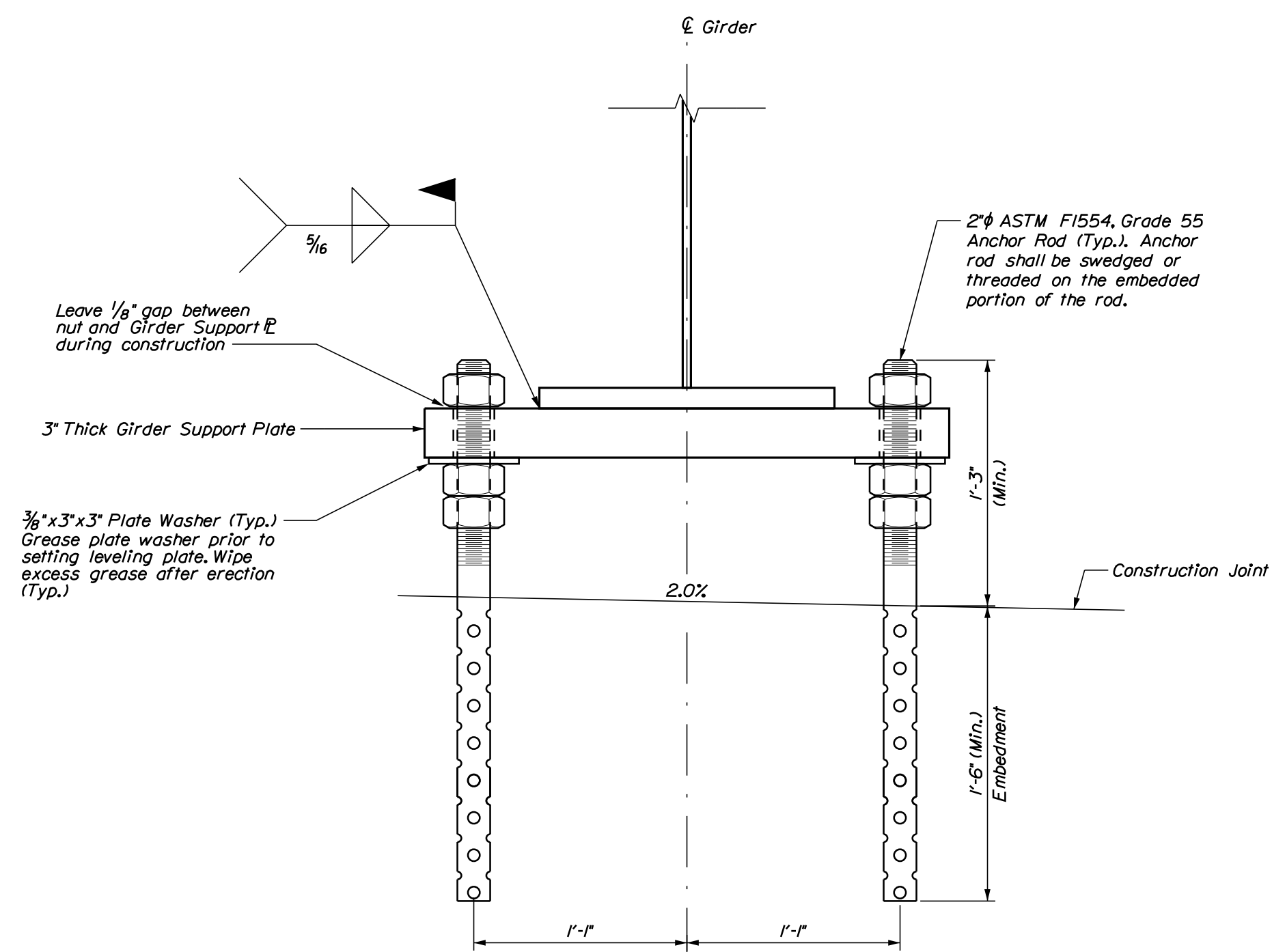
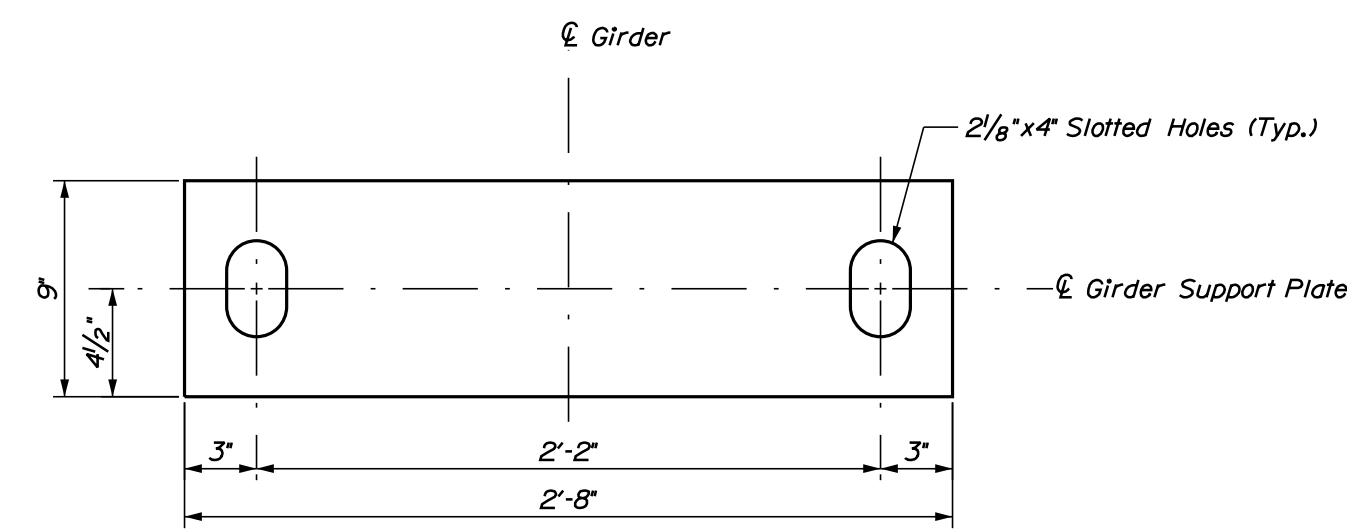
NOTE:

1. Place One Set I Under And Parallel To Construction Joint
2. Place One Set I Above And Parallel To Construction Joint
3. Place One Set 2 Above And Parallel To Construction Joint
4. Place Set 3's Parallel To Construction Joint





* See Rock socketed Pile Details



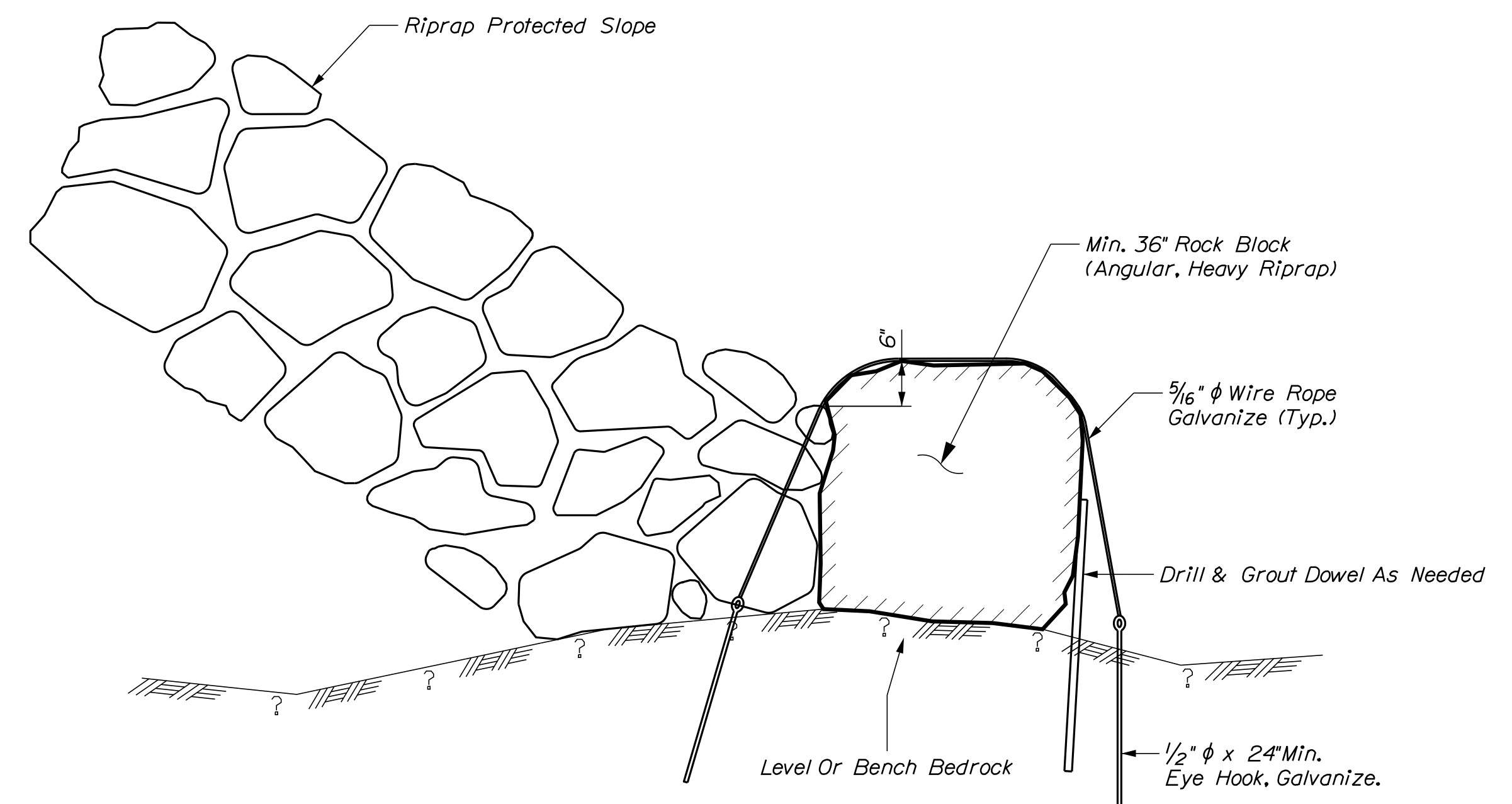
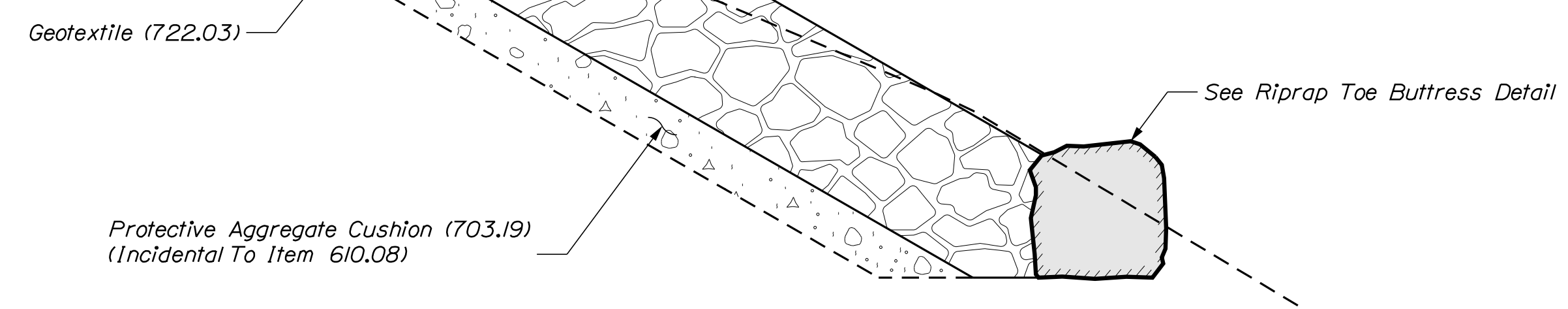
ABUTMENT NO.1 BOTTOM OF GIRDER ELEVATIONS	
Girder	Elevation
G1	242.26
G2	242.13
G3	242.00
G4	241.86
G5	241.73

GIRDER SUPPORT NOTES

1. Payment for fabrication and installation of the girder support assemblies at the abutments will be considered incidental to related contract items.
2. Grease plate washer prior to setting bearing plate. Wipe excess grease after erection.

NOTES:

- 1. Payment for mortared chamfer at approach slabs shall not be paid for directly, but shall be considered incidental to related Contract Items.*
- 2. Asphalt Dampproofing shall meet the requirements of either ASTM D497 Type II, ASTM D1227 Type II-Class I, or ASTM D1227 Type III-Class I. The product shall be applied in accordance with the manufacturer's recommendations.*
- 3. Asphalt Dampproofing shall be applied to the backside of the wingwalls up to 1 foot below grade.*
- 4. Payment for Asphalt Dampproofing will not be made directly, but will be considered incidental to related Contract Items.*



TOE BUTTRESS DETAIL

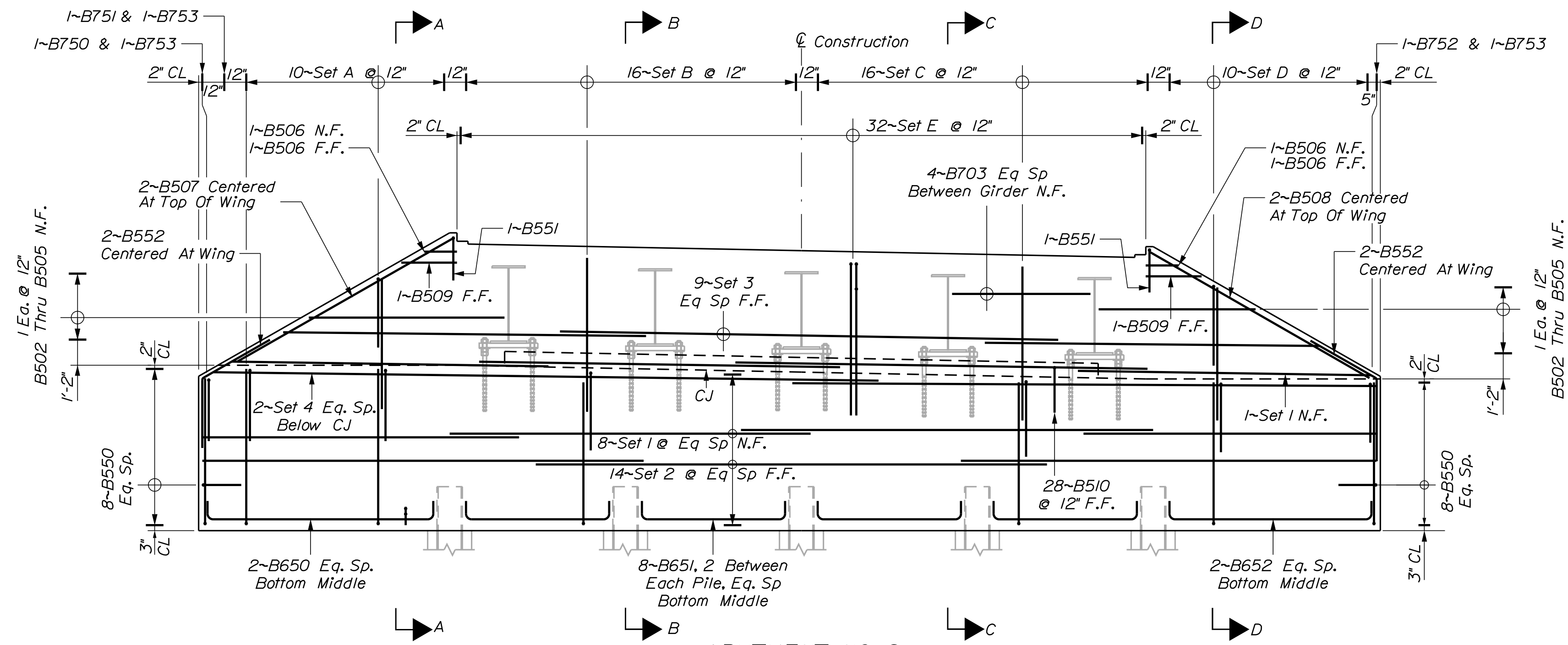
(To Be Constructed In Accordance With Special Provision 513)

NOTE:

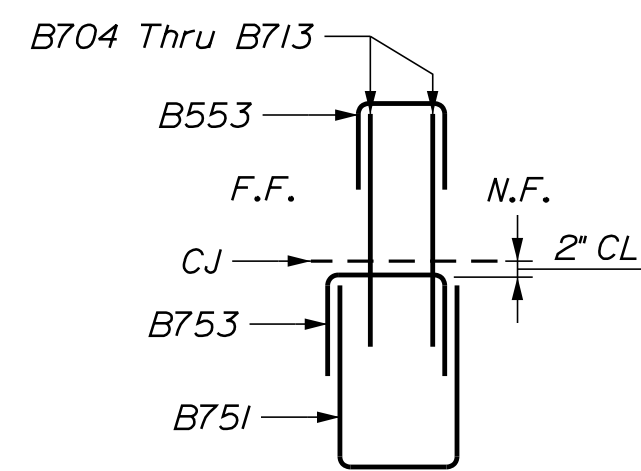
N.F. = Near Face
F.F. = Far Face
E.F. = Each Face
CL = Clear
CJ = Construction Joint
Eq. Sp. = Equally Space

When Two Different Size Rebar Lap Each Other
Use The Bigger Bar Size For Lap Splice.

Rebar = (Black) Plain Steel, Other wise Mark.



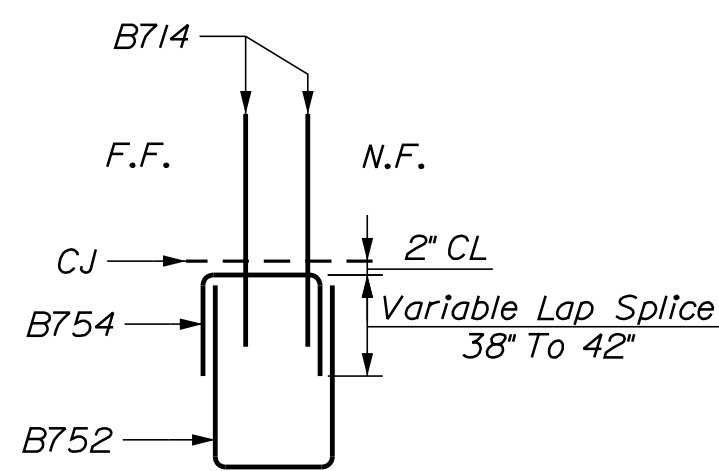
ABUTMENT NO. 2
(Elevation View)



SET A

*Trim B553 If Needed
To Construction Joint*

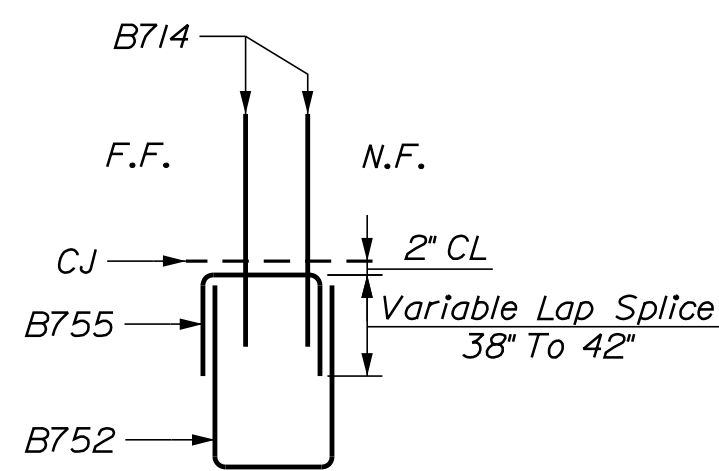
When B75I Interferes With Pile
Replace It With 2~B725
1 N.F. & 1 F.F.



SET B

*Do Not Install B714 N.F. When It
Interferes With Bottom Of Girder.*

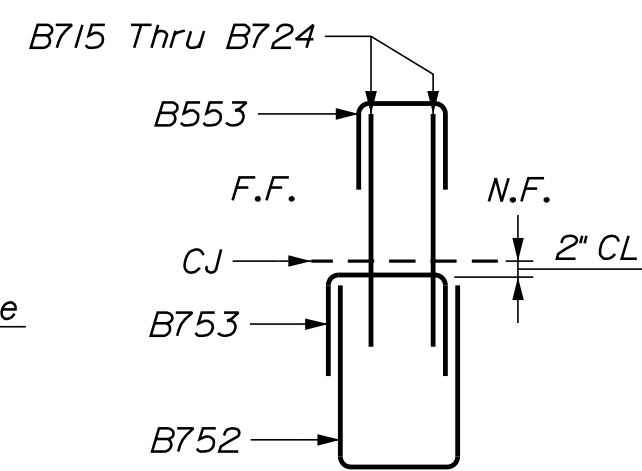
When B752 Interferes With Pile
Replace It With 2~B726
1 N.F. & 1 F.F.



SET C

Do Not Install B714 N.F. When It Interferes With Bottom Of Girder.

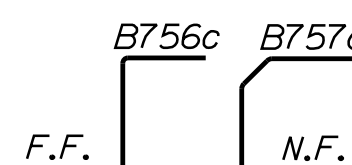
When B752 Interferes With Pile
Replace It With 2~B726
1 N.F. & 1 F.F.



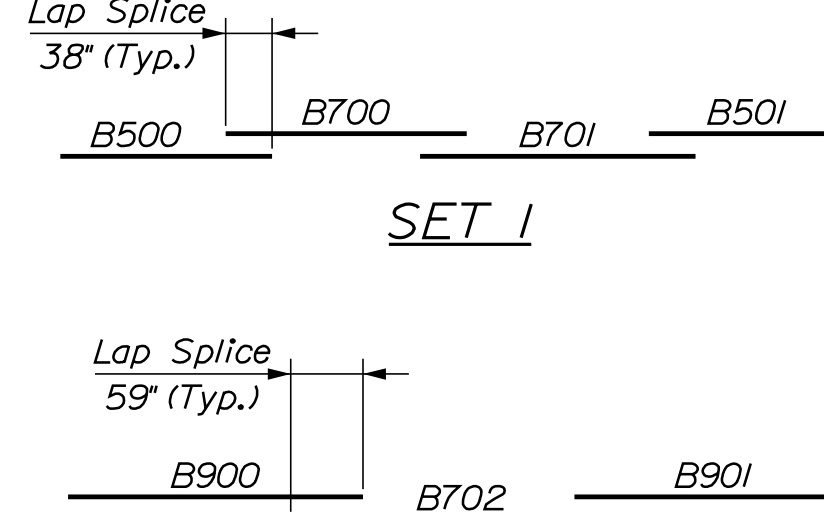
SET D

Trim B553 If Needed
To Construction Joint

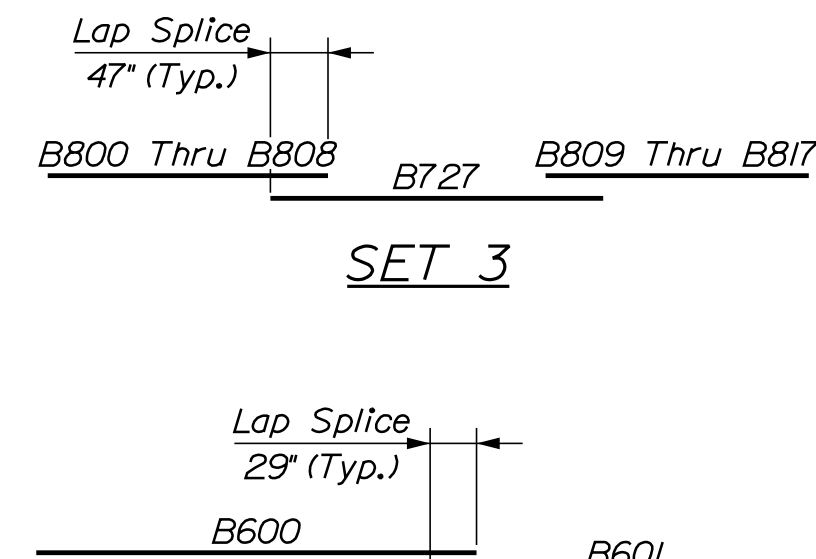
When B752 Interferes With Pile
Replace It With 2~B726
1 N.F. & 1 F.F



SET E

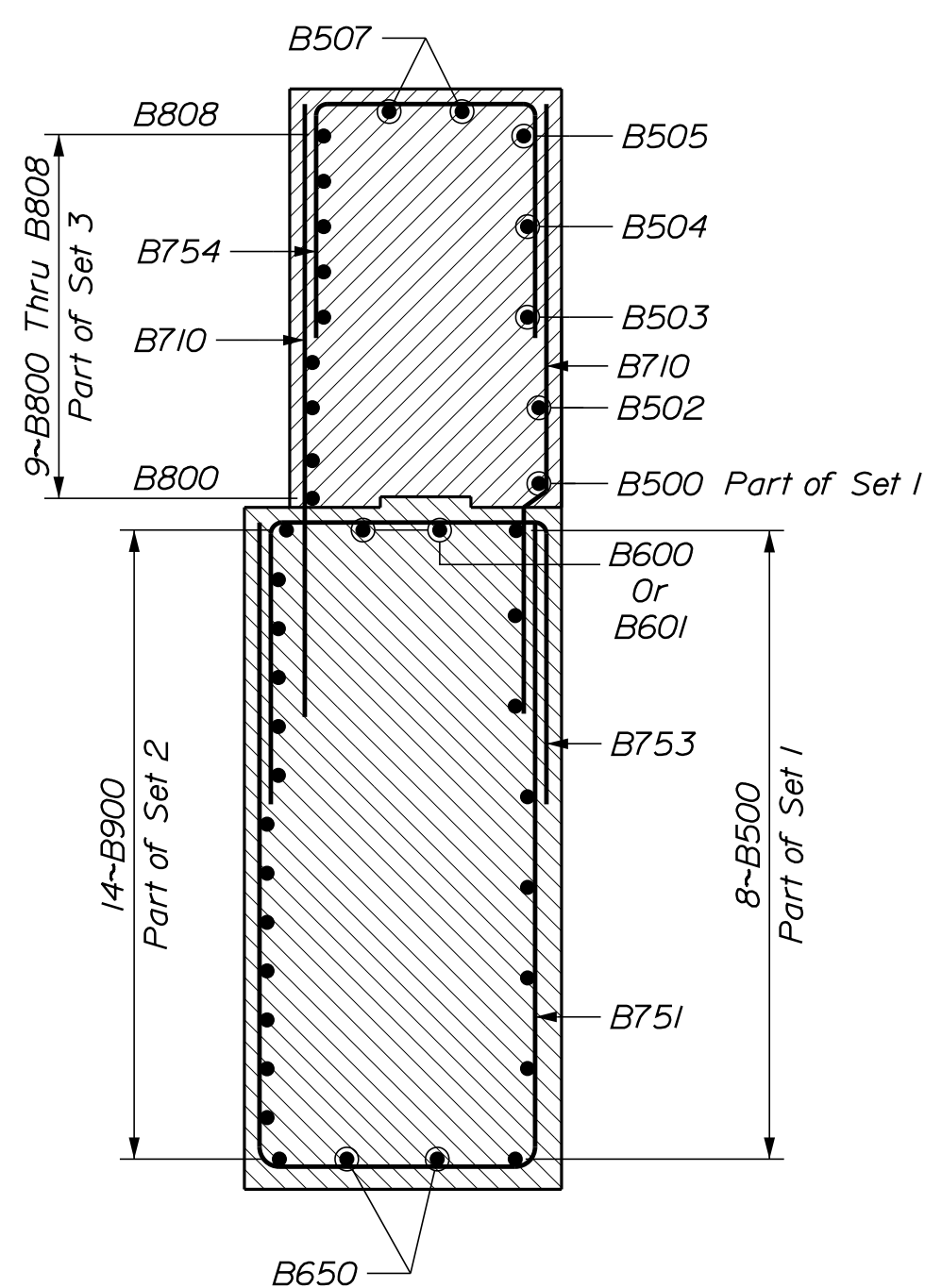


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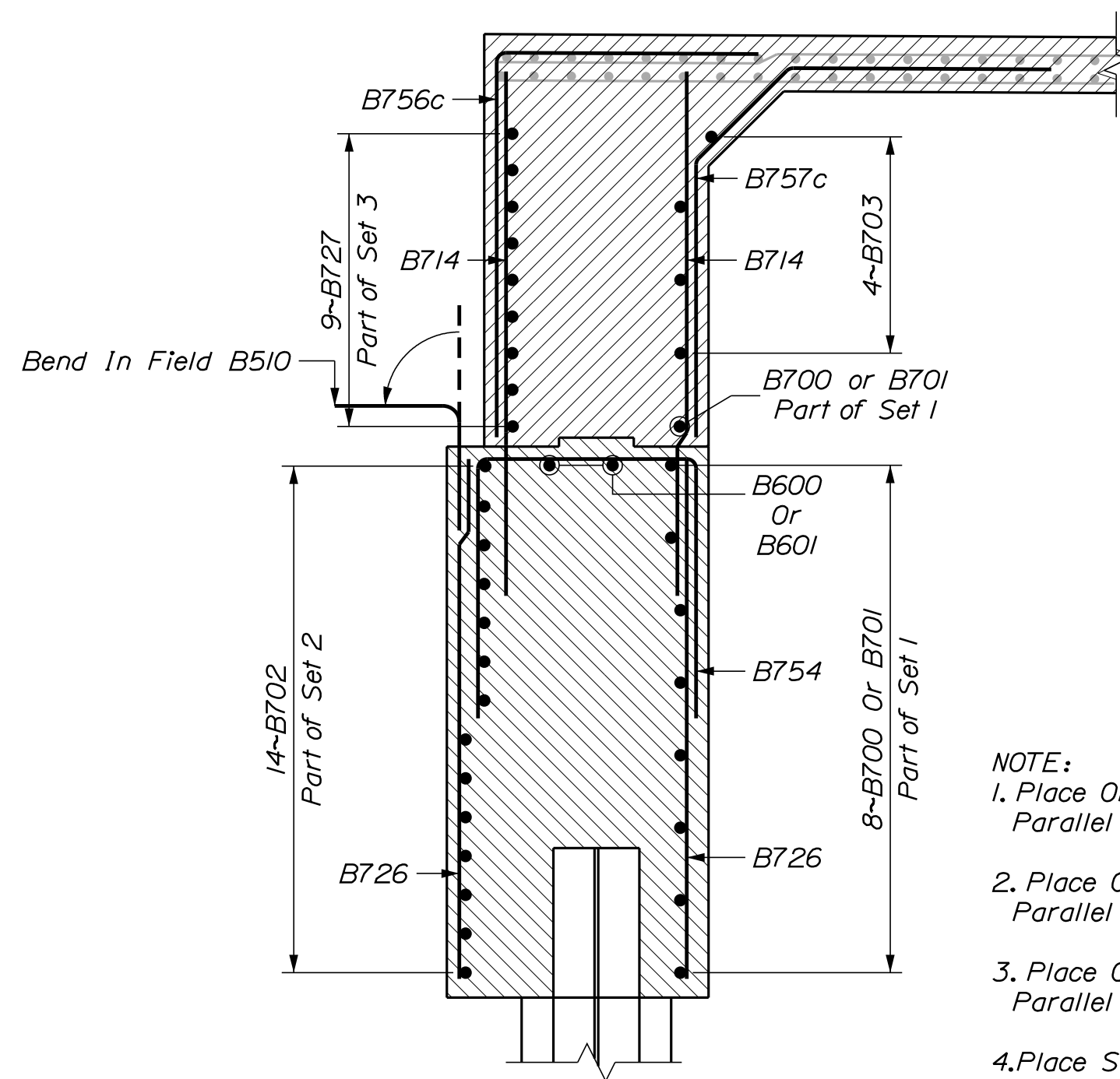
SET 2

SET 3

SFT 4

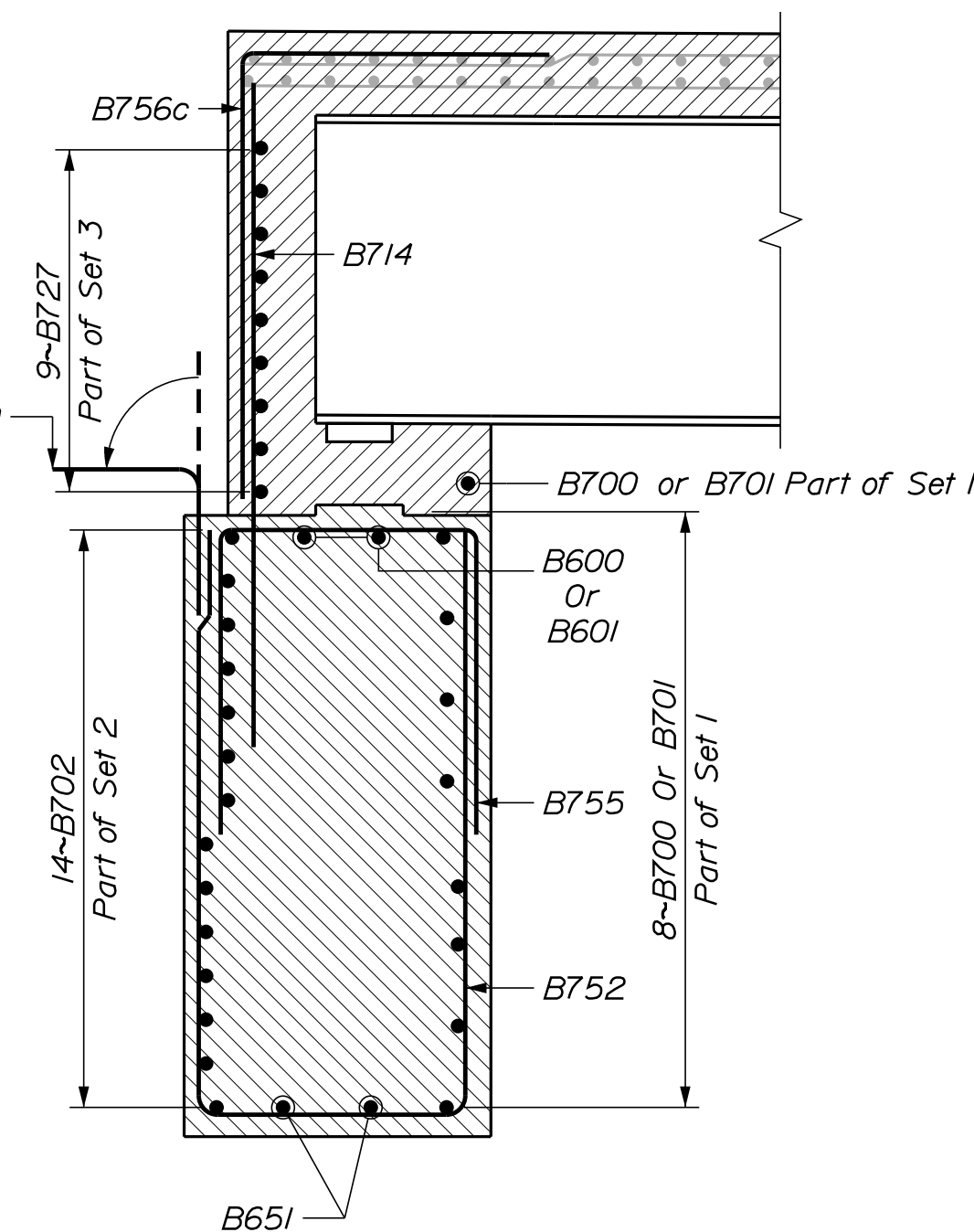


SECTION A - A
North Wingwall

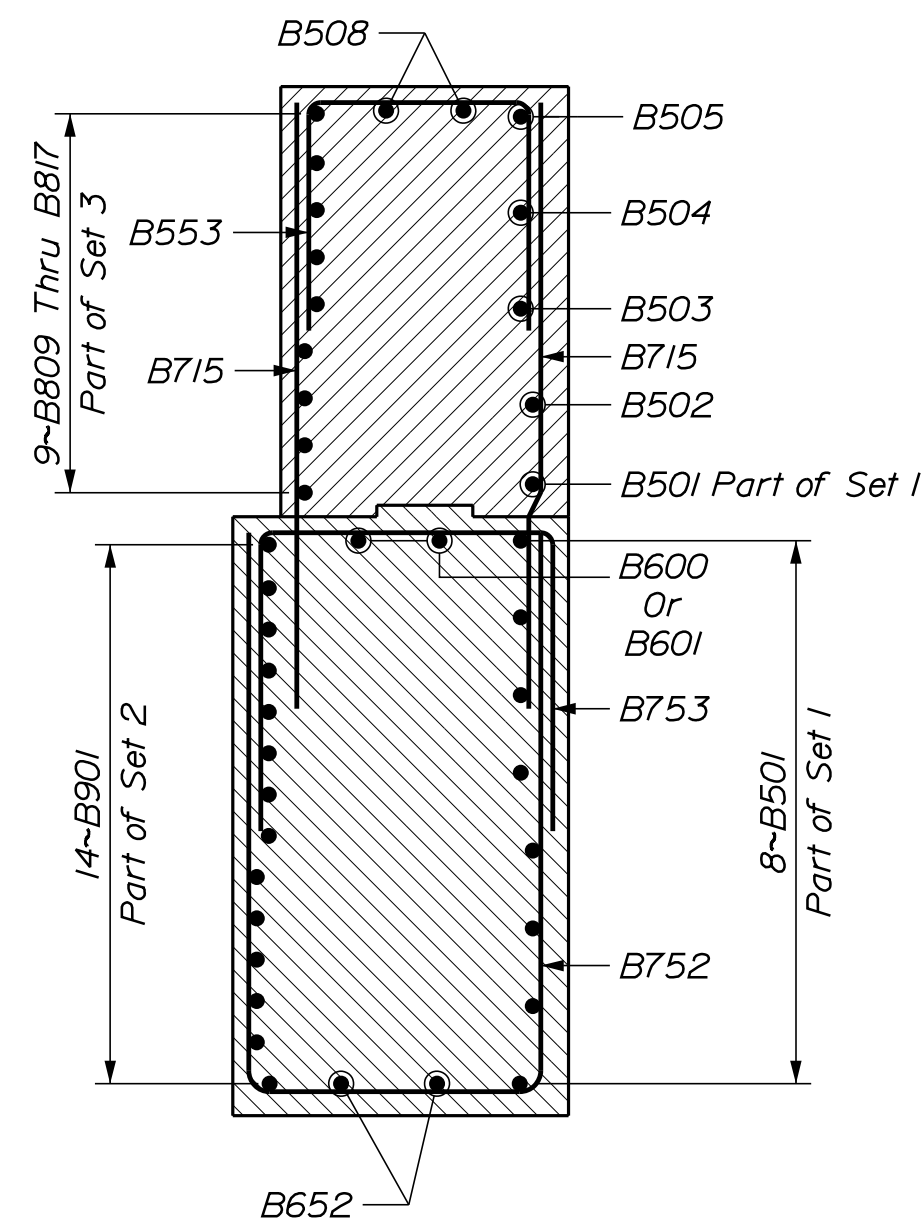


SECTION B - B
At Pile

- NOTE:**
1. Place One Set I Under And Parallel To Construction Joint
 2. Place One Set I Above And Parallel To Construction Joint
 3. Place One Set 2 Above And Parallel To Construction Joint
 4. Place Set 3's Parallel To Construction Joint



SECTION C - C
At Girder



SECTION D - D
South Wingwall

STATE OF MAINE	
DEPARTMENT OF TRANSPORTATION	
02529900	
BRIDGE NO. 3107	WIN
	025299.00
BRIDGE PLANS	

PROJ. MANAGER	BRIAN NICHOLS	BY	DATE
DESIGN-CHECKED	E. BREWER	MRP	Nov. 2024
DESIGN-REVIEWED	RMN	E. BREWER	Nov. 2024
DESIGN-DETAILED	DES2		
DESIGN-DETAILED	DES3		
REVISIONS 1			
REVISIONS 2			
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			
SIGNATURE			
P.E. NUMBER			
DATE			

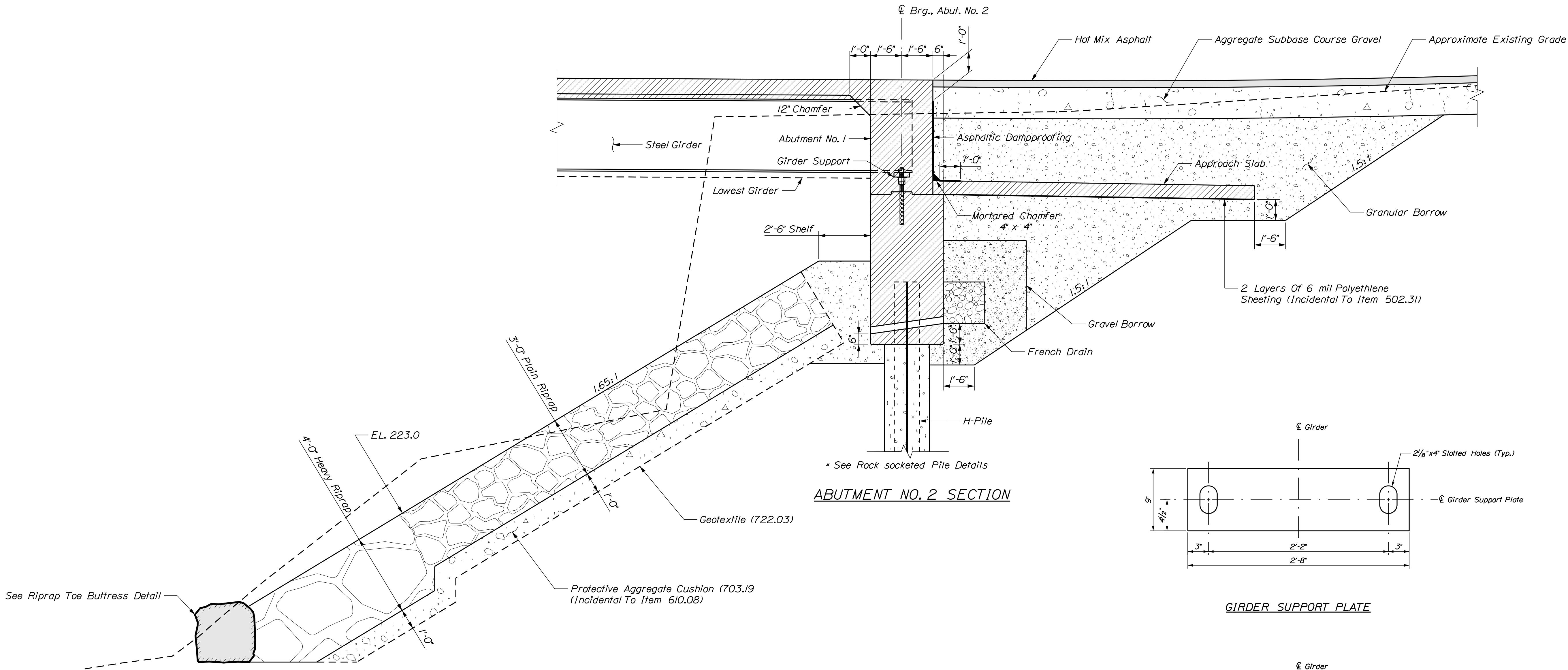
PUDDLE DOCK BRIDGE
FIFTEENMILE STREAM
ALBION KENNEBEC COUNTY

ABUTMENT NO.2 REINFORCING

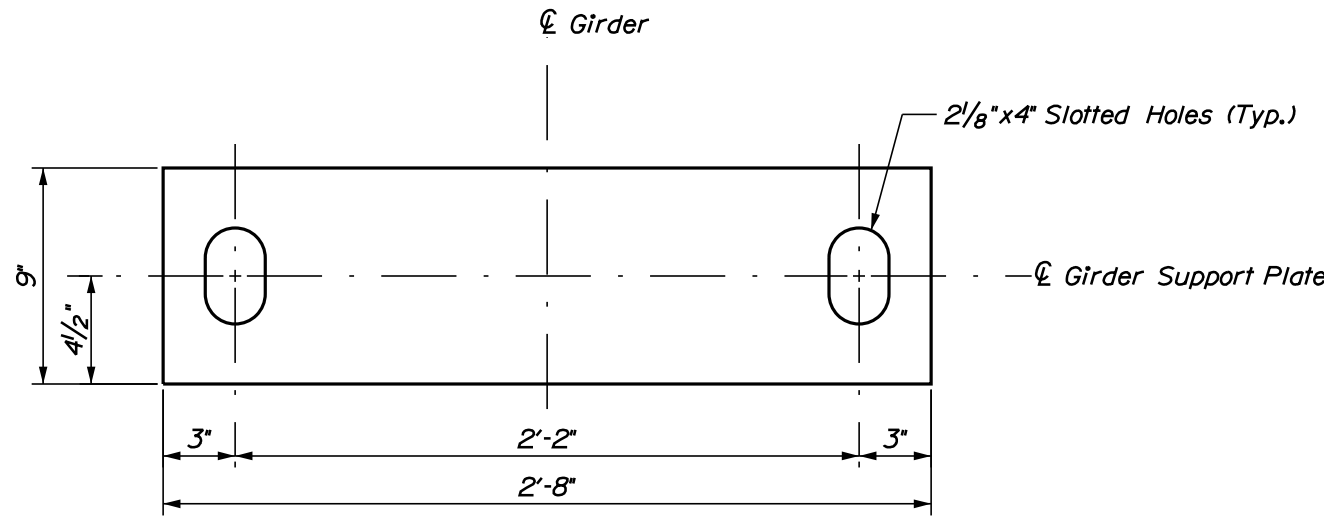
SHEET NUMBER

23

OF 31



ABUTMENT NO. 2 SECTION

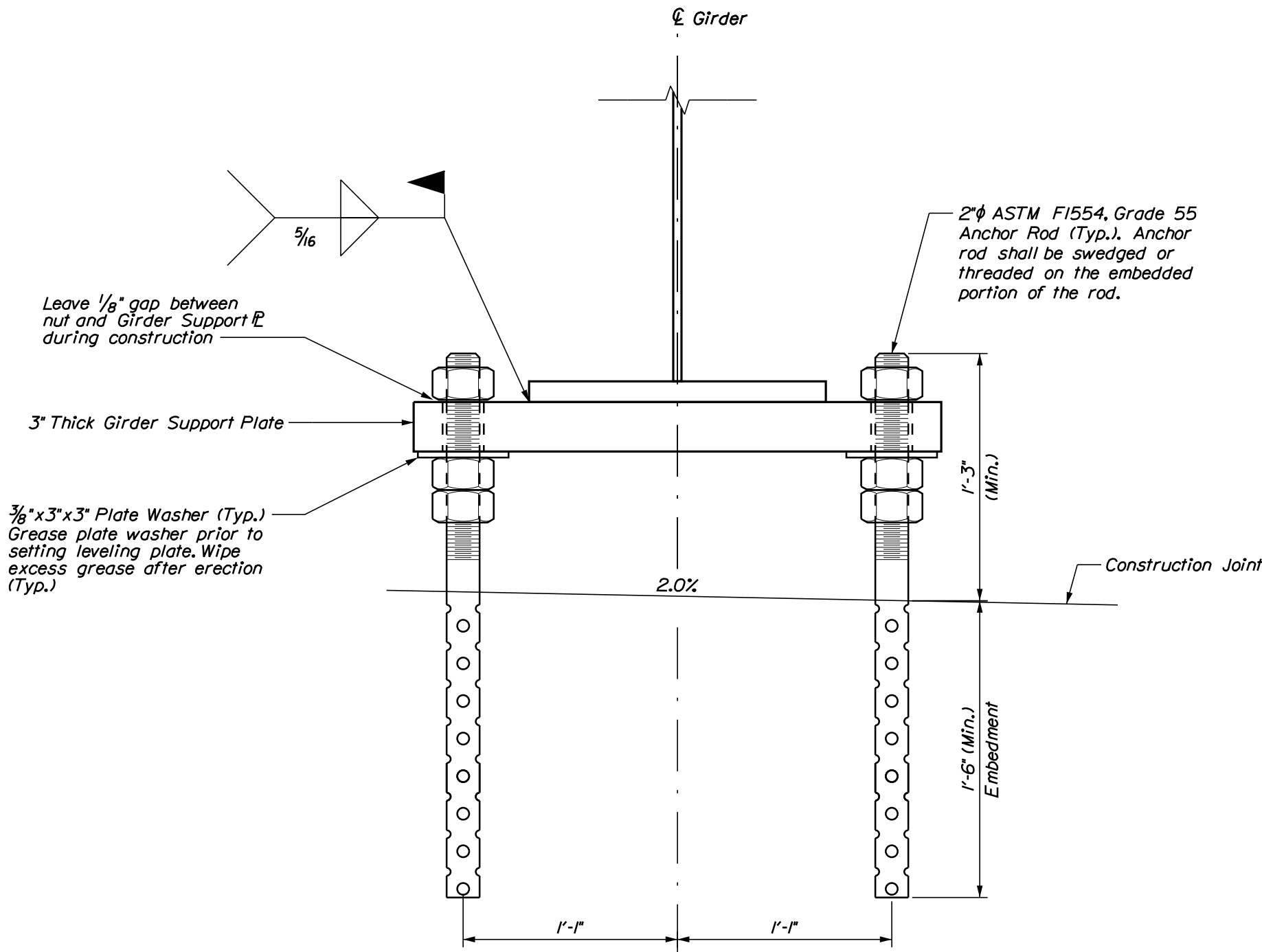


GIRDER SUPPORT PLATE

GIRDER SUPPORT NOTES

1. Payment for fabrication and installation of the girder support assemblies at the abutments will be considered incidental to related contract items.
2. Grease plate washer prior to setting bearing plate. Wipe excess grease after erection.

ABUTMENT NO. 2 BOTTOM OF GIRDER ELEVATIONS	
Girder	Elevation
G1	242.75
G2	241.61
G3	241.48
G4	241.35
G5	241.21



GIRDER SUPPORT DETAIL

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

02529900
WIN
025299.00

BRIDGE NO. 3107
BRIDGE PLANS

PROJ. MANAGER
BRIAN NICHOLS

DESIGN-DETAILED
E. BREWER

CHECKED-REVIEWED
RNN

DESIGNS-DETAILED

REVISIONS 1

REVISIONS 2

REVISIONS 3

REVISIONS 4

FIELD CHANGES

DATE
Nov. 2024

BY
MRP
E. BREWER

DATE
Nov. 2024

SIGNATURE

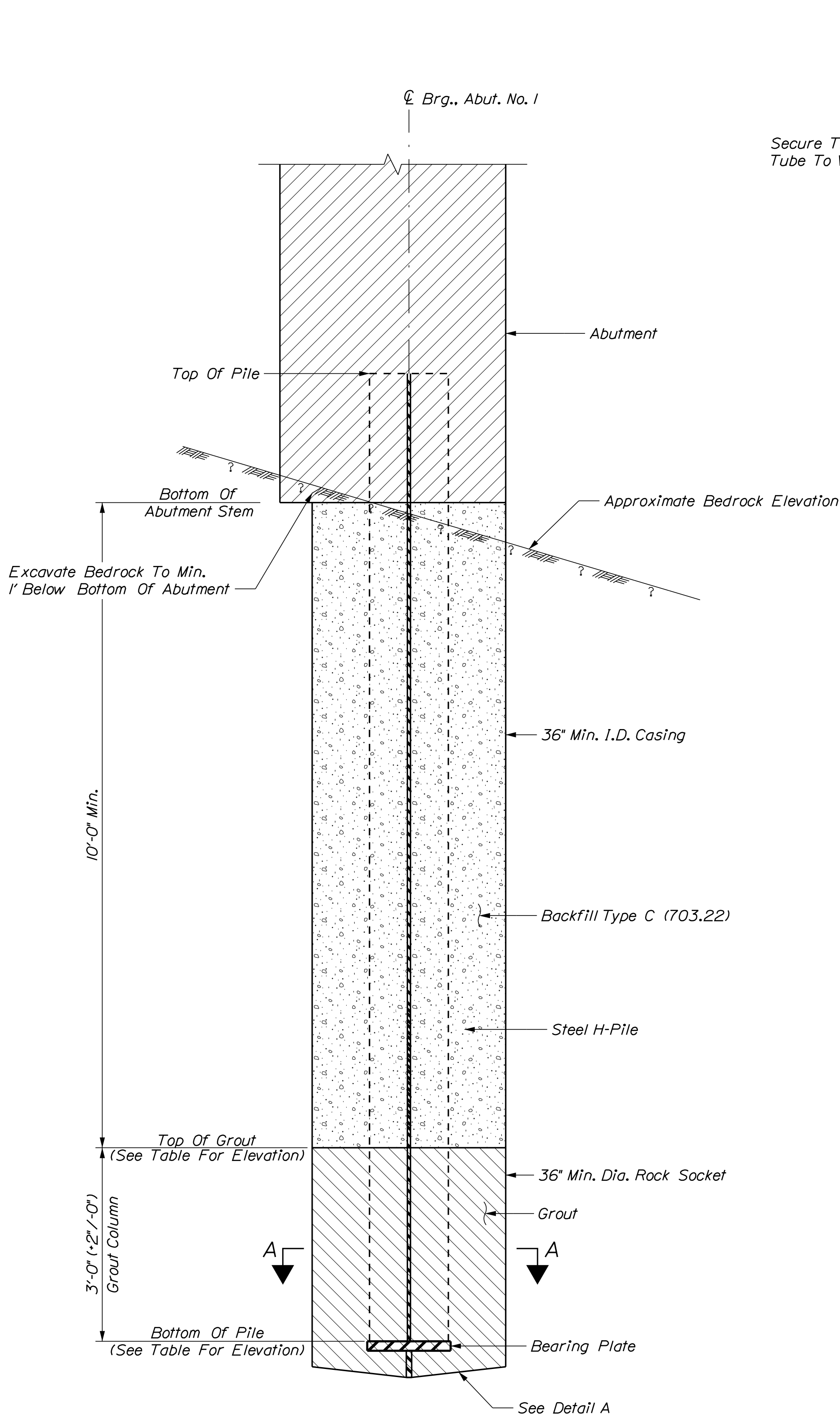
P.E. NUMBER

DATE

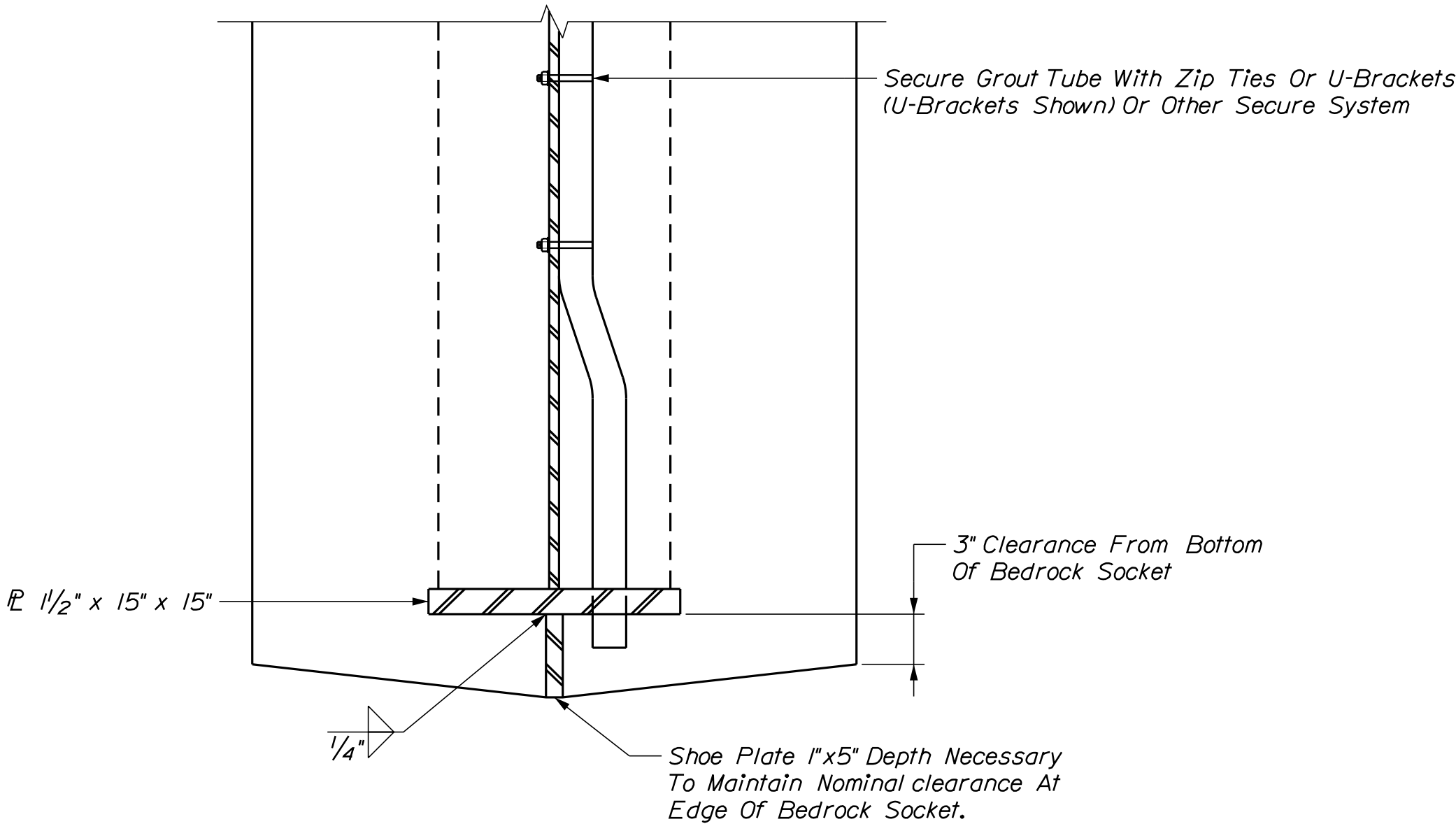
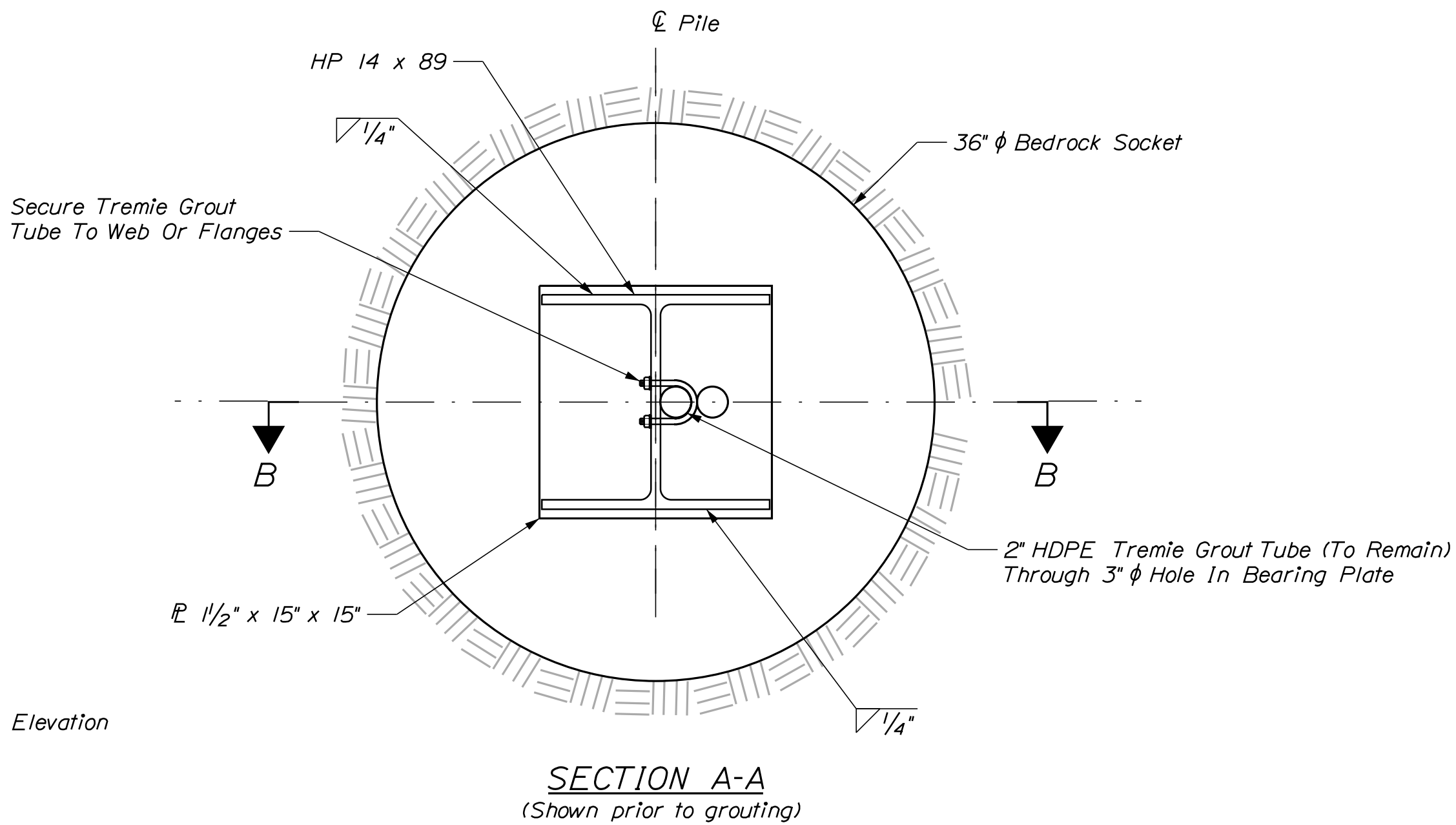
PUDDLE DOCK BRIDGE
FIFTEENMILE STREAM
ALBION
KENNEBEC COUNTY

ABUTMENT NO. 2 SECTION &
GIRDER SUPPORT

SHEET NUMBER
24
OF 31



ROCK SOCKETED PILE DETAIL
Abutment No. 1 Shown, Abutment No. 2 Similar



Abutment	Bottom Of Abutment Stem Elevation (Ft.)	Approximate Top Of Bedrock Elevation (Ft.)	Estimated Top Of Grout Elevation (Ft.)	Estimated Bottom Of Pile Elevation (Ft.)	Approximate Rock Socket Length (Ft.)
1	235.7	233.5	225.7	222.7	10.8
		239.1			16.4
2	233.2	231.9	223.2	220.2	11.7
		234.6			14.4

PILE TABLE NOTES

- These approximate bedrock elevations have been estimated based on the test borings conducted at each abutment.
- Top of rock elevations are provided to the nearest 0.1' and are using widely space explorations. Actual elevations may vary.
- Estimated top of grout elevation is 10' below bottom of abutment stem elevation. If top of rock is more than 10' below bottom of abutment stem, top of grout shall be 1' below the approximate top of rock to provide a minimum rock socket depth of 4' with 3' grouted embedment.
- Top of rock is defined as penetration of full diameter of rock excavation equipment beneath the bedrock surface, as determined by the Resident.
- Approximate top of bedrock elevation is based on the range of bedrock elevations encountered in the explorations at each abutment.

PILE NOTES

- The maximum factored pile load is 357.2 kips at the Strength Limit State.
- Estimate of piles required:

Abutment No. 1: 5 ~ HP 14x89 @ 15 feet (See chart for pile excavation estimates.)
Abutment No. 2: 5 ~ HP 14x89 @ 15 feet (See chart for pile excavation estimates.)
- H-pile material shall be ASTM A572, Grade 50.
- Piles shall not be out of position shown by more than 2 inches in any direction.
- Piles shall be installed in accordance with Special Provision Section 501, Rock-Socketed H-Pile Foundations.

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BRIDGE NO. 3107
BRIDGE PLANS

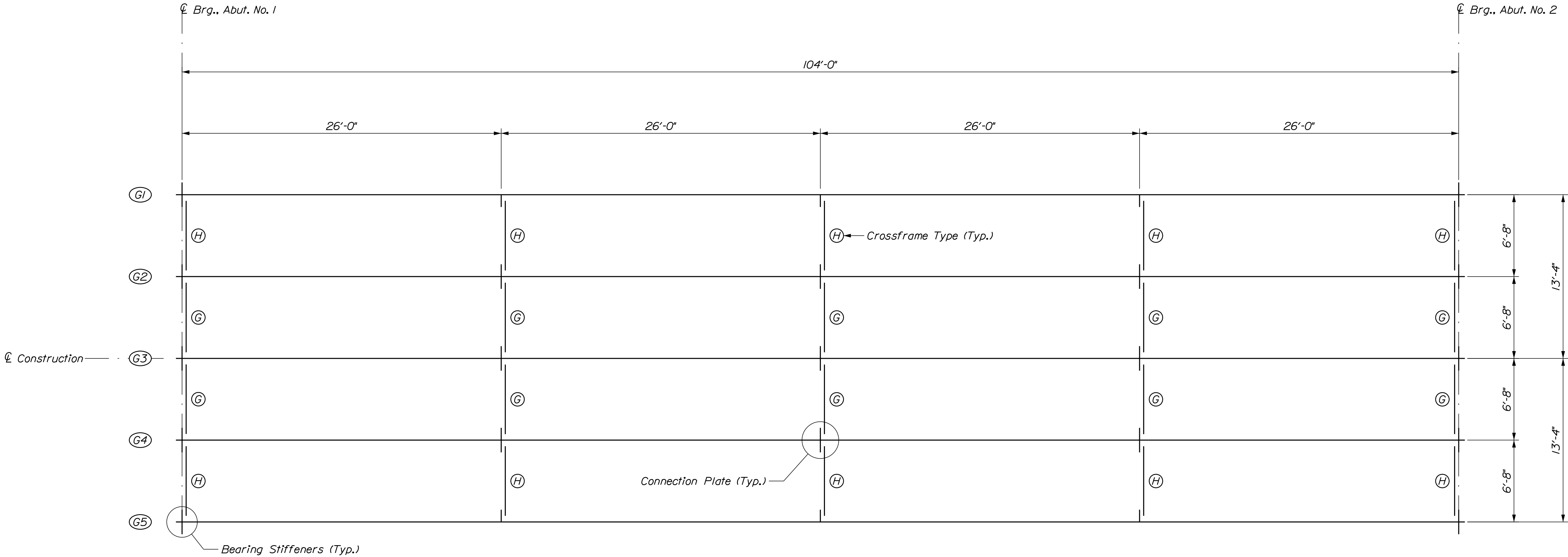
PUDDLE DOCK BRIDGE
FIFTEENMILE STREAM
ALBION KENNEBEC COUNTY

ROCKSOCKET DETAILS

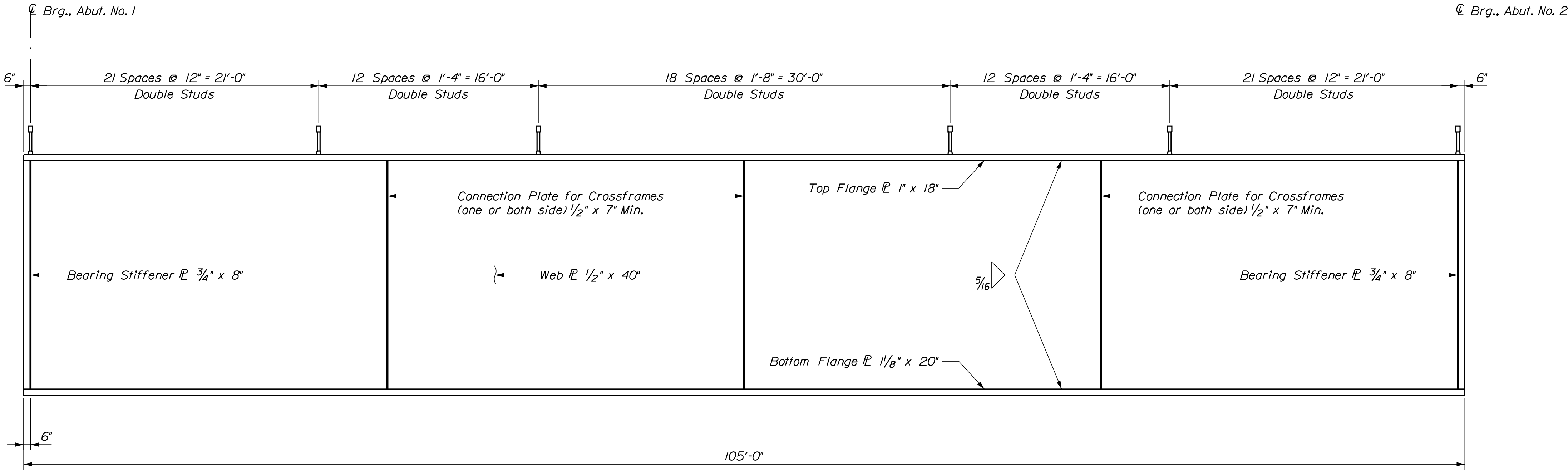
SHEET NUMBER

25

OF 31

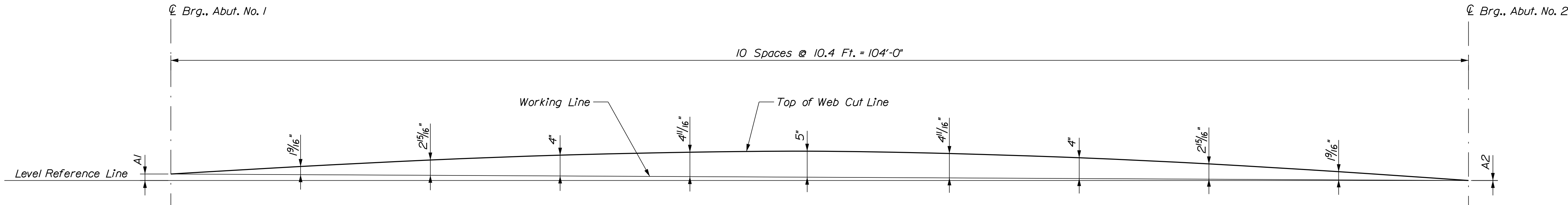


FRAMING PLAN

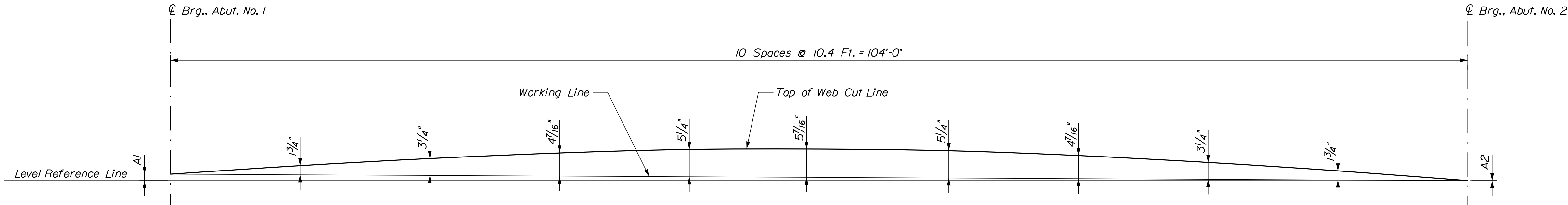


GIRDER ELEVATION
 170 Studs Per Girder x 5 Girders
 (850 Studs Total)

STATE OF MAINE		DEPARTMENT OF TRANSPORTATION		02529900		WIN		025299.00		BRIDGE NO. 3107		BRIDGE PLANS	
PUDDLE DOCK BRIDGE		FIFTEENMILE STREAM		KENNEBEC COUNTY		ALBION		FRAMING & GIRDER DETAILS		SHEET NUMBER		26	
PROJ. MANAGER		BRIAN NICHOLS		BY		DATE		SIGNATURE		P.E. NUMBER		DATE	
DESIGNED-DETAILED		E. BREWER		MRP		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
CHECKED-REVIEWED		E. BREWER		E. BREWER		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
DESIGNED-DETAILED		E. BREWER		E. BREWER		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
REVISIONS 1		E. BREWER		E. BREWER		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
REVISIONS 2		E. BREWER		E. BREWER		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
REVISIONS 3		E. BREWER		E. BREWER		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
REVISIONS 4		E. BREWER		E. BREWER		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
FIELD CHANGES		E. BREWER		E. BREWER		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	



CAMBER DIAGRAM
Exterior Girders



CAMBER DIAGRAM
Interior Girders

Table Of Camber Dimensions (Feet)		
Girder	A1	A2
1	0.52	0.00
2	0.52	0.00
3	0.52	0.00
4	0.52	0.00
5	0.52	0.00

Camber Ordinates "CO" (Inches)											
	℄ A1	Span									℄ A2
Girder	0'	10.4'	20.8'	31.2'	41.6'	52'	62.4'	72.8'	83.2'	93.6'	104'
Exterior	0	1 9/16	2 5/16	4	4 11/16	5	4 11/16	4	2 5/16	1 9/16	0
Interior	0	1 3/4	3 1/4	4 7/16	5 1/4	5 7/16	5 1/4	4 7/16	3 1/4	1 3/4	0

Bottom Of Slab Elevations												
	℄ A1	Span										℄ A2
Girder	0'	10'	20'	30'	40'	50'	60'	70'	80'	90'	100'	104'
1	245.98	246.04	246.08	246.11	246.11	246.08	246.02	245.93	245.82	245.68	245.53	245.48
2	245.85	245.92	245.97	246.01	246.02	245.99	245.93	245.84	245.71	245.56	245.40	245.35
3	245.72	245.78	245.84	245.88	245.88	245.86	245.80	245.70	245.58	245.43	245.26	245.22
4	245.58	245.65	245.71	245.74	245.75	245.72	245.66	245.57	245.44	245.30	245.13	245.08
5	245.45	245.50	245.55	245.58	245.58	245.55	245.49	245.40	245.28	245.14	244.99	244.95

Deflections - Girders (Inches)												
Girder	Dead Load Component	℄ A1	10.4'	20.8'	31.2'	41.6'	52'	62.4'	72.8'	83.2'	93.6'	℄ A2
Exterior	Steel Dead Load	0.00	0.30	0.58	0.79	0.92	0.96	0.92	0.79	0.58	0.30	0.00
	Fluid Dead Load	0.00	1.18	2.21	3.04	3.56	3.73	3.56	3.04	2.21	1.18	0.00
	Superimposed Dead Load	0.00	0.08	0.14	0.20	0.23	0.25	0.23	0.20	0.14	0.08	0.00
Interior	Steel Dead Load	0.00	0.30	0.58	0.79	0.92	0.96	0.92	0.79	0.58	0.30	0.00
	Fluid Dead Load	0.00	1.34	2.52	3.46	4.06	4.25	4.06	3.46	2.52	1.34	0.00
	Superimposed Dead Load	0.00	0.08	0.14	0.19	0.23	0.23	0.23	0.19	0.14	0.08	0.00

STRUCTURAL STEEL NOTES

1. Camber ordinates, as shown, are computed to compensate for all dead load deflections and for the curvature of the finished grade profile.
2. No transverse butt weld splices will be allowed in the flange plates or web plates within 10 feet or 10 percent of the span length (whichever is greater) from the points of maximum negative moment or maximum positive moment. Butt weld splices in flanges shall be not less than 1 foot from transverse butt welds in the web plates and no transverse web or flange butt welds shall be located within 1 foot of other transverse welds (e.g. connection plates to web welds) on either flange or web. No transverse butt weld splices will be allowed in areas of stress reversal.
3. Sections of flange plates or web plates between transverse shop splices or between a transverse shop splice and a field splice shall be at least 20 feet in length unless otherwise shown on the Plans.
4. Bearing stiffeners shall be plumb after erection and dead loading of the structure.
5. Intermediate crossframe or diaphragm connection plates may be either plumb or normal to the top flange.
6. All connection plate and stiffener welds shall be 5/16 inch fillet welds.
7. Coat girder ends and cross frames to a distance of 10 feet from each Abutment centerline of bearing in accordance with Standard Specifications Section 506, Shop Applied Protective Coating - Steel (Zinc Rich Coating System), except NEPCOAT Qualified Products List C may be used. Payment for the coating will be incidental to Pay Item 504.702.
8. After placement of the superstructure concrete, thoroughly clean the abutments of all stains with a method approved by the Resident. Payment will be considered incidental to related Contract items.
9. Provide a drip bar on the high side of each abutment on all girders in accordance with Standard Detail 504.101.
10. Structural steel was designed with a vertical construction load of 50 lb/sf and a lateral wind velocity during construction of 20 mph.
11. Bolted diaphragms or cross frame connections shall be made using 7/8 inch diameter, ASTM F3125, Grade A325 Type 3 high strength bolts. Hole size shall be 15/16 inch diameter. The minimum edge distance shall be 1 1/2 inch unless otherwise shown. Oversized or short-slotted holes are not permitted. Bolt threads shall be excluded from the shear plane of cross frame or diaphragms connections
12. Ends of girder webs shall be vertical under full dead load.

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BRIDGE NO. 3107
BRIDGE PLANS

PROJ. MANAGER
BRIAN NICHOLS

BY
E. BREWER

DATE
Nov. 2024

CHECKED-DETAILED
E. BREWER

NOV. 2024

SIGNATURE

P.E. NUMBER

DATE

PUDDLE DOCK BRIDGE
FIFTEENMILE STREAM
ALBION

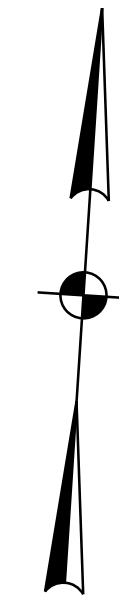
KENNEBEC COUNTY

CAMBER DETAILS &
BOTTOM OF SLAB ELEVATION

SHEET NUMBER

27

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g = Glass Fiber Reinforcing Polymer
c = High Strength, Low Carbon Chromium

SET "A"
1 ~ S700g & 2 ~ S552c (Top Mat)
1 ~ S700g (Bottom Mat)

S700g

S552c → S700g

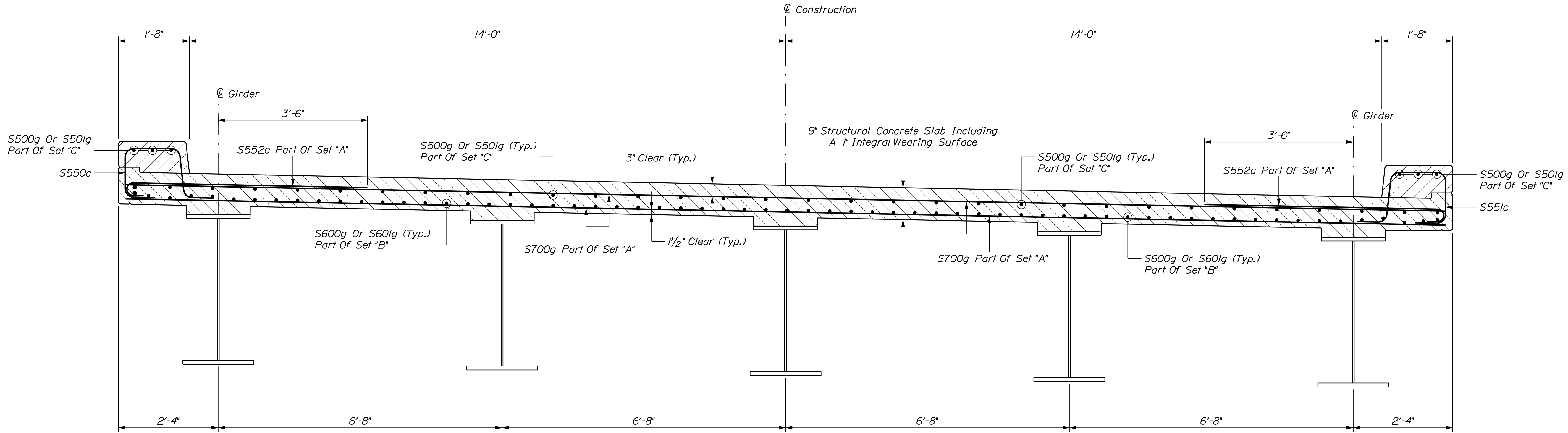
S700g

SET "B"
3 ~ S600g & 1 ~ S601c
(Bottom Mat)

Stagger The Lap Splice Locations.

SET "C"
3 ~ S500g & 1 ~ S50lc
(Top Mat)

Stagger The Lap Splice Locations.



PROPOSED BRIDGE SECTION

SUPERSTRUCTURE NOTES

1. The theoretical blocking used for design of the structure is 2.5 inch at the centerline of bearing of the abutments. Refer to Standard Detail 502(03) for blocking details.
2. Reinforcing steel shall have a minimum concrete cover of 2 inches unless otherwise noted.
3. Form a one inch V-groove on the fascias at the horizontal joint between the curb and slab.
4. The Saw Cut Grooving shall be in the longitudinal direction.
5. The superstructure slab concrete shall be placed continuously and shall be kept plastic until the entire placement has been made.
6. Precast Concrete Deck Panels are not allowed on this project.

STATE OF MAINE		DEPARTMENT OF TRANSPORTATION		02529900		WIN		025299.00		BRIDGE NO. 3107		BRIDGE PLANS	
PUDDLE DOCK BRIDGE		FIFTEENMILE STREAM		KENNEBEC COUNTY		ALBION		SUPERSTURCTURE SECTION & NOTES		SHEET NUMBER		29	
PROJ. MANAGER		BRIAN NICHOLS		BY		DATE		SIGNATURE		P.E. NUMBER		DATE	
DESIGNED-DETAILED		E. BREWER		MRP		Nov. 2024		SIGNATURE		P.E. NUMBER		DATE	
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