

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION



SPECIFICATIONS

Design: Load and Resistance Factor Design per AASHTO LRFD Bridge Design Specifications, Ninth Edition 2020.

DESIGN LOADING

Live Load HL - 93 Modified for Strength I

TRAFFIC DATA

Current (2020) AADT 4680
Future (2040) AADT 5260
DHV - % of AADT 13%
Design Hour Volume 731
Heavy Trucks (% of AADT) 4%
Heavy Trucks (% of DHV) 3%
Directional Distribution (% of DHV) 69%
18 kip Equivalent P 2.0 123
18 kip Equivalent P 2.5 118
Design Speed (mph) 25 MPH

MATERIALS

Concrete:
Curbs Class "LP"
All Other Class "A"
Reinforcing Steel
Deck Bars, Curb Bars, End Diaphragm
and Wingwalls ASTM A1035, Alloy Type CS, Grade 100
All Others ASTM A615/A615M, Grade 60

BASIC DESIGN STRESSES

CONCRETE:
Class "LP" f 'c = 5000 psi
Class "A" f 'c = 4000 psi
REINFORCING STEEL:
ASTM A1035, Grade 100 f y = 100,000 psi
ASTM A615, Grade 60 f y = 60,000 psi

LIST OF DRAWINGS

Title Sheet 1
Estimated Quantities 2
General Construction Notes 3
General Plan 4
Profile 5
Typical Sections 6
Construction Staging 7
Cross Sections 8-15
Maintenance of Traffic 16-17
Abutment 1 Modifications 18
Abutment 2 Modifications 19
Abutment Details 20
Structural Steel Details 21
Superstructure Plan 22
Reinforcing Plan 23
Superstructure Deck End Details 24
Reinforcing Schedule 25
Right of Way Map 26

KENNEBUNK
YORK COUNTY
NASH MILL BRIDGE
OVER
MOUSAM RIVER
MILL STREET
FEDERAL PROJECT NO. 2362900
PROJECT LENGTH 0.076 mi.
BRIDGE NO. 5756

UTILITIES

Kennebunk Light and Power Company
Kennebunk, Kennebunkport & Wells Water District

MAINTENANCE OF TRAFFIC

Maintain one lane of alternating one-way traffic on the existing bridge, using temporary traffic signals.

PROJECT LOCATION	Nash Mill Bridge carrying Mill Street over Mousam River in Kennebunk, ME Latitude: 43°24' 13.54" N, Longitude: 70°35' 9.78" W
PROGRAM AREA	Bridge Program
OUTLINE OF WORK	Bridge deck replacement and wingwall modifications.

STATE OF MAINE DEPARTMENT OF TRANSPORTATION	APPROVED	DATE
	COMMISSIONER: <i>[Signature]</i>	10-30-23
CHIEF ENGINEER: <i>[Signature]</i>		

STATE OF MAINE DEPARTMENT OF TRANSPORTATION	SIGNATURE	P.E. NUMBER	DATE
	<i>[Signature]</i>	12202	10/23/2023

PROJECT INFORMATION	PROGRAM	PROJECT MANAGER	DESIGNER	CONSULTANT	PROJECT RESIDENT	CONTRACTOR	PROJECT COMPLETION DATE
	BRIDGE	D. EATON	A. STOCKTON	WSP			

KENNEBUNK NASH MILL BRIDGE	TITLE SHEET

SHEET NUMBER
1
OF 26

WIN 023629.00

2362900

Date:10/25/2023

Username:

Division: BRIDGE

Filename: \\Codd\BRIDGE\MSTA\001_Title.dgn

ESTIMATED QUANTITIES			
ITEM NO.	DESCRIPTION	QUANTITY	UNIT
202.10	REMOVING EXISTING SUPERSTRUCTURE PROPERTY OF CONTRACTOR	(47 CY)	1
202.12	REMOVING EXISTING STRUCTURAL CONCRETE		31
202.202	REMOVING PAVEMENT SURFACE		690
203.20	COMMON EXCAVATION		310
203.25	GRANULAR BORROW		133
206.082	STRUCTURAL EARTH EXCAVATION - MAJOR STRUCTURES, PLAN QUANTITY		153
304.10	AGGREGATE SUBBASE COURSE - GRAVEL		320
403.2081	12.5 MM POLYMER MODIFIED HOT MIX ASPHALT		96
403.209	HOT MIX ASPHALT 9.5 MM (INCIDENTALS)		5
403.211	HOT MIX ASPHALT (SHIM)		25
403.2131	12.5 MM POLYMER MODIFIED HOT MIX ASPHALT BASE		65
409.15	BITUMINOUS TACK COAT - APPLIED		59
424.304	HIGH MOLECULAR WEIGHT METHACRYLATE CRACK SEALER		35
502.219	STRUCTURAL CONCRETE, ABUTMENTS & RETAINING WALLS	(11 CY)	1
502.26	STRUCTURAL CONCRETE ROADWAY AND SIDEWALK SLAB ON STEEL BRIDGES	(93 CY)	1
502.291	SAW CUT GROOVING	(2091 SF)	1
502.31	STRUCTURAL CONCRETE APPROACH SLABS	(21 CY)	1
502.49	STRUCTURAL CONCRETE CURBS AND SIDEWALKS	(7 CY)	1
503.12	REINFORCING STEEL, FABRICATED AND DELIVERED		4350
503.13	REINFORCING STEEL, PLACING		4350
503.17	MECHANICAL/WELDED SPLICE		724
503.19	LOW-CARBON CHROMIUM REINFORCEMENT, FABRICATED AND DELIVERED		30900
503.20	LOW CARBON CHROMIUM REINFORCEMENT, PLACING		30900
505.08	SHEAR CONNECTORS	(810 EA)	1
507.0821	STEEL BRIDGE RAILING, 3-BAR	(154 LF)	1
507.0822	STEEL APPROACH RAIL, 3-BAR		4
508.13	SHEET WATERPROOFING MEMBRANE	(38 SY)	1
515.21	PROTECTIVE COATING FOR CONCRETE SURFACES	(65 SY)	1
518.60	REPAIR OF VERTICAL SURFACES < 8 INCHES		160
518.80	CRACK REPAIR		40
520.233	EXPANSION DEVICE - SILICONE COATED PRECOMPRESSED FOAM		43
524.301	TEMPORARY STRUCTURAL SUPPORT - LATERAL BRACING		1
524.301	TEMPORARY STRUCTURAL SUPPORT (ROADWAY SUPPORT DURING STAGED CONSTRUCTION)		1
526.305	PORTABLE CONCRETE BARRIER, BRACED TYPE I	(180 LF)	1
527.34	WORK ZONE CRASH CUSHIONS		2
606.1301	31" W-BEAM GUARDRAIL - MID-WAY SPLICE (STEEL POST, 8" OFFSET BLOCKS, SINGLE FACED)		260
606.1303	31" W-BEAM GUARDRAIL - MID-WAY SPLICE (STEEL POST, 8" OFFSET BLOCKS, 15' RADIUS AND LESS)		32
606.1304	31" W-BEAM GUARDRAIL - MID-WAY SPLICE (STEEL POST, 8" OFFSET BLOCKS, OVER 15' RADIUS)		41
606.1721	BRIDGE TRANSITION - TYPE I		4
606.265	TERMINAL END - SINGLE RAIL - GALVANIZED STEEL		2
606.353	REFLECTORIZED FLEXIBLE GUARDRAIL MARKER		8
606.78	LOW VOLUME GUARDRAIL END		2
613.319	EROSION CONTROL BLANKET		210
615.07	LOAM		13
618.14	SEEDING, METHOD NUMBER 2		2
619.12	MULCH		2
619.14	EROSION CONTROL MIX		25
627.733	4" WHITE OR YELLOW PAINTED PAVEMENT MARKING LINE		1390
627.77	REMOVE EXISTING PAVEMENT MARKING		780
627.78	TEMPORARY PAVEMENT MARKING LINE, WHITE OR YELLOW		1100
629.05	HAND LABOR, STRAIGHT TIME		10
631.12	ALL-PURPOSE EXCAVATOR (INCLUDING OPERATOR)		10
631.14	GRADER (INCLUDING OPERATOR)		10
631.15	ROLLER EARTH BASE COURSE (INCLUDING OPERATOR)		10
631.172	TRUCK-LARGE (INCLUDING OPERATOR)		10
631.21	ROAD BROOM (INCLUDING OPERATOR & HAULER)		10
639.19	FIELD OFFICE TYPE B		1
643.72	TEMPORARY TRAFFIC SIGNAL		1
652.33	DRUM		23
652.34	CONE		46
652.35	CONSTRUCTION SIGNS		400
652.361	MAINTENANCE OF TRAFFIC CONTROL DEVICES		1
652.38	FLAGGER		360
652.41	PORTABLE CHANGEABLE MESSAGE SIGN		2
656.75	TEMPORARY SOIL EROSION AND WATER POLLUTION CONTROL		1
659.10	MOBILIZATION		1

2. *The clearing limits as shown on the plans are approximate. The exact limits will be established in the field by the Resident. Payment for clearing will be considered incidental to Contract items.*
3. *All utility facilities will be adjusted by the respective utilities unless otherwise Noted.*
4. *Do not excavate for Aggregate Subbase Course where existing material is suitable as determined by the Resident.*
5. *In areas where the Resident directs the Contractor not to excavate to the subgrade line shown on the plans, payment for removing existing pavement, grubbing, shaping, ditching, and compacting the existing subbase and layers of new subbase 6 inches or less thick will be made under appropriate equipment rental items.*
6. *Unless otherwise noted, place loam 2 inches deep on all new or reconstructed side slopes or as directed by the Resident.*
7. *Guardrail end treatments shall be installed concurrently with the placement of each section of beam guardrail. No exposed ends are allowed.*
8. *Guardrail posts as shown in the Standard Details shall be modified from the indicated length of 7 feet to a length of 8 feet with an embedment of 5.25 feet. Payment will be considered incidental to the guardrail pay items.*
9. *Protective Coating for Concrete Surfaces shall be applied to the following areas:*

All exposed surfaces of concrete curbs, fascias down to the drip notch, top of abutment backwalls and wingwalls, and to one foot below the ground on vertical walls against earth.
10. *Apply High Molecular Weight Methacrylate Crack Sealer to concrete curbs and concrete wearing surface. Apply to curb faces with a brush. This work will be paid under Pay Item 424.304, High Molecular Weight Methacrylate Crack Sealer.*
11. *Project information referred to below may be accessed at the following MaineDOT web address: <http://www.maine.gov/mdot/contractors/>.*
12. *The existing bridge plans may be accessed at the MaineDOT web address. The plans are reproductions of the original drawings as prepared for the construction of the bridge. It is very unlikely that the plans will show any construction field changes or any alterations which may have been made to the bridge during its life span.*
13. *Quantities included for pay items measured and paid for by Lump Sum are estimated quantities and are provided by MaineDOT for additional informational purposes only. Lump Sum pay items will be paid for at Contract Bid amount, with no addition or reduction in payment to the Contractor if the actual final quantities are different from the MaineDOT provided estimated quantities, except as follows:*
 - a. *If a Lump Sum Pay Item is eliminated, the requirements of Standard Specifications Section 109.2, Elimination of Items, will take precedence.*
 - b. *If other Contract Documents specifically allow a change in payment for a Lump Sum pay item, those requirements will be followed.*
 - c. *If a design change results in changes to estimated quantities for Lump Sum pay items, price adjustments will be made in accordance with Standard Specifications Section 109.7, Equitable Adjustments to Compensation and Time.*
14. *The Contractor shall submit a bridge deck removal plan to the Resident at least 10 days prior to the start of demolition work. The plan shall outline the methods and equipment to be used to remove and dispose of all materials included in the existing bridge deck. No work related to the removal of the bridge deck shall be undertaken by the Contractor until MaineDOT has reviewed the bridge deck removal plan for appropriateness and completeness. Payment for all work necessary developing, submitting and finalizing the removal plan will be considered incidental to the bridge deck removal pay item.*

15. The steel portions of the existing bridge may be coated with a lead-based paint system. The Contractor is responsible for the containment, proper management and disposal of all lead-contaminated hazardous waste generated by the process of rehabilitating the bridge. The Contractor is responsible for implementing appropriate OSHA mandated personal protection standards related to this process. Payment for all labor, materials, equipment and other costs required to remove and dispose of lead-contaminated waste will be considered incidental to Contract related items.
16. The Contractor shall plan and conduct the work accordingly so that upon final completion of the project there is no drop-off from the edge of shoulder pavement. All remaining or disturbed material on slopes or in ditches on the project shall be capable of attaining a growth of grass that is acceptable according to Standard Specification 618.10. No separate payment will be made for this work.
17. Erosion Control Mix may be substituted in those areas normally receiving loam and seed as directed by the Resident. Placement shall be in accordance with Standard Specification Section 619, Mulch. Payment will be made under Item 619.14, Erosion Control Mix.
18. Where it is apparent that runoff will cause continual erosion, Erosion Control Blanket, seeded gutters, riprap downspouts, and other gutters lined with Stone Ditch Protection shall be constructed after paving and shoulder work is completed. Payment will be made under the appropriate Contract items.
19. Place a 24 inch wide strip of Erosion Control Blanket on the sideslopes adjacent to the edge of pavement and behind the wingwalls.
20. Where new pavement joins existing pavement the existing pavement shall be sawcut along a smooth line to a neat, even, vertical joint, as directed by the Resident. Broken or raveled edges will not be permitted. All work necessary for the preparation of this joint will be considered incidental to the related contract items.
21. Concrete repairs are required at the abutments and will be paid under Item 518.60. The areas and limits of repair shall be established in the field by the Resident.
22. When removing existing bridge drains, the Contractor shall remove the tack welds flush with the existing girder.
23. Existing signs within the project limits shall be removed and reset as directed by the Resident. Payment for removal and reinstallation of the signs will be considered incidental to the Contract. No separate payment will be made.
24. Clean off existing bearings and have Resident inspect prior to placing any reinforcing in any end diaphragm or forming of concrete end diaphragm.

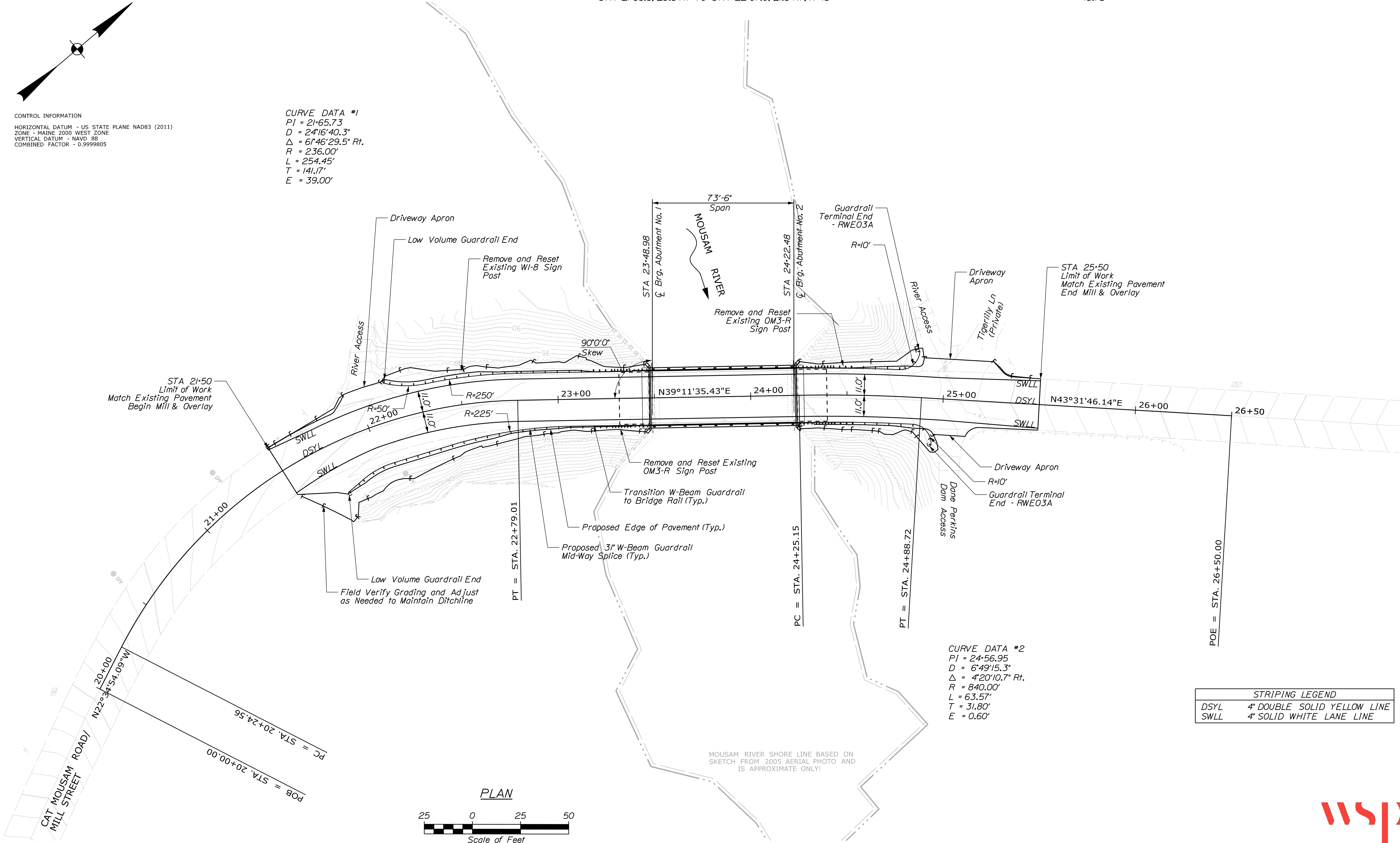
כ"ס

SUPERELEVATION TABLE					
LT SHOULDER		LT TRAVELWAY	STATION	RT TRAVELWAY	RT SHOULDER
MATCH	EXISTING	MATCH	EXISTING	MATCH	EXISTING
-2.1		+3.8	21+50	-6.2	
-4.0		+3.8	21+75	-6.0	
-4.0		+3.8	21+85	-6.0	
-4.0		+3.8	22+35	-4.7	
+0.4		+3.8	22+75	-3.7	
+3.1		+3.1	23+00	-3.1	
+3.1		+3.1	24+25	-3.1	
+2.4		+2.4	24+50	-3.1	
+2.4		+2.4	24+75	-3.1	
MATCH	EXISTING	MATCH	EXISTING	MATCH	EXISTING
			25+50		

ITEM 606.1301 - 3" W-Beam Guardrail - Mid-Way Splice (Steel Post, 8" Offset Blocks, Single Faced)	LF
STA 22+33.9, 14.0' LT TO STA 23+18.7, 14.0' LT	87.5
STA 21+76.5, 26.4' RT TO STA 21+86.6, 23.8' RT	9.38
STA 22+07.0, 21.3' RT TO STA 23+18.7, 14.0' RT	106.25
STA 24+52.3, 14.0' LT TO STA 24+76.9, 14.0' LT	25.0
STA 24+53.2, 14.0' RT TO STA 24+85.0, 14.0' RT	31.25
ITEM 606.1303 - 3" W-Beam Guardrail - Mid-Way Splice (Steel Post, 8" Offset Blocks, 15' Radius and Less)	LF
STA 24+76.9, 14.0' LT TO STA 24+86.6, 24.0' LT, R=10'	15.63
STA 24+85.0, 14.0' RT TO STA 24+95.1, 23.9' RT, R=10'	15.63
ITEM 606.1304 - 3" W-Beam Guardrail - Mid-Way Splice (Steel Post, 8" Offset Blocks, Over 15' Radius)	LF
STA 22+14.3, 19.6' LT TO STA 22+33.9, 14.0' LT, R=50'	21.88
STA 21+86.6, 23.8' RT TO STA 22+07.0, 21.3' RT, R=45'	18.75

CONTROL INFORMATION
HORIZONTAL DATUM - US STATE PLANE NAD83 (2011)
ZONE - MAINE 2000 WEST ZONE
VERTICAL DATUM - NAVD 88
COMBINED FACTOR - 0.9999805

CURVE DATA #1
PI = 21+65.73
D = 24°16'40.3"
Δ = 61°46'29.5" Rt.
R = 236.00'
L = 254.45'
T = 141.17'
E = 39.00'



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2362900

BRIDGE NO. 5756
WIN
023629.00
BRIDGE PLANS

STATE OF MAINE
TIMOTHY HIGGINSON
No. 12266

Signature

10/23/2023

DATE

9/20/23

BY

W. GERHOLD

PROJ. MANAGER

D. EATON

CHECKED-DETAILED

J. KIERA

DESIGN-DETAILED

T. HIGGINSON

DESIGN-DETAILED

REVISIONS 1

REVISIONS 2

REVISIONS 3

REVISIONS 4

FIELD CHANGES

SIGNATURE

12558

P.E. NUMBER

10/23/2023

DATE

NASH MILL BRIDGE
MOUSAM RIVER
YORK COUNTY

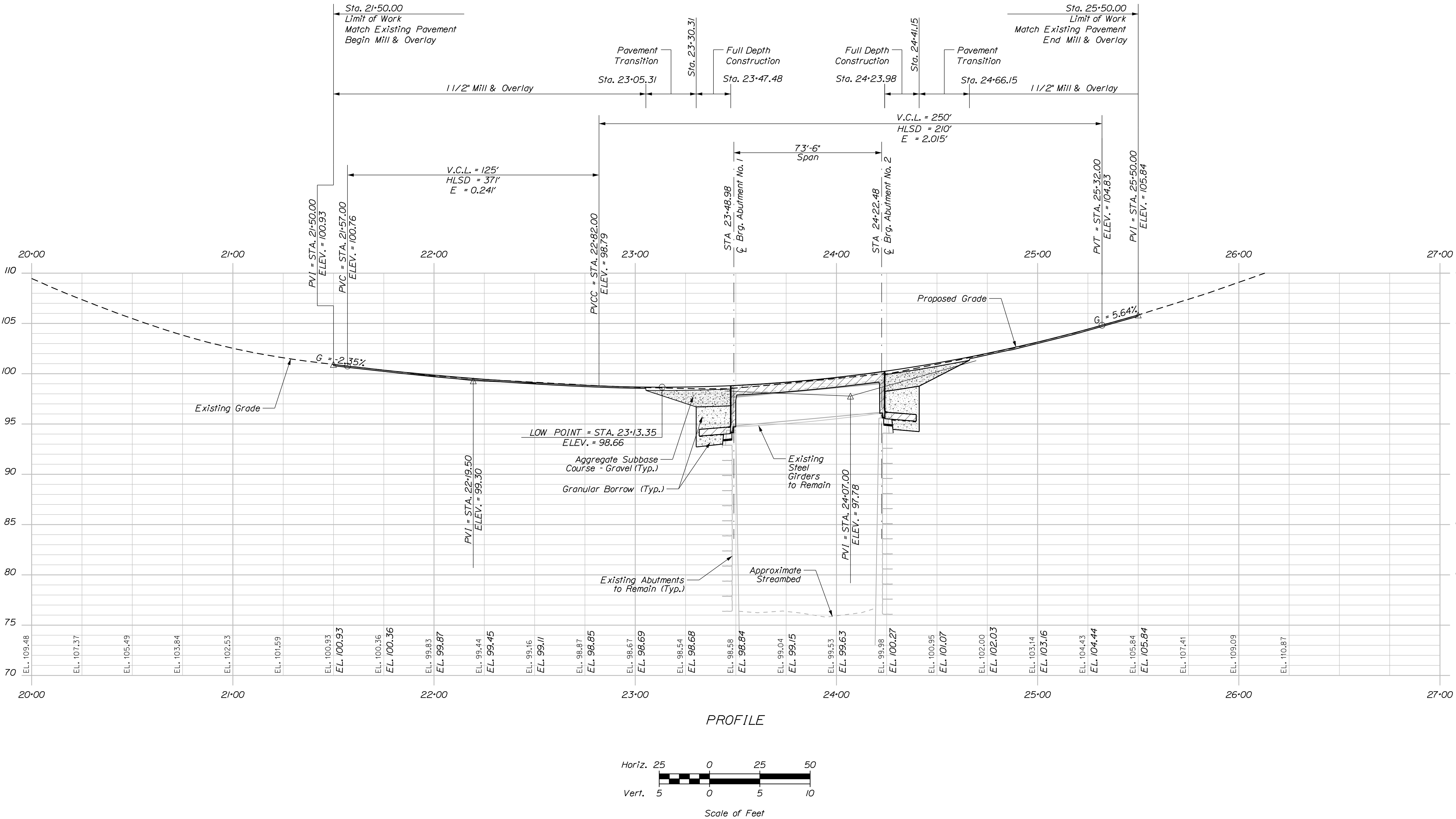
KENNEBUNK

GENERAL PLAN

SHEET NUMBER

4

OF 26



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2362900

WIN
023629.00

BRIDGE NO. 5756
BRIDGE PLANS

STATE OF MAINE
TIMOTHY
HIGGINSON
NO. 12206

10/23/2023

PROJ. MANAGER
DESIGNED-Detailed
CHECKED-Reviewed
DESIGNED-Detailed
REVISIONS 1
REVISIONS 2
REVISIONS 3
REVISIONS 4
FIELD CHANGES

DATE
9/2023

BY
J. KHERA

D. EATON
T. HIGGINSON

DATE
9/2023

BY
T. HIGGINSON

DATE
10/23/2023

BY
T. HIGGINSON

SIGNATURE
12558

P.E. NUMBER
10/23/2023

DATE
10/23/2023

DATE
10/23/2023

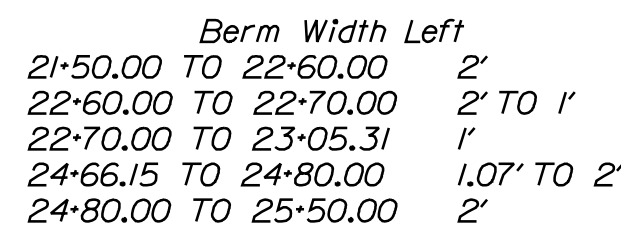
NASH MILL BRIDGE
MOUSAM RIVER
KENNEBUNK
YORK COUNTY

PROFILE

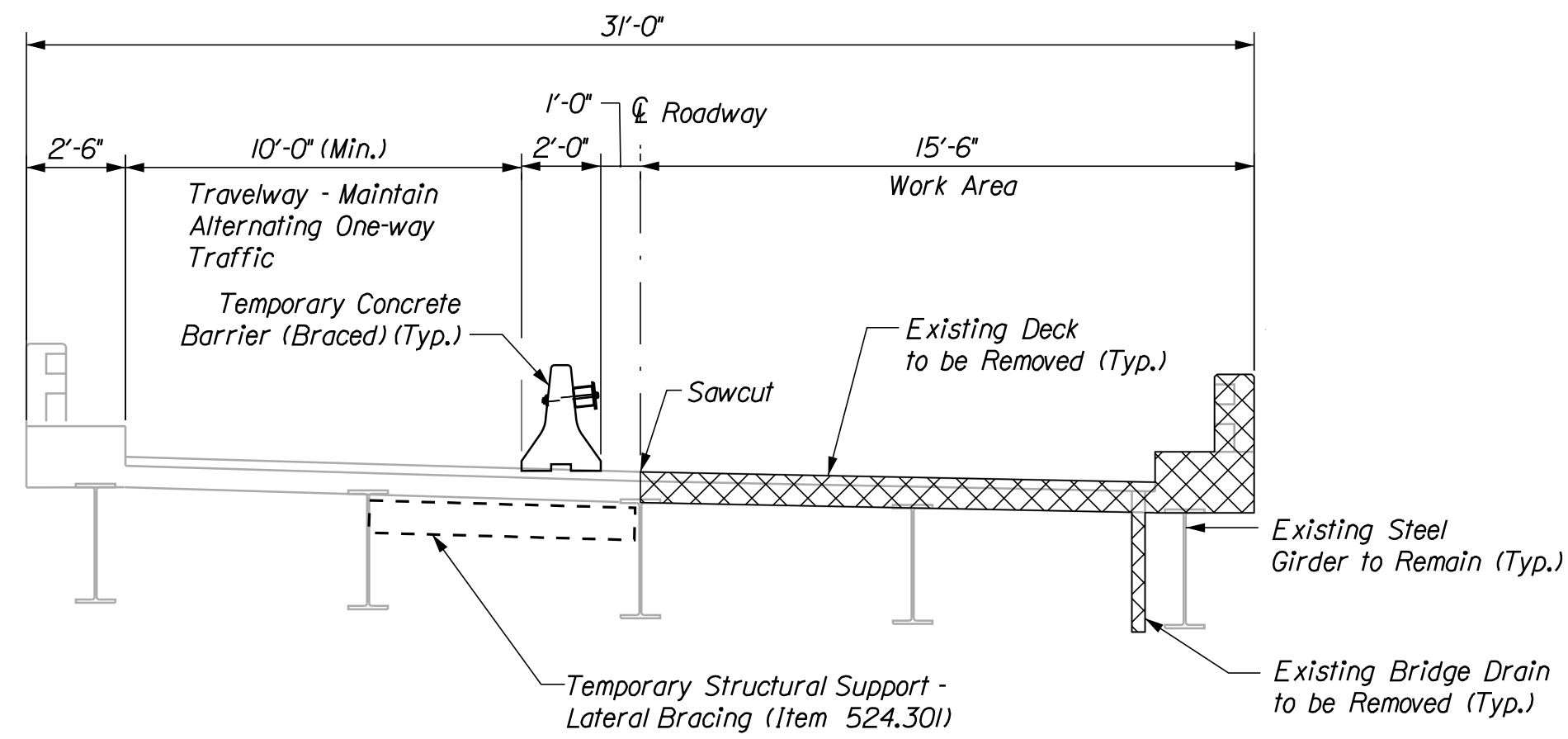
SHEET NUMBER

5

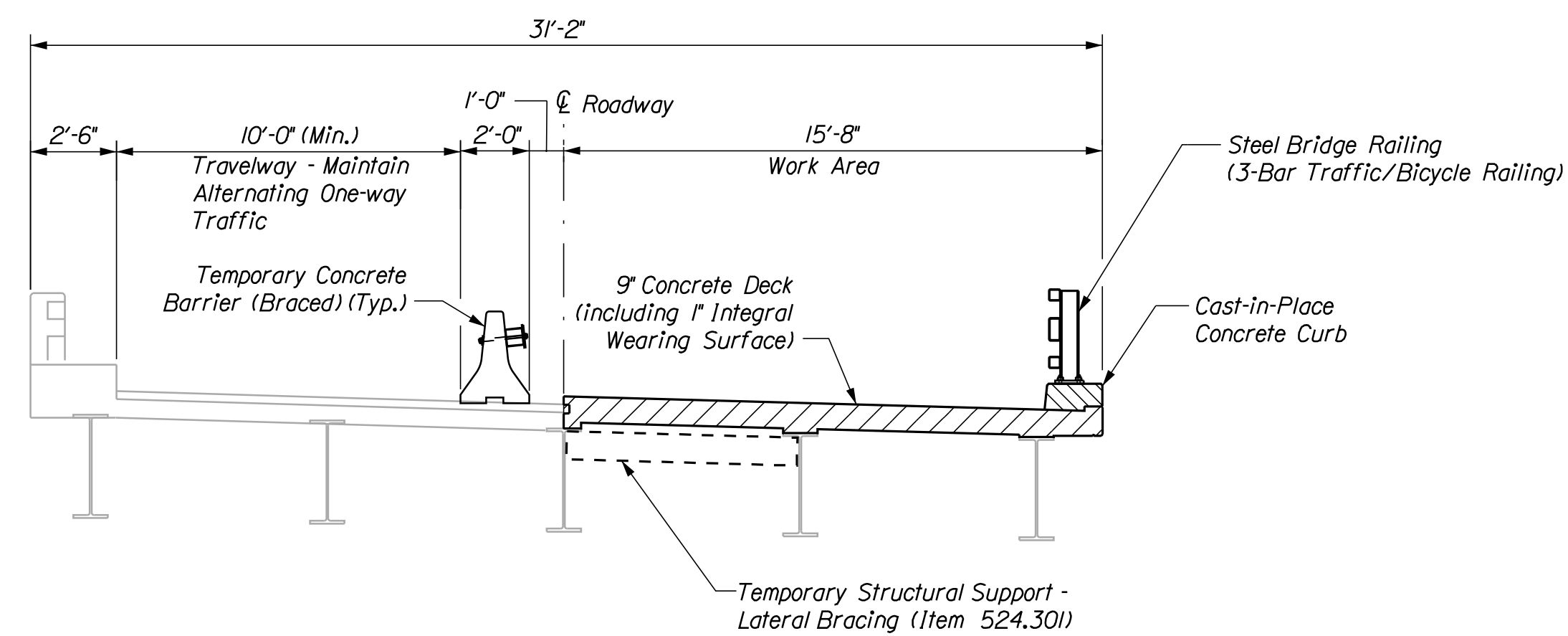
OF 26



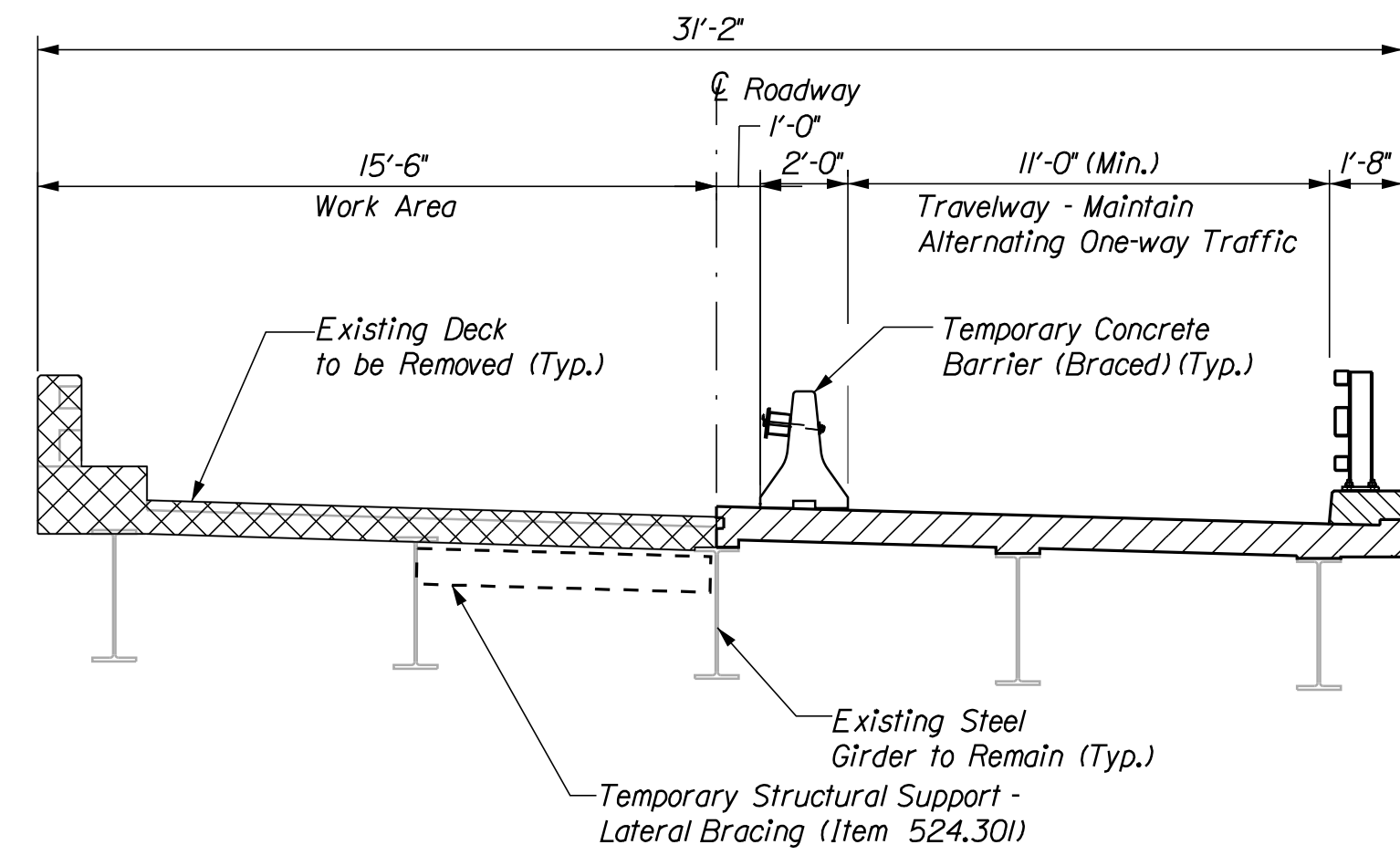
- WS | D



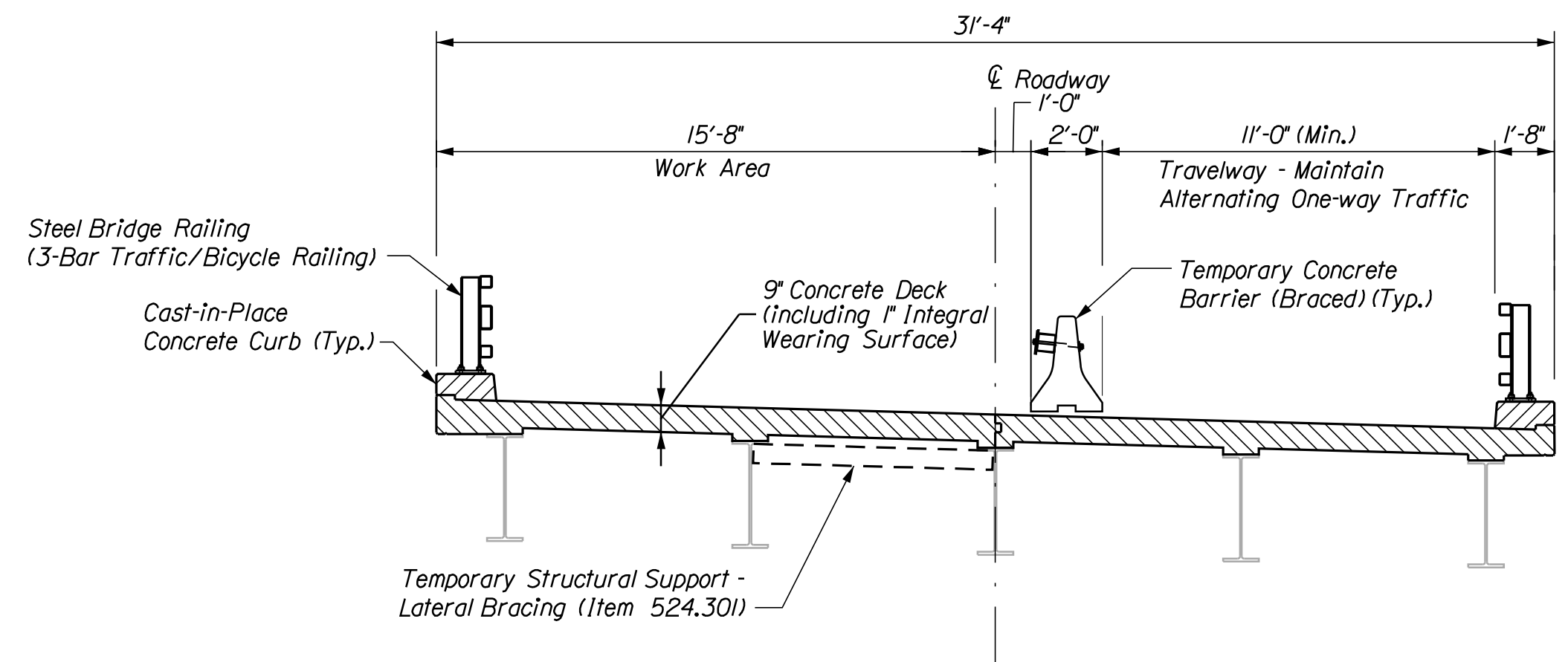
STAGE 1 DEMOLITION



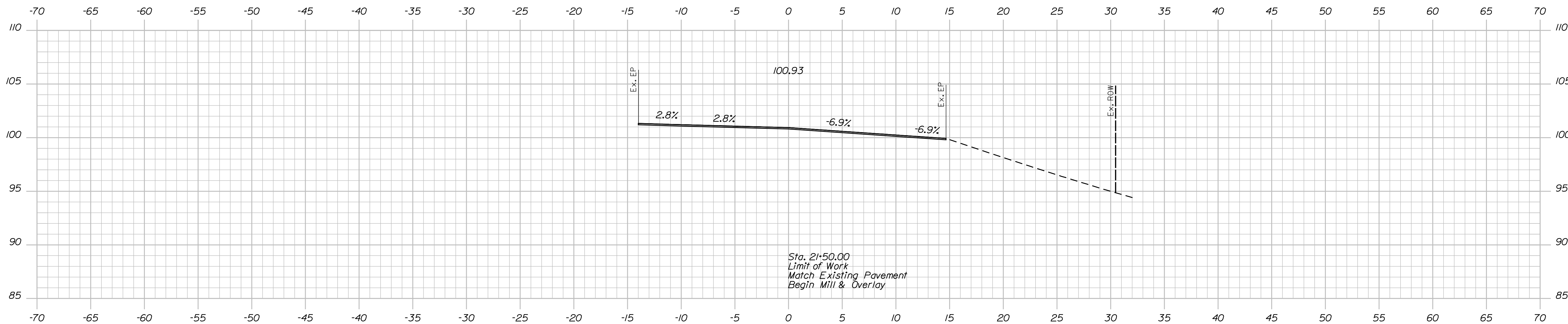
STAGE 1 CONSTRUCTION



STAGE 2 DEMOLITION



STAGE 2 CONSTRUCTION



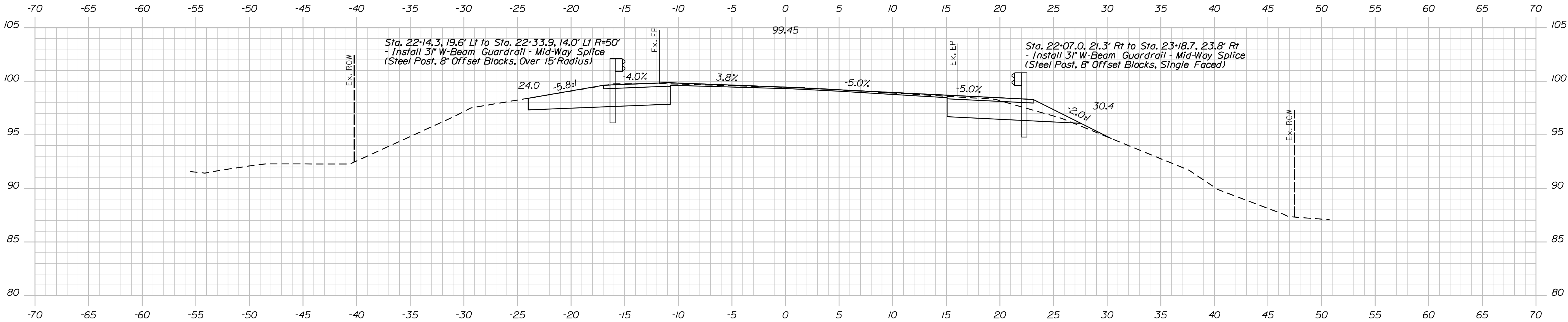
Sta. 21+50.00 to Sta. 21+75.00

Date:10/25/2023

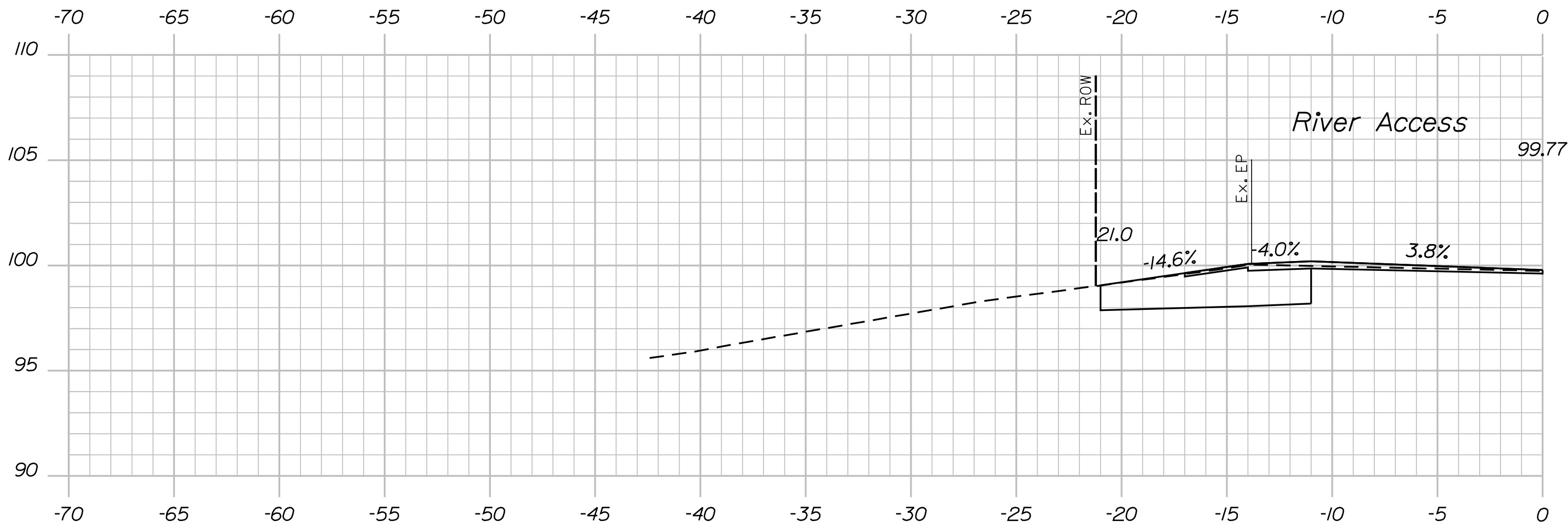
Username:

Division: BRIDGE

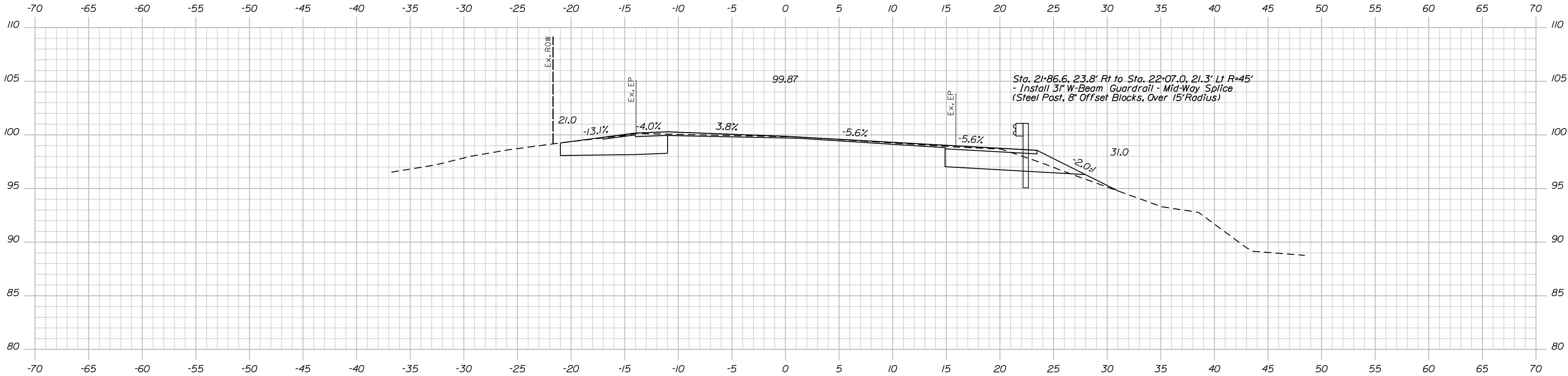
Filename: ... \BRIDGE\MSTA\009_Xsect_02.dgn



22+25.00



22+05.00



22+00.00

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2362900

BRIDGE NO. 5756
WIN
023629.00
BRIDGE PLANS

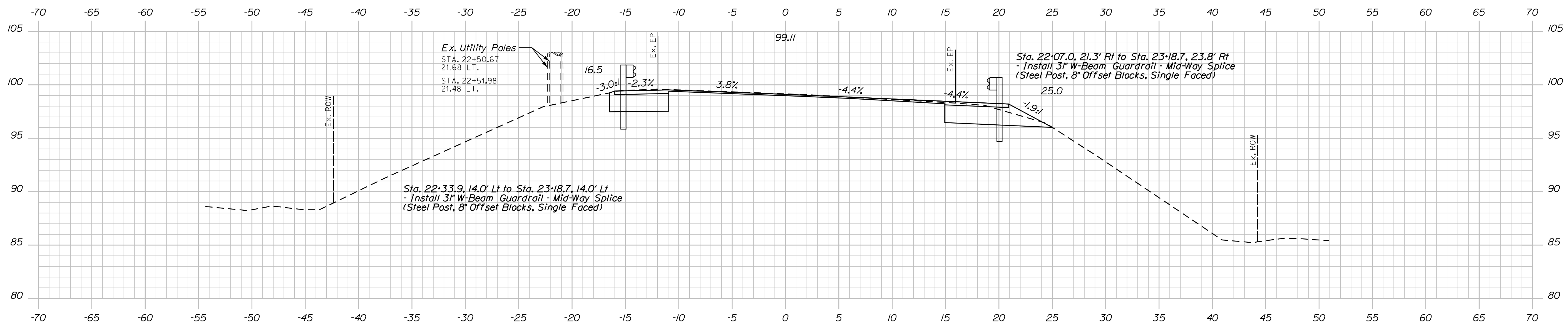
Signature: *[Signature]*
P.E. NUMBER: 12558
DATE: 10/23/2023

PROJ. MANAGER	D. EATON	BY	DATE
DESIGNED-DETAILED	J. KHERA	J. KHERA	9/2023
CHECKED-REVIEWED	T. HIGGINSON	T. HIGGINSON	9/2023
DESIGNS-DETAILED			
REVISIONS 1			
REVISIONS 2			
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			

NASH MILL BRIDGE
MOUSAM RIVER
KENNEBUNK YORK COUNTY

CROSS SECTIONS

SHEET NUMBER
9
OF 26

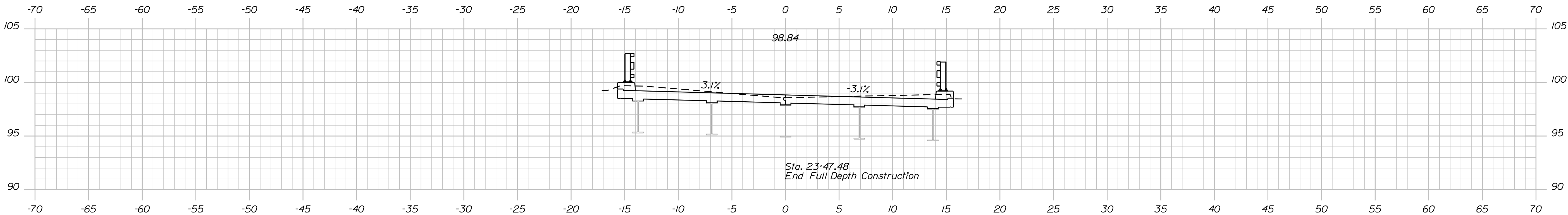


Date:10/25/2023

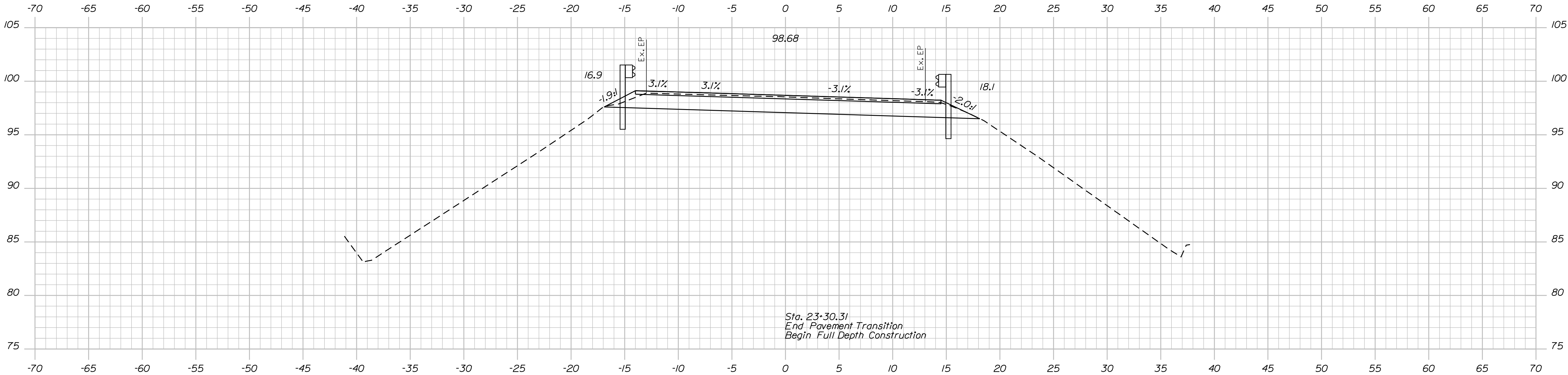
Username:

Division: BRIDGE

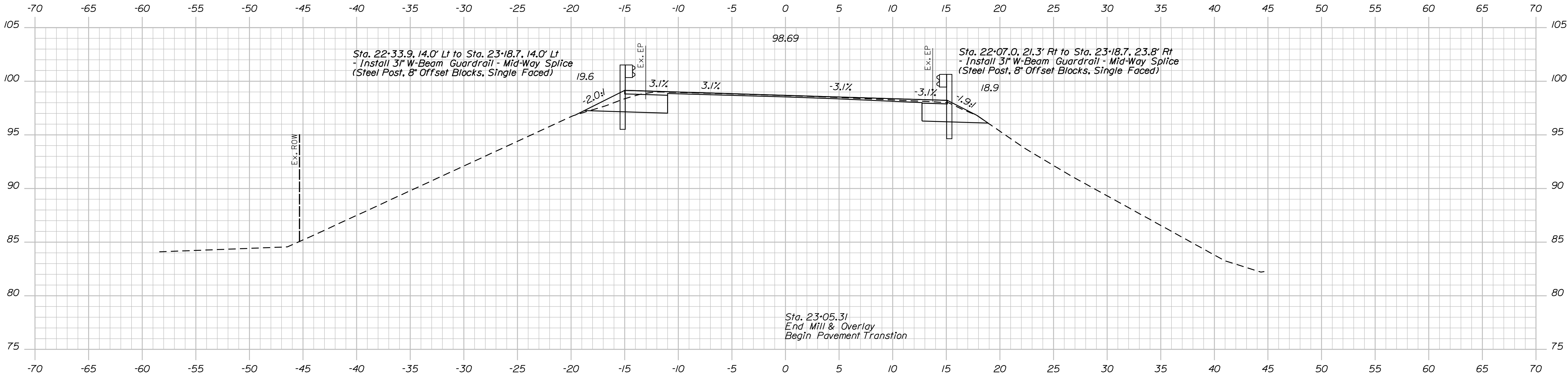
Filename: ... \BRIDGE\MSTA\011_Xsect_04.dgn



23+50.00



23+25.00



23+00.00

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2362900

WIN
023629.00
BRIDGE NO. 5756
BRIDGE PLANS

Signature: [Signature]

SIGNATURE

P.E. NUMBER

DATE

PROJ. MANAGER	D. EATON	BY	DATE
CHECKED	J. KHERA	J. KHERA	9/2023
DESIGNED	T. HIGGINS	T. HIGGINS	9/2023
REVISIONS 1			
REVISIONS 2			
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			

NASH MILL BRIDGE
MOUSAM RIVER
KENNEBUNK YORK COUNTY

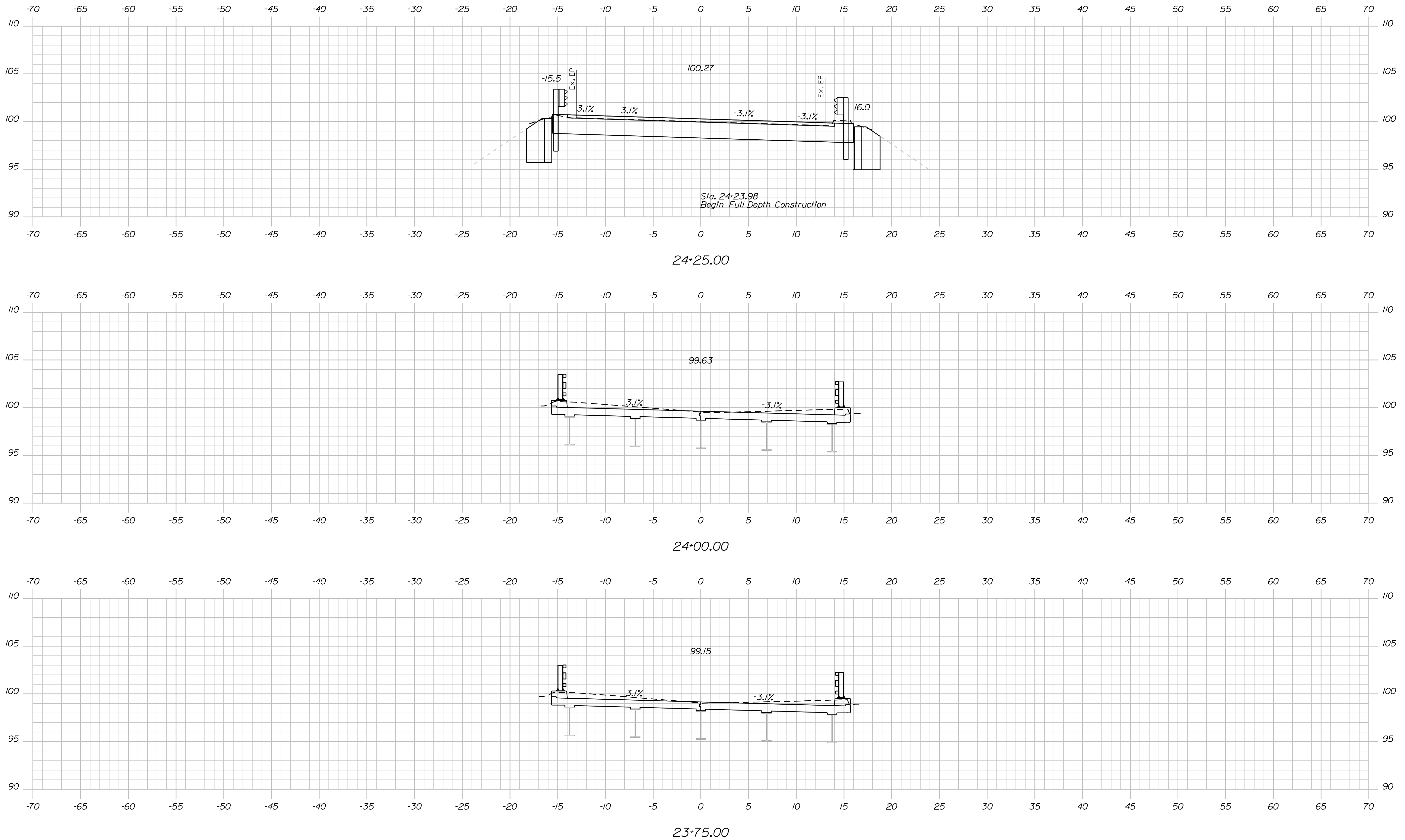
CROSS SECTIONS

SHEET NUMBER

11


OF 26

Filename: ... \BRIDGE\MSTA\012_Xsect_05.dgn



STATE OF MAINE	
DEPARTMENT OF TRANSPORTATION	
2362900	
BRIDGE NO. 5756	WIN 023629.00
BRIDGE PLANS	



	SIGNATURE
12558	P.E. NUMBER
10/23/2023	DATE

PROJ. MANAGER	D. EATON	BY	DATE
DESIGN-DETAILED	J. KHERA	J. HIGGINS	9/2023
CHECKED-REVIEWED	T. HIGGINS		9/2023
DESIGN-DETAILED2			
DESIGN-DETAILED3			
DESIGN-DETAILED3			
REVIEWS 1			
REVIEWS 2			
REVIEWS 3			
REVIEWS 4			
FIELD CHANGES			

KENNEBUNK
NASH MILL BRIDGE
MOUSAM RIVER
YORK COUNTY

CROSS SECTIONS

SHEET NUMBER

12

OF 26

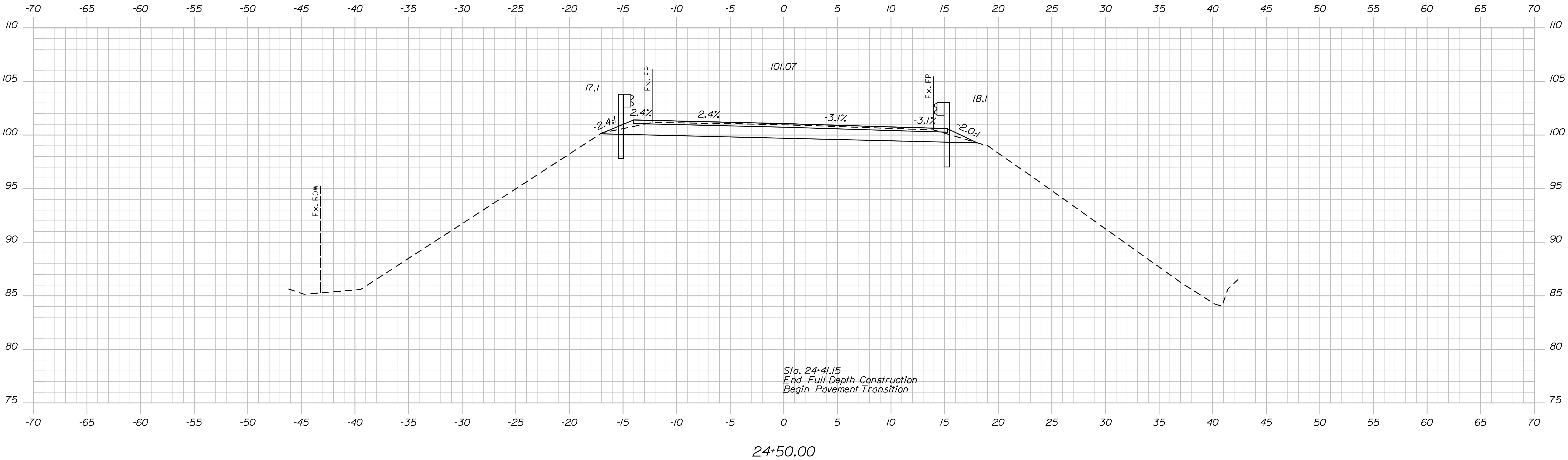
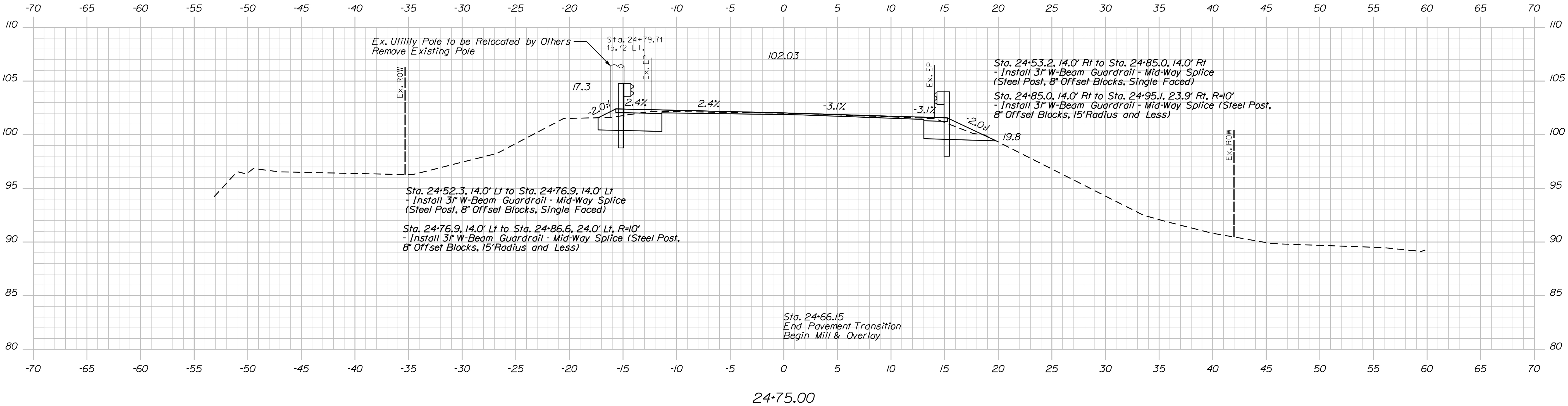
Sta. 23+75.00 to Sta. 24+25.00

Date:10/25/2023

Username:

Division: BRIDGE

Filename: ... \BRIDGE\NSTA\013_Xsect_06.dgn



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2362900

WIN
023629.00

BRIDGE NO. 5756
BRIDGE PLANS

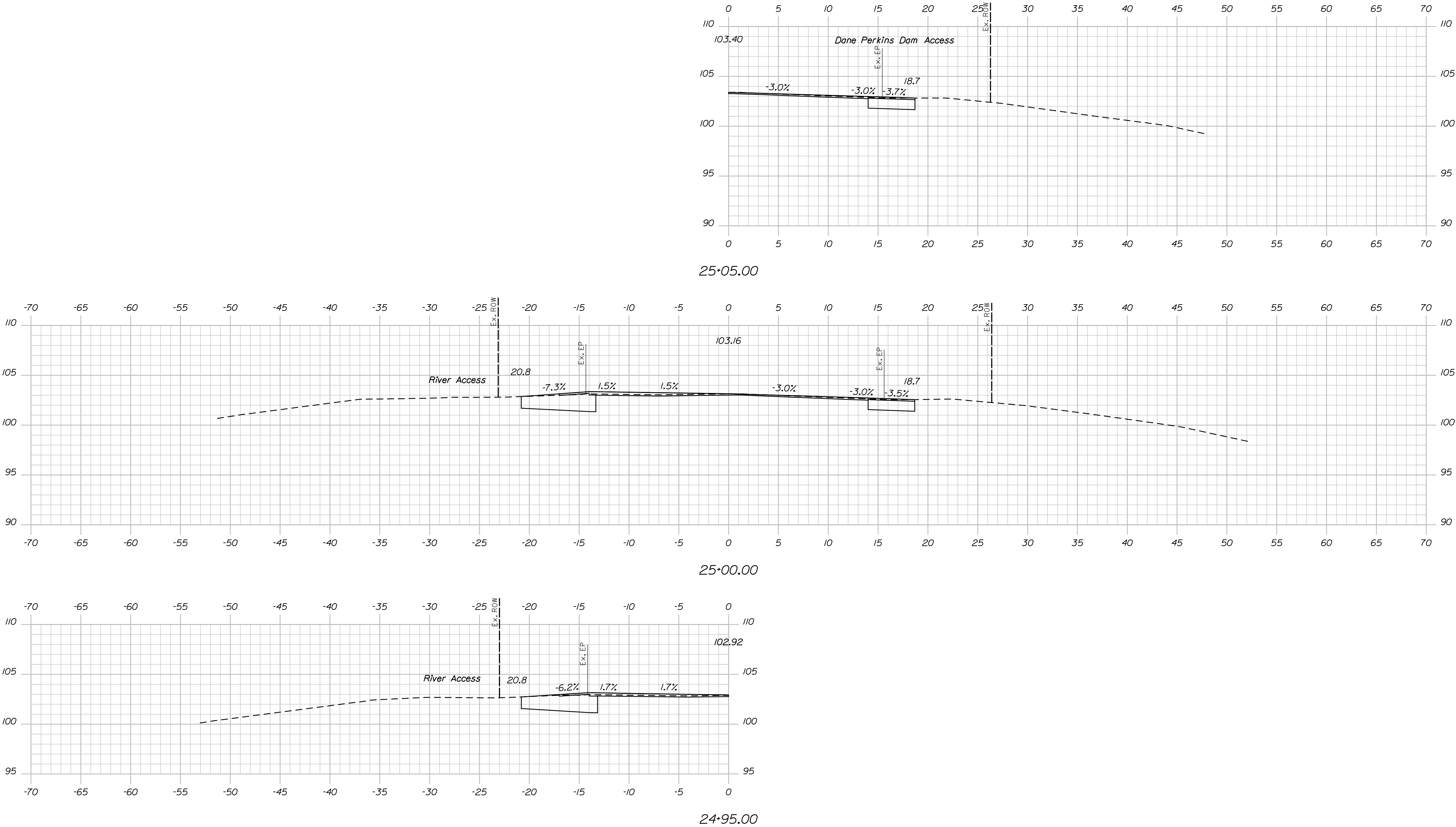
Signature: [Signature]
P.E. NUMBER: 12558
DATE: 10/23/2023

PROJ. MANAGER	D. EATON	BY	DATE
DESIGN-DETAILED	J. KHERA	J. KHERA	9/2023
CHECKED-REVIEWED	T. HOGANSON	T. HOGANSON	9/2023
DESIGNS-DETAILED			
REVISIONS 1			
REVISIONS 2			
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			

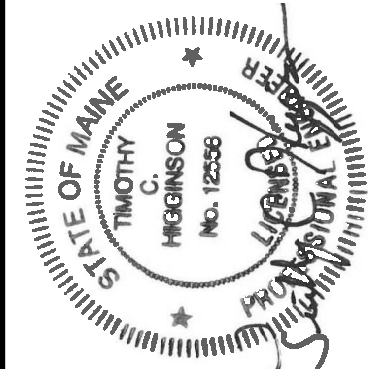
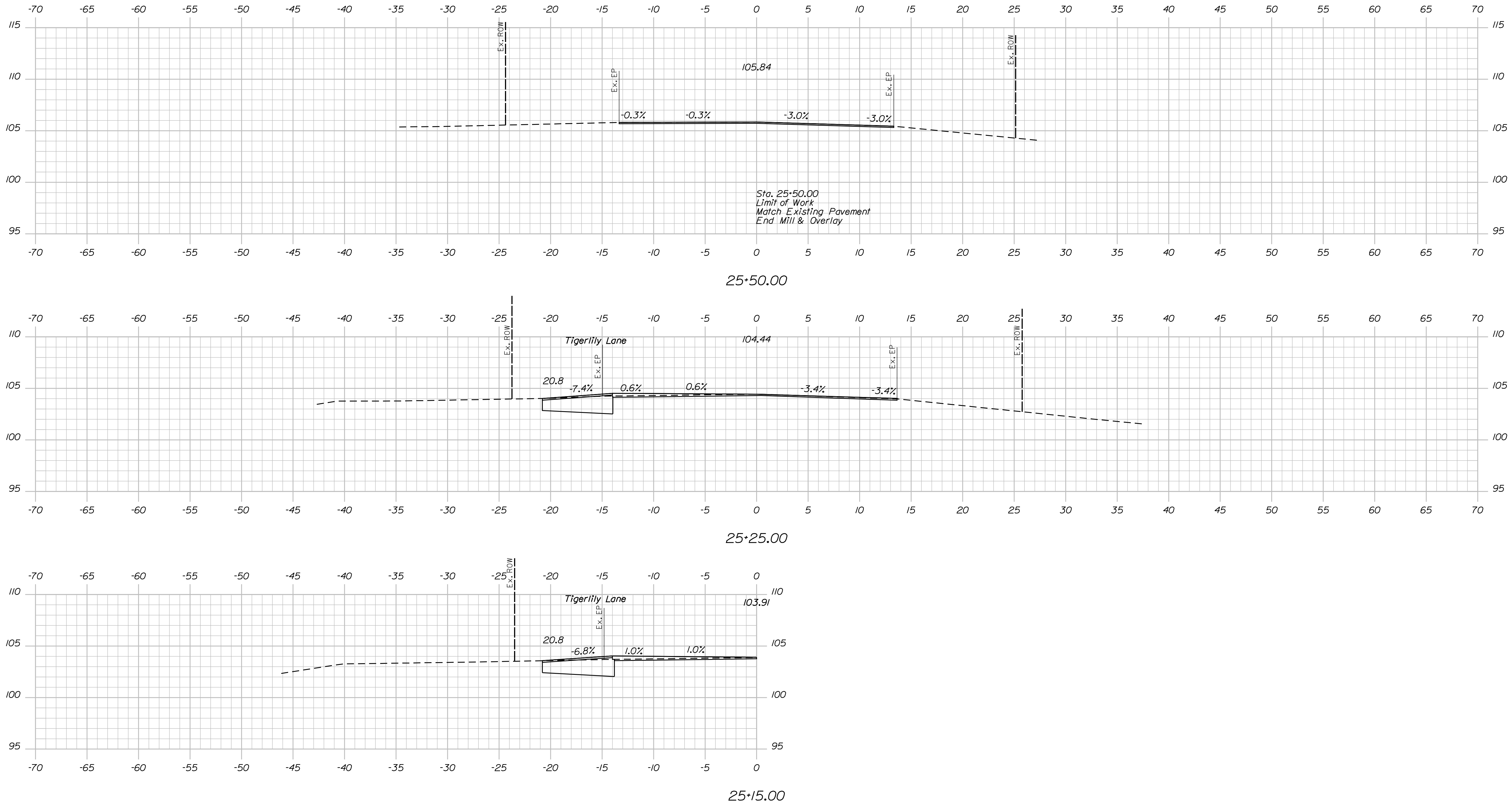
NASH MILL BRIDGE
MOUSAM RIVER
KENNEBUNK YORK COUNTY


CROSS SECTIONS

SHEET NUMBER
13
OF 26



STATE OF MAINE DEPARTMENT OF TRANSPORTATION		2362900		BRIDGE NO. 5756		WIN		023629.00		BRIDGE PLANS	
		SIGNATURE		P.E. NUMBER		DATE					
		12558		10/23/2023							
PROJ. MANAGER		BY		DATE							
DESIGN-DETAILED		J. KHERA		9/2023							
CHECKED-REVIEWED		T. HIGGINS		9/2023							
DESIGN-DETAILED											
DESIGN-DETAILED											
REVISIONS 1											
REVISIONS 2											
REVISIONS 3											
REVISIONS 4											
FIELD CHANGES											
NASH MILL BRIDGE MOUSAM RIVER KENNEBUNK YORK COUNTY		CROSS SECTIONS									
SHEET NUMBER											
14											
OF 26											




SIGNATURE
12558
P.E. NUMBER
10/23/2023
DATE

PROJ. MANAGER	D. EATON	BY	DATE
DESIGNED-TAILED	J. KHERA		9/2023
CHECKED-REVIEWED	T. HIGGINS		9/2023
DESIGNS-DET-TAILED?			
DESIGNS-TAILED?			
REVISIONS 1			
REVISIONS 2			
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			

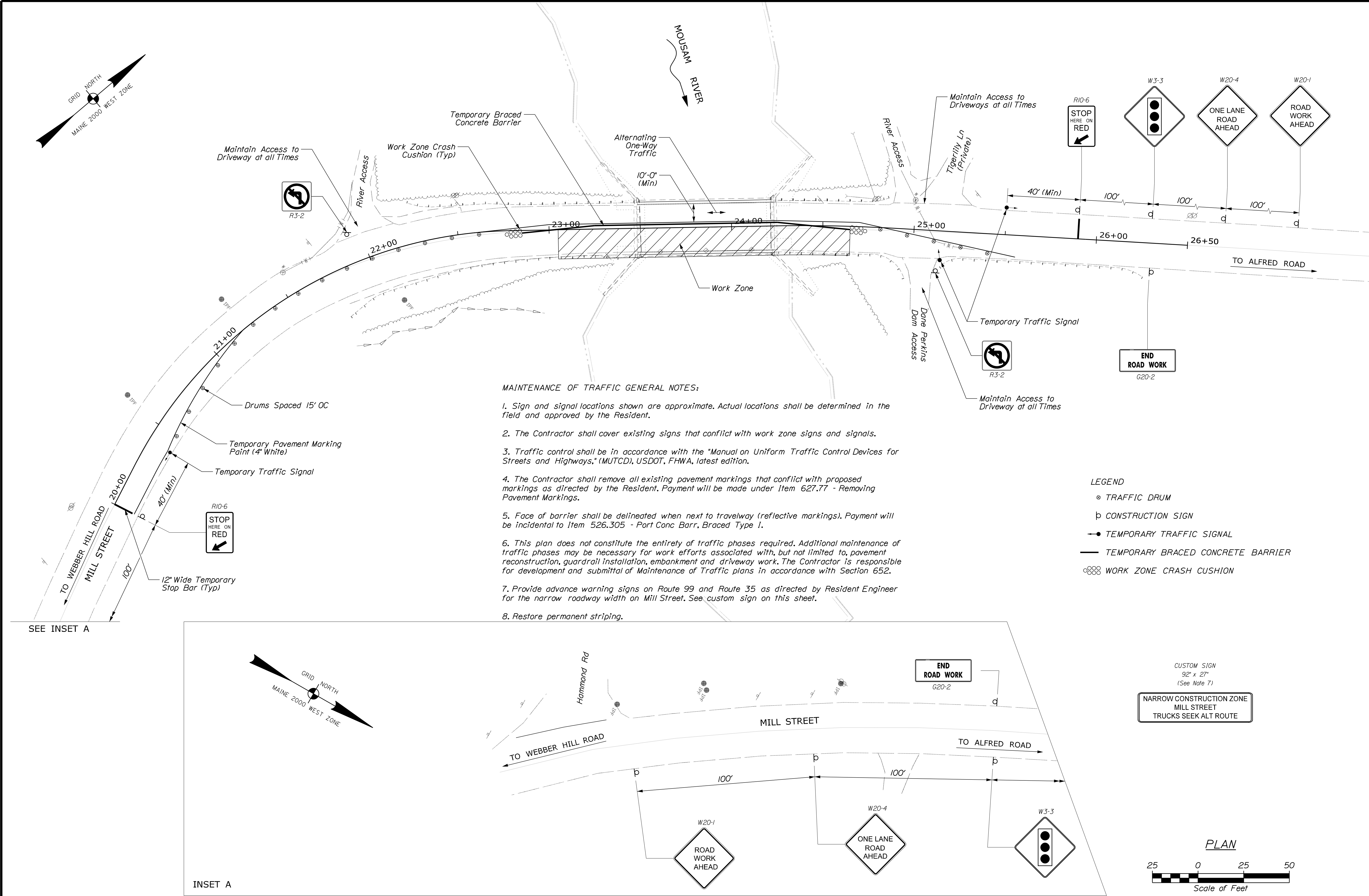
NASH MILL BRIDGE
MOUSAM RIVER
KENNEBUNK YORK COUNTY

CROSS SECTIONS

SHEET NUMBER

15

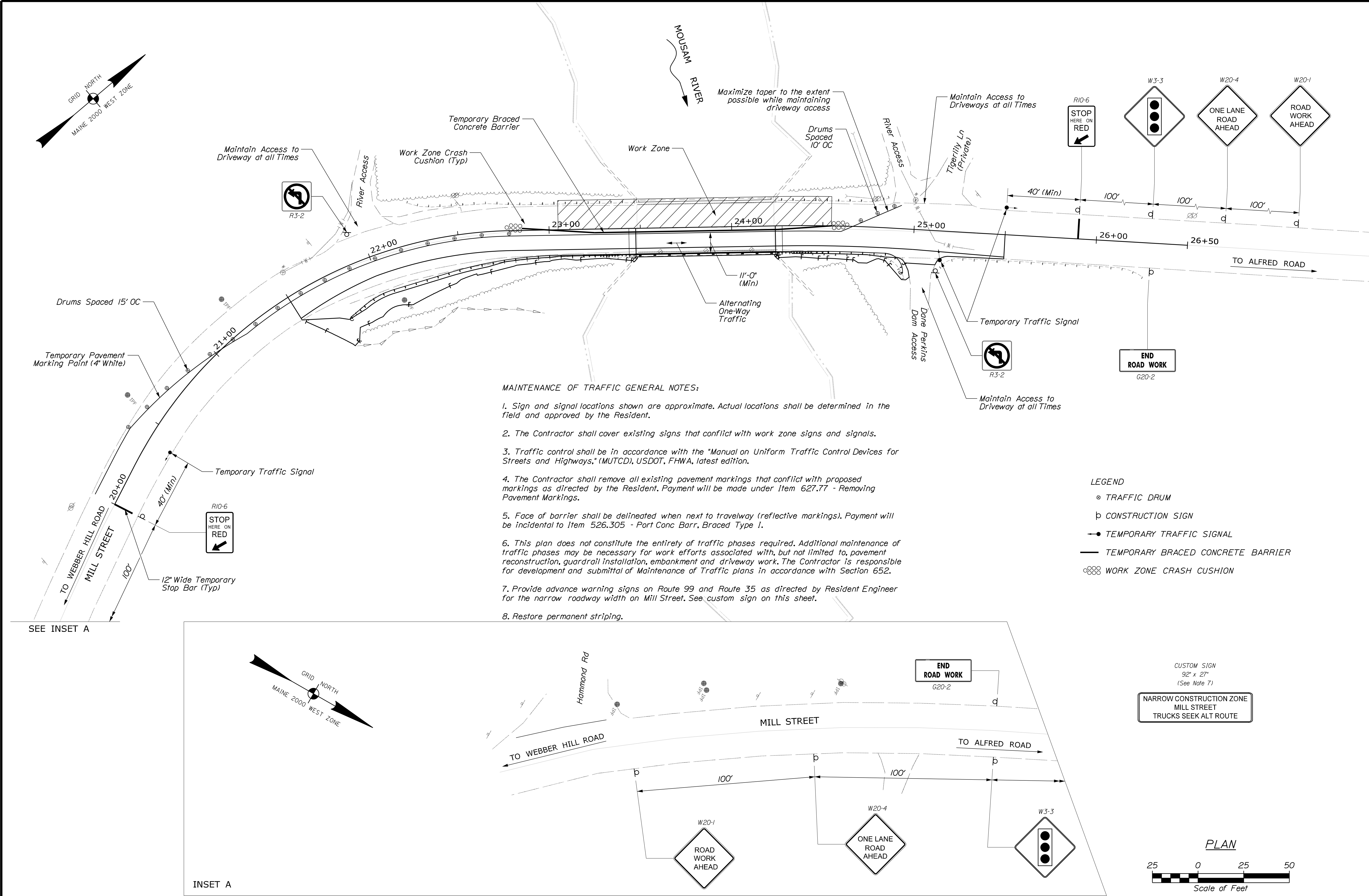
OF 26



MAINTENANCE OF TRAFFIC STAGE I



STATE OF MAINE DEPARTMENT OF TRANSPORTATION		2362900		BRIDGE NO. 5756		WIN		023629.00		BRIDGE PLANS	
NASH MILL BRIDGE MOUSAM RIVER KENNEBUNK YORK COUNTY		SIGNATURE 12558		P.E. NUMBER 10/23/2023		DATE		10/23/2023		DATE	
DESIGN-DETAILED 9/2023		DESIGN-REVIEWED 9/2023		DESIGN-DETAILED 9/2023		DESIGN-REVIEWED 9/2023		DESIGN-DETAILED 9/2023		DESIGN-REVIEWED 9/2023	
PROJ. MANAGER D. EATON		BY C. CARNEY		DATE 9/2023		DATE 9/2023		DATE 9/2023		DATE 9/2023	
SHEET NUMBER		16		OF 26							

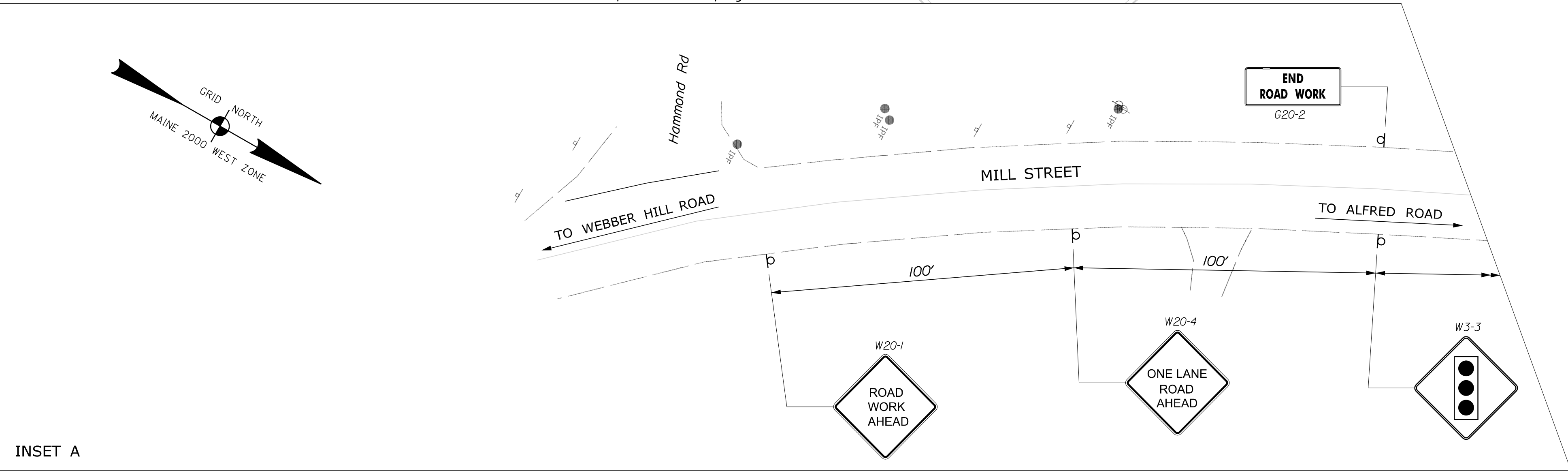


MAINTENANCE OF TRAFFIC GENERAL NOTES:

1. Sign and signal locations shown are approximate. Actual locations shall be determined in the field and approved by the Resident.
2. The Contractor shall cover existing signs that conflict with work zone signs and signals.
3. Traffic control shall be in accordance with the "Manual on Uniform Traffic Control Devices for Streets and Highways," (MUTCD), USDOT, FHWA, latest edition.
4. The Contractor shall remove all existing pavement markings that conflict with proposed markings as directed by the Resident. Payment will be made under Item 627.77 - Removing Pavement Markings.
5. Face of barrier shall be delineated when next to travelway (reflective markings). Payment will be incidental to Item 526.305 - Port Conc Barr, Braced Type I.
6. This plan does not constitute the entirety of traffic phases required. Additional maintenance of traffic phases may be necessary for work efforts associated with, but not limited to, pavement reconstruction, guardrail installation, embankment and driveway work. The Contractor is responsible for development and submittal of Maintenance of Traffic plans in accordance with Section 652.
7. Provide advance warning signs on Route 99 and Route 35 as directed by Resident Engineer for the narrow roadway width on Mill Street. See custom sign on this sheet.
8. Restore permanent striping.

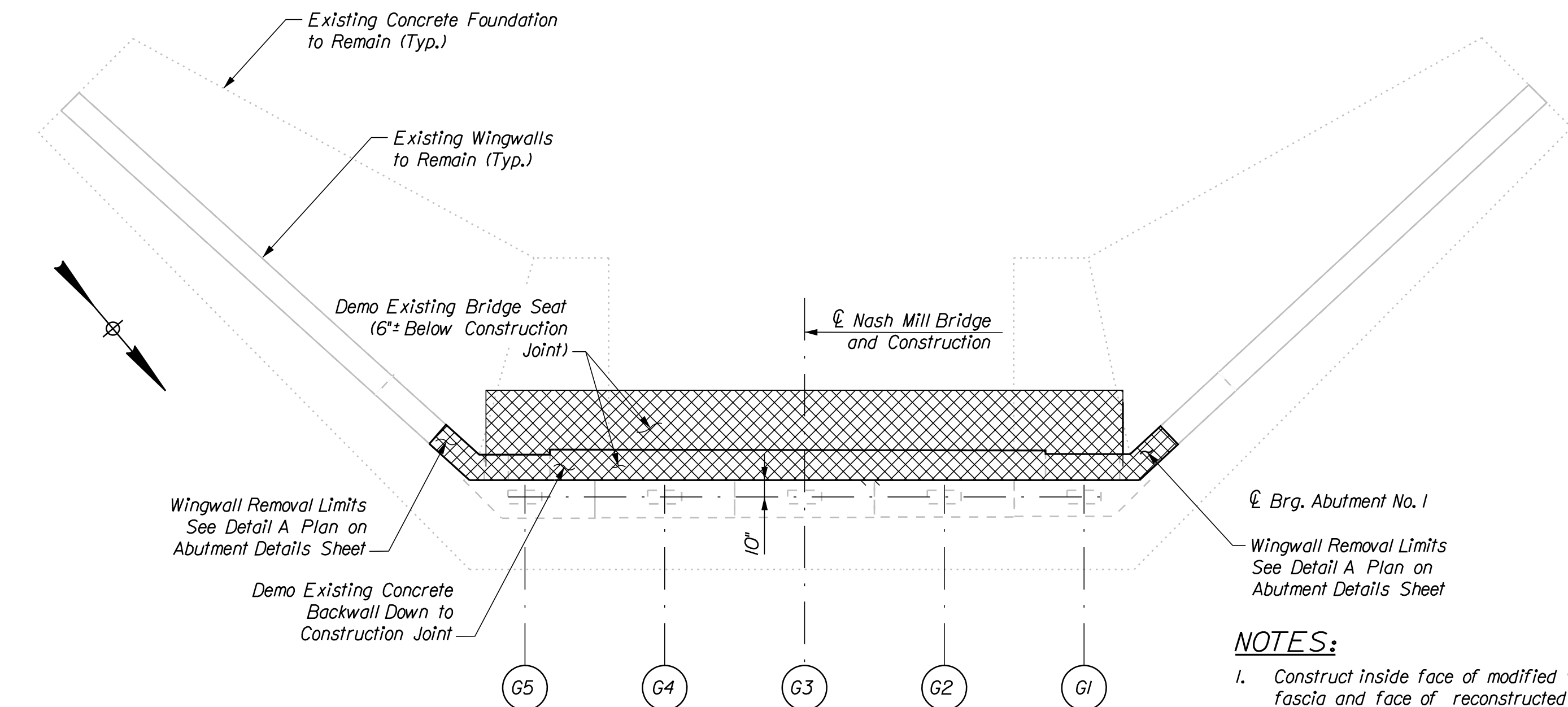
LEGEND

- ⊗ TRAFFIC DRUM
- p CONSTRUCTION SIGN
- TEMPORARY TRAFFIC SIGNAL
- TEMPORARY BRACED CONCRETE BARRIER
- ⊗⊗⊗ WORK ZONE CRASH CUSHION

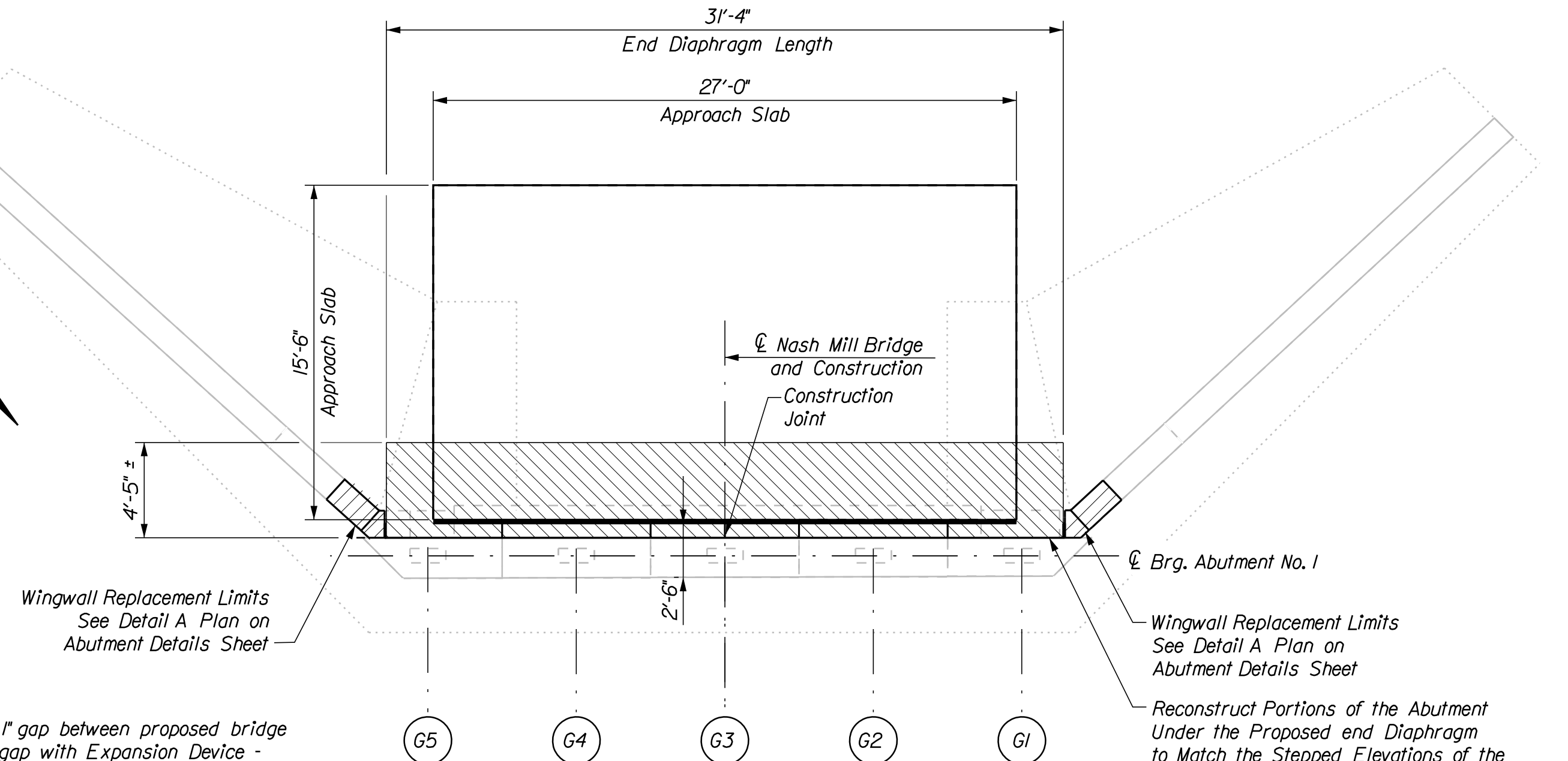


MAINTENANCE OF TRAFFIC STAGE 2

STATE OF MAINE DEPARTMENT OF TRANSPORTATION		2362900		WIN		023629.00		BRIDGE NO. 5756		BRIDGE PLANS	
NASH MILL BRIDGE MOUSAM RIVER KENNEBUNK YORK COUNTY		SIGNATURE 12558		P.E. NUMBER 10/23/2023		DATE		17		OF 26	
MAINTENANCE OF TRAFFIC		SHEET NUMBER		17		OF 26					



ABUTMENT No. 1 DEMOLITION PLAN



ABUTMENT No. 1 CONSTRUCTION PLAN

NOTES:

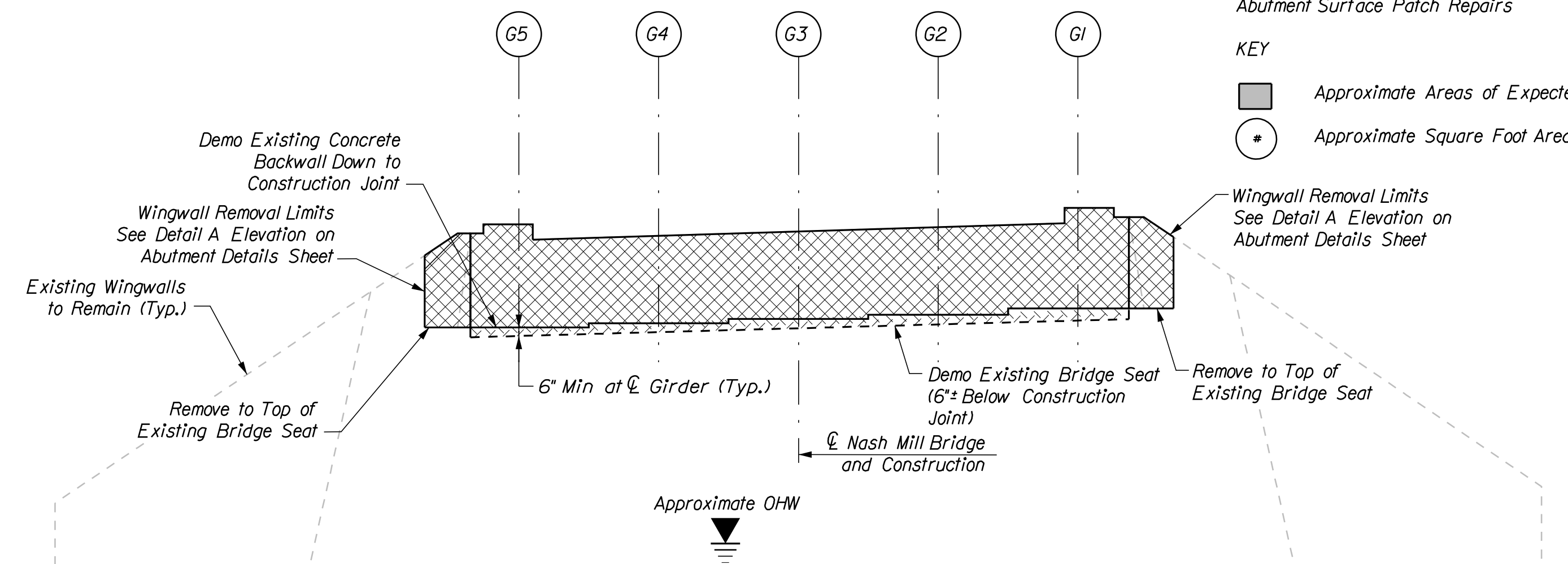
1. Construct inside face of modified wingwall to leave 1" gap between proposed bridge fascia and face of reconstructed wingwall. Seal 1" gap with Expansion Device - Silicone Coated Precompressed Foam.
2. The estimated abutment repair quantities are based on the existing Highway Bridge Inspection Report. If differences are experienced, the Resident shall be notified and quantities shall be adjusted accordingly.
3. Areas identified by visual inspection requiring concrete repair are shown. Abutments shall be inspected jointly by the Contractor and Resident. Deteriorated concrete shall be removed to sound concrete and repaired utilizing Item(s) 518.60 and/or 518.80 as directed by the Resident. Actual repair areas will be determined and agreed on by the Contractor and the Resident during construction.

ESTIMATED REPAIR QUANTITIES (SF)

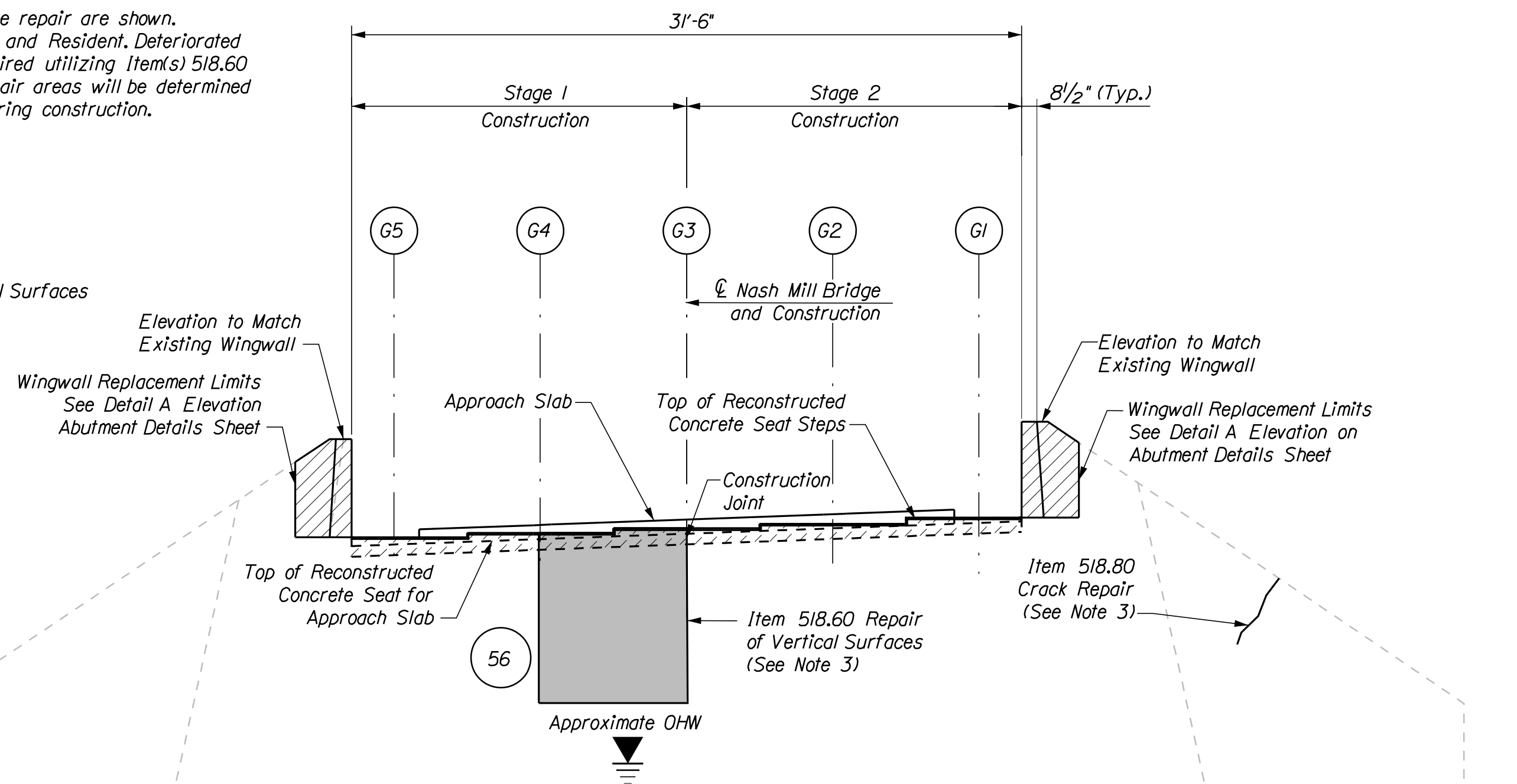
Abutment Surface Patch Repairs 56

KEY

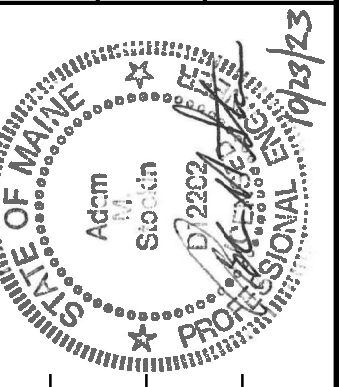
- Approximate Areas of Expected Repair of Vertical Surfaces
- Approximate Square Foot Area of Repair



ABUTMENT No. 1 DEMOLITION ELEVATION



ABUTMENT No. 1 CONSTRUCTION ELEVATION



SIGNATURE
12202
P.E. NUMBER
10/23/2023
DATE

PROJ. MANAGER	DESIGN-DETAILED	CHECKED	REVIEWED	DATE	BY
D. EATON	N. GOMEZ	W. GERHOLD	T. POLSON	9/20/23	9/20/23
DESIGN-DETAILED	DESIGN-DETAILED	DESIGN-DETAILED	DESIGN-DETAILED		
REVISIONS 1	REVISIONS 2	REVISIONS 3	REVISIONS 4		
FIELD CHANGES					

NASH MILL BRIDGE
MOUSAM RIVER
KENNEBUNK YORK COUNTY
ABUTMENT 1 MODIFICATIONS

SHEET NUMBER

18

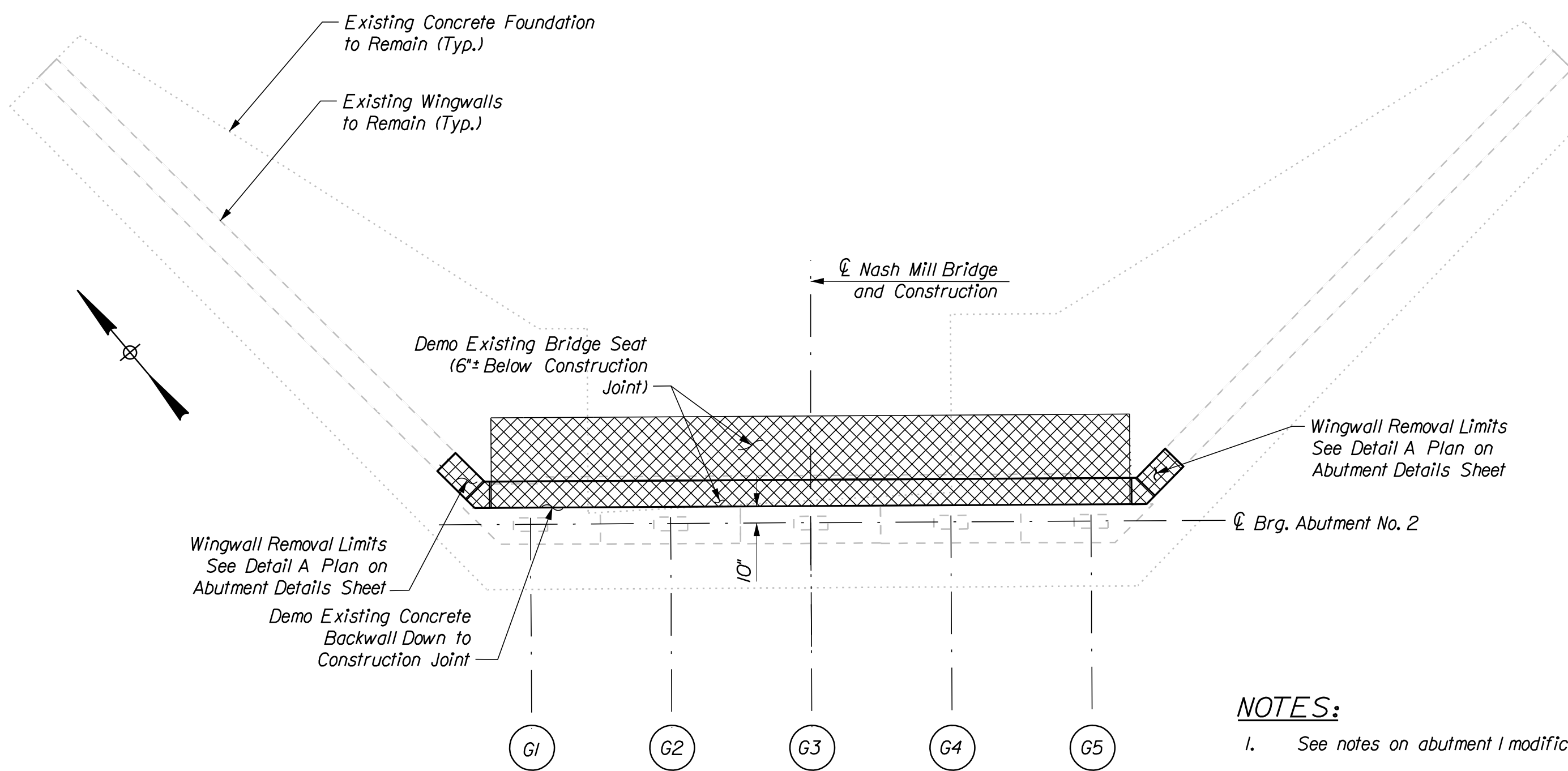
OF 26

Date:10/25/2023

Username:

Division: BRIDGE

Filename: ... \MSTAD019_Abument_2Mod.dgn



ABUTMENT No. 2 DEMOLITION PLAN

NOTES:

1. See notes on abutment 1 modification sheet.

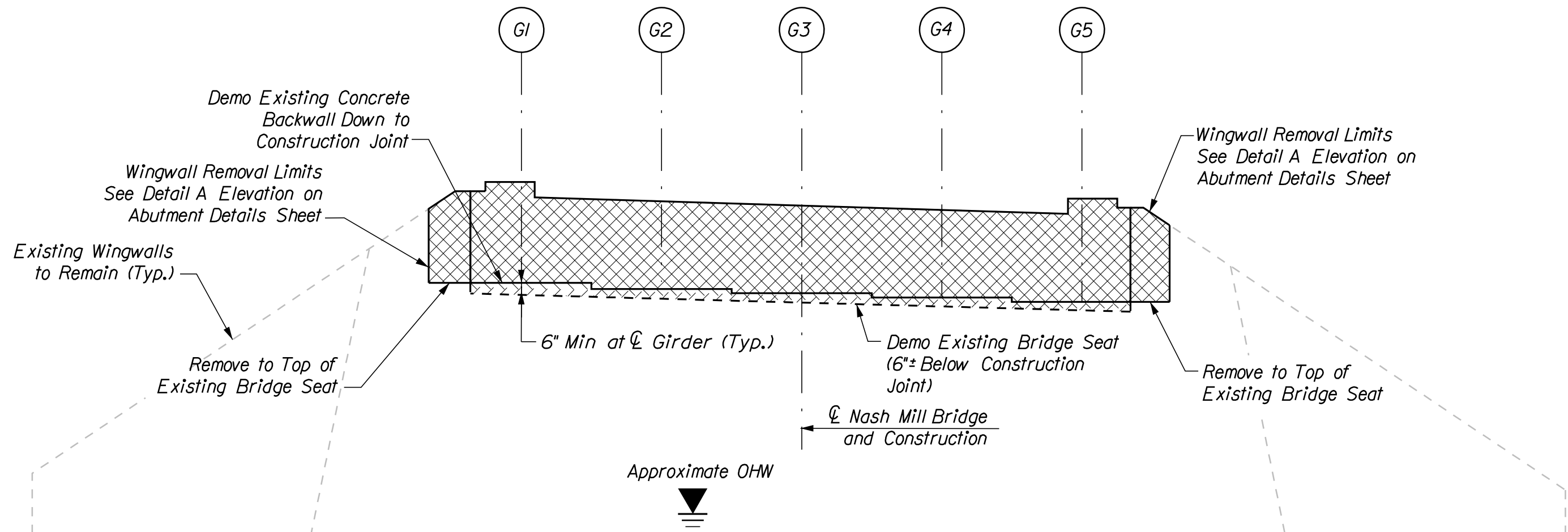
Estimated Repair Quantities (SF)

Abutment Surface Patch Repairs 96

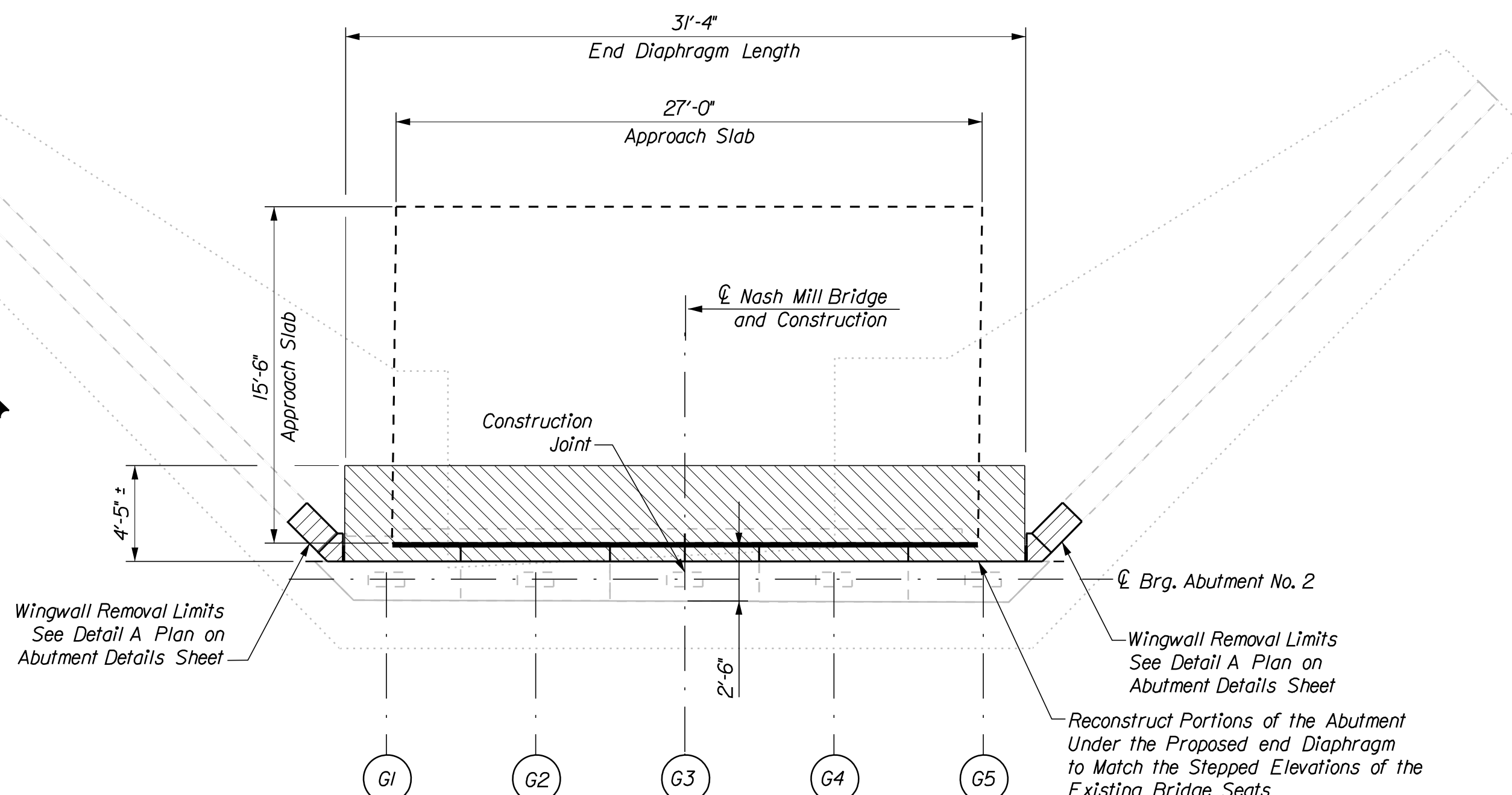
KEY

Approximate Areas of Expected Repair of Vertical Surfaces

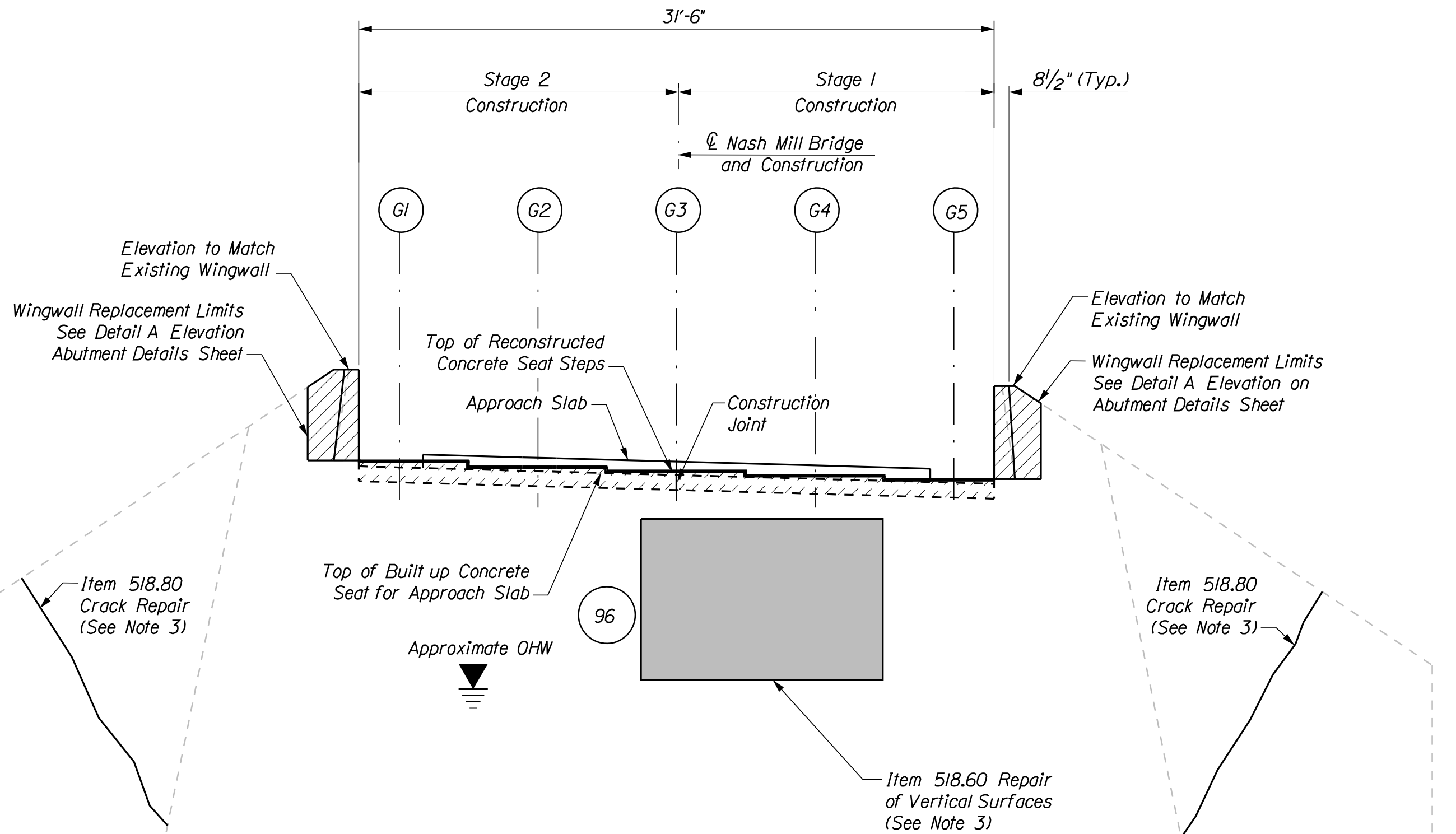
Approximate Square Foot Area of Repair



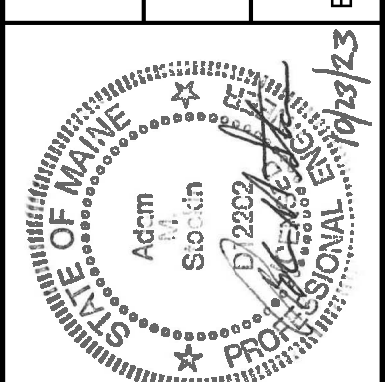
ABUTMENT No. 2 DEMOLITION ELEVATION



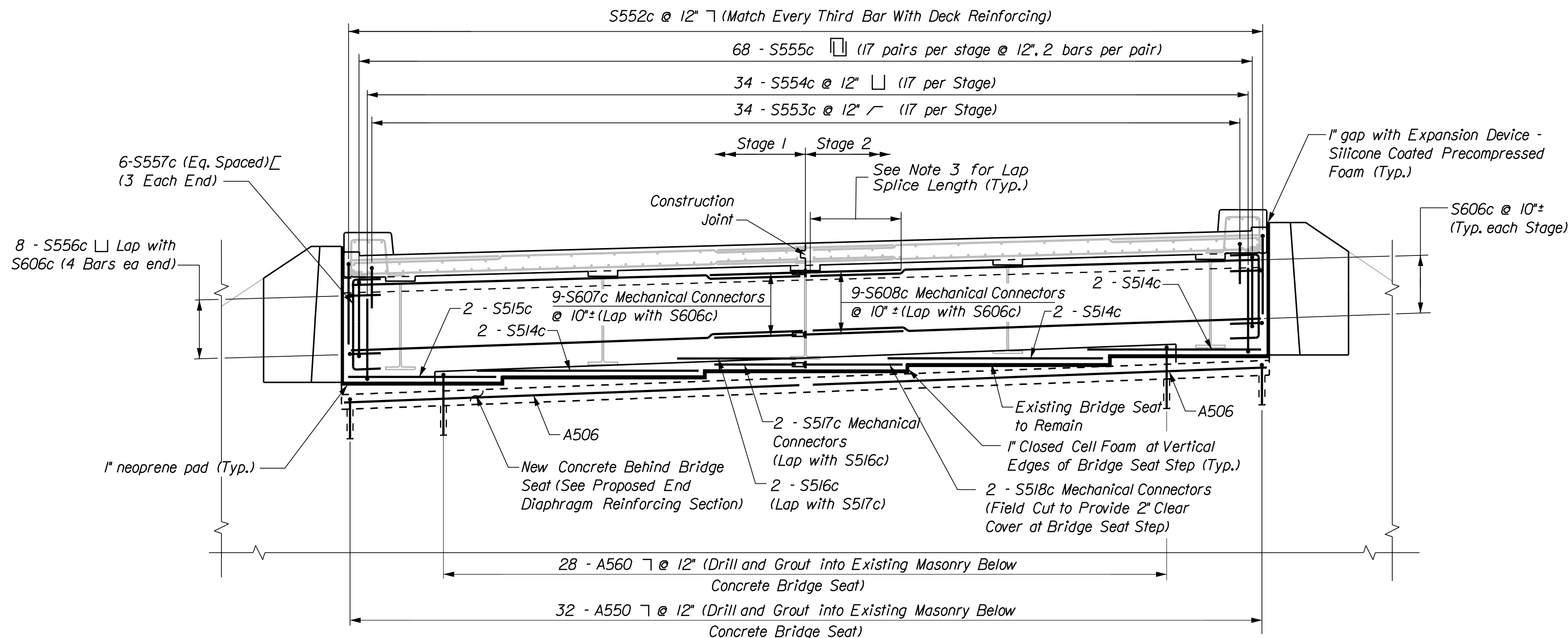
ABUTMENT No. 2 CONSTRUCTION PLAN



ABUTMENT No. 2 CONSTRUCTION ELEVATION



PROJ. MANAGER	DESIGN-DETAILED	CHECKED-REVIEWED	DESIGN-DETAILED	DESIGN-DETAILED	REVISIONS 1	REVISIONS 2	REVISIONS 3	REVISIONS 4	FIELD CHANGES
D. EATON	N. GOMEZ	T. POLSON	T. POLSON						
BY	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE
W. GERHOLD	9/20/23	9/20/23							
T. POLSON	9/20/23								
SIGNATURE	12202	P.E. NUMBER	10/23/2023	DATE					

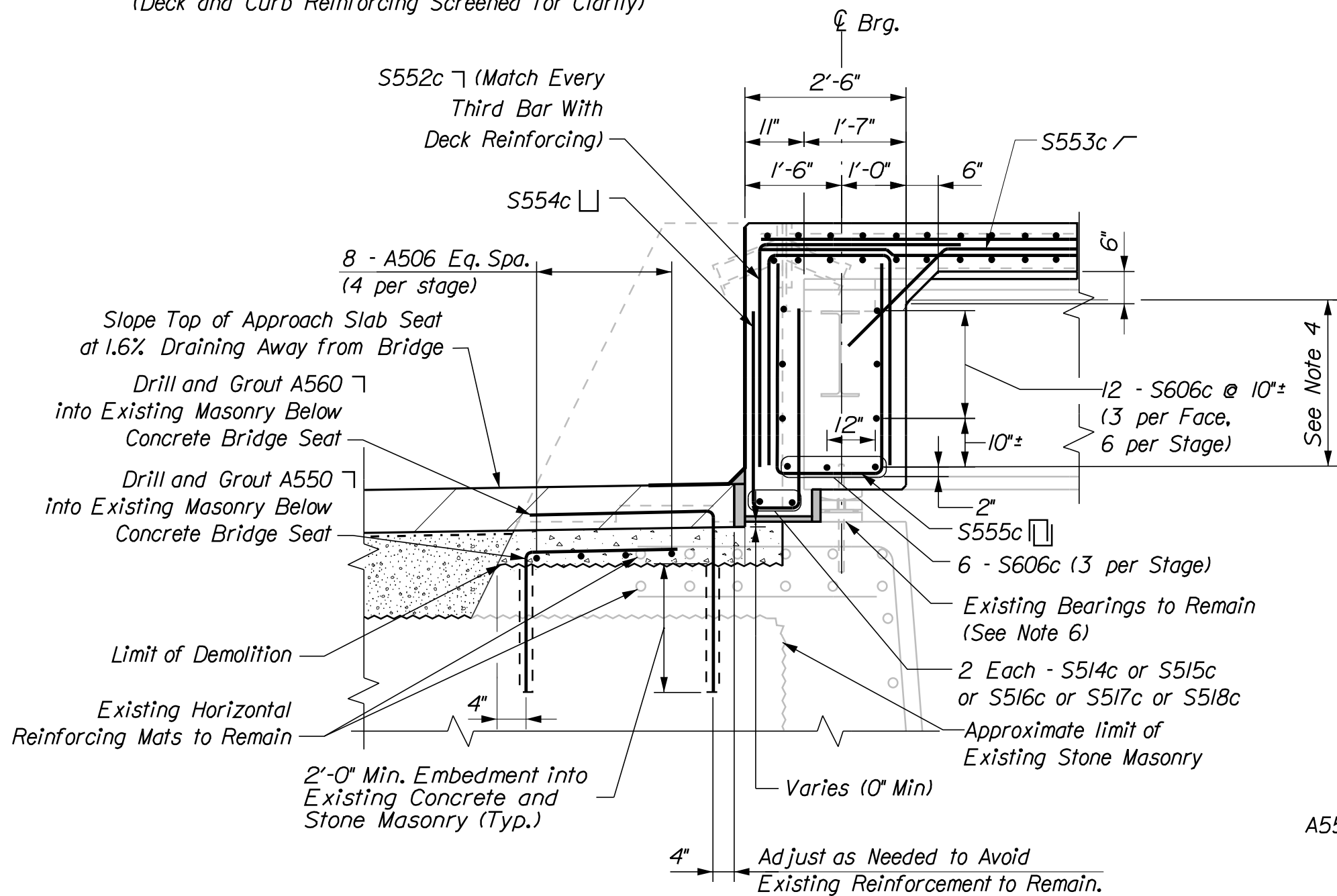


ABUTMENT AND END DIAPHRAGM REINFORCING ELEVATION

(Abutment No. 1 shown, Abutment No. 2 similar but opposite)
(Deck and Curb Reinforcing Screened for Clarity)

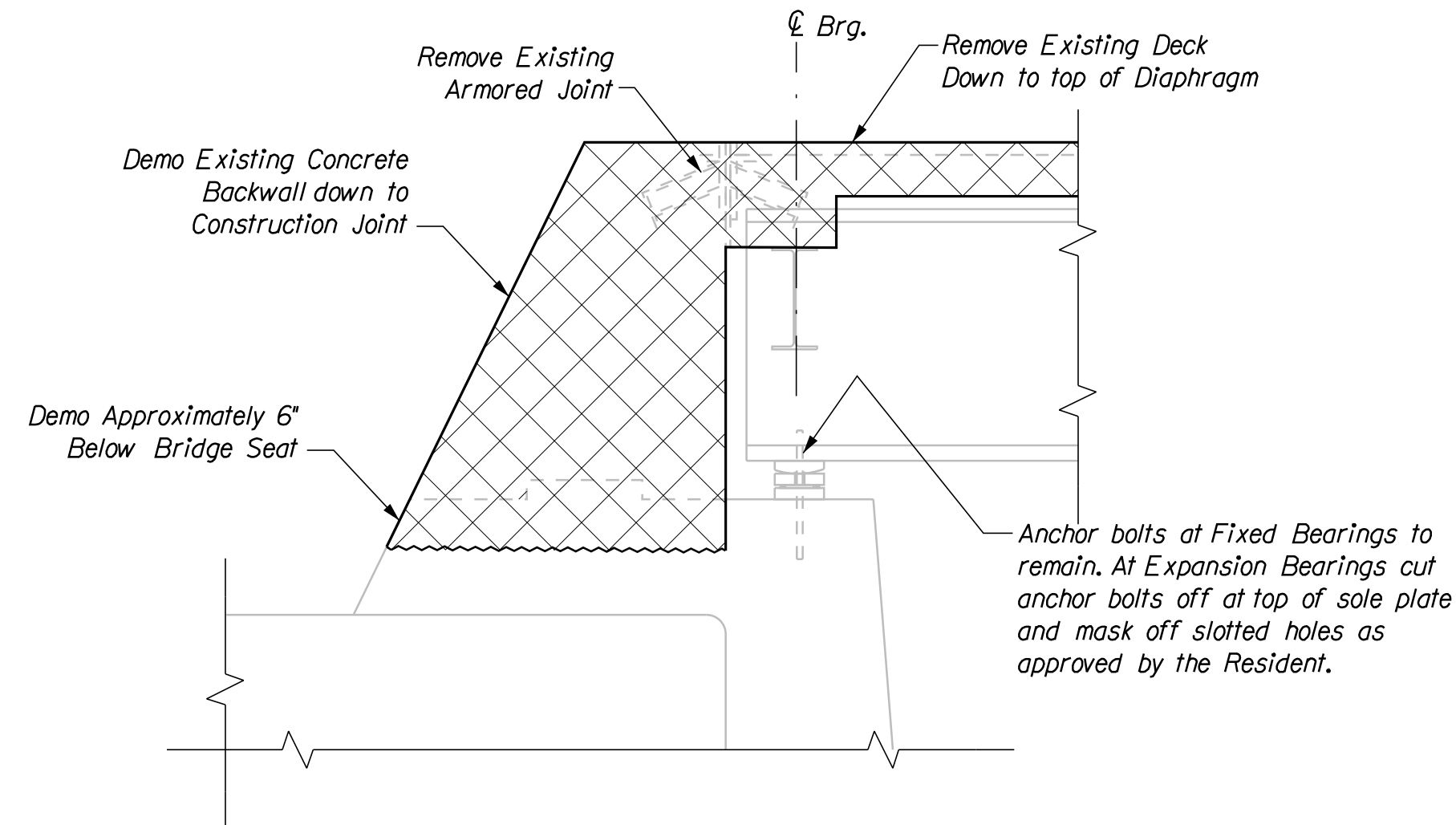
NOTES:

- Neoprene pads shall be either polychloroprene or natural polyisoprene of 50+5 Shore A durometer hardness, and shall conform to the requirements of Section 18.2 of the LRFD Bridge Construction Specifications, Third Edition.
- Neoprene pads, closed cell foam, mortared chamfer and preformed expansion joint filler will not be paid for directly but will be incidental to related contract items.
- Minimum lap splice length shall be:
2'-7" for #5 bars
3'-1" for #6 bars
- Drill (5) 1 3/4" diameter holes in the Girder 3 web and (5) 1" diameter holes in each of the remaining girder webs to accommodate S607c & S608c Mechanical Connectors and S606c bars.
- Existing bearings shown schematically. Existing bearings to remain.
- NF = Near Face
FF = Far Face
- Abutment No. 1 reinforcement detailed with bar mark "A". Abutment No. 2 reinforcement similar but with bar mark "B".
- The Contractor shall use care not to damage the existing reinforcing steel which is to remain. Any damaged reinforcing steel shall be replaced as directed by the Resident at no expense to the Department.
- The Contractor shall locate, by non-destructive methods, reinforcing steel in the existing concrete before drilling and grouting new reinforcing steel and anchor rods. All costs associated with this work shall be incidental to related contract items.
- Reinforcing steel shall have 2 inches cover unless otherwise noted.
- Existing concrete abutments and wingwalls to be removed as shown on the plans shall be sawcut one inch deep prior to removing existing concrete. All costs associated with this work shall be incidental to related contract items.
- Where drilling and anchoring of reinforcement is specified, the Contractor shall use a material listed on the Maine Department of Transportation Qualified Product List of Concrete Adhesive Anchor Systems. Proposed anchoring material and embedment depth shall be submitted for approval. No separate payment will be made for drilling and anchoring of reinforcing steel, but shall be incidental to the related Contract pay item.
- All dimensions based on or related to the existing bridge shall be verified in the field by the Contractor.
- All exposed edges of concrete shall have a 3/4 inch chamfer unless otherwise noted. Chamfers for wingwall modifications shall match existing chamfers.

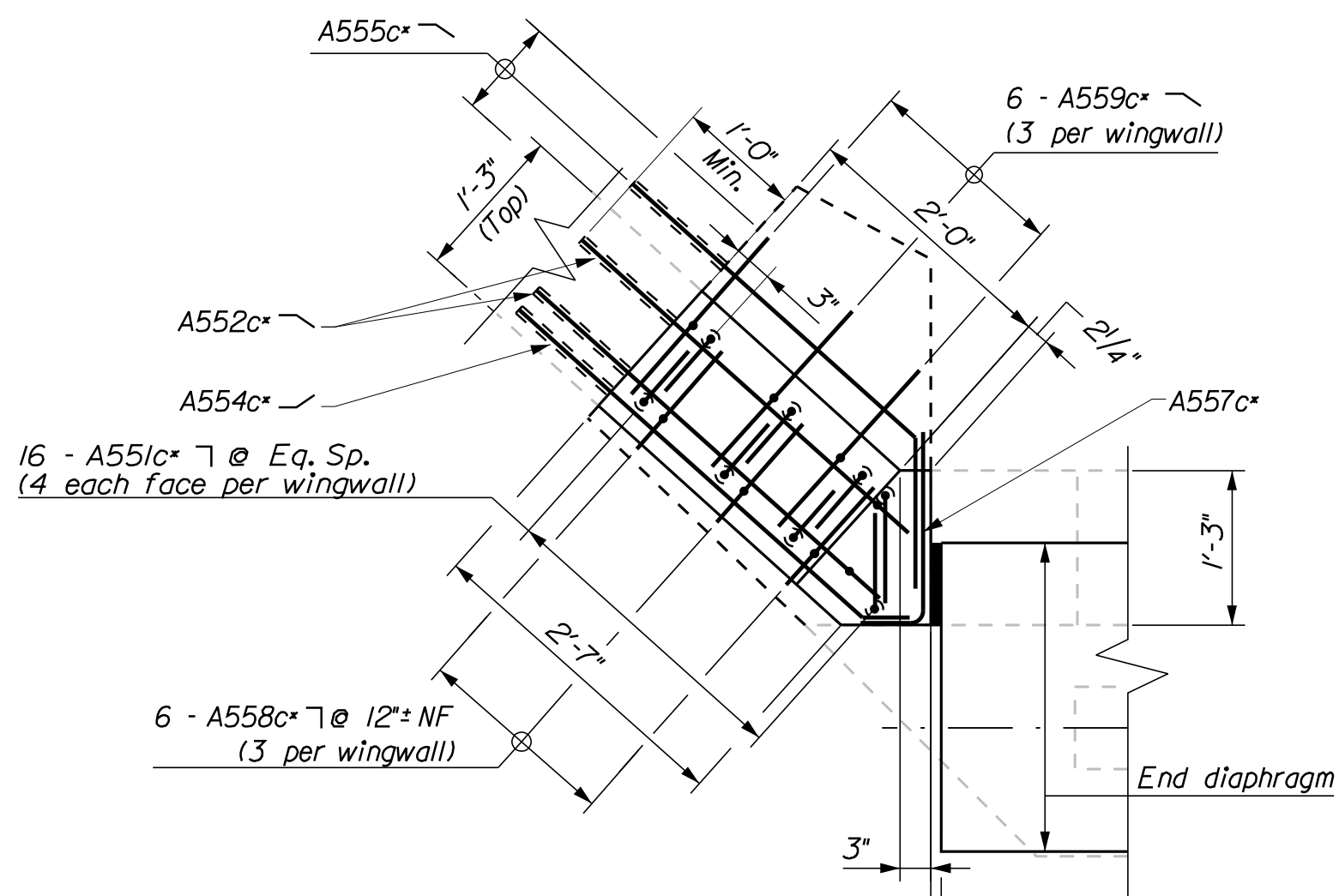


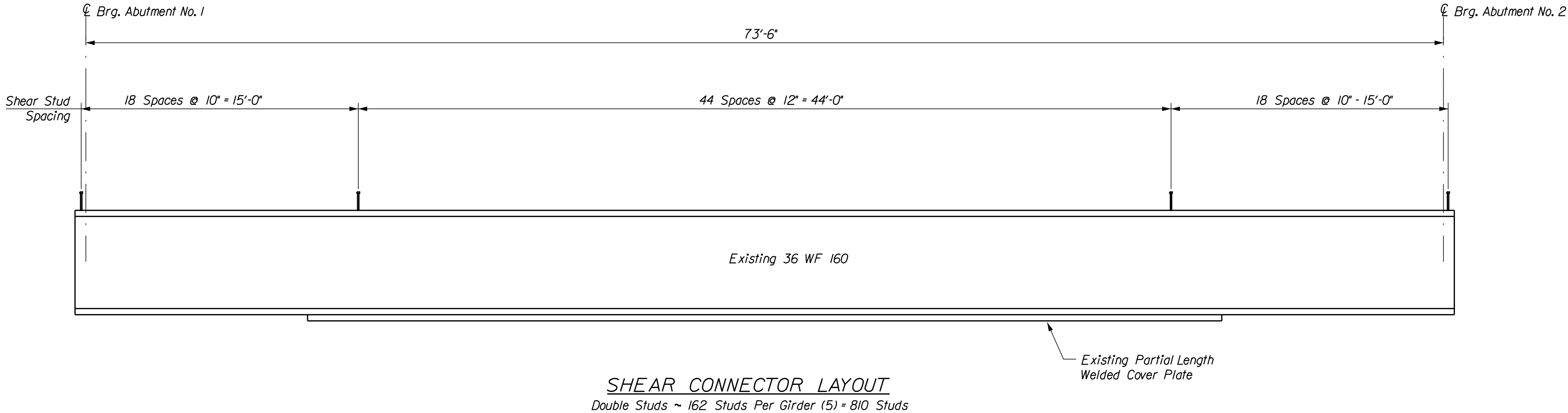
PROPOSED END DIAPHRAGM REINFORCING SECTION

Abutment No. 1 shown (Abutment No. 2 similar but opposite)



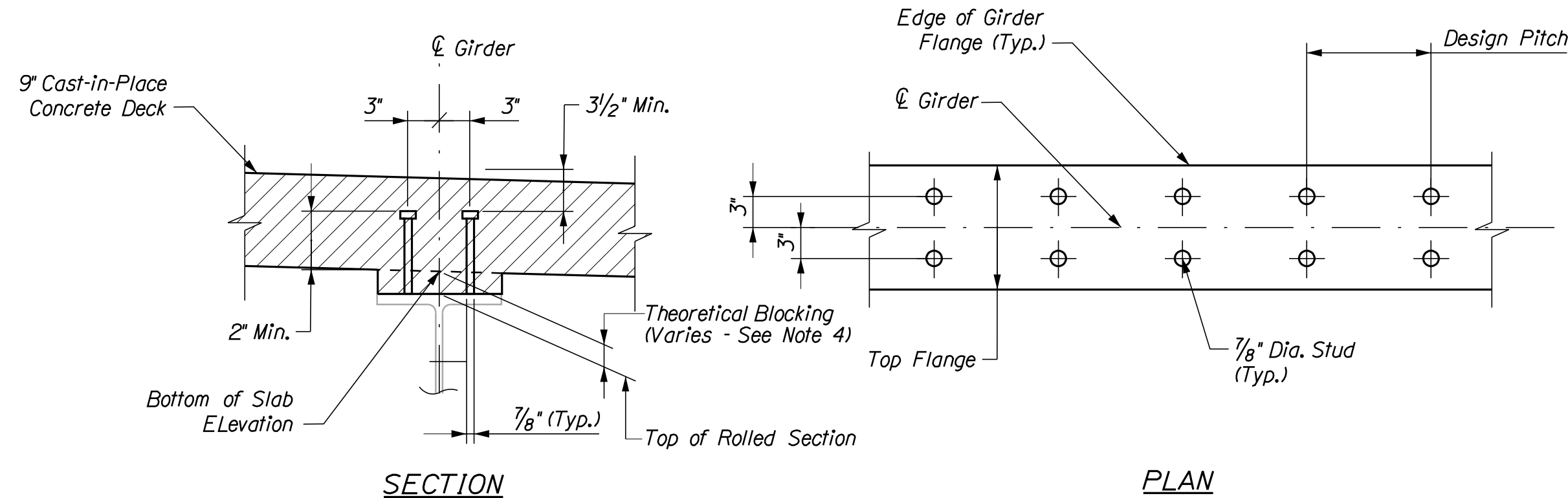
ABUTMENT REMOVAL SECTION





NOTES

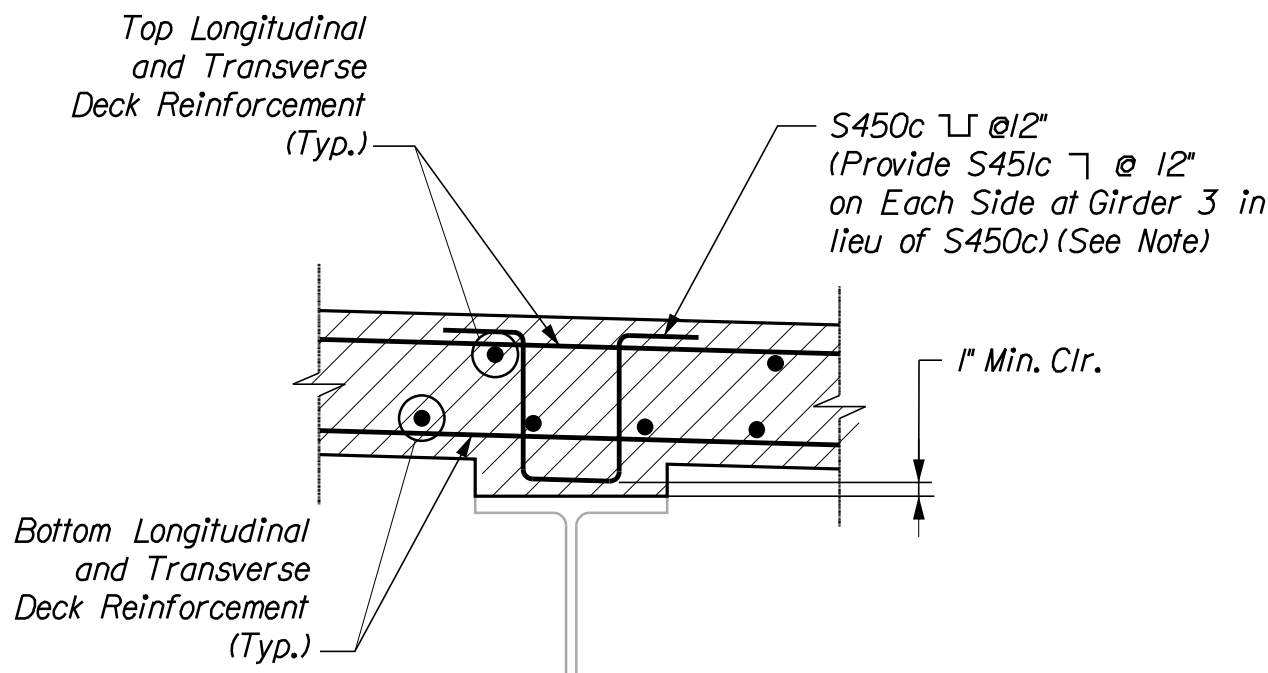
- Girder haunches are anticipated to have variable depths and may require stud lengths ranging from 6" to 8". Payment for additional shear connectors will be considered incidental to Item 505.08 "Shear Connectors".
- Temporary top flange bracing placed at midspan of the existing girders is required for construction of the new deck and paid for under Item 524.301, Temporary Structural Support - Lateral Bracing. The temporary bracing shall be removed after concrete deck has cured.
- Prior to installing the proposed shear studs, the Contractor shall clean the top flange so that it is free of debris, Rust, scale, oil and other contaminants that would adversely the welding operation. Payment for cleaning the top flange for installation of proposed shear studs will be considered incidental to shear stud item.
- The theoretical blocking used for design of the structure is $5\frac{3}{8}"$ and $5\frac{5}{8}"$ at the centerline of bearing of Abutment 1 and Abutment 2, respectively. Refer to Standard Detail 502(03) for blocking details. See Haunch Reinforcement detail on Structural Steel Details sheet when blocking depth exceeds 4 inches.



SECTION

PLAN

SHEAR CONNECTOR DETAILS



Note: Where girder haunch depths exceed 4" haunch reinforcement shall be provided, locations of girder haunch reinforcement shall be verified by the Resident. A nominal quantity of haunch reinforcement bars is specified, but actual number of bars required is to be determined by the Resident.

BOTTOM OF SLAB ELEVATIONS												
	℄ Brg. Abut. 1	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	℄ Brg. Abut. 2	
Girder 1	98.51	98.58	98.67	98.77	98.89	99.02	99.16	99.32	99.49	99.68	98.88	
Girder 2	98.29	98.37	98.45	98.56	98.67	98.80	98.95	99.10	99.28	99.46	99.66	
Girder 3	98.08	98.15	98.24	98.34	98.46	98.59	98.73	98.89	99.06	99.25	99.45	
Girder 4	97.86	97.94	98.02	98.13	98.24	98.37	98.52	98.67	98.85	99.03	99.23	
Girder 5	97.65	97.72	97.81	97.91	98.03	98.16	98.30	98.46	98.63	98.82	99.02	

UNFACTORED GIRDER DEFLECTIONS												
Girder 1 & Girder 5 Deflections (in)												
	℄ Brg. Abut. 1	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	℄ Brg. Abut. 2	
Cast-In-Place Slab Deflection	0	1/2	15/16	1 5/16	1 1/2	1 9/16	1 1/2	1 5/16	15/16	1/2	0	
Superimposed Deflection	0	1/16	1/16	1/8	1/8	1/8	1/8	1/8	1/16	1/16	0	
Total Deflection	0	9/16	1	1 3/8	1 5/8	1 11/16	1 5/8	1 3/8	1	9/16	0	
Girder 2 & Girder 4 Deflections (in)												
	℄ Brg. Abut. 1	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	℄ Brg. Abut. 2	
Cast-In-Place Slab Deflection	0	1/2	15/16	1 5/16	1 1/2	1 9/16	1 1/2	1 5/16	15/16	1/2	0	
Superimposed Deflection	0	1/16	1/16	1/16	1/8	1/8	1/8	1/16	1/16	1/16	0	
Total Deflection	0	9/16	1	1 3/8	1 5/8	1 11/16	1 5/8	1 3/8	1	9/16	0	
Girder 3 - Stage 1 Deflections (in)												
	℄ Brg. Abut. 1	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	℄ Brg. Abut. 2	
Cast-In-Place Slab Deflection	0	3/16	5/16	7/16	1/2	9/16	1/2	7/16	5/16	3/16	0	
Superimposed Deflection	0	0	1/16	1/16	1/16	1/16	1/16	1/16	1/16	0	0	
Total Deflection	0	3/16	3/8	1/2	9/16	5/8	9/16	1/2	3/8	3/16	0	
Girder 3 - Stage 2 Deflections (in)												
	℄ Brg. Abut. 1	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	℄ Brg. Abut. 2	
Cast-In-Place Slab Deflection	0	3/16	5/16	7/16	1/2	1/2	1/2	7/16	5/16	3/16	0	
Superimposed Deflection	0	0	1/16	1/16	1/16	1/16	1/16	1/16	1/16	0	0	
Total Deflection	0	3/16	5/16	7/16	1/2	9/16	1/2	7/16	5/16	3/16	0	

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2362900

WIN
023629.00

BRIDGE NO. 5756
BRIDGE PLANS

PROJ. MANAGER
D. EATON

CHECKED-REVIEWED
E. CARON
T. POLSON

DESIGN-REVIEWED
N. GOMEZ

DESIGN-REVIEWED
N. GOMEZ

REVISIONS
1
2
3
4

FIELD CHANGES

NASH MILL BRIDGE
MOUSAM RIVER
KENNEBUNK

YORK COUNTY

STRUCTURAL STEEL DETAILS

SHEET NUMBER

21

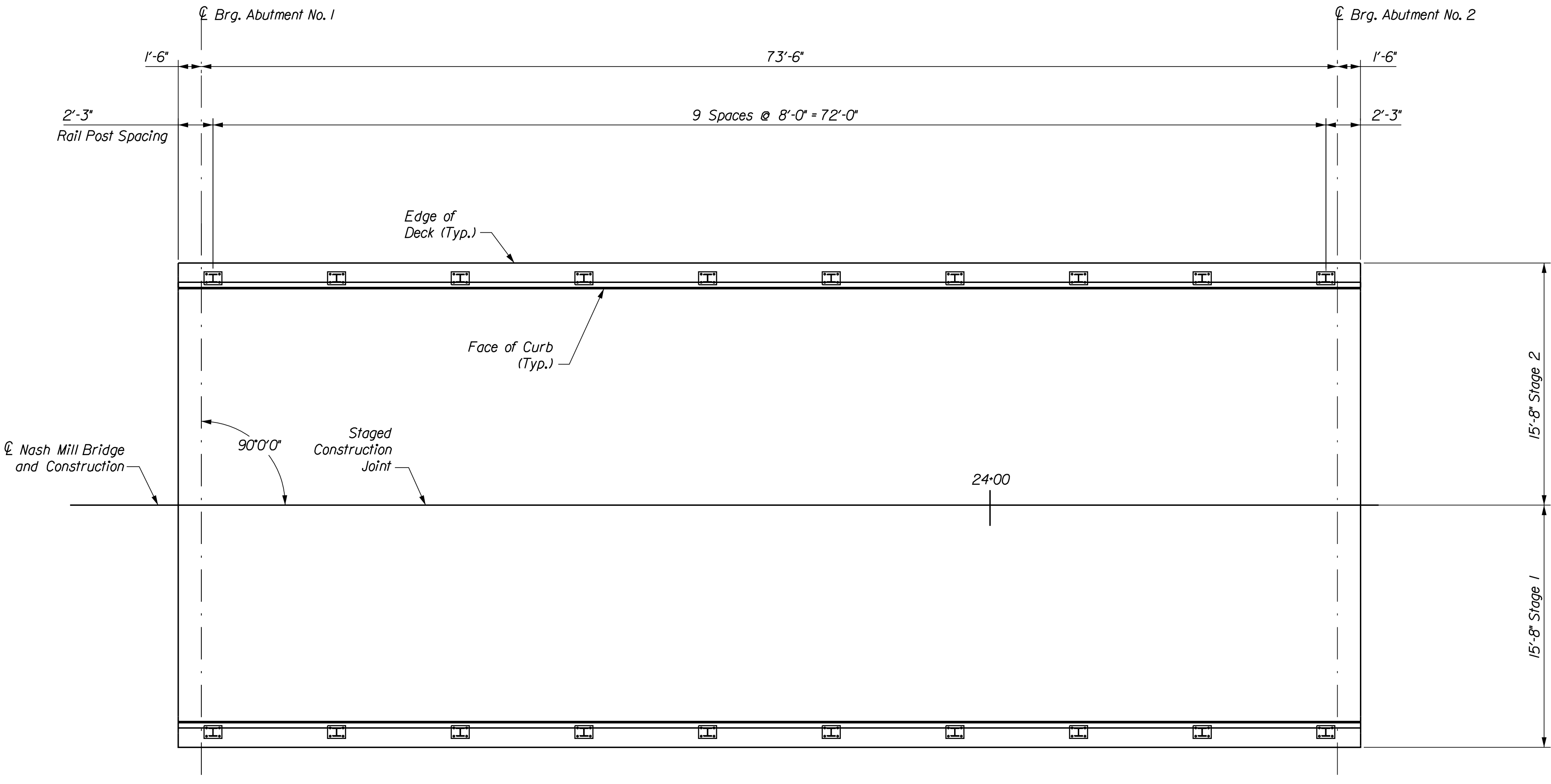
OF 26

Date:10/25/2023

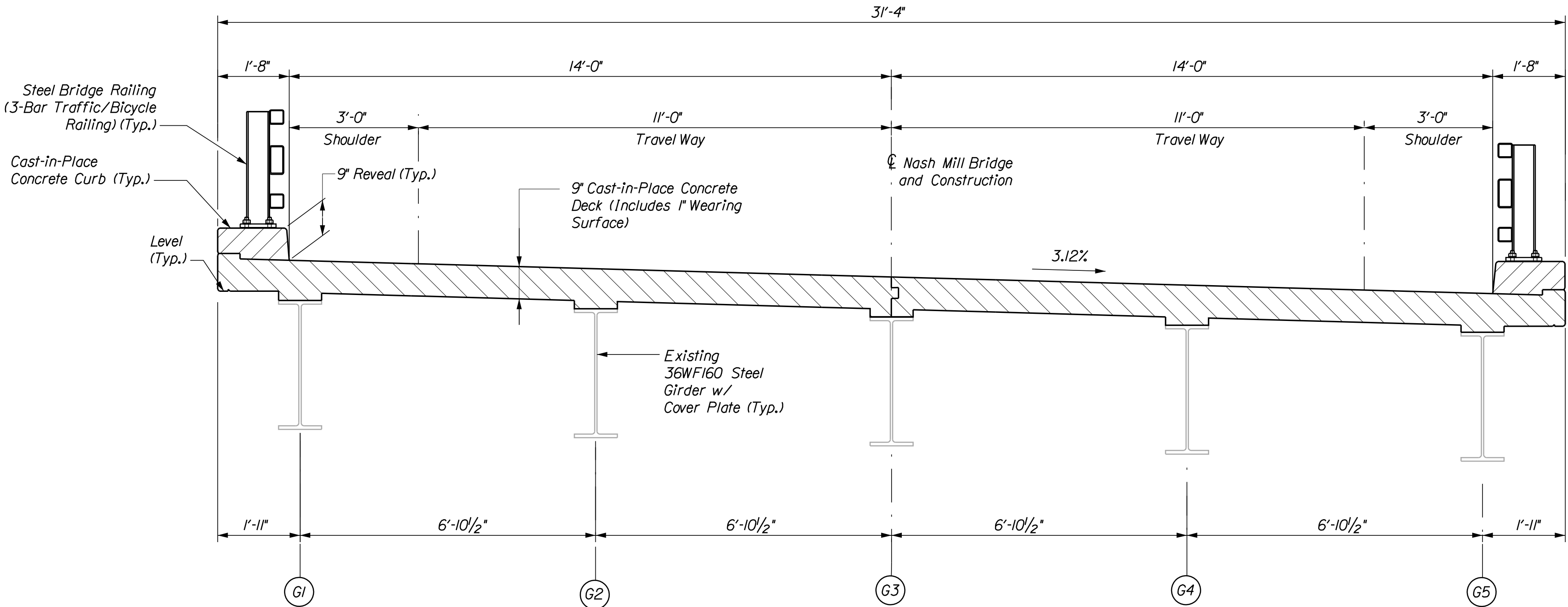
Username:

Division: BRIDGE

Filename: ... \MSTAN022_Superstructure.dgn



DECK PLAN



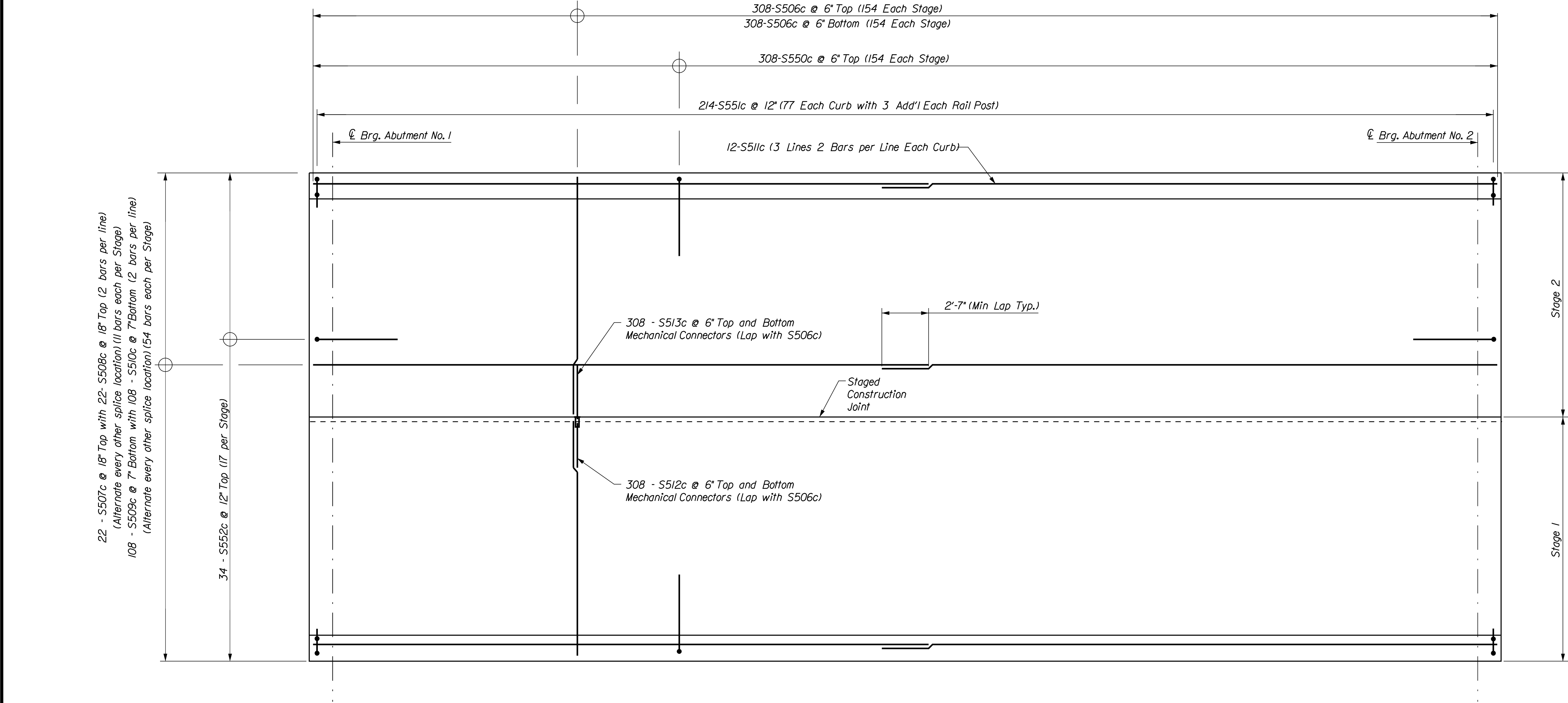
PROPOSED BRIDGE SECTION

SUPERSTRUCTURE NOTES:

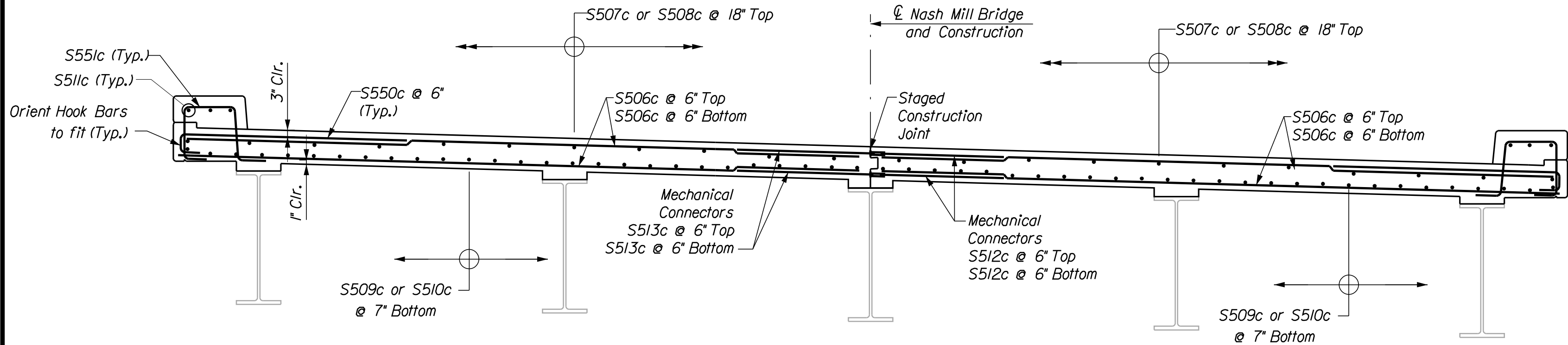
- Reinforcing steel shall have a minimum concrete cover of 2 inches unless otherwise noted.
- Form a one inch V-groove on the fascias at the horizontal joint between the curb and slab.
- The superstructure slab concrete, including end diaphragm, shall be placed continuously and shall be kept plastic until the entire placement has been made.
- Precast Concrete Deck Panels are not allowed on this project.
- Anchor rods for the steel bridge rail posts shall be shortened by 1 inch to provide additional clearance between the top of the deck and bottom of the anchor rod.
- The saw cut grooving shall be in the longitudinal direction.

STATE OF MAINE DEPARTMENT OF TRANSPORTATION		2362900		BRIDGE NO. 5756		BRIDGE PLANS	
NASH MILL BRIDGE MOUSAM RIVER KENNEBUNK YORK COUNTY		SUPERSTRUCTURE PLAN		SHEET NUMBER		22	
PROJ. MANAGER DESIGN-DETAILED CHECKED-REVIEWED DESIGNS-DETAILED REVISIONS 1 REVISIONS 2 REVISIONS 3 REVISIONS 4 FIELD CHANGES		D. EATON E. CARON N. GOMEZ		BY W. GERHOLD T. POLSON		DATE 9/20/23 9/20/23	
SIGNATURE 12202		P.E. NUMBER 10/23/2023		DATE 10/23/2023			

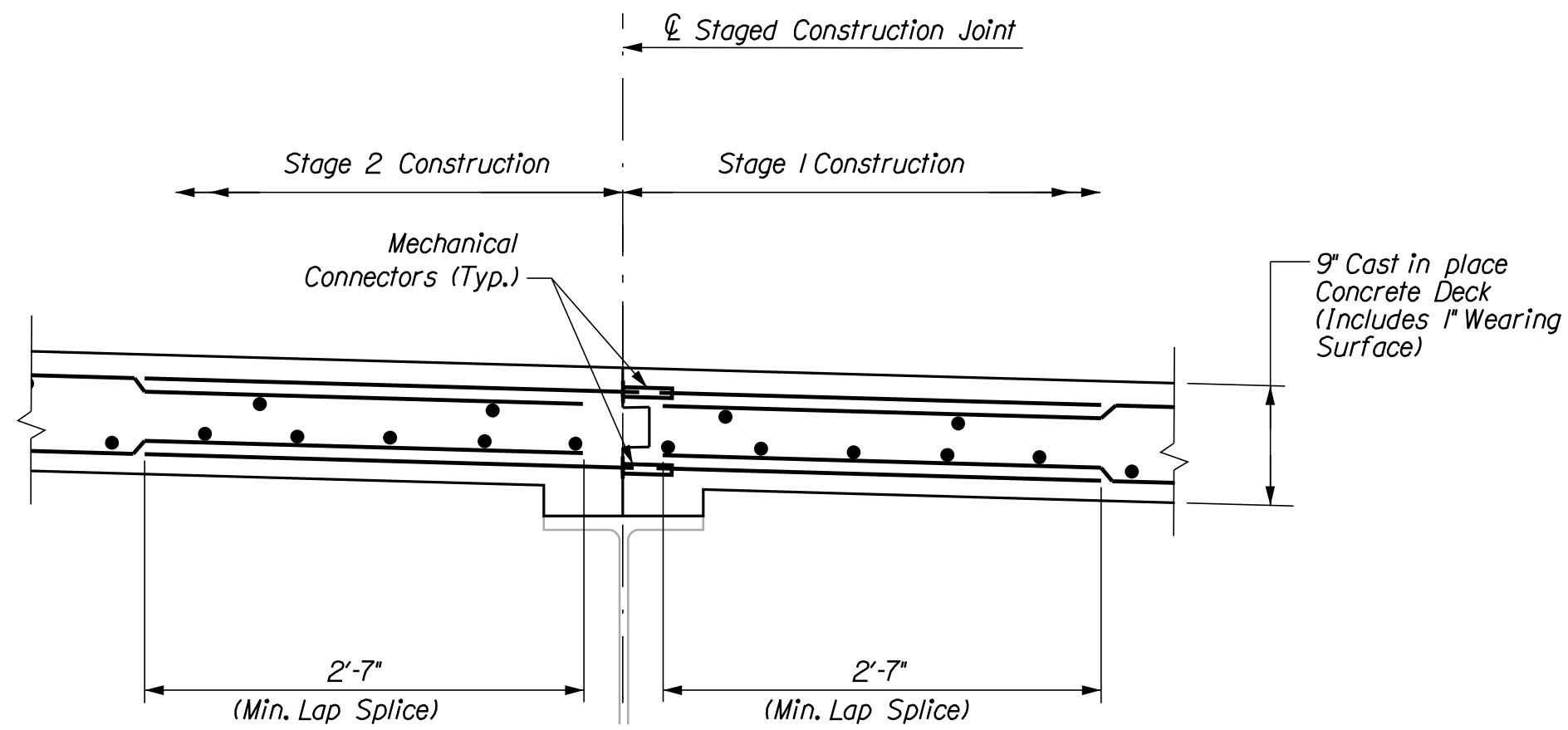




SUPERSTRUCTURE REINFORCING PLAN



SUPERSTRUCTURE REINFORCING SECTION



LONGITUDINAL CONSTRUCTION JOINT DETAIL





Date:10/25/2023

Username:

Division: BRIDGE

Filenome: ... \BRIDGE\WSTA\025_Reinfesch.dgn

STRAIGHT BARS								BENT BARS															
MARK	QTY.	LENGTH	LOCATION	MARK	QTY.	LENGTH	LOCATION	MARK	QTY.	LENGTH	TYPE	A	B	C	D	E	F	G	H	O	R	LOCATION	
Superstructure								Superstructure															
S506c	616	15'-3"	Top and bottom transverse					S450c	140	3'-4"	SL	0'-8"	0'-8"	0'-8"	0'-8"			0'-8"					Haunch reinforcement
S507c	22	45'-0"	Top longitudinal					S451c	70	1'-4"	L	0'-8"	0'-8"										Haunch rein. at girder 3
S508c	22	33'-10'	Top longitudinal																				
S509c	108	45'-0"	Bottom longitudinal					S550c	308	5'-8"	C	0'-7"	5'-1"	0'-0"									Deck overhang transverse
S510c	108	33'-10"	Bottom longitudinal					S551c	214	5'-3"	SC	0'-10"	1'-2"	1'-3"	1'-2"			0'-10"		1'-4"			Curb stirrup
S511c	12	39'-5"	Longitudinal curb reinforcement					S552c	68	6'-0"	L	3'-0"	3'-0"										End diaphragm hanger
S512c	308	3'-0"	Stage 1 deck mechanical coupler					S553c	68	6'-0"	V				3'-0"	3'-0"			2'-1"				End diaphragm top of front face
S513c	308	3'-6"	Stage 2 deck mechanical coupler					S554c	68	5'-7"	S	0'-0"	2'-6"	0'-7"	2'-6"			0'-0"					End diaphragm rear stirrup
S514c	12	8'-8"	Bottom of end diaphragm stub					S555c	136	8'-6"	S	0'-0"	3'-2"	2'-2"	3'-2"			0'-0"					End diaphragm stirrup
S515c	4	5'-0"	Bottom of end diaphragm stub					S556c	16	4'-6"	S	0'-0"	1'-3"	2'-0"	1'-3"			0'-0"					End diaphragm end stirrups
S516c	4	5'-5"	Bottom of end diaphragm stub					S557c	12	5'-9"	S	0'-0"	1'-3"	3'-3"	1'-3"			0'-0"					End dia. vertical end stirrups
S517c	4	3'-0"	Stage 1 end dia. mech. coupler					Abutment No. 1															
S518c	4	3'-6"	Stage 2 end dia. mech. coupler					A550	32	4'-8"	L	2'-4"	2'-4"										Back of abutment dowels
S606c	36	15'-3"	End diaphragm primary					A551c*	16	6'-11"	L	6'-0"	0'-11"										Vertical wingwall bars
S607c	18	3'-6"	Stage 1 end dia. mech. coupler					A552c*	4	4'-8"	V				1'-2"	3'-6"			1'-6"				Wingwall top bars
S608c	18	4'-0"	Stage 2 end dia. mech. coupler					A553c	2	1'-10"	T3	0'-6"	0'-9"	0'-7"					0'-5"				Top wingwall stirrup
Approach Slab								A554c*	8	4'-5"	V				3'-9"	0'-8"			0'-6"			Horizontal wingwall bars, NF	
AS501	64	12'-8"	Bottom transverse					A555c*	8	4'-0"	V				3'-1"	0'-11"			0'-8"			Horizontal wingwall bars, FF	
AS502	32	3'-0"	Stage 1 mechanical coupler					A556c	2	2'-2"	T3	0'-10"	0'-6"	0'-10"					0'-7"				Top wingwall stirrup
AS503	32	3'-6"	Stage 2 mechanical coupler					A557c*	8	4'-1"	L	3'-0"	1'-1"									Wingwall horizontal L's	
AS601								A558c*	6	4'-6"	L	1'-1"	3'-5"									Wingwall NF battered bars	
								A559c*	6	5'-6"	V				0'-11"	4'-7"			4'-5"			Wingwall FF battered bars	
								A560	28	3'-10"	L	1'-0"	2'-10"										Approach slab dowels
Abutment No. 1								Abutment No. 2															
A506	8	15'-5"	Approach slab seat long.					B550	32	4'-8"	L	2'-4"	2'-4"										Back of abutments dowels
Abutment No. 2								B551c*	16	6'-11"	L	6'-0"	0'-11"										Vertical wingwall bars
B506	8	15'-5"	Approach slab seat long.					B552c*	4	4'-8"	V				1'-2"	3'-6"			1'-6"				Wingwall top bars
								B553c	2	1'-10"	T3	0'-6"	0'-9"	0'-7"					0'-5"				Top wingwall stirrup
								B554c*	8	4'-5"	V				3'-9"	0'-8"			0'-6"				Horizontal wingwall bars, NF
								B555c*	8	4'-0"	V				3'-1"	0'-11"			0'-8"				Horizontal wingwall bars, FF
								B556c	2	2'-2"	T3	0'-10"	0'-6"	0'-10"					0'-7"				Top wingwall stirrup
								B557c*	8	4'-1"	L	3'-0"	1'-1"									Wingwall horizontal L's	
								B558c*	6	4'-6"	L	1'-1"	3'-5"									Wingwall NF battered bars	
								B559c*	6	5'-6"	V				0'-11"	4'-7"			4'-5"				Wingwall FF battered bars
								B560	28	3'-10"	L	1'-0"	2'-10"										Approach slab dowels
						</																	

