

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION



SPECIFICATIONS

Design: Load and Resistance Factor Design per AASHTO LRFD Bridge Design Specifications, Fifth Edition 2010.

DESIGN LOADING

Live Load HL - 93 Modified

TRAFFIC DATA

Current (2011) AADT 2040
Future (2031) AADT 2450
DHV - % of AADT 14
Design Hour Volume 343
Heavy Trucks (% of AADT) 17
Heavy Trucks (% of DHV) 17
Directional Distribution (% of DHV) 52
18 kip Equivalent P 2.0 356
18 kip Equivalent P 2.5 339
Design Speed (mph) 60

MATERIALS

Concrete:
Curbs & Transition Barriers Class "LP"
All Other Class "A"
Reinforcing Steel:
Curbs ASTM A 615 and ASTM A 775
All Other ASTM A 615/A 615M, Grade 60

BASIC DESIGN STRESSES

Concrete f 'c = 4,350 psi
Reinforcing Steel f.y. = 60,000 psi

ABBOT
PISCATAQUIS COUNTY
PAGE - CARR BRIDGE
OVER
PISCATAQUIS RIVER
STATE ROUTES 6 & 15
FEDERAL PROJECT NO. BR-1930(600)
PROJECT LENGTH 0.085 mi.
BRIDGE NO. 5725

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UTILITIES

Central Maine Power Company Fairpoint Communications
Corner Stone Communications LLC.

MAINTENANCE OF TRAFFIC

Maintain a minimum of at least one 13'-0" wide lane of alternating two - way traffic using traffic signals.

PROJECT LOCATION:	Located 0.10 of a mile southerly of Pond Road. 45° 12' 0" N - 69° 27' 36.00" W
PROGRAM AREA:	Bridge
OUTLINE OF WORK:	Bridge Rehabilitation including deck replacement, bearing replacement with required concrete repairs and placement of scour countermeasures.

WIN 19306.00

BR-1930(600)

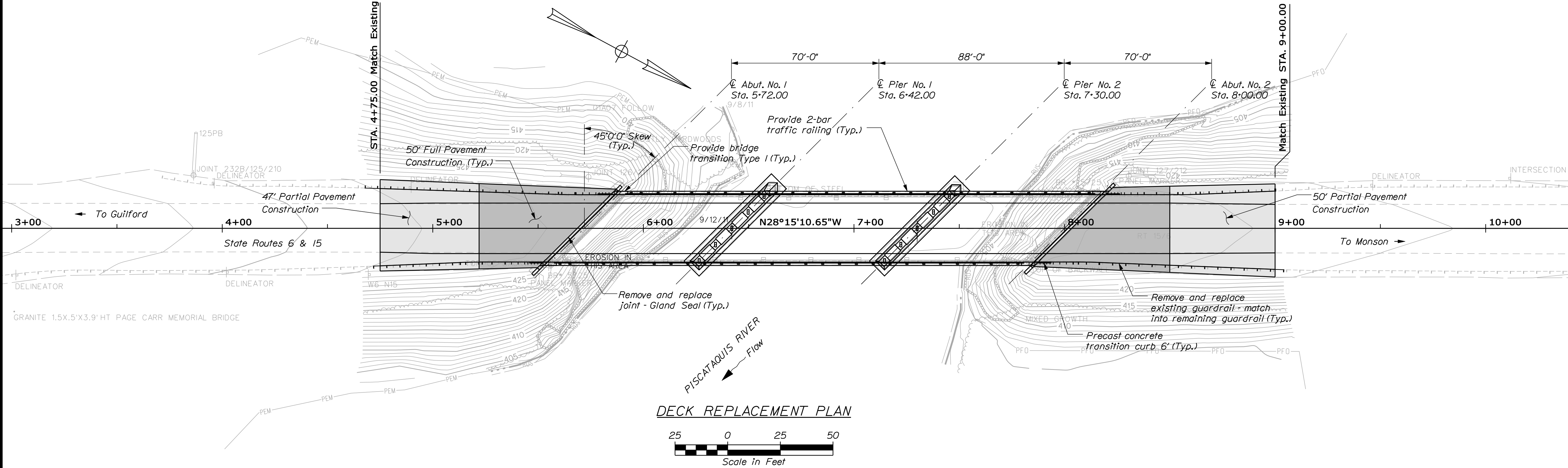
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APPROVED	DATE
COMMISSIONER: <i>[Signature]</i>	6/29/12
CHIEF ENGINEER: <i>[Signature]</i>	6/25/12

SIGNATURE <i>Michael Wright</i>	DATE 10/25/2012
8089	

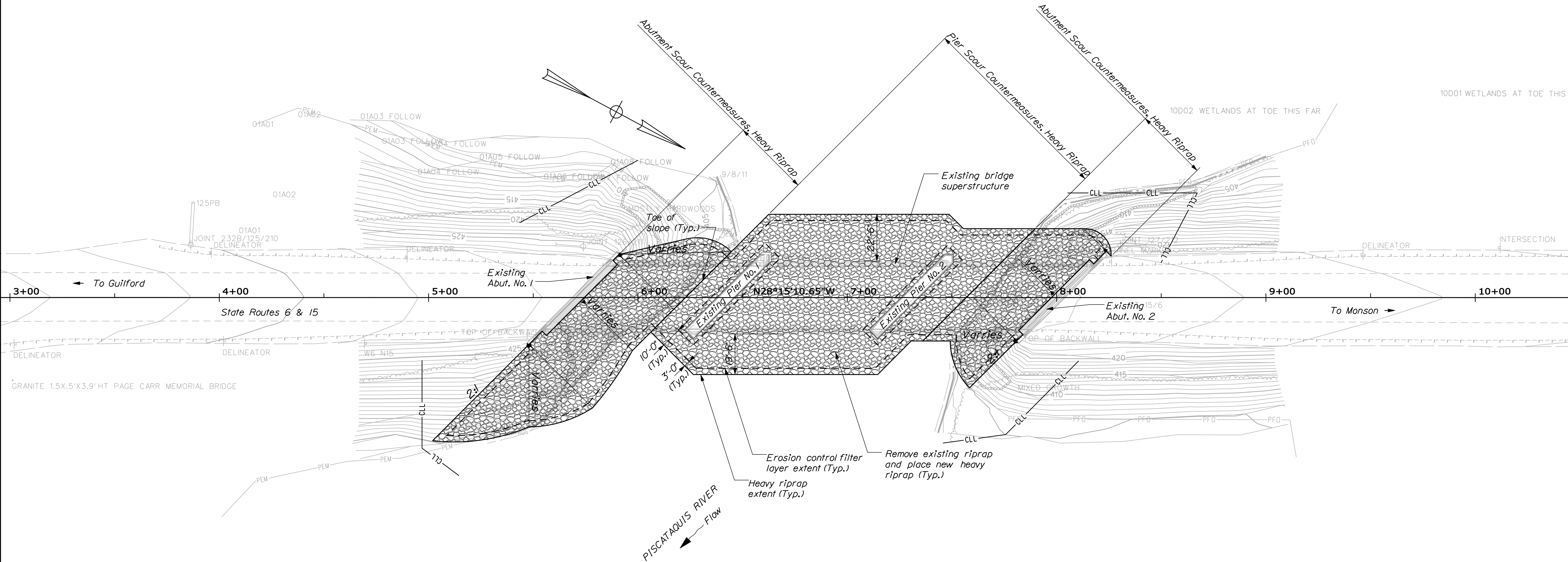
PROJECT INFORMATION	
PROGRAM	BRIDGE
PROJECT MANAGER	STEVE BODGE
DESIGNER	BRIAN REEVES
CONSULTANT	
PROJECT RESIDENT	
CONTRACTOR	
PROJECT COMPLETION DATE	

ABBOT PAGE - CARR BRIDGE	TITLE SHEET
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OF 15



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PAGE - CARR BRIDGE PISCATAQUIS RIVER ABBOT		PISCATAQUIS COUNTY		GENERAL PLAN		SHEET NUMBER		
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SCOUR COUNTERMEASURE PLAN



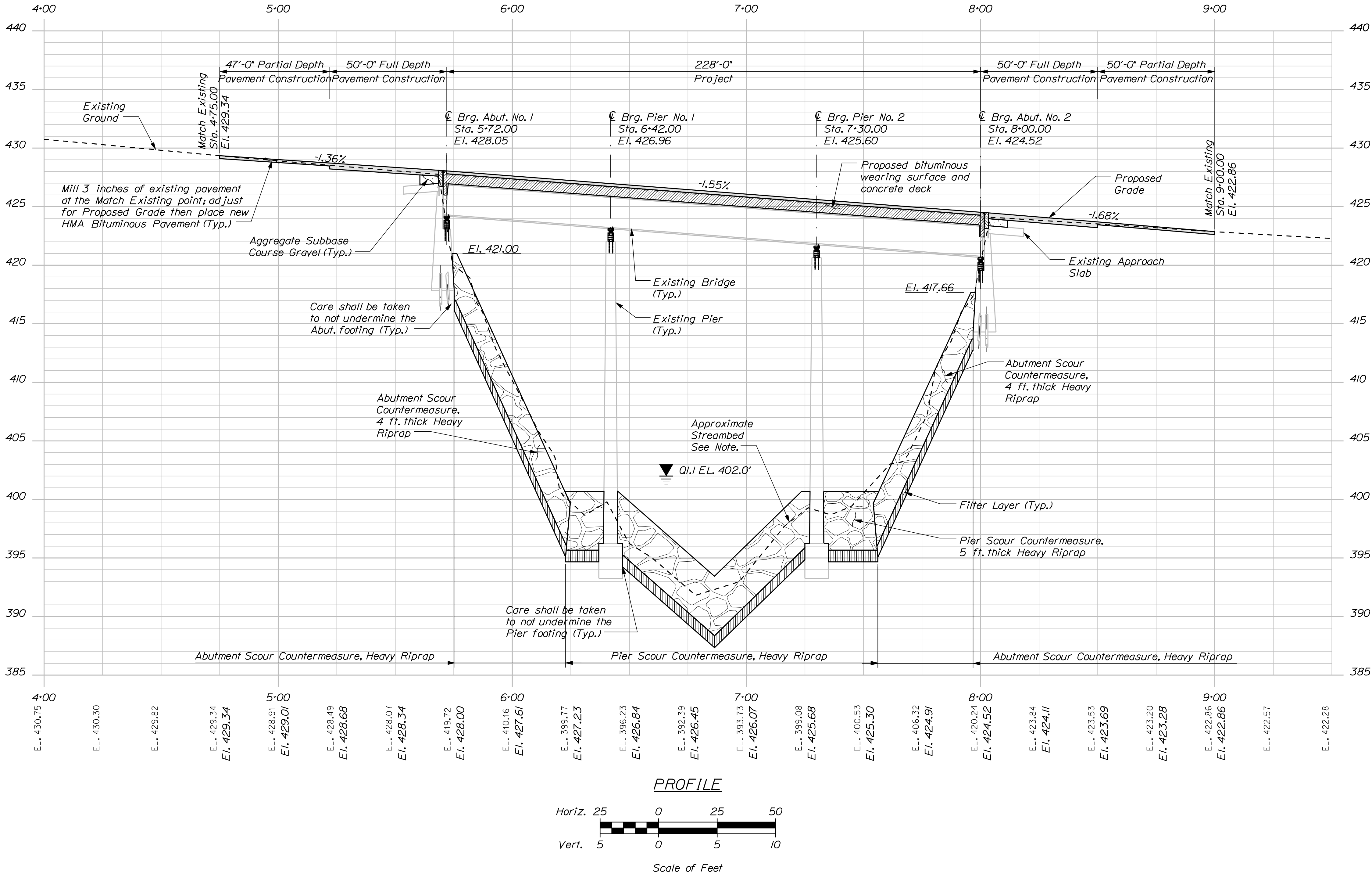
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	BY D. Sullivan		P.E. NUMBER	
PAGE - CARR BRIDGE PISCATAQUIS RIVER PISCATAQUIS COUNTY ABBOT SCOUR COUNTERMEASURES PLAN	PROJ. MANAGER S. Godge		DATE	
	CHECKED-REVIEWED B. Reeves		DATE	
SHEET NUMBER 4 OF 15	DESIGN-DETAILED		DATE	
	DESIGN-DETAILED		DATE	
REVISIONS 1	DESIGN-DETAILED		DATE	
	REVISIONS 2		DATE	
REVISIONS 3	DESIGN-DETAILED		DATE	
	REVISIONS 4		DATE	
FIELD CHANGES	DESIGN-DETAILED		DATE	
	REVISIONS 5		DATE	

Date:11/16/2012

Username: david.sullivan

Division: BRIDGE

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PROFILE NOTES:
 1. The assumed streambed elevations shown represent the upstream and downstream cross-section taken in August 1995. For further cross-sections reference see Appendix D (Hydrology/Hydraulic/ Scour Data/POA) of the Preliminary Design Report (PDR).

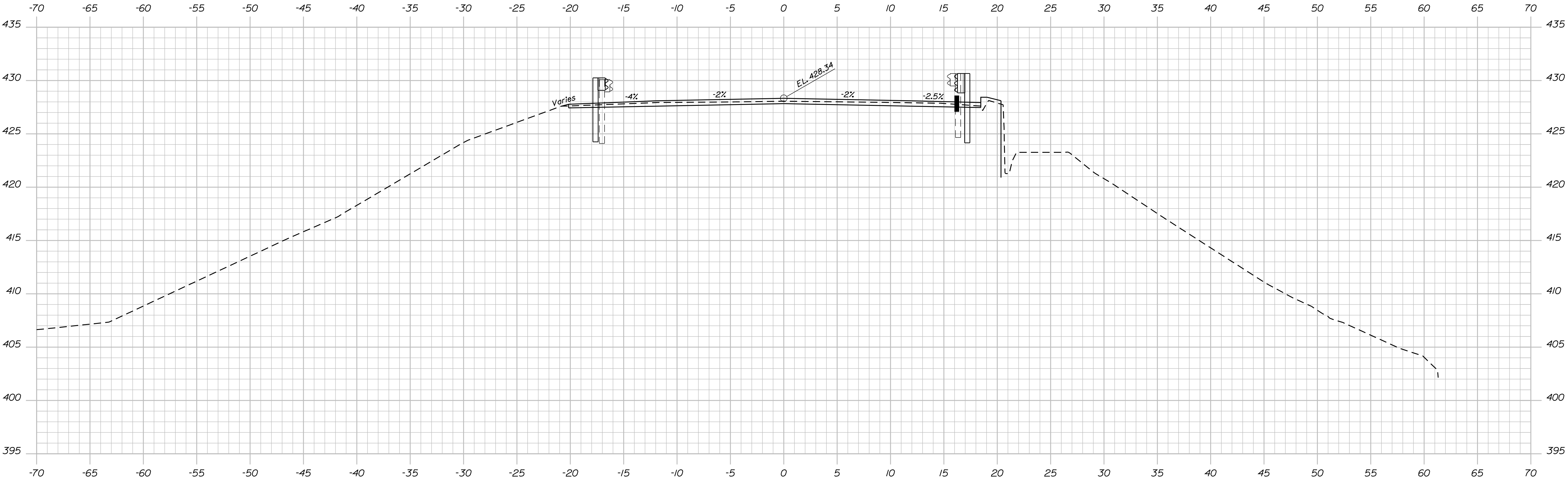
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PISCATAQUIS COUNTY		B. Reeves			
ABBOT		Erd. & Anlt.		SIGNATURE	
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		DESIGN3-DETAILED3			
		REVISONS 1			P.E. NUMBER
		REVISONS 2			
		REVISONS 3			
		REVISONS 4			DATE
		FIELD CHANGES			
PROFILE					BRIDGE NO. 5725
					PIN 19306.00
					BRIDGE PLANS

Date:11/6/2012

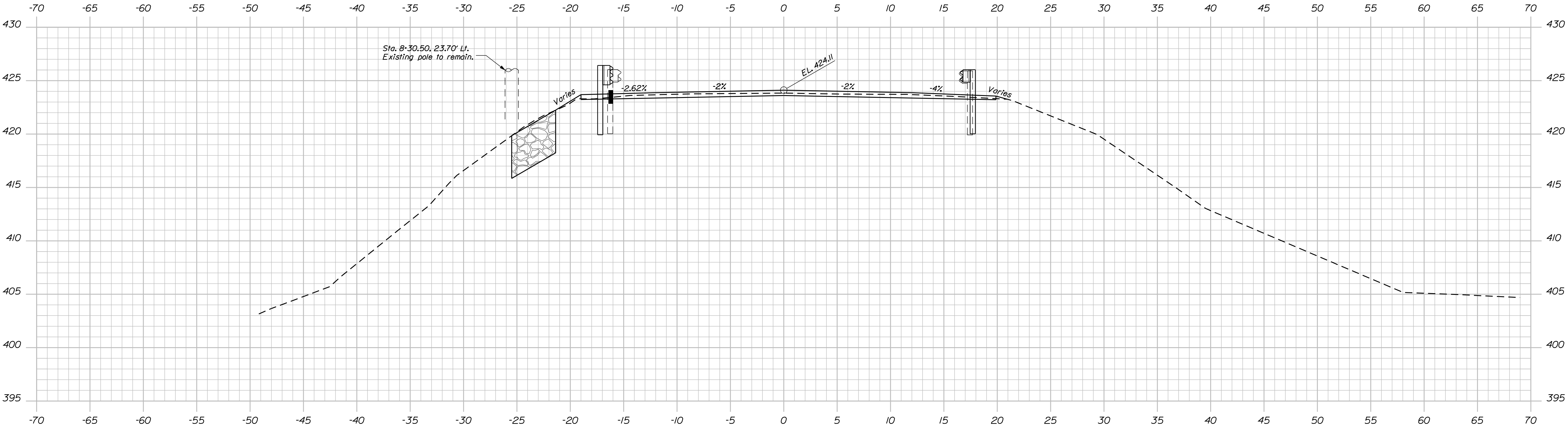
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5+50.00



8+25.00

STATE OF MAINE		DEPARTMENT OF TRANSPORTATION	
BR-1930(600)		P.E. NUMBER	
BRIDGE NO. 5725		DATE	
PIN 19306.00		FIELD CHANGES	
PISCATAQUIS COUNTY		CROSS SECTIONS	
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2. Any damage to the structure caused by attachment of the concrete barrier shall be repaired and paid for under related contract item.



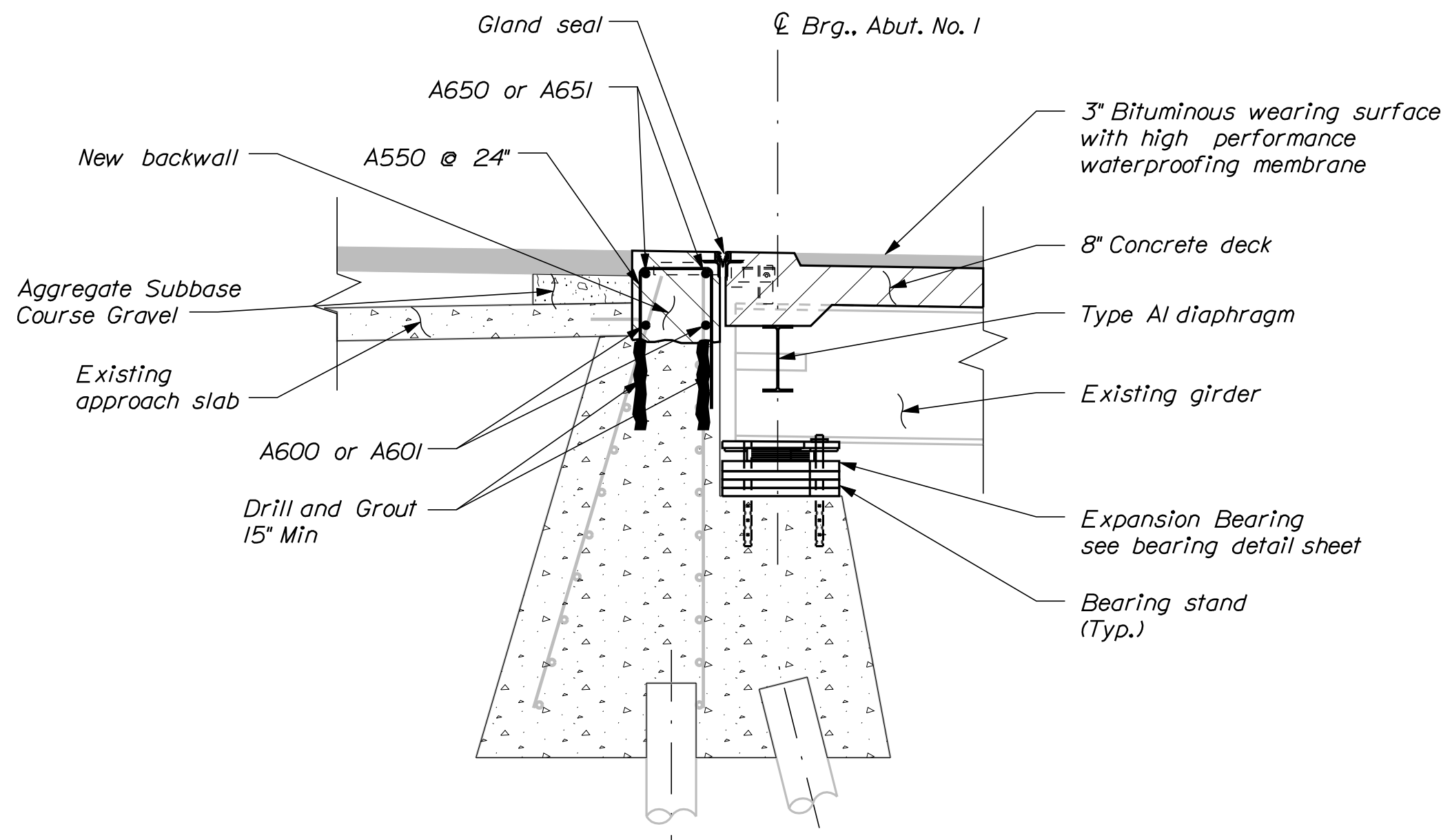
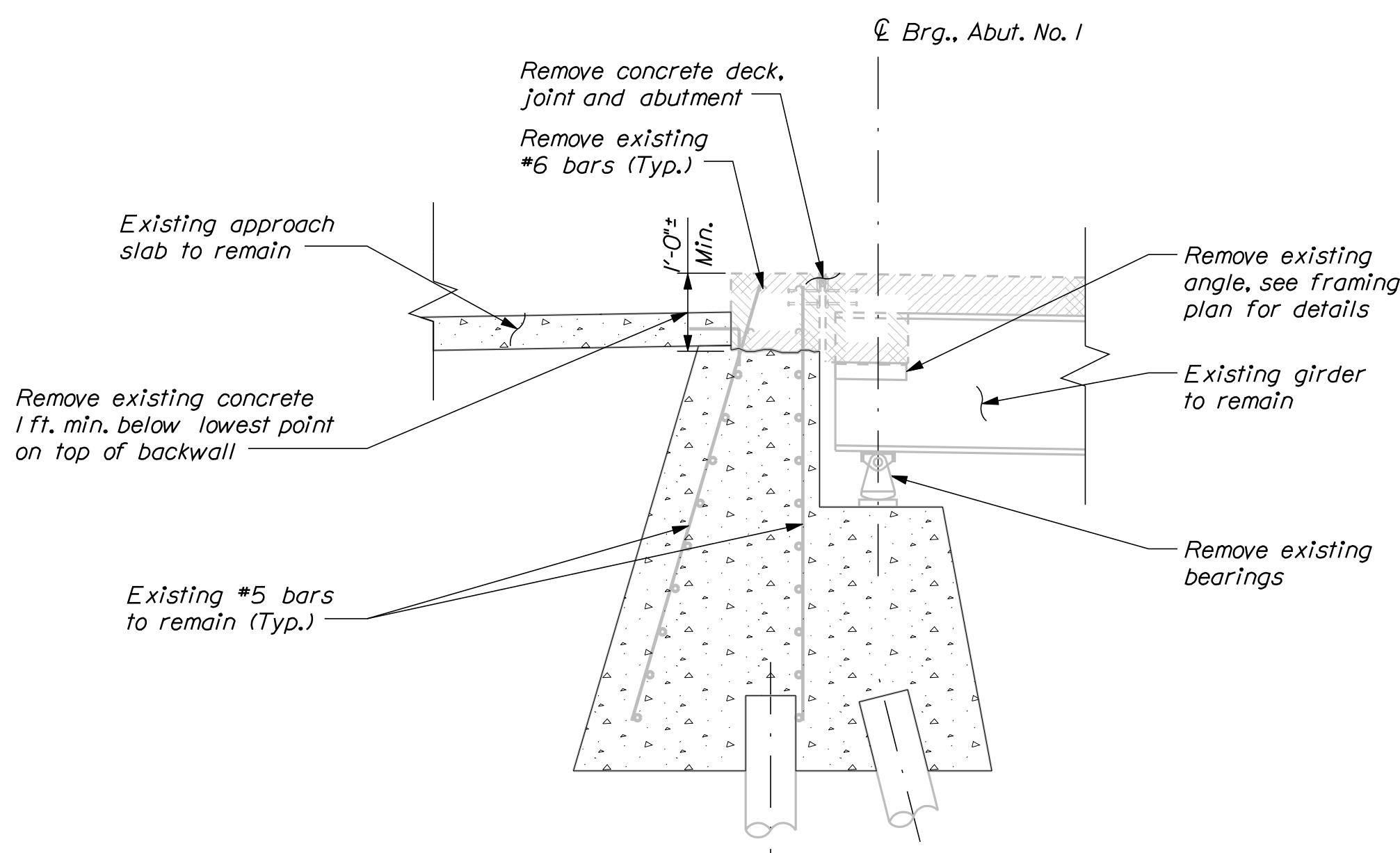
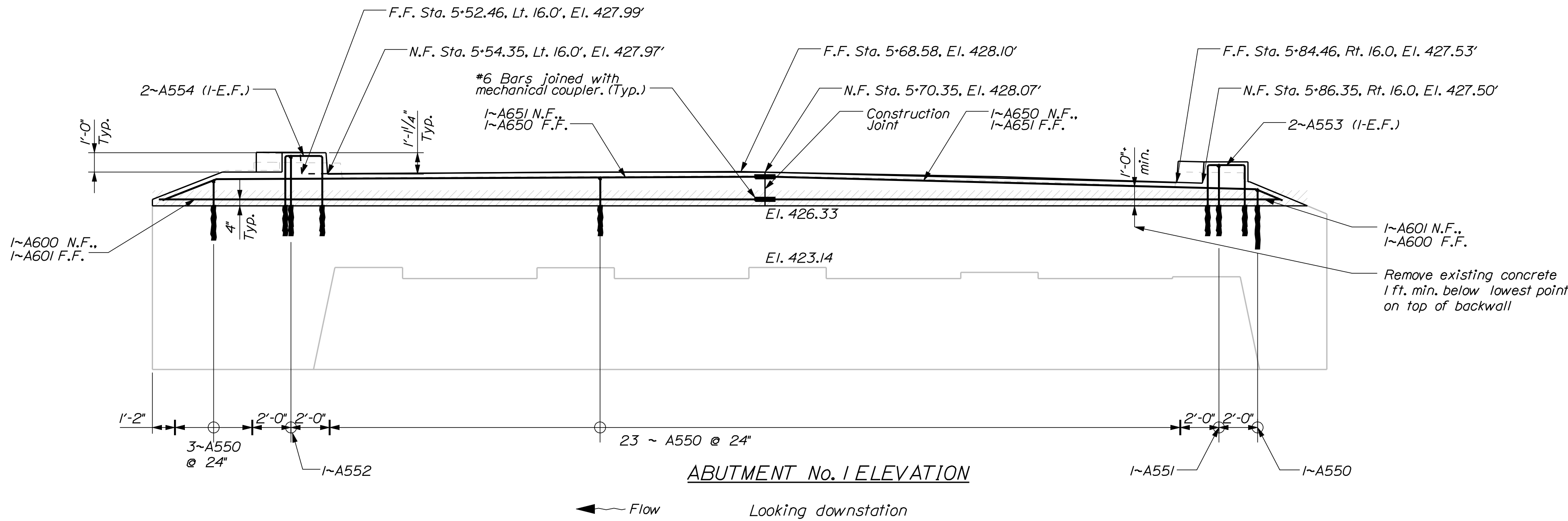
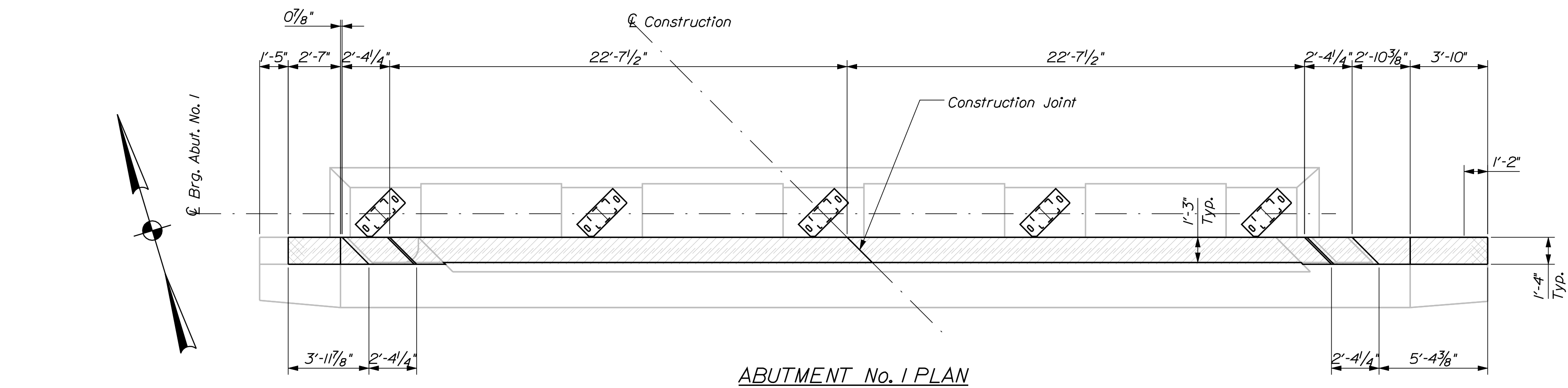
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ABUTMENT NOTES

1. Reinforcing steel shall have a minimum concrete cover of 2 inches in the walls and 3 inches in the footings unless otherwise noted.
2. Cover joints where water stops are not required in accordance with Standard Details Section 502.
3. Some reinforcing steel will need to be field cut, payment will be incidental to placement Item 503.13 Reinforcing Steel, Placing.
4. Approach slab is to remain along with any associated reinforcing steel
5. Field adjust reinforcing steel for clearance around the gland seal

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BR-1930(600)	
PIN 19306.00	BRIDGE NO. 5725
BRIDGE PLANS	

PROJ. MANAGER	BY	DATE	SIGNATURE
DESIGN-DETAILED	D. Sullivan	OCT 2012	
CHECKED-REVIEWED	B. Reeves		
DESIGN-DETAILED	M. Poulin		
REVISIONS 1			P.E. NUMBER
REVISIONS 2			DATE
REVISIONS 3			
REVISIONS 4			
FIELD CHANGES			

PAGE - CARR BRIDGE	
PISCATAQUIS RIVER	
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ABBOT	
ABUTMENT No. 1 PLANS	

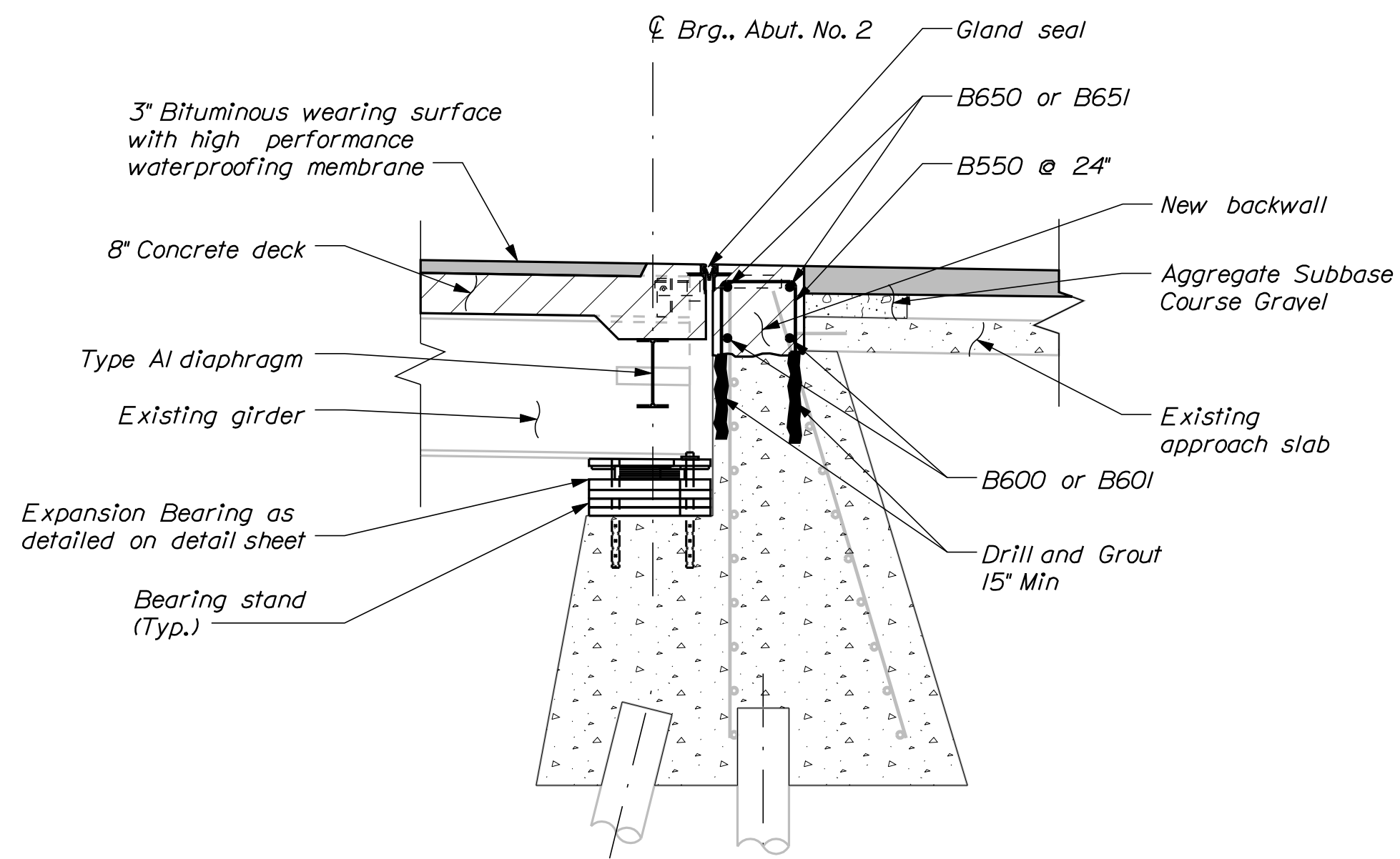
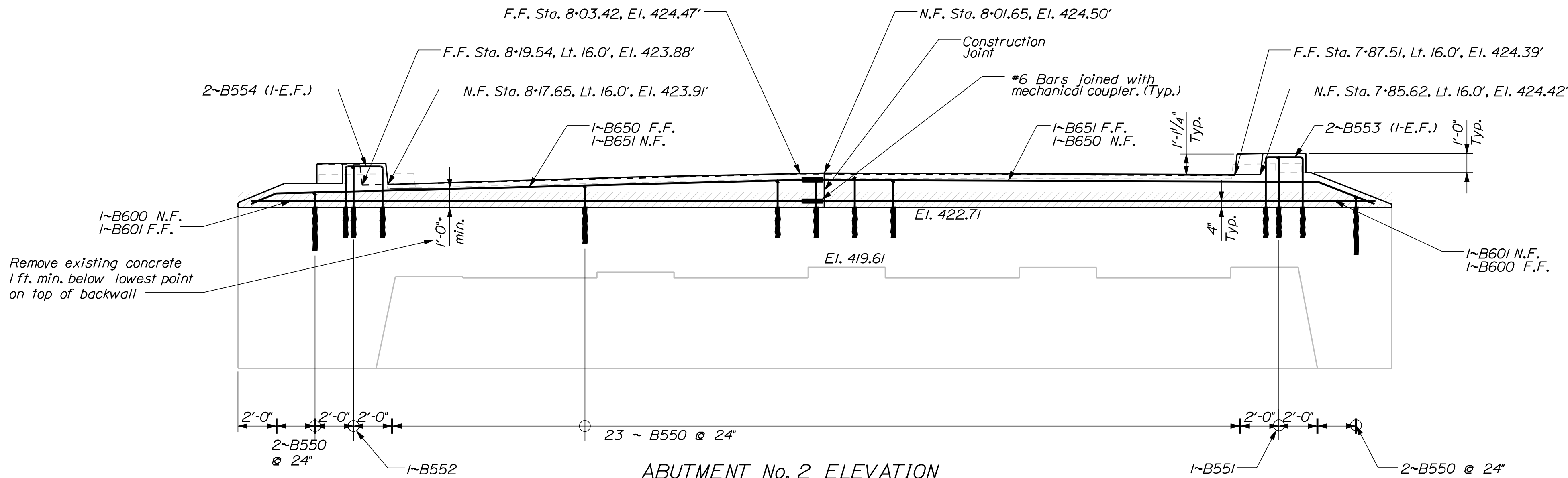
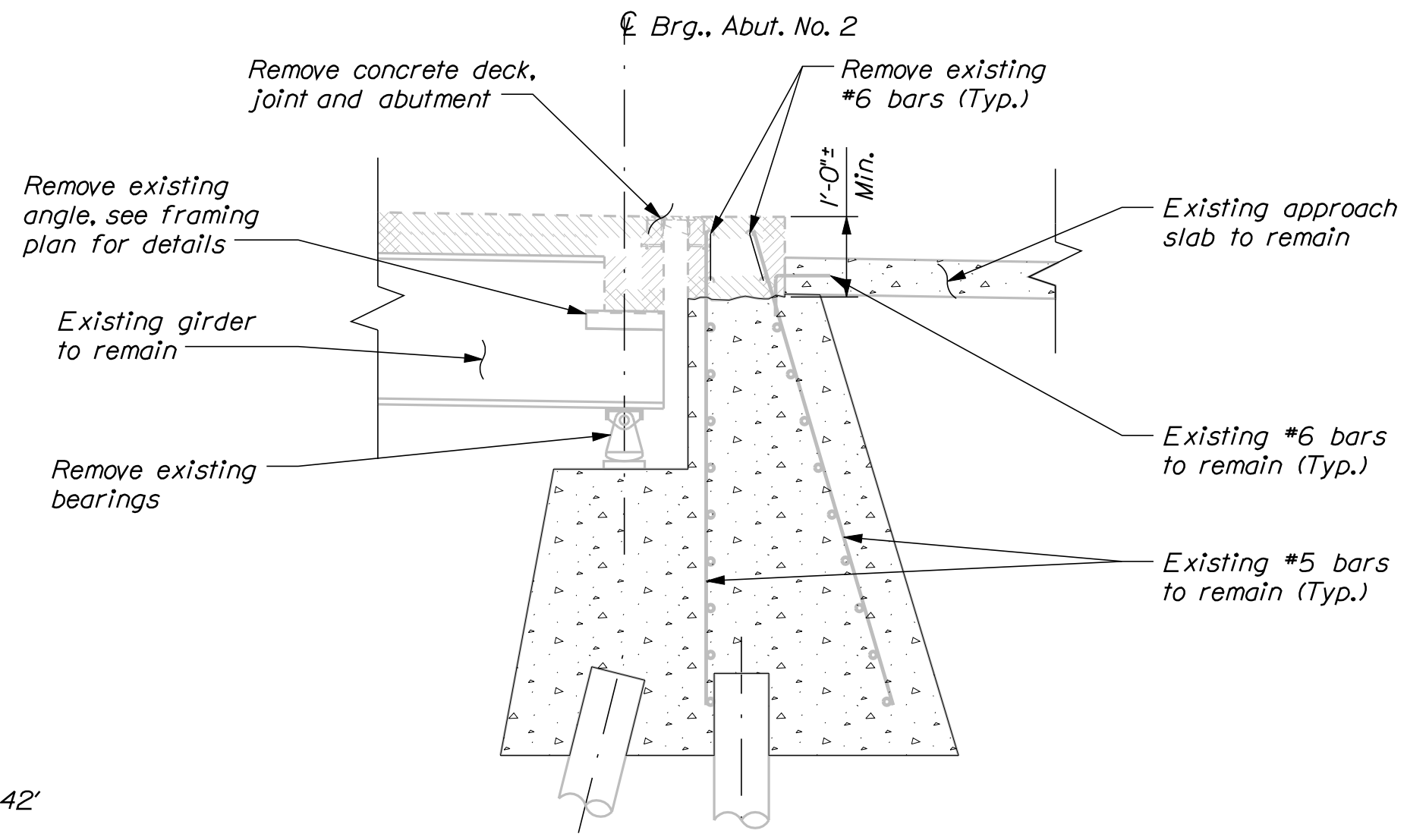
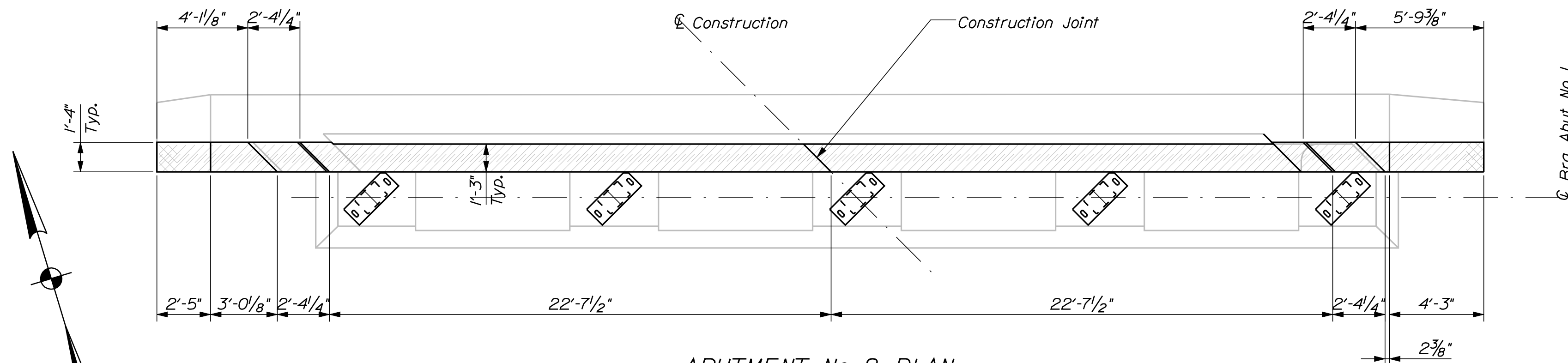
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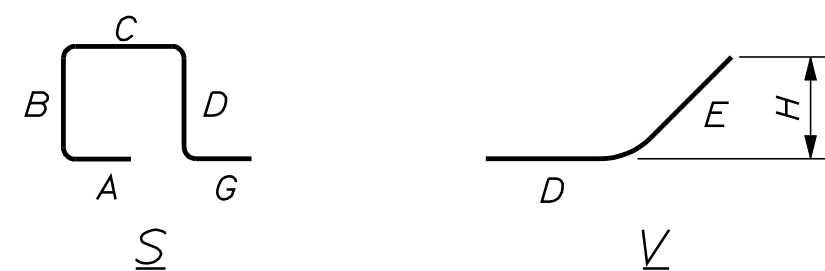
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ABUTMENT REINFORCING STEEL														
MARK	QTY.	LENGTH	TYPE	A	B	C	D	E	F	G	H	O	R	LOCATION
ABUTMENT No. 1														
A600	2	31'-7"												Horizontal Transverse
A601	2	26'-10"												Horizontal Transverse
A550	27	6'-6"	S		2'-10"	0'-10"	2'-10"							Vertical Longitudinal Stirup
A551	1	7'-8"	S		3'-5"	0'-10"	3'-5"							Vertical Long. Stirup Upstream
A552	1	8'-8"	S		3'-11"	0'-10"	3'-11"							Vertical Long. Stirup Downstream
A553	2	8'-9"	S		3'-5"	1'-11"	3'-5"							Vertical Long. Stirup Upstream
A554	2	9'-9"	S		3'-11"	1'-11"	3'-11"							Vertical Long. Stirup Downstream
A650	2	27'-1"	V				25'-7"	1'-6"			0'-6/2"			Transverse Upstream
A651	2	31'-7"	V				28'-7"	3'-0"			1'-1"			Transverse Downstream
ABUTMENT No. 2														
B600	2	29'-5"												Horizontal Transverse
B601	2	29'-4"												Horizontal Transverse
B550	27	6'-6"	S		2'-10"	0'-10"	2'-10"							Vertical Longitudinal Stirup
B551	1	7'-2"	S		4'-0"	0'-10"	4'-0"							Vertical Long. Stirup Upstream
B552	1	8'-2"	S		3'-6"	0'-10"	3'-6"							Vertical Long. Stirup Downstream
B553	2	8'-3"	S		4'-0"	1'-11"	4'-0"							Vertical Long. Stirup Upstream
B554	2	9'-3"	S		3'-6"	1'-11"	3'-6"							Vertical Long. Stirup Downstream
B650	2	26'-6"	V				28'-1"	1'-5"			1'-3"			Transverse Upstream
B651	2	27'-7"	V				26'-5"	3'-2"			1'-3"			Transverse Downstream



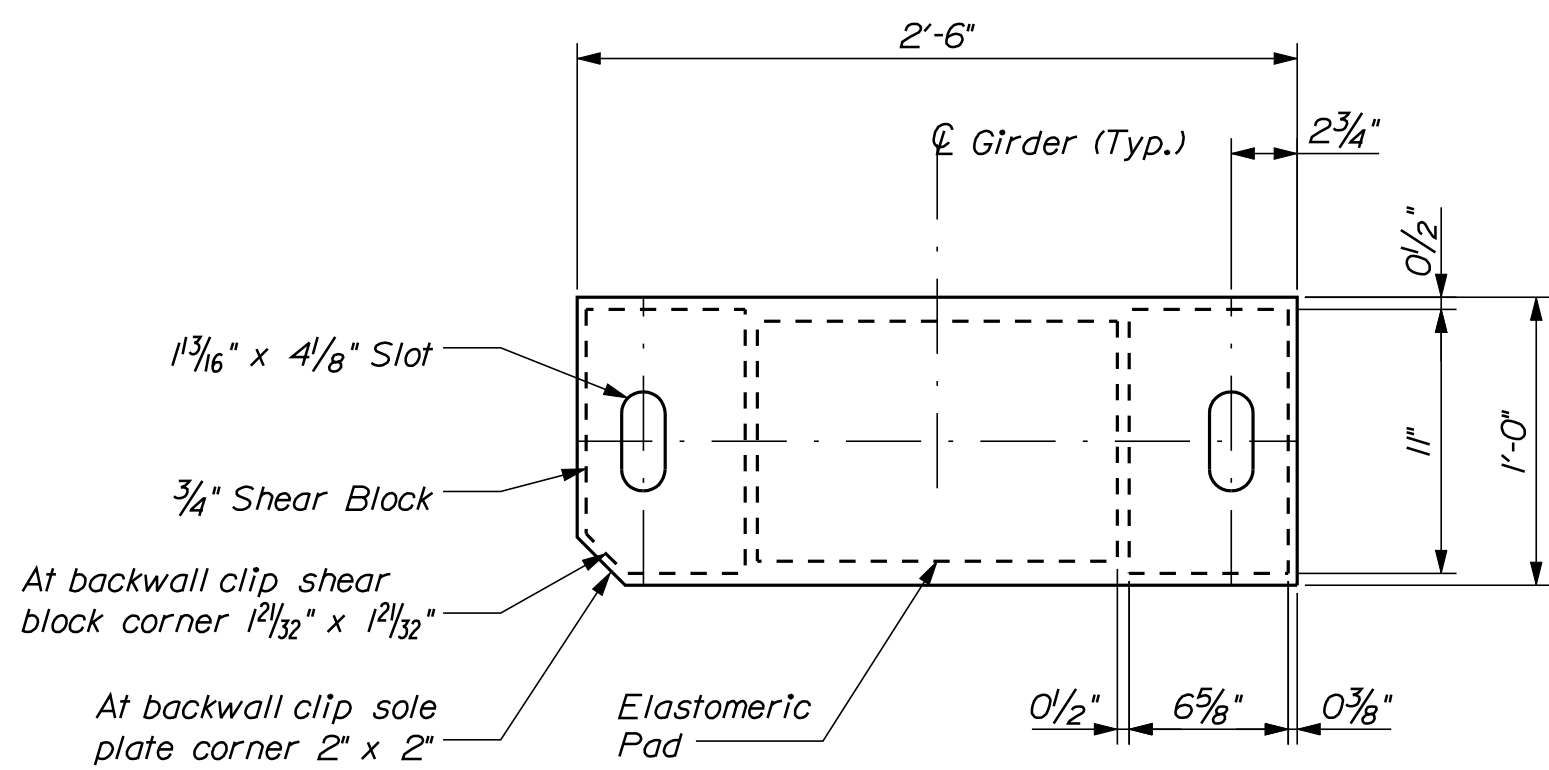
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CHECKED-REVIEWED	Erd. & Anh.	M. Poulin	
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DATE			

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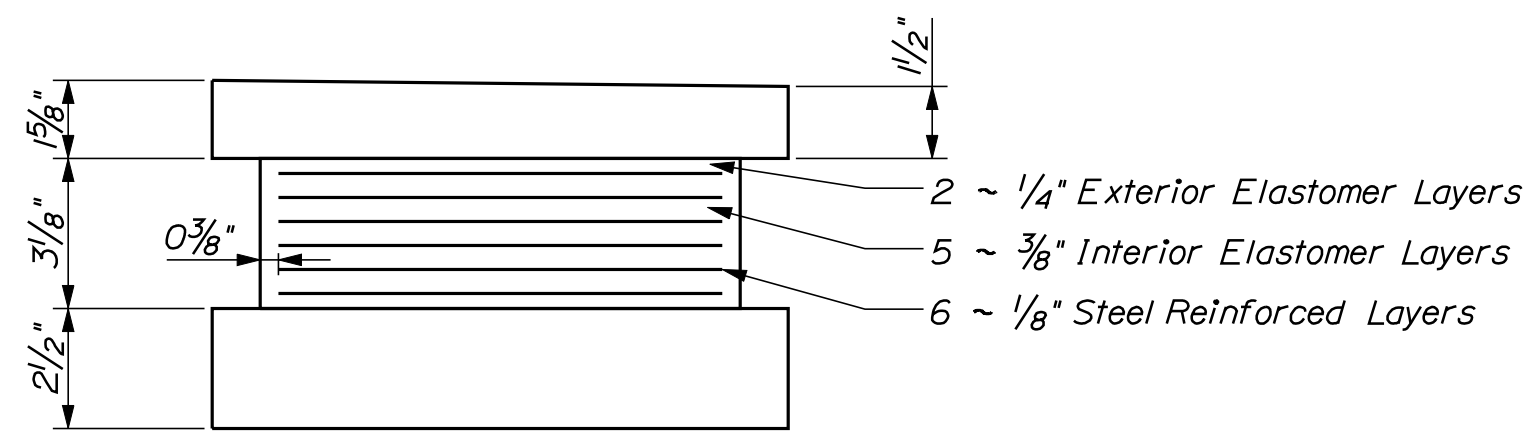
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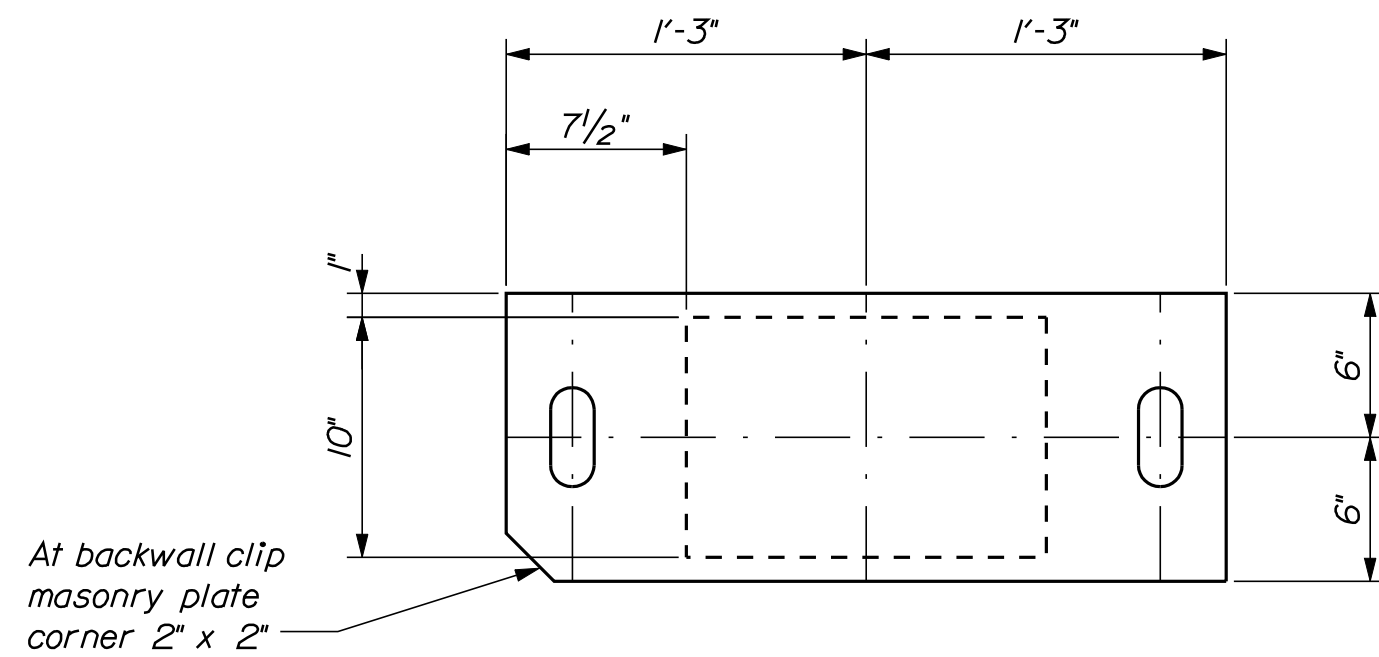
EXPANSION BEARING SOLE PLATE PLAN

Abutment No. 1 Bearing shown; Abutment No. 2 Bearings similar except clipped corners will be diagonally opposite. Pier No. 1 Bearings similar.

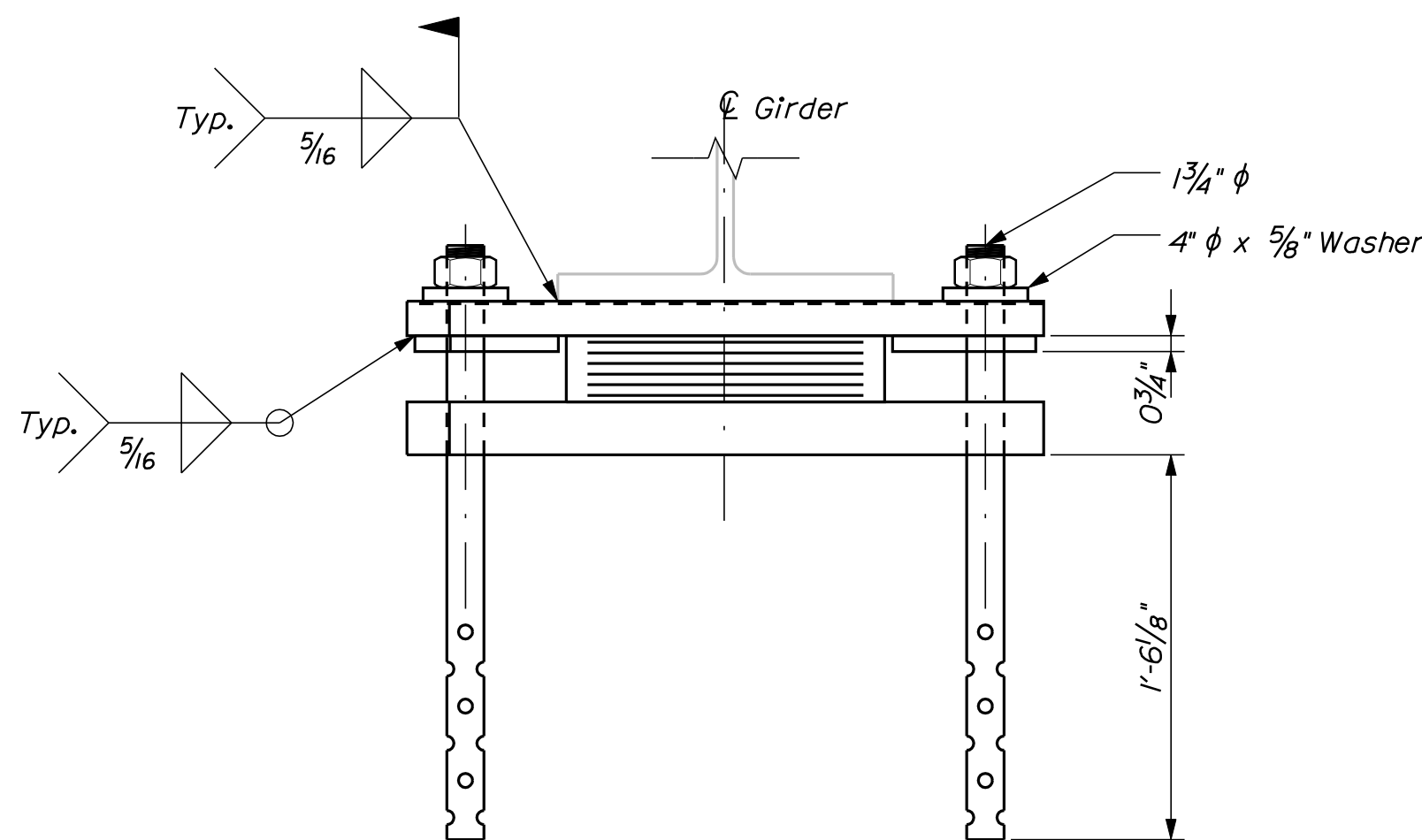


EXPANSION BEARING SECTION

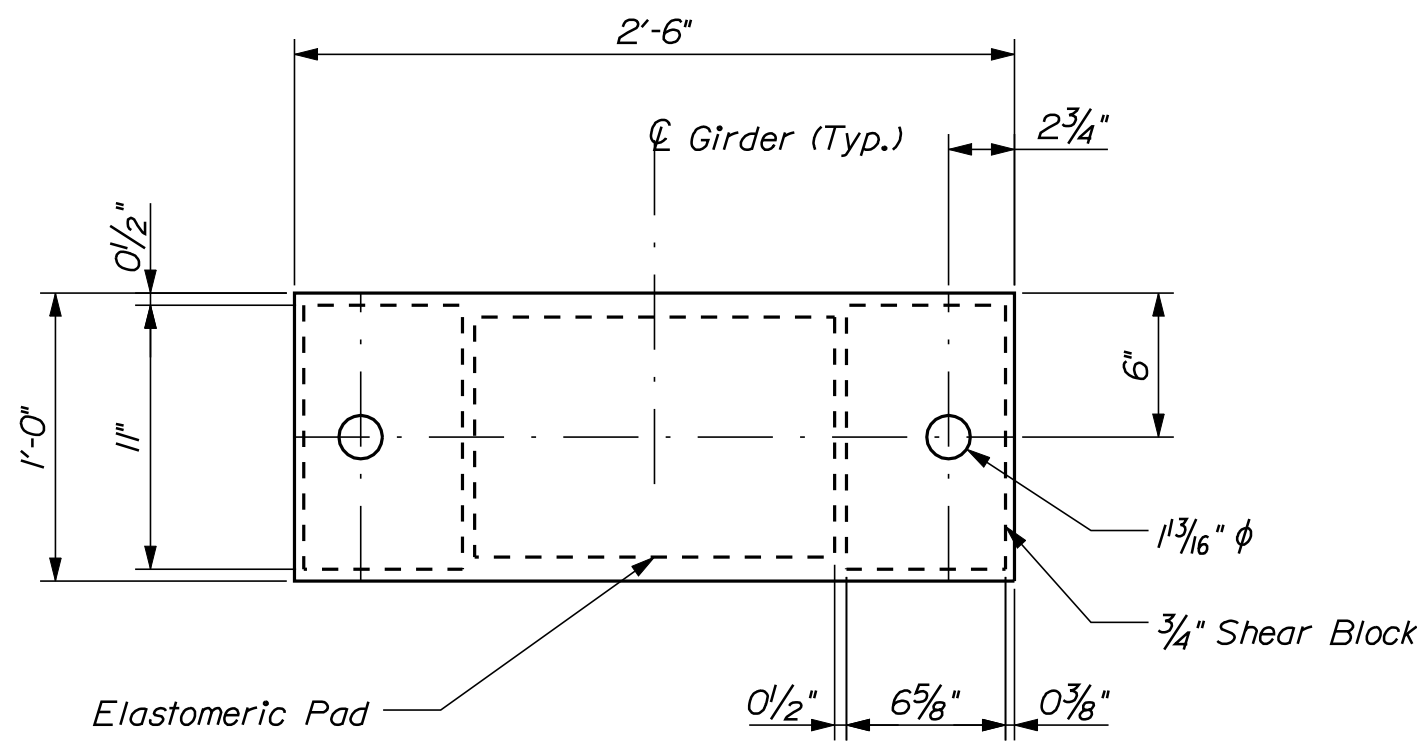
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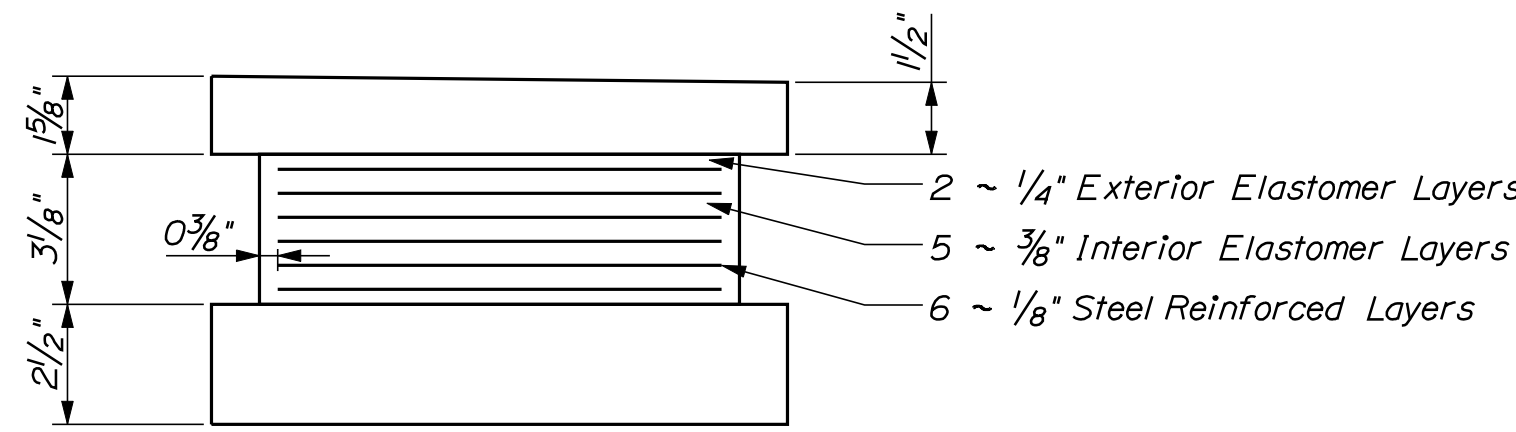
EXPANSION BEARING MASONRY PLATE PLAN



EXPANSION BEARING ELEVATION

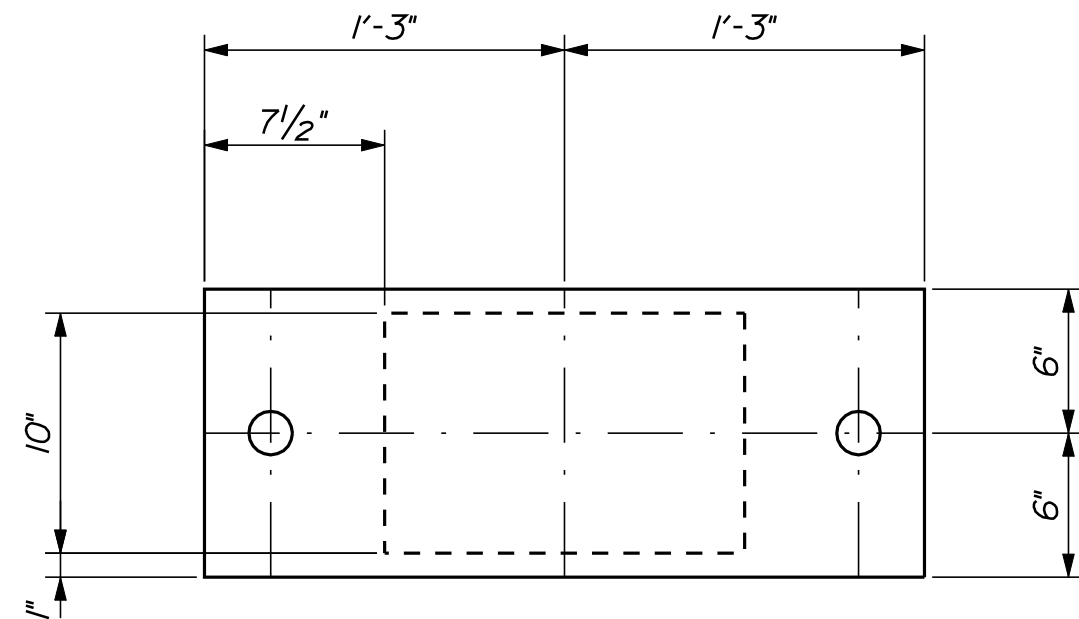


PIER No. 2 FIXED BEARING SOLE PLATE PLAN

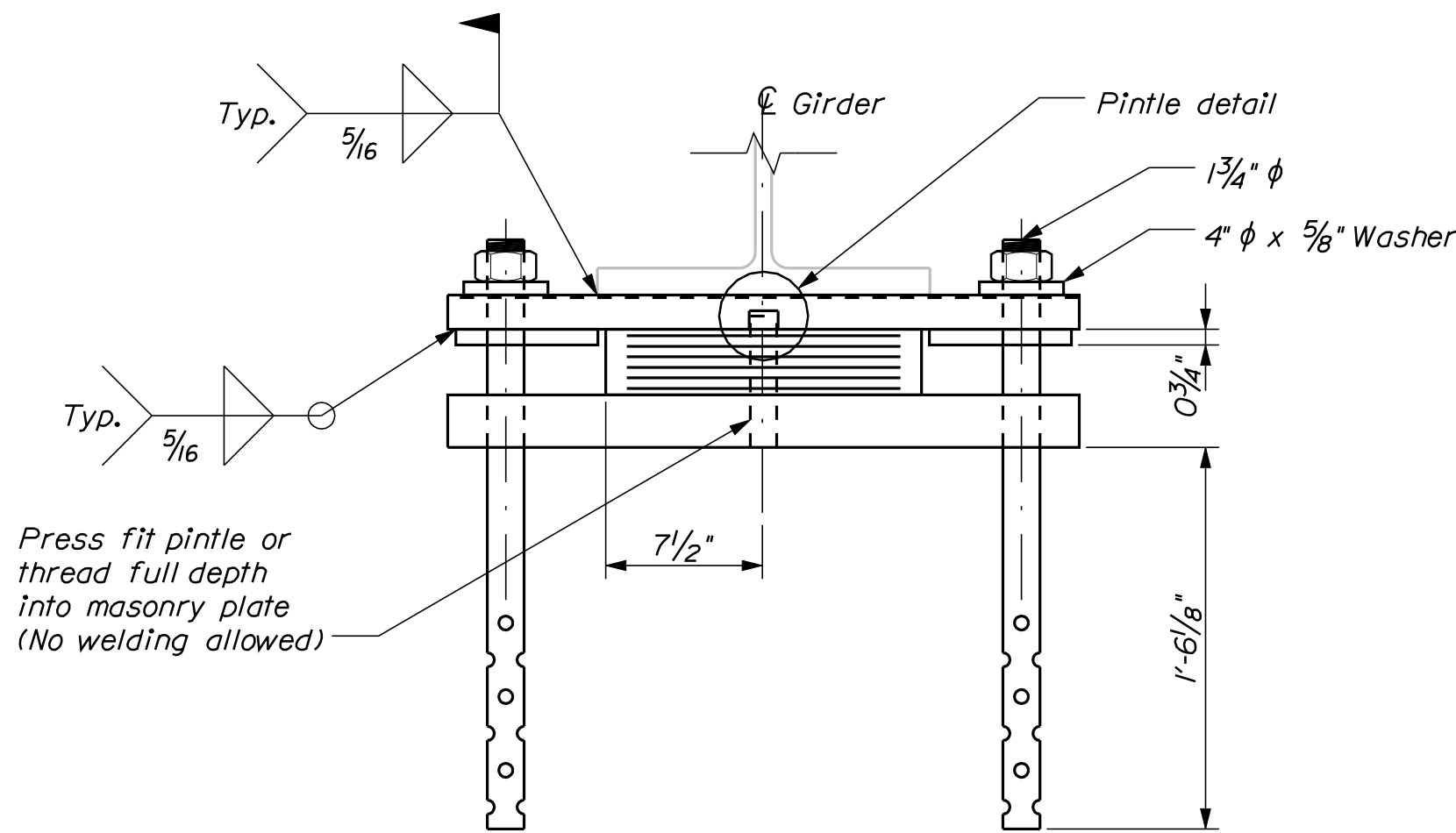


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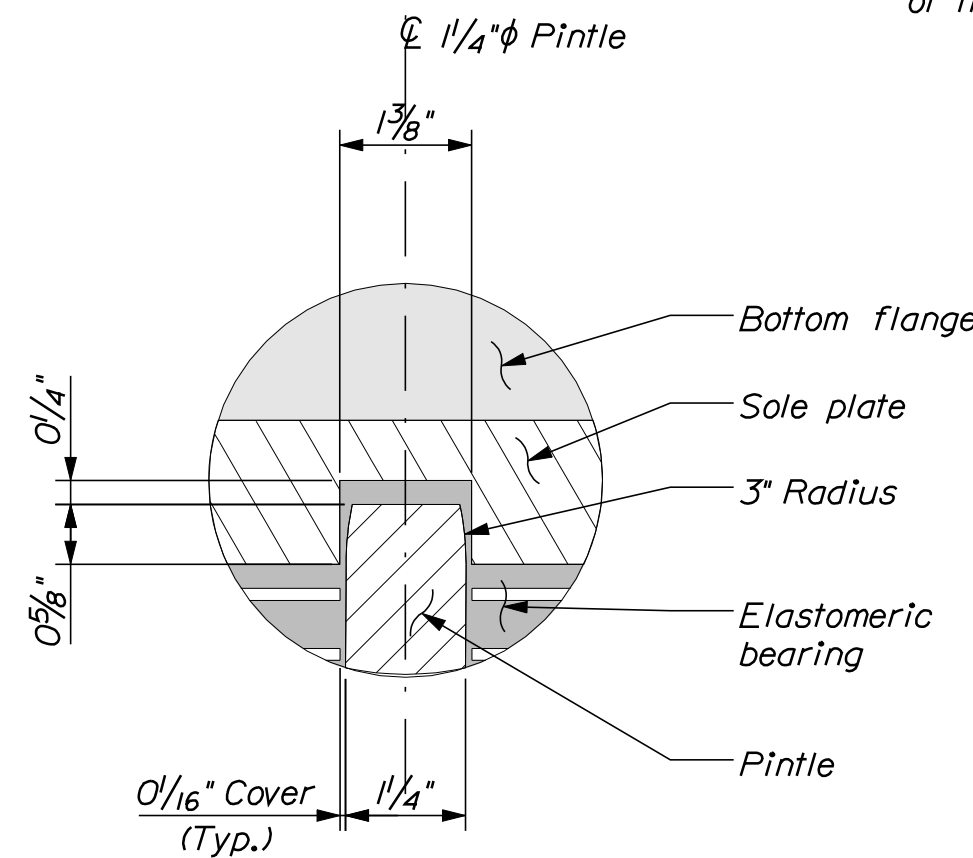
Upstation →



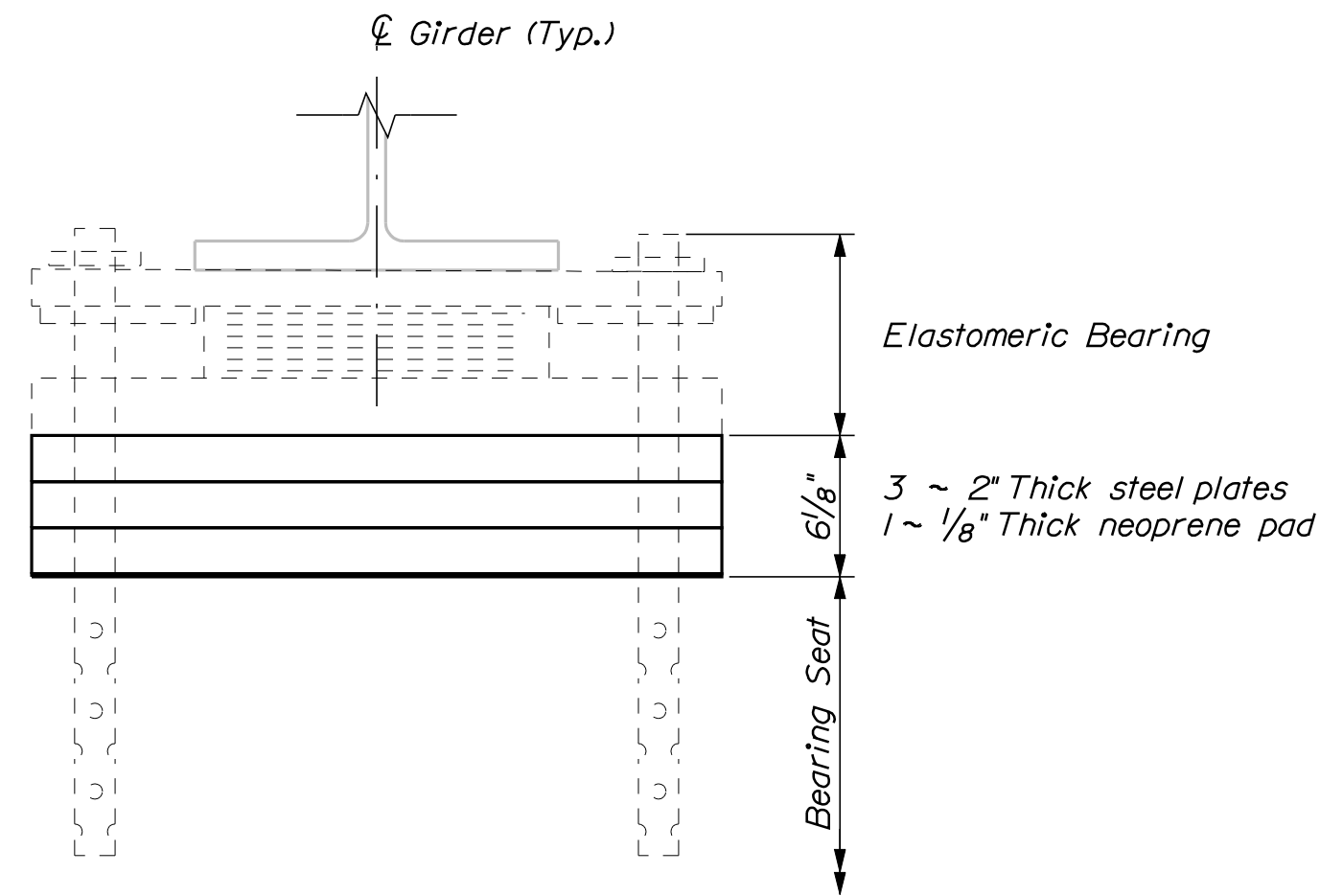
PIER No. 2 FIXED BEARING MASONRY PLATE PLAN



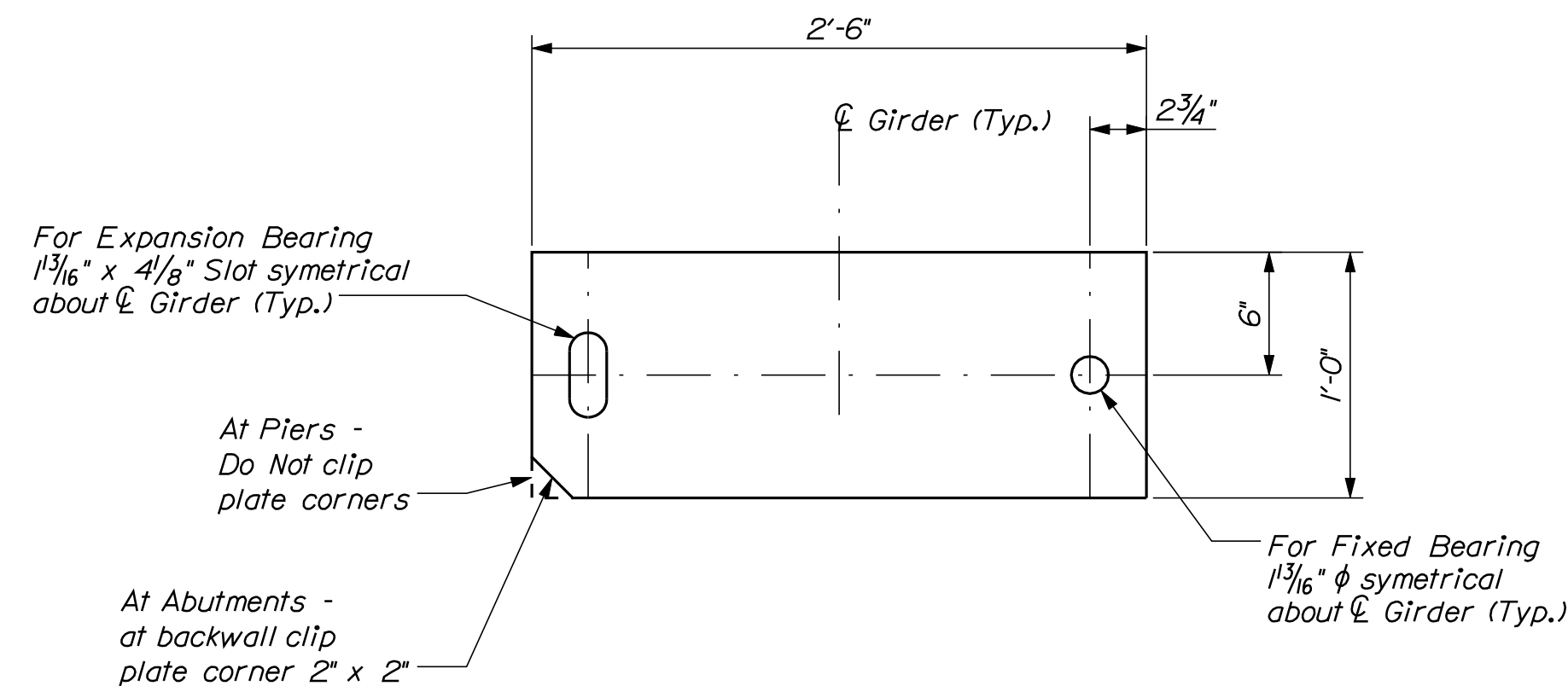
PIER No. 2 FIXED BEARING ELEVATION



PINTLE DETAIL



BEARING STAND SECTION



BEARING STAND PLAN (TYP.)

Abutment No. 1 Bearing plates shown; Abutment No. 2 Bearing plates similar except clipped corners will be diagonally opposite, against backwall. Pier No. 1 Bearing plates similar, without clipped corners.

ELASTOMERIC BEARING NOTES

- The shear modulus of the elastomer shall be 130 psi. ± 15%.
- Vulcanizing of the elastomer to the steel plates shall be done during the primary mold process.
- Masonry plates, sole plates and shear pins shall meet the requirements of ASTM A 709/A 709M, Grade 50 or 50W. Anchor rods shall meet the requirements of ASTM F 1554, Grade 105 and shall be swaged on the embedded portion of the rod.
- Masonry plates shall be galvanized in accordance with Section 506. Sole plates for steel superstructures shall be treated in the same manner as the structural steel. Anchor rods, washers, nuts and shear pins shall be galvanized to ASTM A 153 or ASTM B 695, Class 50, Type I.
- All bearings shall be marked prior to shipping. The marks shall include the bearing location on the bridge and a direction arrow that points upstation. All marks shall be permanent and shall be visible after the bearing is installed.
- Bearings shall be covered during transit.
- The bearings are designed so that the superstructure may be erected when the ambient air temperature is within the range of 65 °F and 90 °F. If the ambient air temperature is outside this range, the bearings shall be reset as directed by the Resident.
- All necessary precautions shall be taken to protect bearing components from field weld flash and spatter. Heat from welding operations shall be controlled such that steel adjacent to the elastomer does not exceed 200 °F. The temperature shall be verified by the use of temperature indicating crayons or other suitable means.
- Upset the threads on the anchor rods after assembly of the bearing.
- Bearings at Abutment No. 1, Pier No. 1 and Abutment No. 2 shall be expansion type as detailed. Pier No. 2 bearing shall be fixed.
- The Contractor shall verify the elevation of the bottom of the flange at each bearing, design the proper shim plates, or other bearing support types, so as to not change that elevation and have the design approved by the Department prior to the replacement of the bearings.

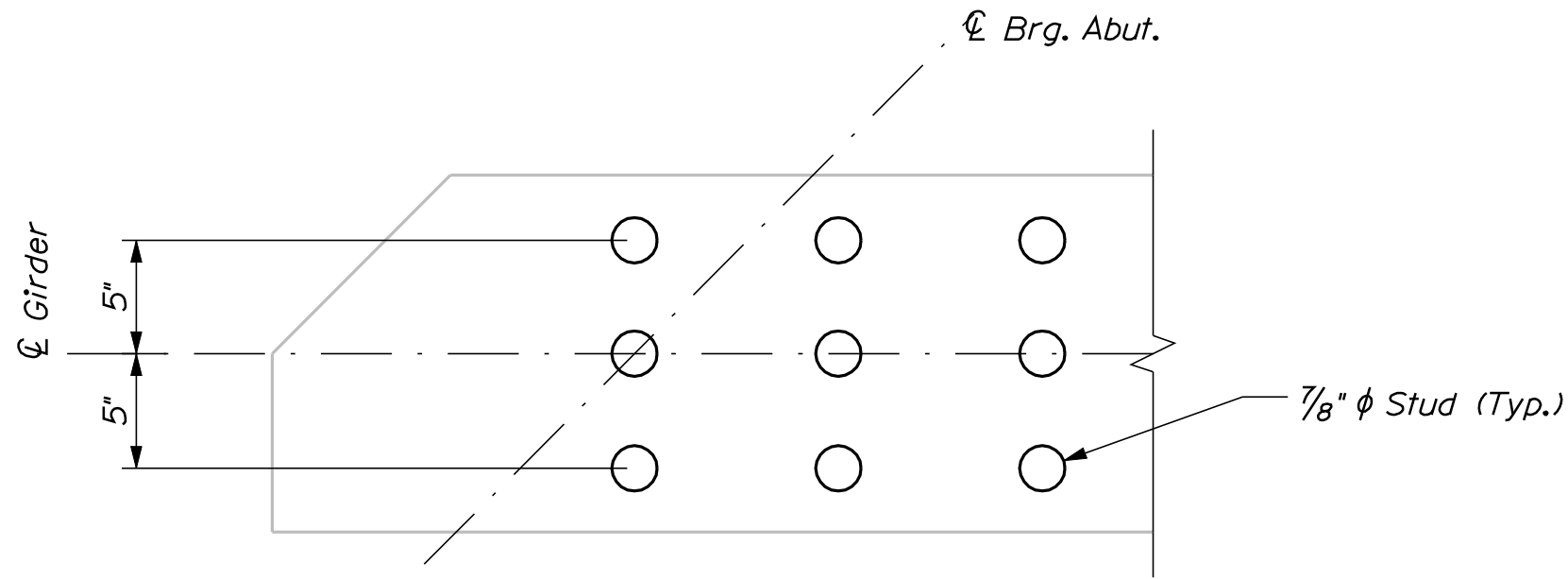
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BR-1930(600)				PIN				19306.00			
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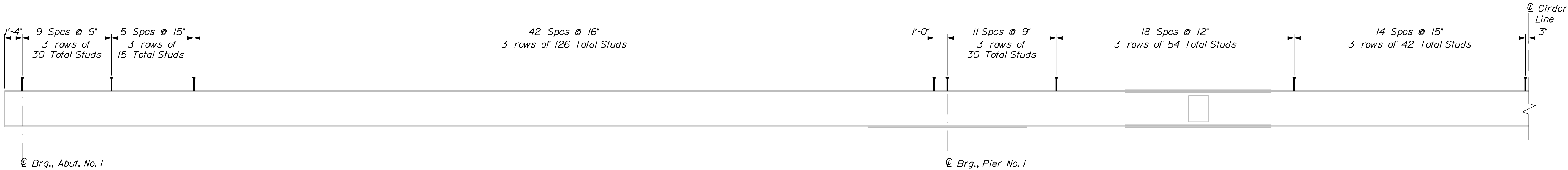
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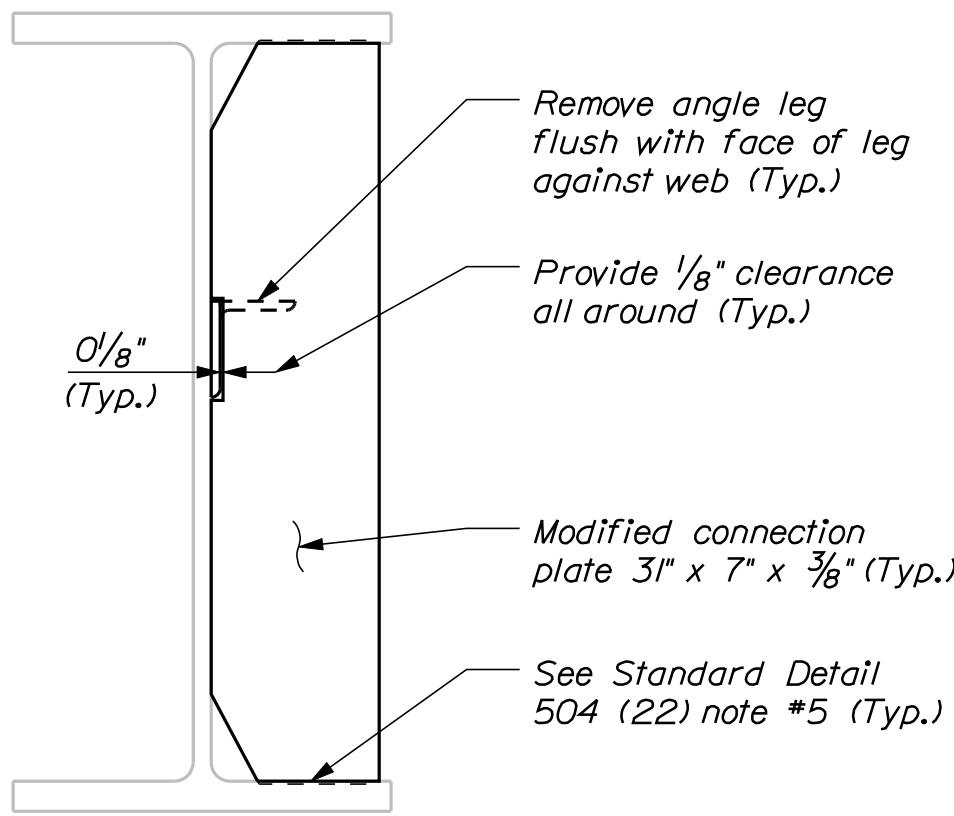
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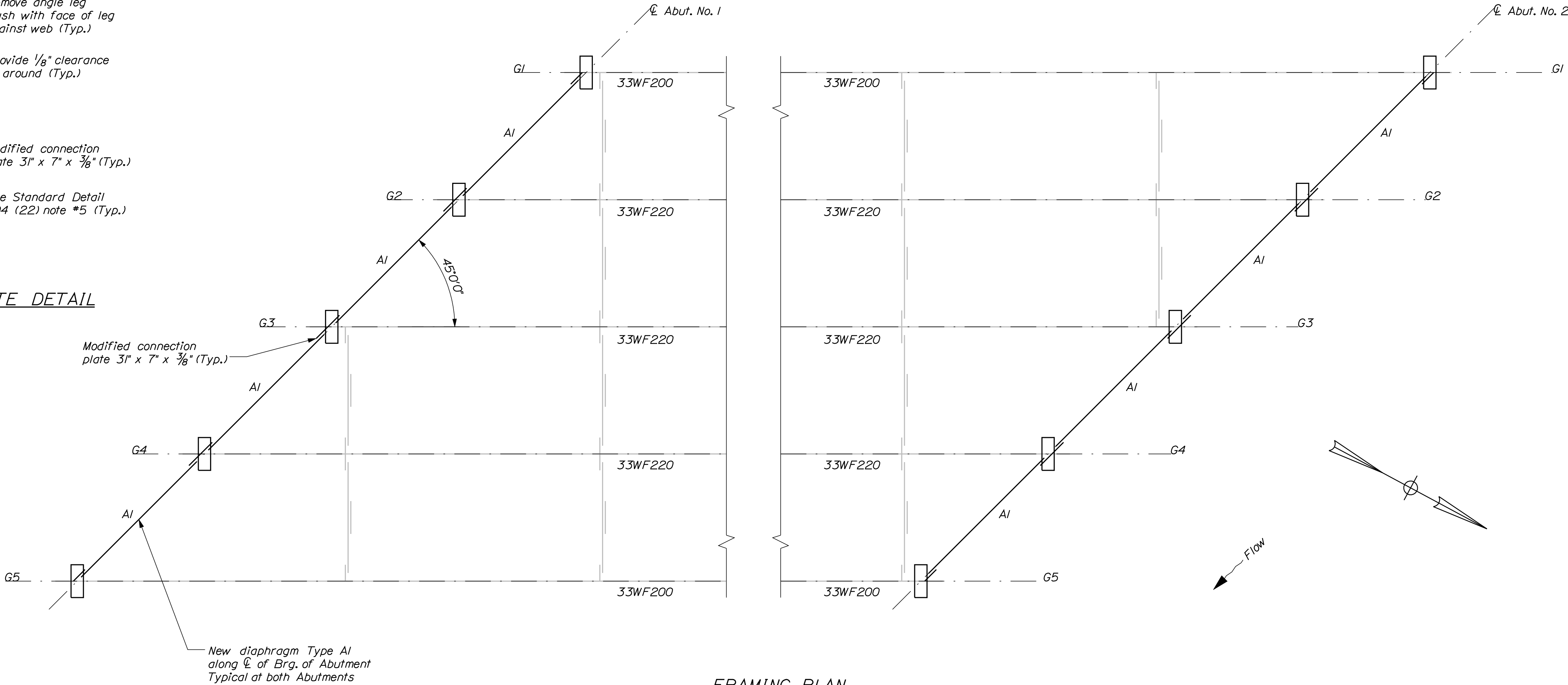
TRANSVERSE SHEAR STUD SPACING



SHEAR CONNECTOR LAYOUT



CONNECTION PLATE DETAIL



FRAMING PLAN

STRUCTURAL STEEL NOTES

1. Connection plates shall be plumb after erection and dead loading of the structure.
2. Paint shall be shop applied except in areas shown on plans. Some field painting may be necessary including areas around field welds and touch up of damaged areas. Any field painting necessary shall not be paid for directly but shall be considered incidental to Item No. 506.144 - Field Painting of Existing and New Structural Steel.
4. Detailing, fabricating and erection of new diaphragms Type A1 along centerline of bearing shall follow Standard Specification 504- Structural Steel.
5. The Contractor shall verify the spacing of the existing Girders for the correct diaphragm lengths.

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																				19306.00	
																				BRIDGE PLANS	

BOTTOM OF SLAB ELEVATIONS

Span No. 1											
	℄ Brg	.1 Span	.2 Span	.3 Span	.4 Span	.5 Span	.6 Span	.7 Span	.8 Span	.9 Span	℄ Brg.
G1	426.563	426.471	426.375	426.275	426.196	426.068	425.949	425.828	425.705	425.586	425.473
G2	426.838	426.746	426.651	426.553	426.446	426.343	426.227	426.104	425.980	425.861	425.748
G3	427.113	427.021	426.926	426.828	426.721	426.618	426.502	426.379	426.255	426.136	426.023
G4	427.058	426.986	426.891	426.793	426.686	426.583	426.467	426.344	426.220	426.101	425.988
G5	427.013	426.931	426.855	426.755	426.649	426.538	426.429	426.308	426.185	426.066	425.953

Span No. 2											
	℄ Brg	.1 Span	.2 Span	.3 Span	.4 Span	.5 Span	.6 Span	.7 Span	.8 Span	.9 Span	℄ Brg.
G1	425.473	425.351	425.229	425.117	424.990	424.855	424.720	424.567	424.419	424.261	424.113
G2	425.748	425.626	425.505	425.393	425.266	425.132	424.996	424.843	424.536	424.536	424.388
G3	426.027	425.899	425.782	425.664	425.541	425.411	425.269	425.120	424.966	424.811	424.667
G4	425.992	425.864	425.747	425.629	425.506	425.376	425.234	425.085	424.931	424.776	424.632
G5	425.956	425.829	425.711	425.593	425.470	425.339	425.198	425.048	424.895	424.741	424.596

Span No. 3											
	℄ Brg	.1 Span	.2 Span	.3 Span	.4 Span	.5 Span	.6 Span	.7 Span	.8 Span	.9 Span	℄ Brg.
G1	424.113	424.016	423.915	423.818	423.719	423.618	423.519	423.405	423.285	423.151	423.023
G2	424.388	424.291	424.190	424.094	423.997	423.893	423.796	423.683	423.561	423.436	423.308
G3	424.663	424.566	424.465	424.369	424.272	424.168	424.071	423.958	423.836	423.711	423.583
G4	424.628	424.531	424.430	424.334	424.237	424.133	424.026	423.923	423.801	423.676	423.548
G5	424.593	424.496	424.395	424.298	424.199	424.098	423.989	423.885	423.765	423.641	423.513

TABLE OF DEFLECTIONS

All Values are in Inches

GIRDERS G2, G3 & G4											
Span No. 1											
	℄ Brg	7'	14'	21'	28'	35'	42'	49'	56'	63'	℄ Brg.
Fluid D/L	0.00	0.18	0.32	0.42	0.45	0.43	0.36	0.25	0.12	0.03	0.00
Superimp. D/L	0.00	0.04	0.08	0.12	0.13	0.12	0.11	0.07	0.03	0.01	0.00
Total D/L	0.00	0.22	0.40	0.54	0.58	0.55	0.47	0.32	0.15	0.04	0.00
Span No. 2											
	℄ Brg	7'	14'	21'	28'	35'	42'	49'	56'	63'	℄ Brg.
Fluid D/L	0.00	0.07	0.23	0.39	0.52	0.57	0.52	0.39	0.23	0.07	0.00
Superimp. D/L	0.00	0.03	0.1	0.15	0.18	0.2	0.18	0.15	0.1	0.03	0.00
Total D/L	0.00	0.1	0.33	0.54	0.7	0.77	0.7	0.54	0.33	0.1	0.00
Span No. 3											
	℄ Brg	7'	14'	21'	28'	35'	42'	49'	56'	63'	℄ Brg.
Fluid D/L	0.00	0.03	0.12	0.25	0.36	0.43	0.45	0.42	0.32	0.18	0.00
Superimp. D/L	0.00	0.01	0.03	0.07	0.11	0.12	0.13	0.12	0.08	0.04	0.00
Total D/L	0.00	0.04	0.15	0.32	0.47	0.55	0.58	0.54	0.4	0.22	0.00

GIRDERS G1 & G5											
Span No. 1											
	℄ Brg	7'	14'	21'	28'	35'	42'	49'	56'	63'	℄ Brg.
Fluid D/L	0.00	0.16	0.28	0.37	0.41	0.39	0.32	0.22	0.11	0.03	0.00
Superimp. D/L	0.00	0.06	0.11	0.14	0.15	0.15	0.12	0.08	0.04	0.01	0.00
Total D/L	0.00	0.22	0.39	0.51	0.56	0.54	0.44	0.3	0.15	0.04	0.00
Span No. 2											
	℄ Brg	7'	14'	21'	28'	35'	42'	49'	56'	63'	℄ Brg.
Fluid D/L	0.00	0.06	0.21	0.35	0.47	0.51	0.47	0.35	0.21	0.06	0.00
Superimp. D/L	0.00	0.04	0.11	0.18	0.22	0.24	0.22	0.18	0.11	0.04	0.00
Total D/L	0.00	0.1	0.32	0.53	0.69	0.75	0.69	0.53	0.32	0.1	0.00
Span No. 3											
	℄ Brg	7'	14'	21'	28'	35'	42'	49'	56'	63'	℄ Brg.
Fluid D/L	0.00	0.03	0.11	0.22	0.32	0.39	0.41	0.37	0.28	0.16	0.00
Superimp. D/L	0.00	0.01	0.04	0.08	0.12	0.15	0.15	0.14	0.11	0.06	0.00
Total D/L	0.00	0.04	0.15	0.3	0.44	0.54	0.56	0.51	0.39	0.22	0.00

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PISCATAQUIS RIVER
ABBOT
PISCATAQUIS COUNTY
BOTTOM OF SLAB ELEVATIONS

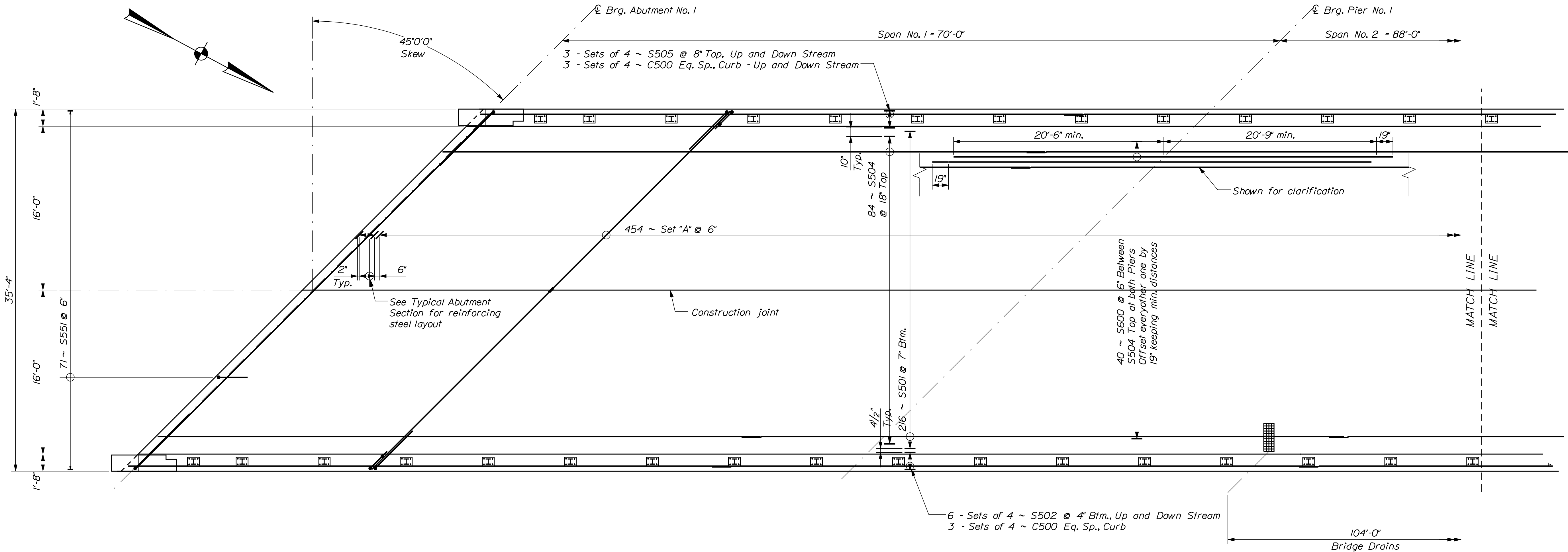
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OF 15

PROJ. MANAGER
DESIGN-DETAILED
CHECKED-REVIEWED
DESIGNS DETAILLED
DESIGNS DETAILLED
REVISIONS 1
REVISIONS 2
REVISIONS 3
REVISIONS 4
FIELD CHANGES

BY
D. Sullivan
M. Poulin

DATE
OCT 2012

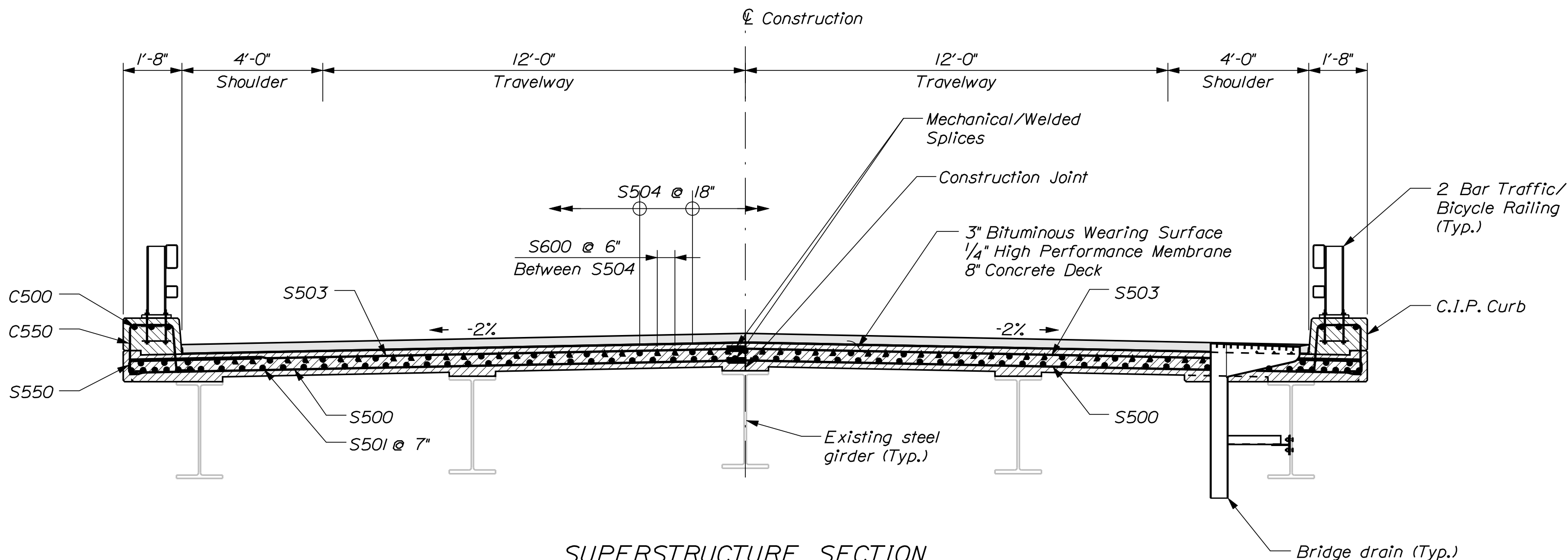
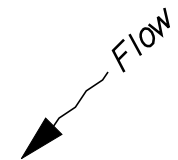
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DATE



Set "A" -> 2- S500 Btm., 2- S503 Top, 2- S550, 2 - C550
Set "B" -> 2- S500 Btm., 2- S503 Top, 2- S552, 2 - C550
Set "C" -> 2- S500 Btm., 2- S503 Top, 2- S553, 2 - C550

SUPERSTRUCTURE PLAN

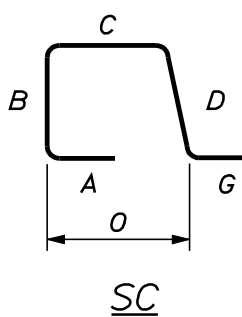
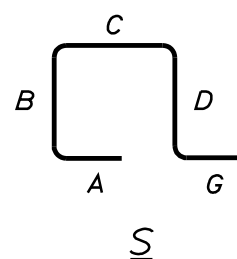
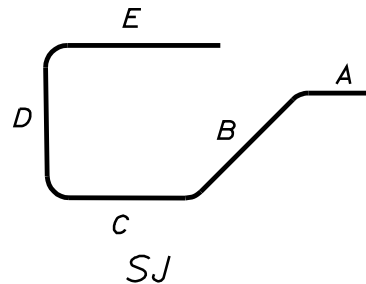
Rotate 180° about center of bridge for reinforcing steel layout.
See full superstructure plan for bridge drain layout.



SUPERSTRUCTURE SECTION

SUPERSTRUCTURE REINFORCING STEEL															
MARK	QTY	LENGTH	TYPE	A	B	C	D	E	F	G	H	O	R	LOCATION	
S500	924	24'-9"												Bottom Transverse	
S501	216	58'-9"												Bottom Longitudinal	
S502	48	58'-9"												Bottom Longitudinal Overhang	
S503	924	24'-9"												Top Transverse	
S504	84	58'-9"												Top Longitudinal	
S505	24	58'-9"												Top Longitudinal Overhang	
* C500	24	58'-9"												Transverse Curb	
S600	40	42'-10"												Top Longitudinal at Piers	
S550	924	6'-0"	S	0'-3"	0'-5"	5'-4"								Transverse Overhang	
S551	142	6'-7 1/4"	SJ	0'-8"	1'-5"	1'-4 1/4"	1'-5 1/2"	1'-8"						Longitudinal Superstructure Ends	
S552	12	7'-0 1/2"	S	0'-3"	1'-5 1/2"	5'-4"								Trans. Overhang Superstructure Ends	
S553	4	6'-9"	S	0'-3"	1'-2"	5'-4"								Trans. Overhang Superstructure Ends	
* C550	924	5'-8"	SC	0'-6"	1'-4"	2'-0"	1'-4"			0'-6"		2'-0"		Transverse Curb	

NOTE: C500 and C550 will be Epoxy-Coated Reinforcing Steel and will be paid for under Item # 503.14 Epoxy-Coated Reinforcing Steel, Fabricated and Delivered and Item # 503.15 Epoxy-Coated Reinforcing Steel, Placing



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SUPERSTRUCTURE PLANS

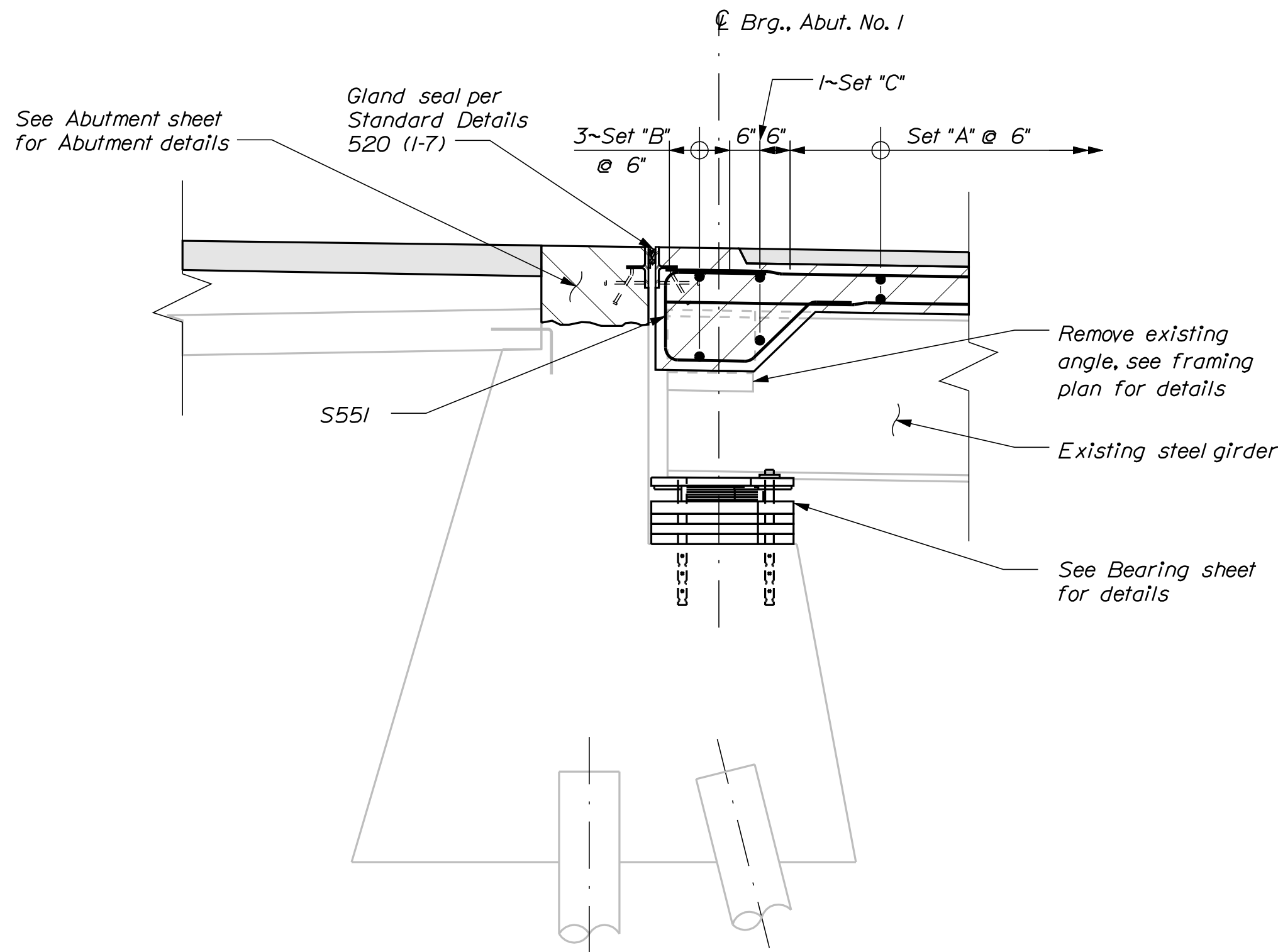
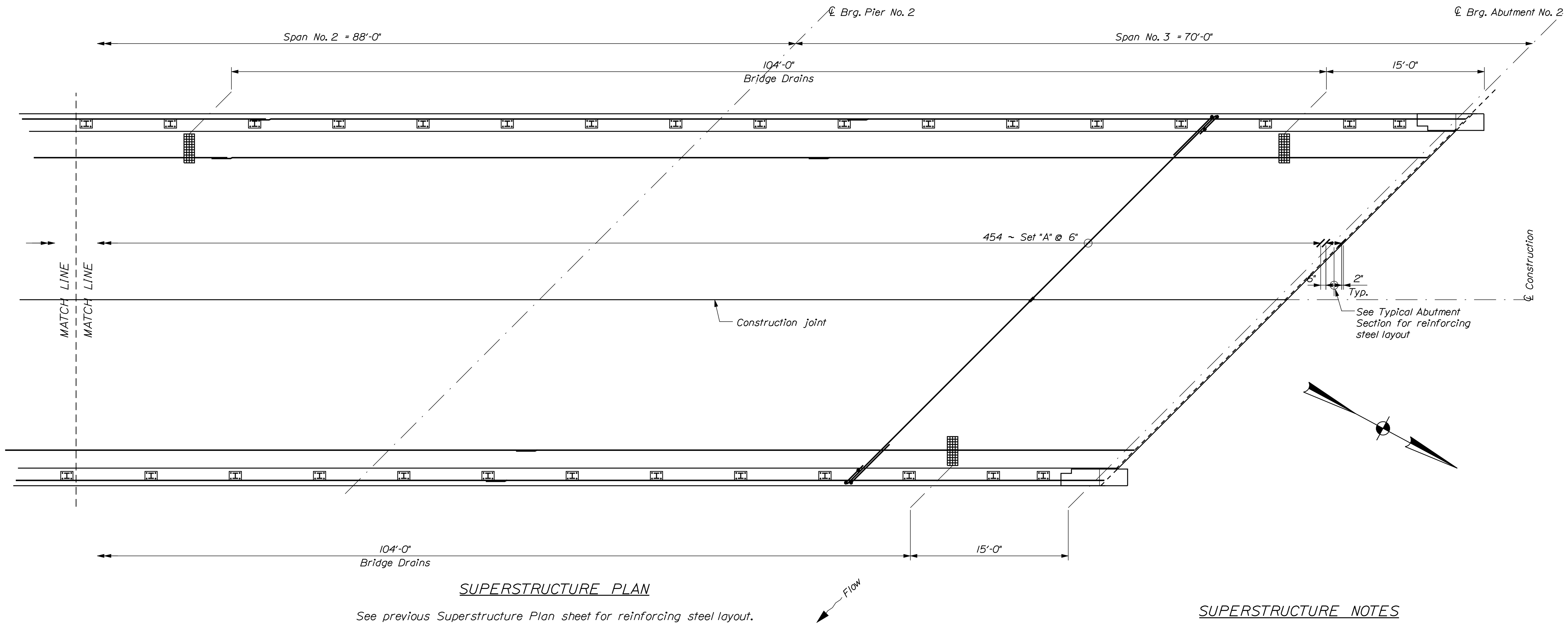
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Date:11/6/2012

Username: david.sullivan

Division: BRIDGE

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SUPERSTRUCTURE NOTES

1. The theoretical blocking used for design of the structure is 1 1/2 inch at the centerline of bearing of the abutments and piers. Refer to Standard Detail 502(02) for blocking details.
2. Reinforcing steel shall have a minimum concrete cover of 2 inches unless otherwise noted.
3. Adjust reinforcing steel to fit around the bridge drains in a manner approved by the Resident. Do not cut transverse reinforcing bars.
4. Form a one inch V-groove on the fascias at the horizontal joint between the curb and slab.
5. The superstructure slab concrete for each span shall be placed continuously and shall be kept plastic until the entire placement has been made.
6. At the Contractor's option, Precast Deck Panels may be used in place of the full depth cast-in-place deck slab, in accordance with Special Provisions Section 502, Structural Concrete - Precast Deck Panels, and in accordance with the Standard Details.
7. Payment for reinforcing steel fabricated, delivered, and placed in the cast-in-place portion of the structural concrete slab will be considered incidental to the appropriate Standard Specifications Section 502 pay item.
8. The seal(s) to be furnished shall have minimum Movement Rating(s) as follows:
Abutment No. 1 = 1.5 inch
Abutment No. 2 = 0.75 inch
9. The Resident shall approve the seals prior to fabrication of the Expansion Device.
10. Provide 3 additional stirrups in the curbs at each Transition Barrier location.
11. The Contractor shall install Transition Barrier vertical closed stirrups, as shown in Standard Details Section 526, prior to the placement of the curb or sidewalk concrete.

PAGE - CARR BRIDGE PISCATAQUIS RIVER PISCATAQUIS COUNTY ABBOT		PROJ. MANAGER DESIGN-DETAILED CHECKED-REVIEWED Erd. & Anlh. M. Poulin		S. Budge B. Reeves D. Sullivan	BY D. Sullivan M. Poulin	DATE OCT. 2012	STATE OF MAINE DEPARTMENT OF TRANSPORTATION
SUPERSTRUCTURE PLANS		DESIGN2-DETAILED2		SIGNATURE			BR-1930(600)
		DESIGN3-DETAILED3					
		REVISIONS 1		P.E. NUMBER			
		REVISIONS 2					
		REVISIONS 3		DATE			
		REVISIONS 4					
FIELD CHANGES		BRIDGE NO. 5725			PIN 19306.00	BRIDGE PLANS	
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