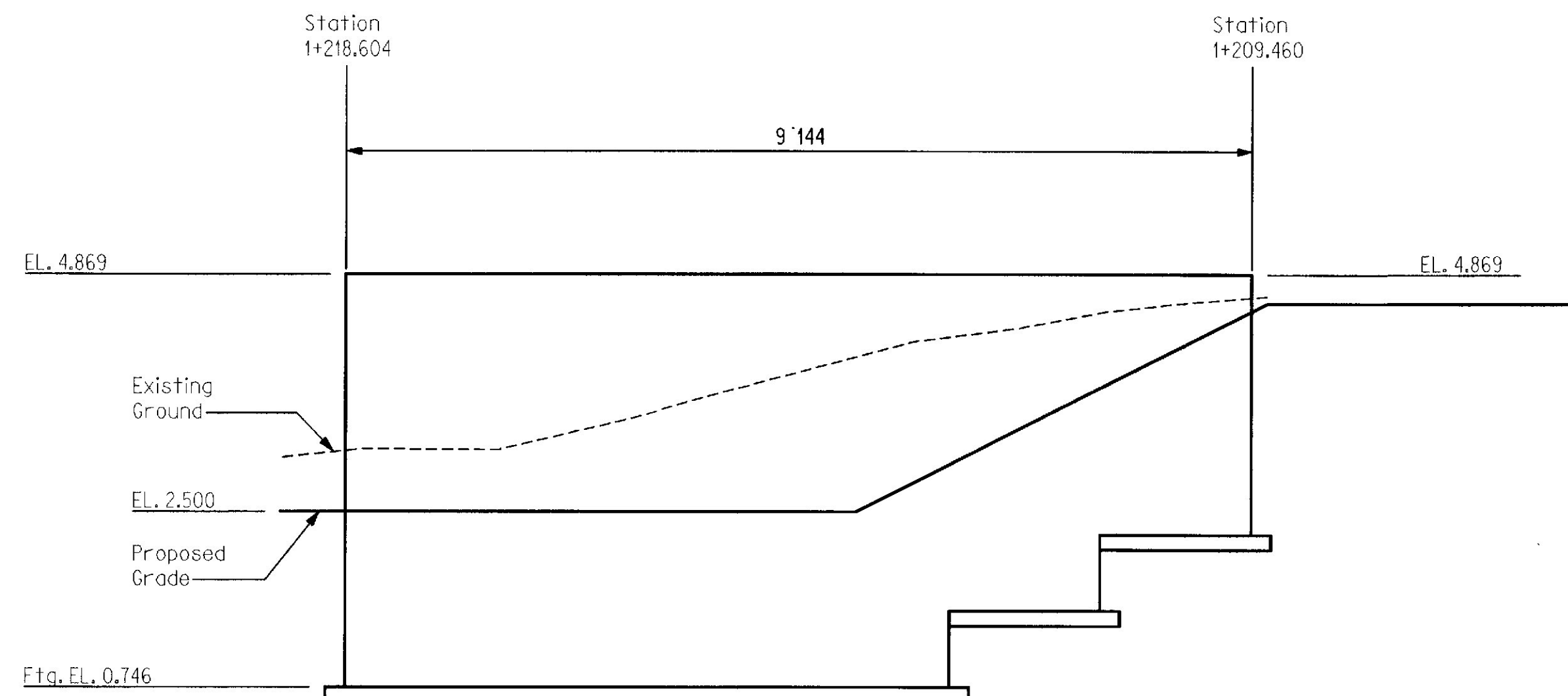


METRIC

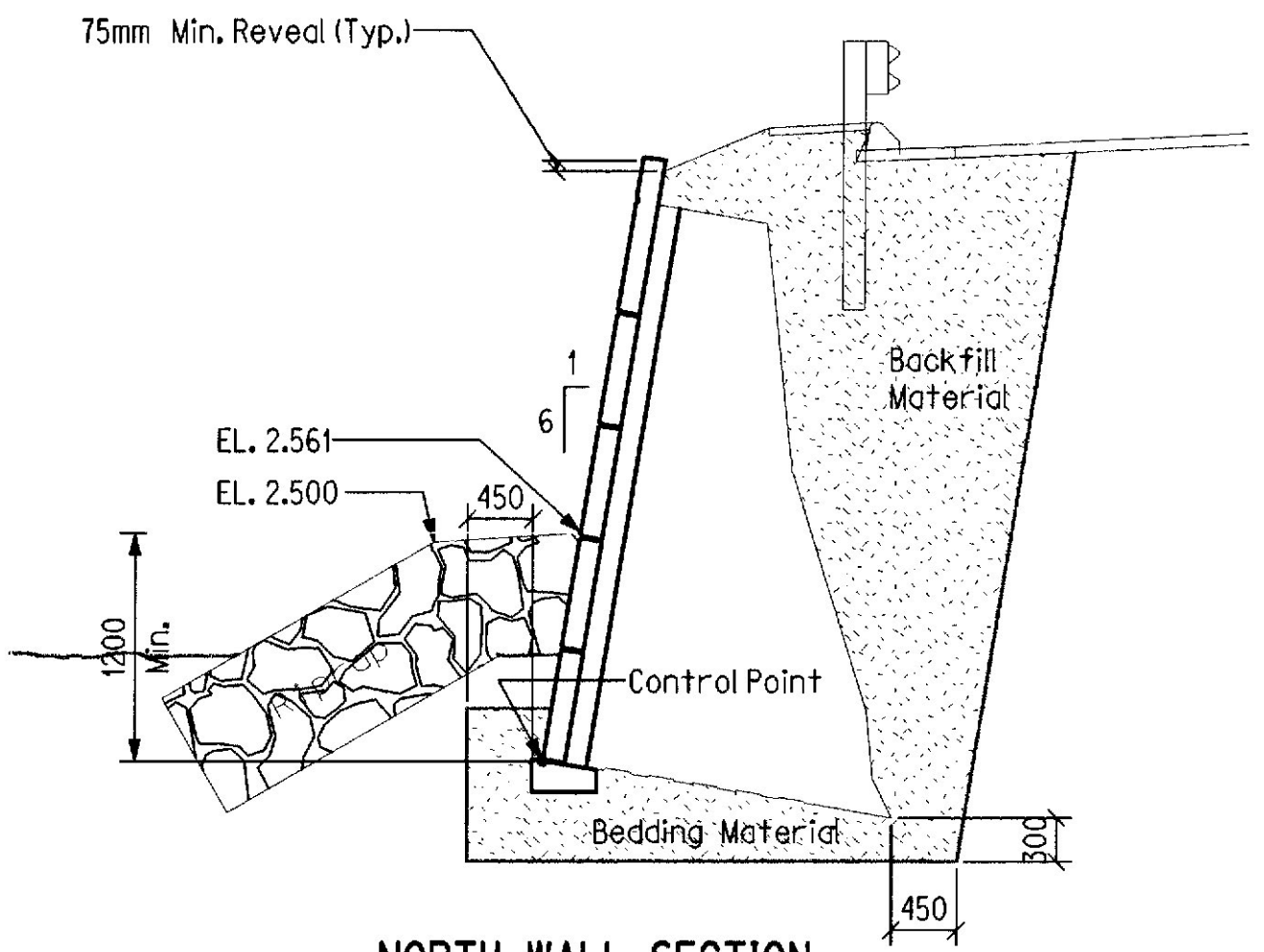
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

PHWA REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-795600IX	18	51

007956.00

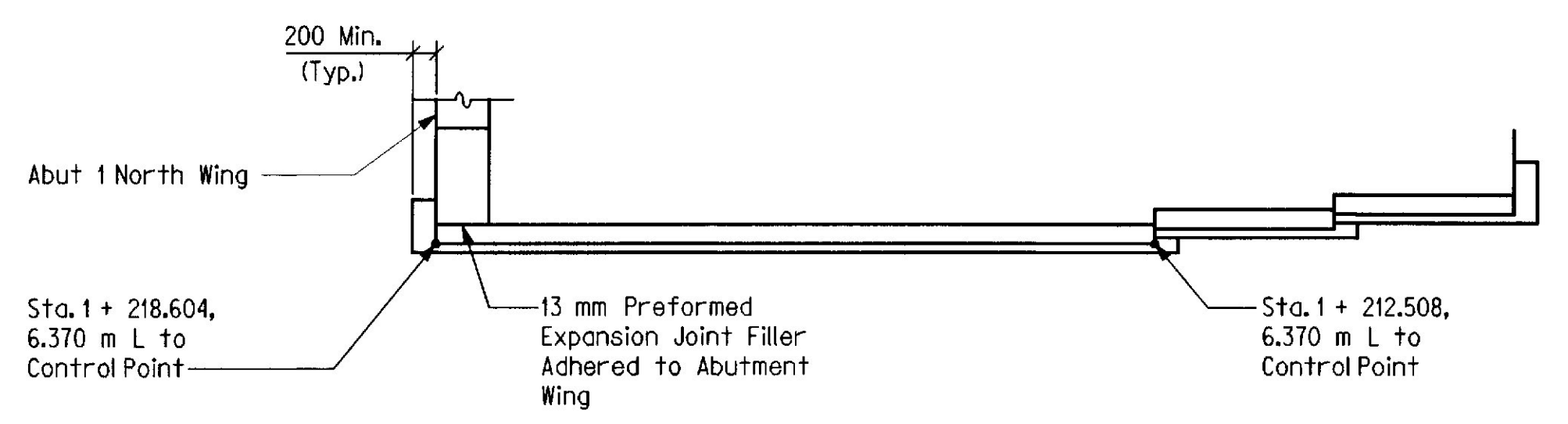


NORTH WALL ELEVATION

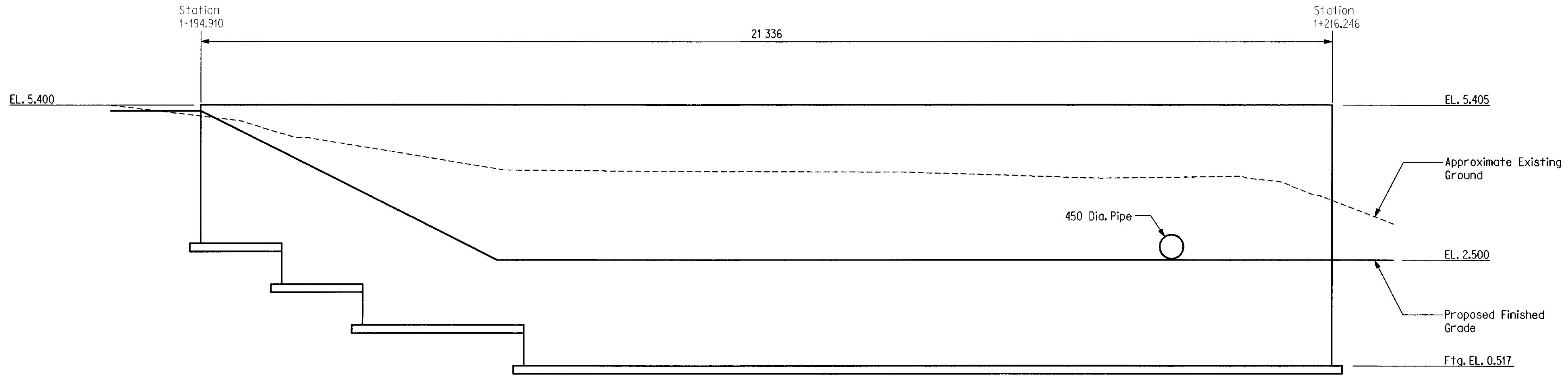


NORTH WALL SECTION

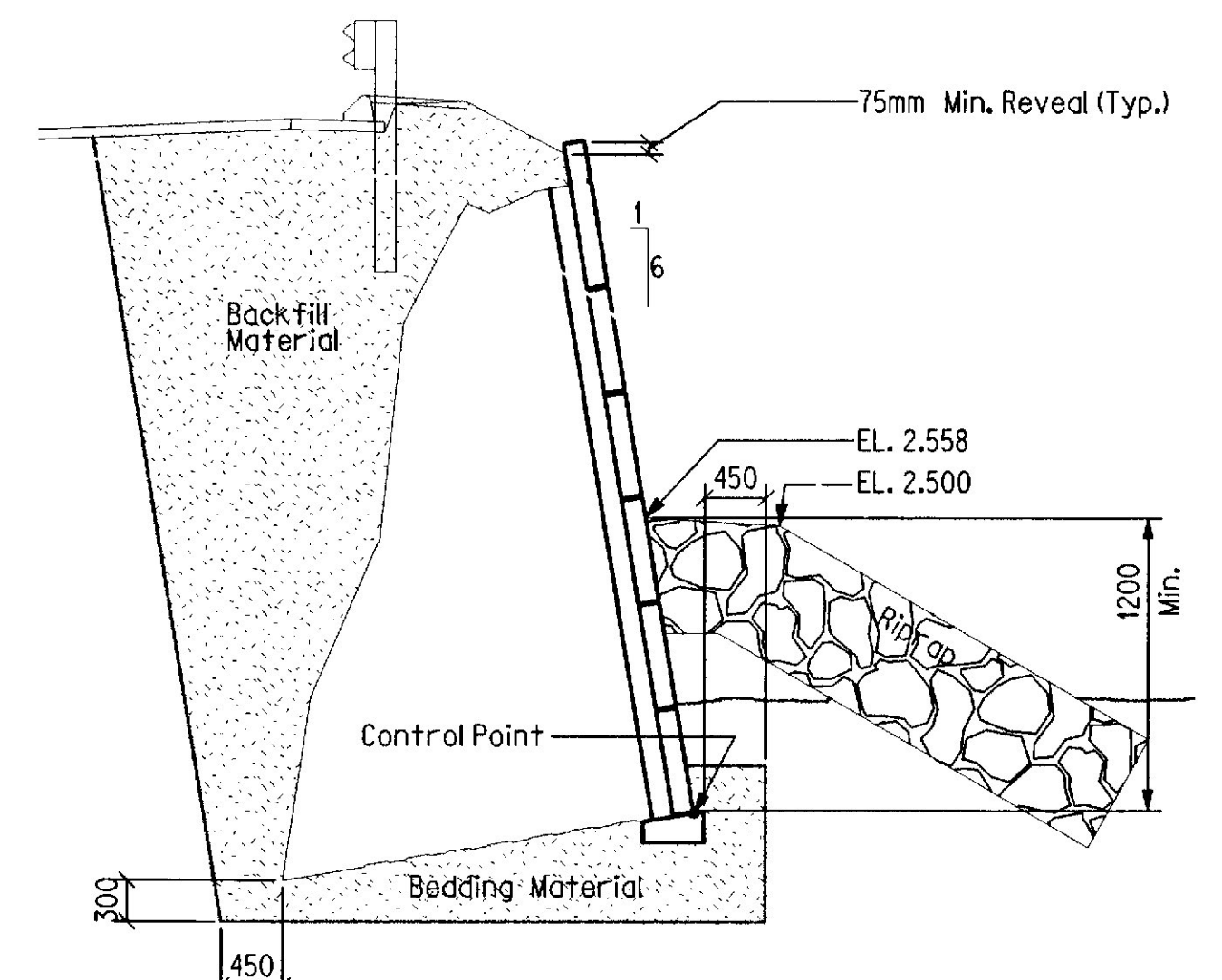
- MODULAR GRAVITY WALL NOTES**
- The Contractor shall provide a prefabricated modular reinforced concrete gravity wall in accordance with Special Provision 635. Plan details are shown for estimating purposes only.
 - The precast units shall be manufactured by the following, or equal: "T-Wall" as manufactured by Superior Concrete Co., Inc. of Auburn, Maine, "Doublewal" as manufactured by a licensee of Doublewal Corporation, Plainville, Connecticut.
 - A 300 mm wide drainage geotextile fabric shall be secured to the back of the units, at all horizontal joints up to EL. 3.0. Payment for the fabric and installation will be considered incidental to related contract items.
 - All reinforcing steel shall be epoxy-coated. The epoxy coating will be considered incidental to Item 635.14, Prefabricated Concrete Modular Gravity Wall.
 - Concrete used for the prefabricated concrete modular gravity wall shall be class LP.
 - Control point location is based on 150 mm thick wall face.
 - Chases in the face of modular gravity wall for 450 mm Diameter Option III Pipe shall be provided at Stations 1+213.22 RT (Invert EL. 2.53 +) and 1+281.38 LT (Invert EL. 2.60 +) to facilitate catch basin drainage outlets.



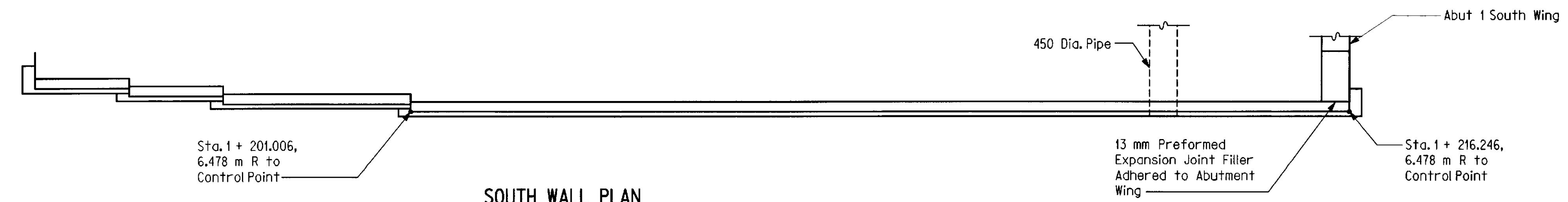
NORTH WALL PLAN



SOUTH WALL ELEVATION



SOUTH WALL SECTION



SOUTH WALL PLAN

PROJECT DESIGN ENGINEER	DATE
DESIGN-DETAILED	5/00
CHECKED	5/00
REVISIONS	
FIELD CHANGES	

PLANS

BRIDGE NO. 2927

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

**WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY**

ABUT 1 RET. WALL

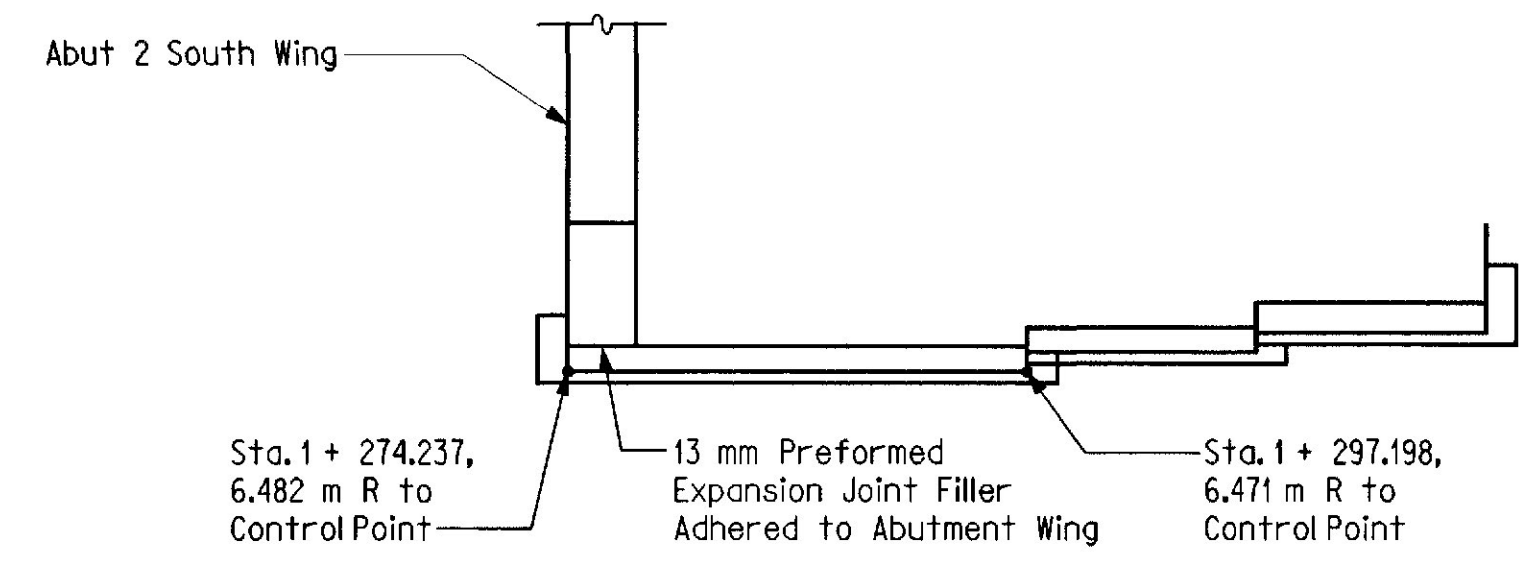
SHEET OF 18

Username: Dana Damren Date: 30 MAY 2000
 Division: BRIDGE
 Filename: ... \018_twall1.dgn

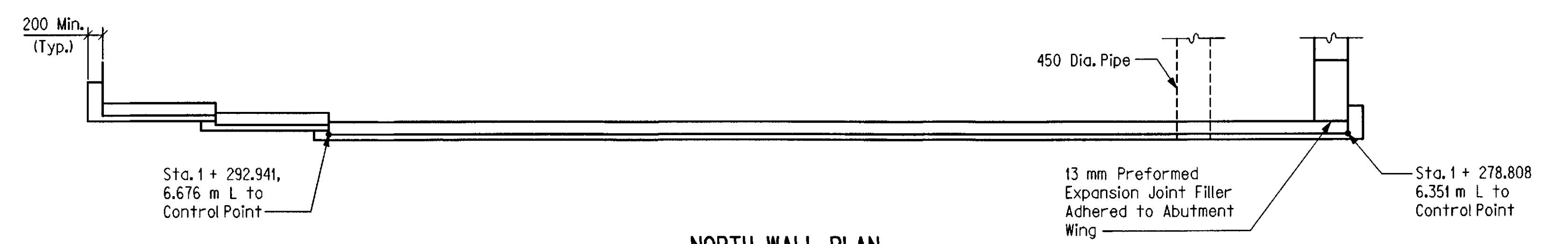
METRIC 1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

FHWA REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-7956/001X	19	51

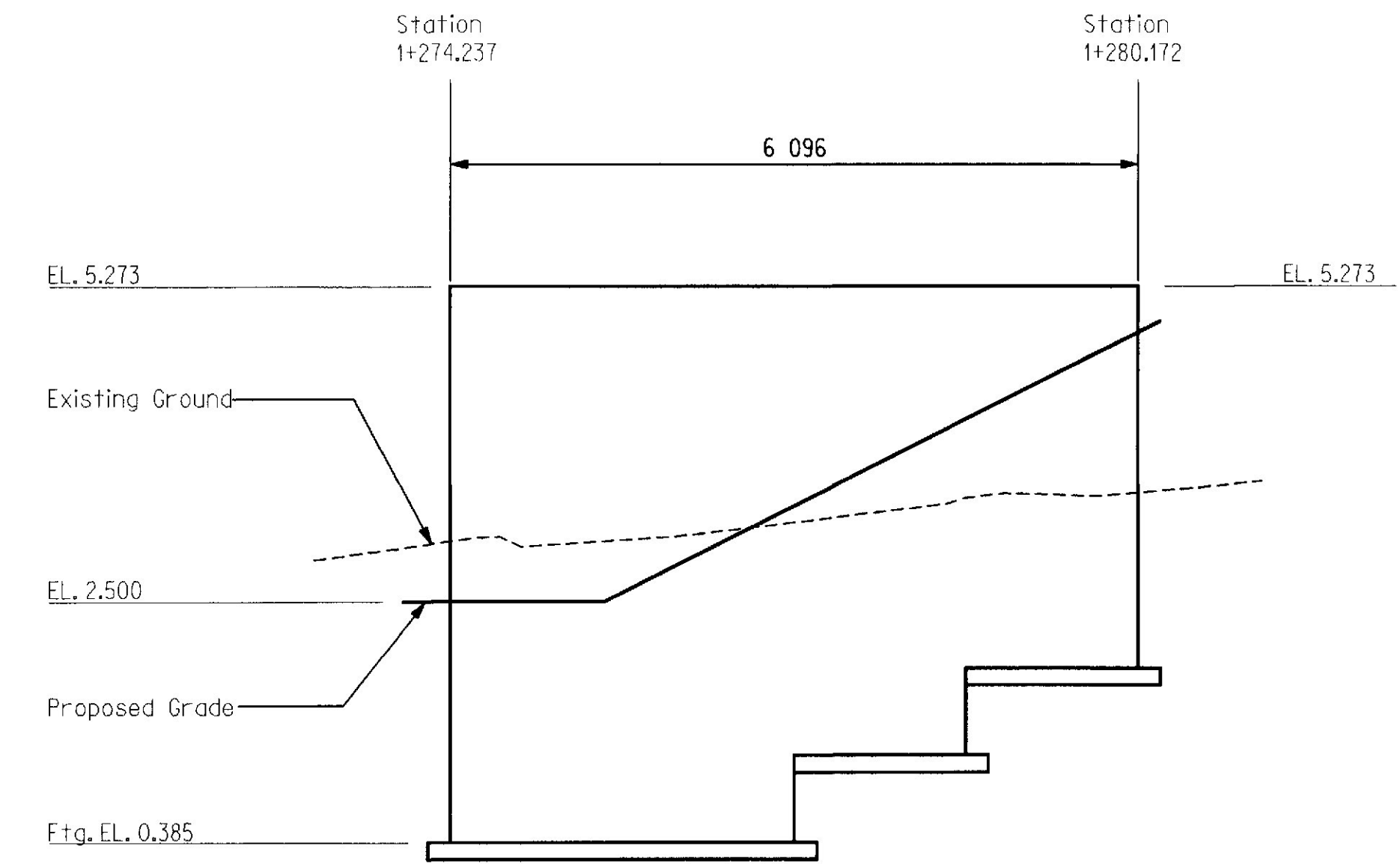
007956.00



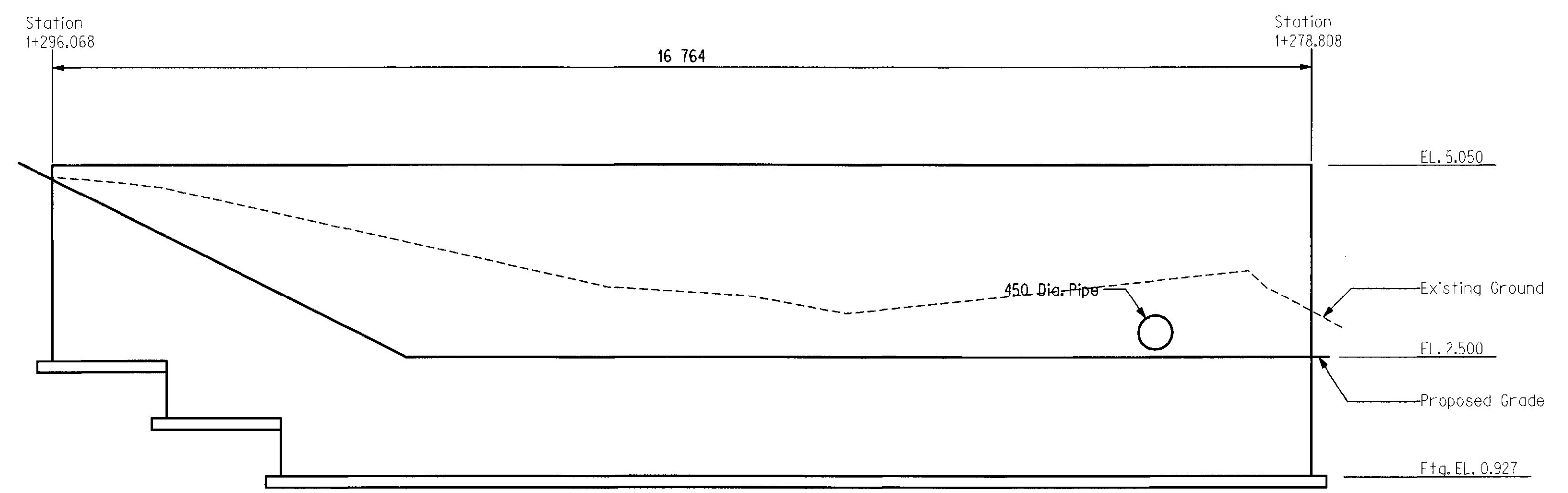
SOUTH WALL PLAN



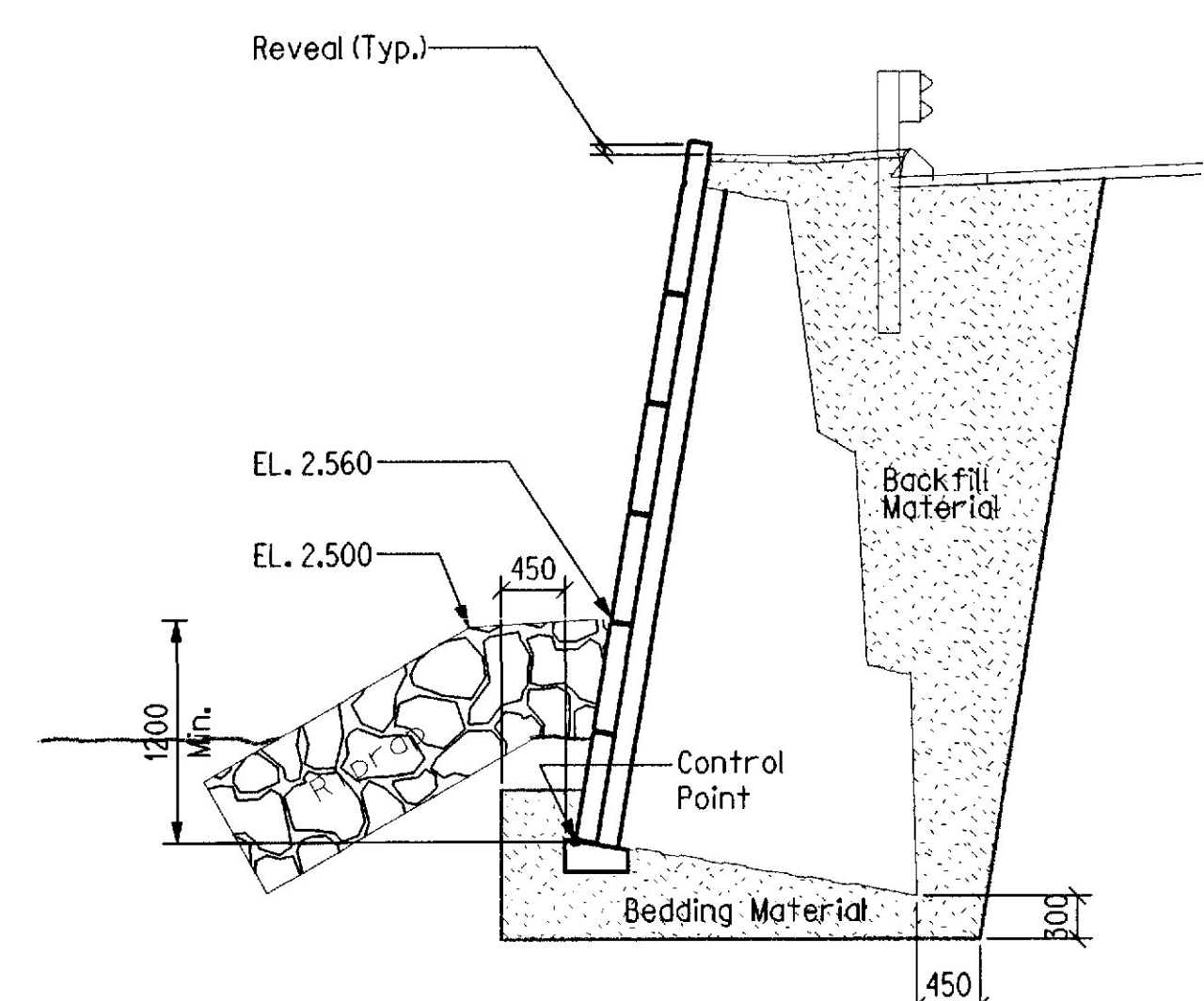
NORTH WALL PLAN



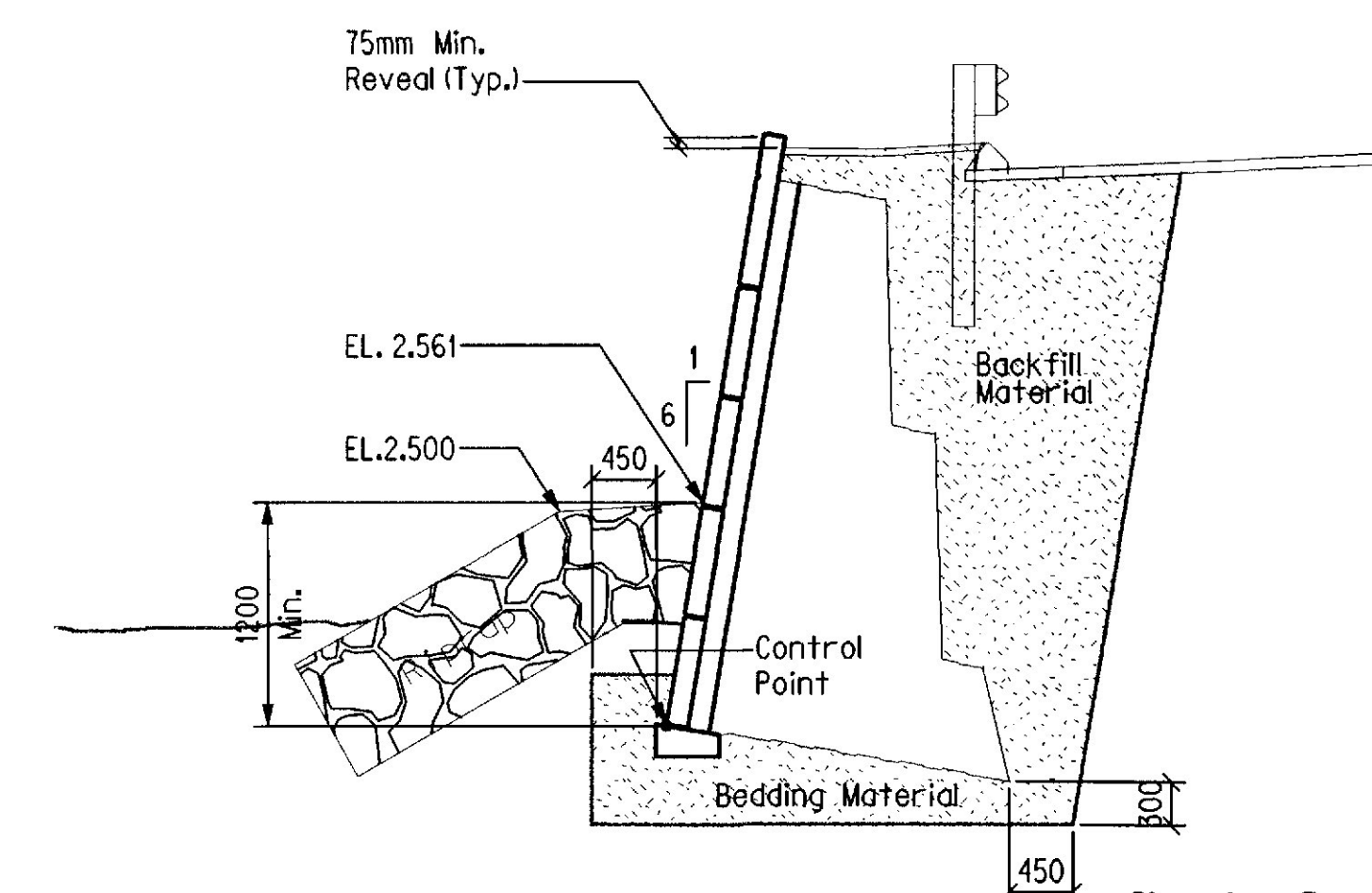
SOUTH WALL ELEVATION



NORTH WALL ELEVATION



SOUTH WALL SECTION



NORTH WALL SECTION

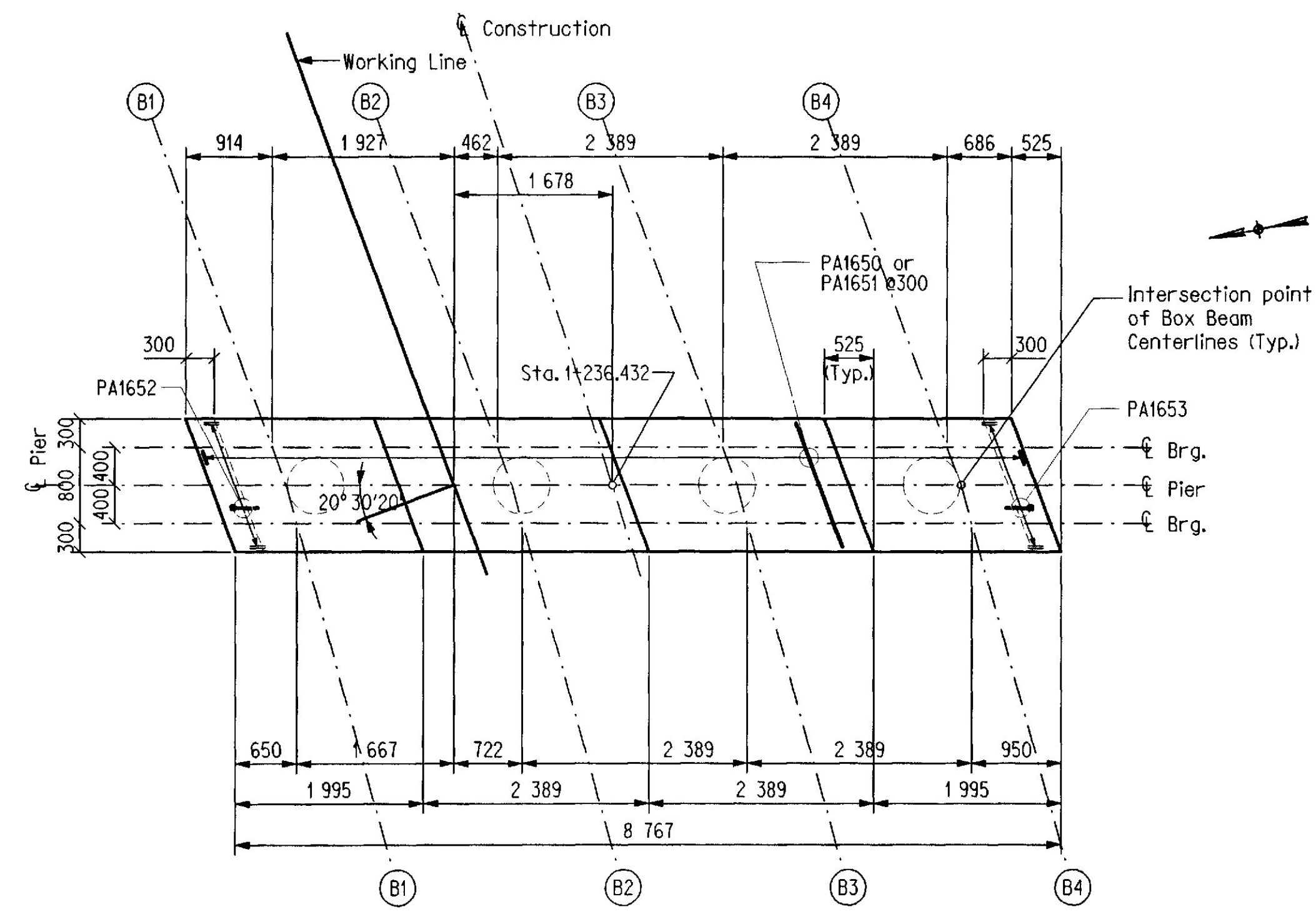
Dimensions Typ. all Sections

BRIDGE NO. 2927
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY
ABUT 2 RET. WALLS

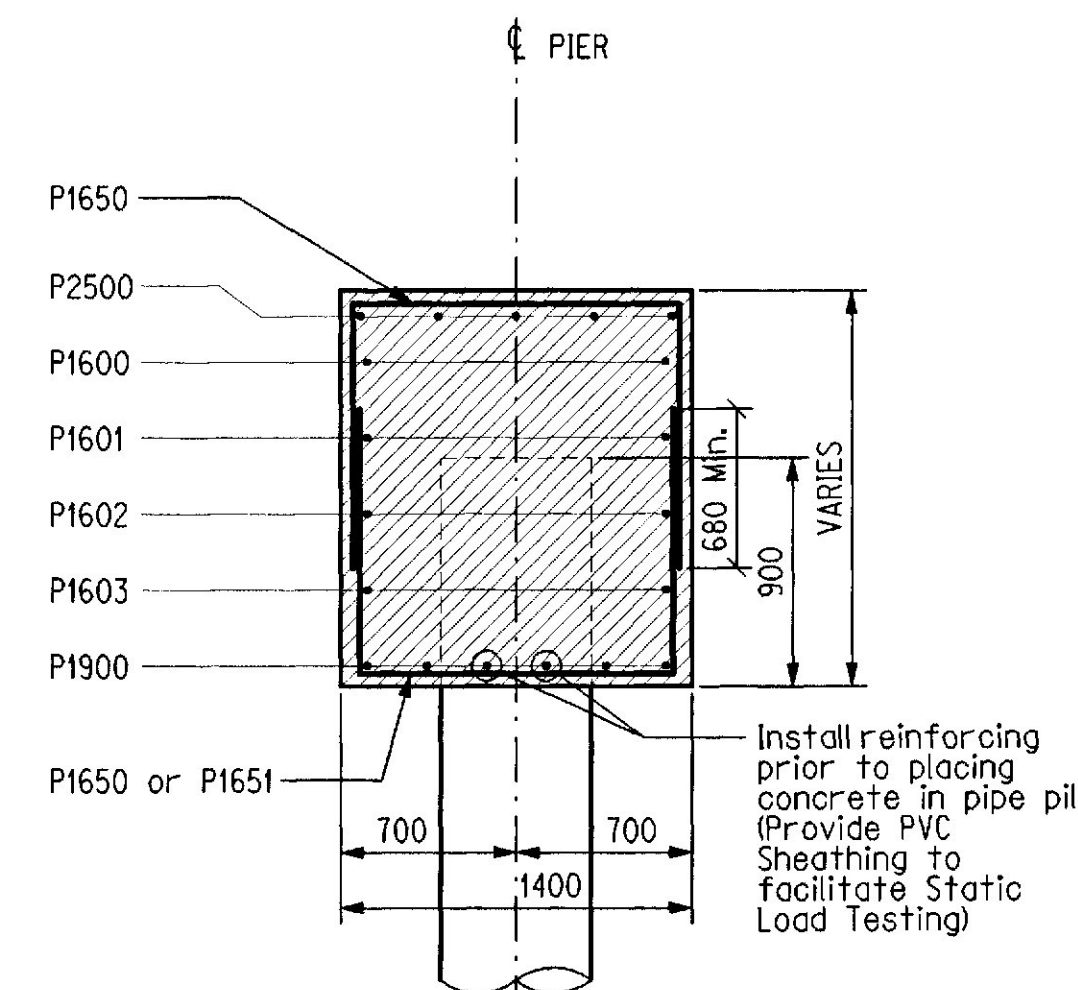
PROJECT DESIGN ENGINEER	BY	DATE
DESIGN-DETAILED	CEN/JCC	5/00
CHECKED	RWR	5/00
REVISIONS		
FIELD CHANGES		

PLANS

Filename: ... \019_twall2.dgn Division: BRIDGE Username: Kevin McLoggan Date: 30 MAY 2000

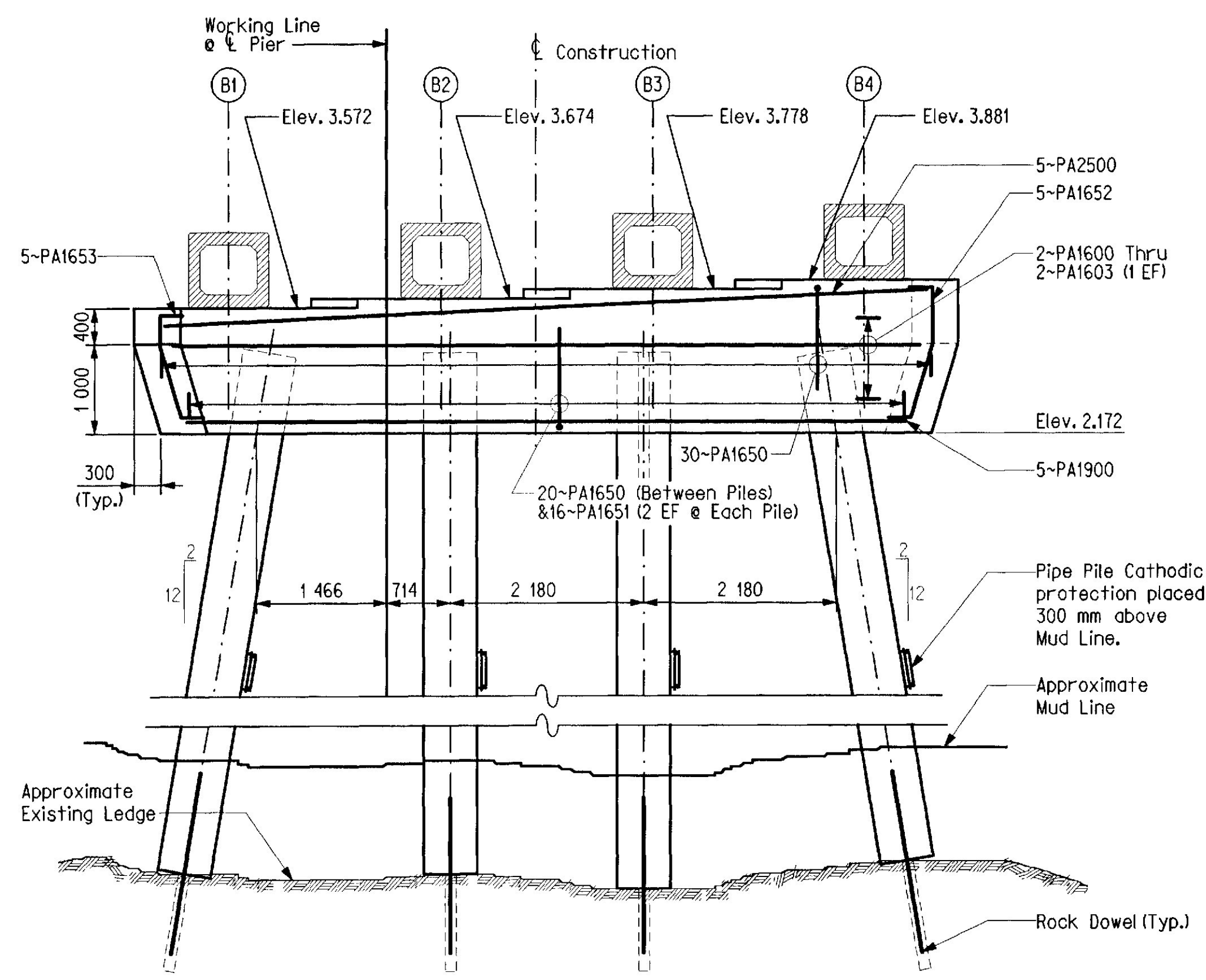


PIER 1 PLAN



TYPICAL PIER SECTION

'P' Bars (above) denote 'PA' for Pier No. 1 and 'PB' for Pier No. 2.



PIER NO. 1 ELEVATION

PIER NOTES

1. Reinforcing Steel shall have a 50 mm cover unless otherwise indicated.

DESIGN CRITERIA

1. Critical AASHTO Loading - Strength I and Extreme Event II
2. Buoyancy - Water level assumed at Elevation 1.4
3. Stream flow - Velocity of 1.5 m/s skewed at 0° to longitudinal centerline of pier.
4. Wind - 160 km/h.
5. Ice - Thickness 225 mm, pressure 0.77 MPa at EL. 1.4

PILE NOTES

1. Estimates of pile length are determined from available soils information with no allowances for uncertain pile penetration.
Battered piles = 12.5 meters each
Interior piles = 12.5 meters each.
2. Piles shall not be out of position by more than 50 mm in any direction.
3. Pile installation procedure shall be in accordance with Special Provision 504 - Rock dowels and Supplemental Specification Section 501 Foundation Piles. Piles shall be driven to sound ledge and cleaned of soil or other debris. If cleaning operations disturb bearing material, or indicates the pile is not on bedrock, the pile shall be redriven to the criteria established by the wave equation and the PDA. After cleaning, holes in bedrock shall be drilled for rock dowels, cleaned and rock dowels installed and grouted. Install rebar and Class A concrete in pipe piles in accordance with 501.20 and 501.21.
4. Piles shall be fitted with an approved cast steel pile driving shoe, conforming to the strength requirements of ASTM A148, Grade 90-60.
5. A wave equation analysis shall be performed by the Contractor to estimate driving stresses and driving criteria for review by the Engineer. The wave equation analysis shall be performed for driving open ended pipe pile to the factored Service 1 pile load of 3719 kN (service load including FS=2.25). Preparation and submission of the analysis shall be considered incidental to Item 501.701.
6. The Contractor shall perform 2 dynamic tests with a Pile Driving Analyzer (PDA) to verify axial capacity and driving stresses. The first plumb pipe pile of Pier 1 and Pier 2 shall be driven to the factored pile load of 3719 kN (Service Load with a FS of 2.25).
7. Prior to installation of production piles, a static load test shall be performed to verify the ultimate pile capacity. At the Contractor's option, the first production pile shall be static load tested. The static load test shall be performed after the pipe pile is driven and tested with a PDA to the criteria in Note 6, and after clean-out, rock-dowel installation, reinforcing and concreting operations are complete. Static load testing shall be in accordance with ASTM D 1143, using the quick load test method and Section 501 'Foundation Piles,' except that the test shall be taken to the factored maximum axial load of 3160 kN (Strength I max. Axial pile load with a resist. factor of 0.80 per LRFD) or plunging failure, whichever occurs first.
8. Holes shall be drilled in piles to allow placement of longitudinal cap reinforcement.
9. Pile material shall be ASTM A252 Grade 2 Fy=250 Mpa.
10. Embedment of piles in pier cap may vary from 850mm to 950mm and the actual embedment length will be included in the measurement for payment.
11. Payment for acquisition, delivery and installation of cathodic protection shall be considered incidental to Item No 502.253, Pile Protective Coating, no separate payment will be made for this work.
12. The concrete and reinforcing placed inside the pipe piles shall be considered incidental to Pay Item 501.701, Steel Pipe Piles in Place.
13. Pile protective coating shall be applied full length to each pipe pile. Color shall be a dark brown to closely match the existing timber pile.

ROCK DOWEL NOTES

1. The rock dowels shall have a 35mm nominal threadbar diameter and 3000 mm minimum bonded length.
2. Rock dowel design, testing, and installation shall be in accordance with the post-tensioning manual, 5th edition.
3. Rock dowels shall have a solid threaded bar meeting ASTM A722M, Type II, with a minimum yield strength of 1030 MPa (150 KSI).
4. Plastic sheathing and sleeves shall be polyvinyl chloride. Corrugated sheathing shall have a minimum tensile and compressive strength of 7000 psi. Sleeves shall be schedule 40 PVC plastic pipe conforming to ASTM D1785.
5. Grout for rock dowel installation shall have a non-shrink additive, maximum water-cement ratio of 0.45 by weight and have a minimum unconfined compressive strength of 27.5 MPa at time of testing.
6. No torch cutting of threadbar is permitted. All dowel rods shall be saw cut in accordance with the manufacturer's recommendations.
7. Contractor may be required to grout and redrill areas of poor quality rock to stabilize hole for rock dowel installation.

BRIDGE NO. 2927

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGadahoc COUNTY
PIER NO. 1

SHEET OF AUGUSTA, MAINE

PROJECT DESIGN ENGINEER	DATE
DESIGN-DETAILED	5/00
CHECKED	5/00
REVISIONS	
FIELD CHANGES	
PLANS	

PROJECT NAME: UGDA LAMPEN
DATE: 26 JUL 2000
USER: ENRICH

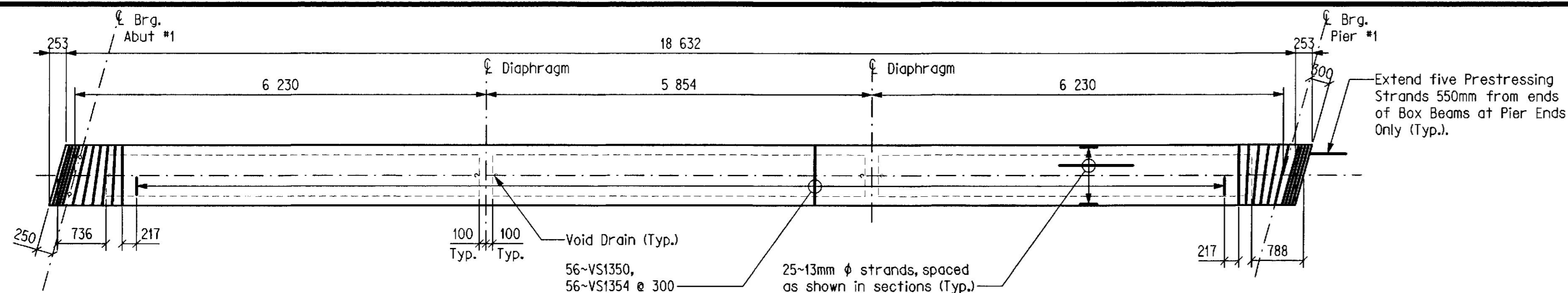
METRIC 1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

FHWA REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-7956100X	22	51

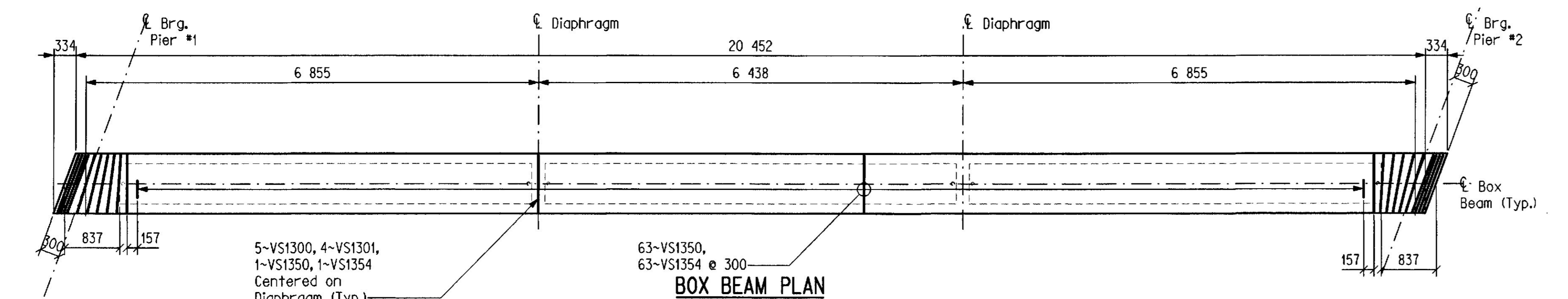
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PRECAST BOX BEAM NOTES

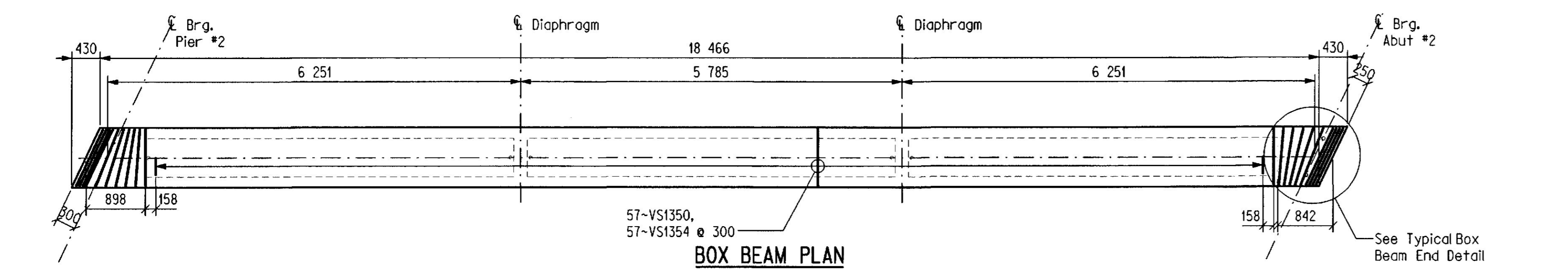
1. Prestressing strands shall be 13 mm diameter seven wire strand, conforming to AASHTO M203 (ASTM A416), Grade 270 Low Relaxation, Initial Force = 138 000 N per strand
2. Minimum concrete strength at release, f'_{ci} = 30000 kPa at 28 days, f'_{cs} = 42000 kPa.
3. Class P concrete shall contain a calcium nitrite admixture. See Special Provision 502 (Use of Calcium Nitrite Admixture).
4. Reinforcing steel, sleeves, threaded inserts and steel strands used in the prestressed girders shall be paid for under item 535.62 Prestressed Structural Concrete Box Beam.
5. The top surface of the upper flange of the prestressed beams shall be raked to a surface roughness of + 6mm except blocking points.
6. Install a 26mm diameter non-metallic void drain in the bottom of each void at both ends.
7. All transverse reinforcement extending out of box beams shall be epoxy coated. Payment shall be incidental to related contract items.



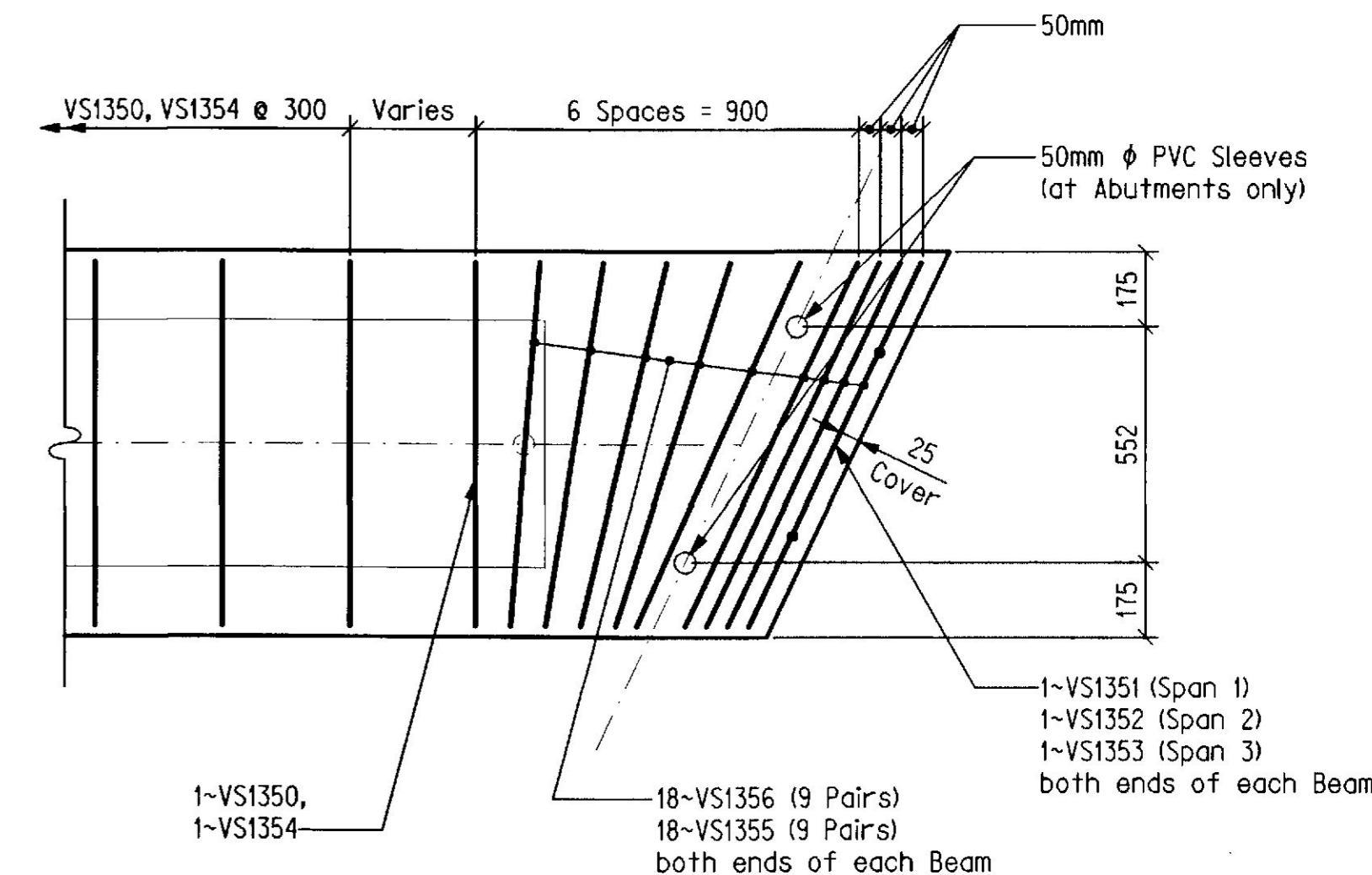
BOX BEAM PLAN SPAN 1



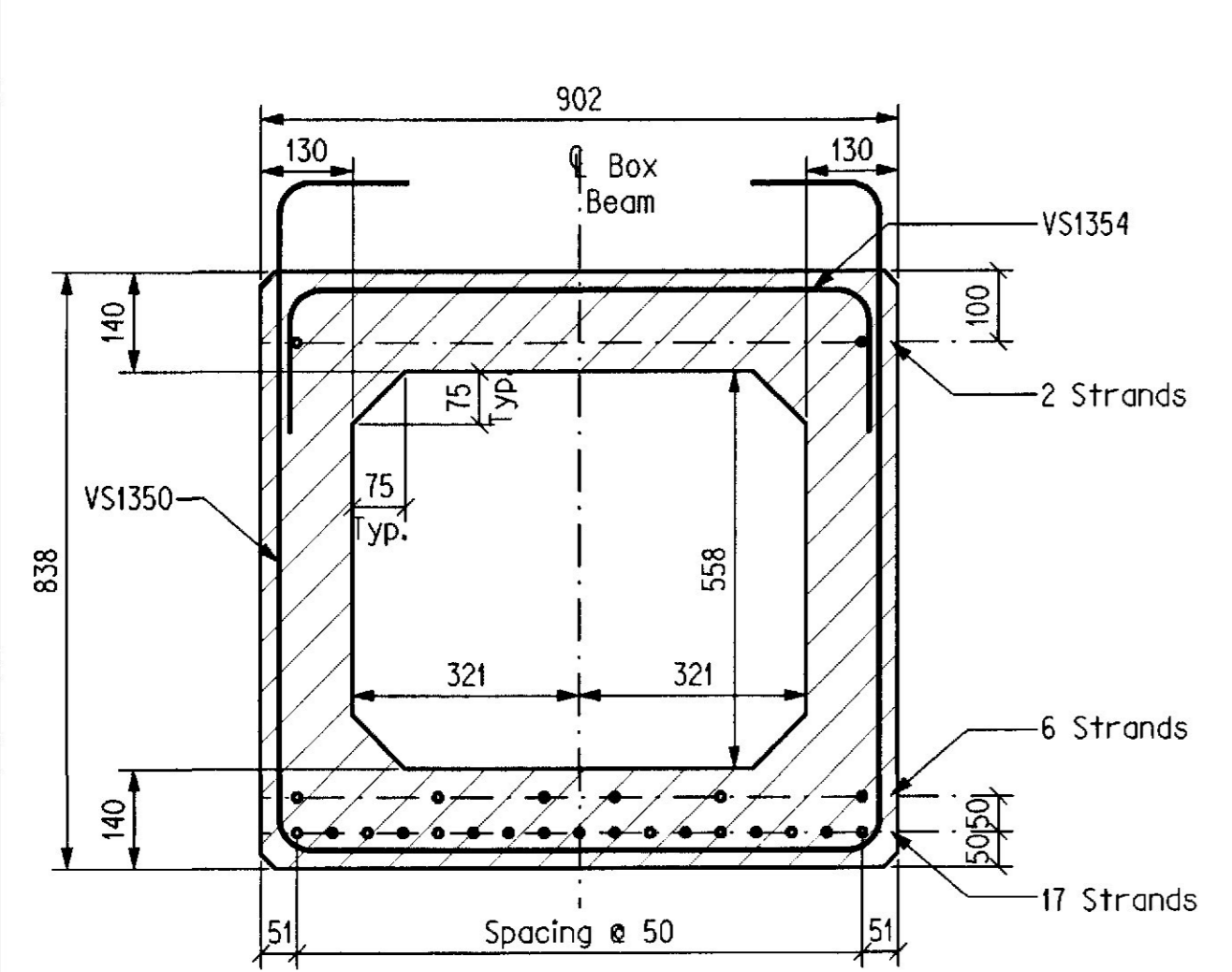
BOX BEAM PLAN SPAN 2



BOX BEAM PLAN SPAN 3

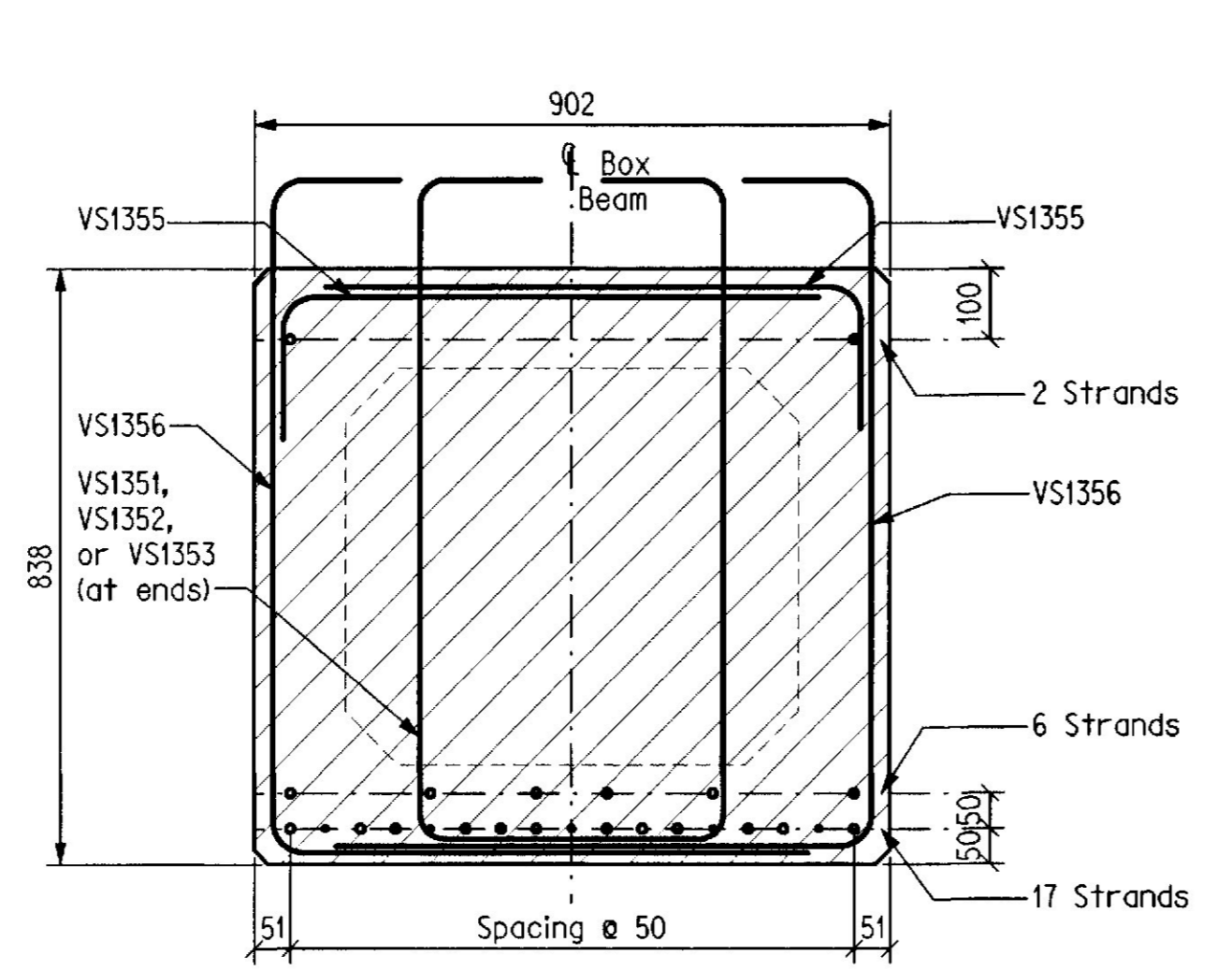


TYPICAL BOX BEAM END DETAIL



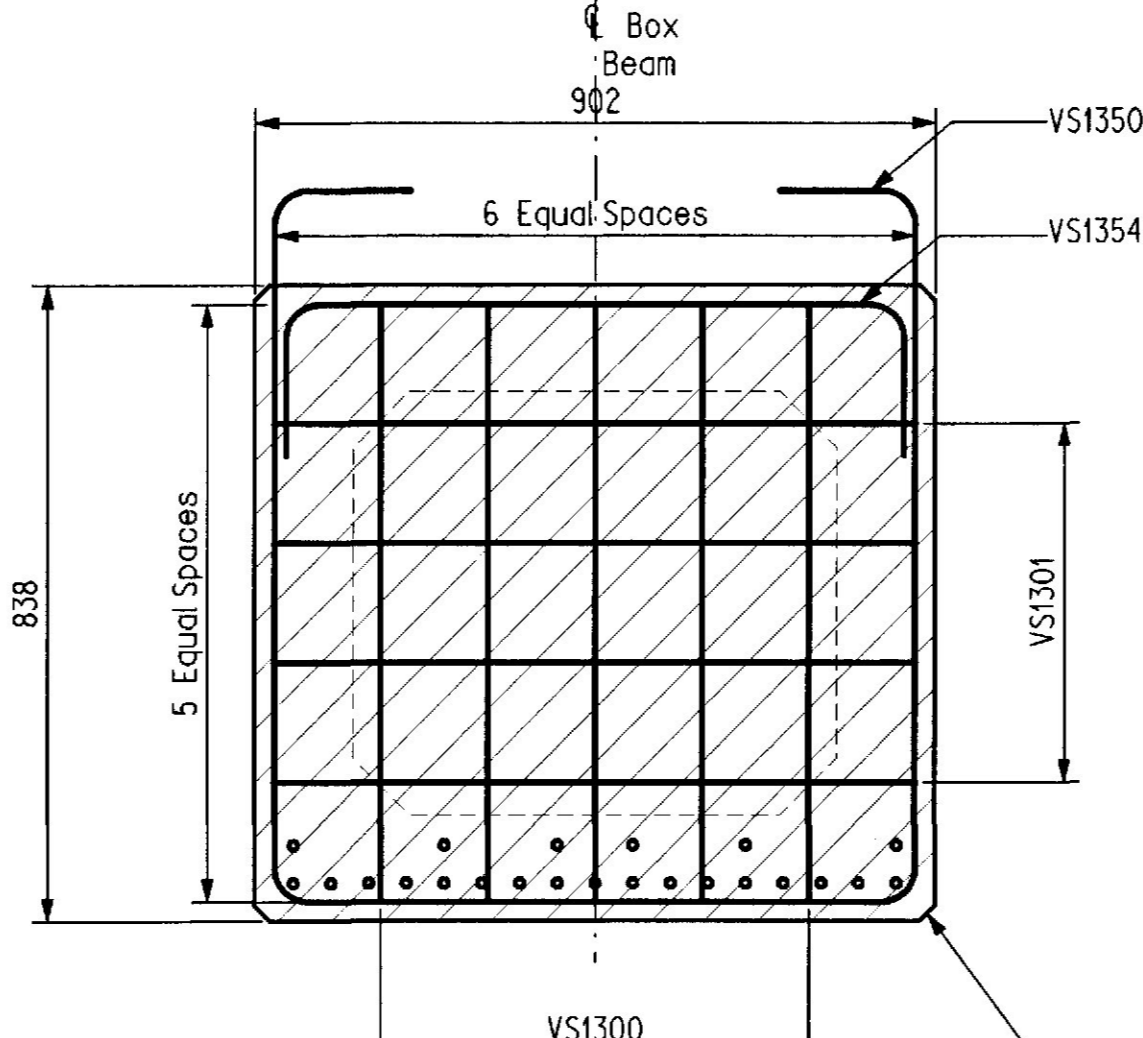
TYPICAL BOX BEAM - MIDSPAN SECTION

• Prestressing Strand



TYPICAL BOX BEAM - END SECTION

• Prestressing Strand
• Prestressing Strand (Debonded 2 m)



TYPICAL DIAPHRAGM SECTION

PROJECT DESIGN ENGINEER	DATE
BY	5/00
DESIGN-DETAILED	5/00
CHECKED	5/00
REVISIONS	
FIELD CHANGES	

BRIDGE NO. 2927

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY

PRECAST BOX BEAMS

SHEET OF AUGUSTA, MAINE

Filename: ...022_precast.dgn Division: BRIDGE Username: Dana Damren Date: 30 MAY 2000

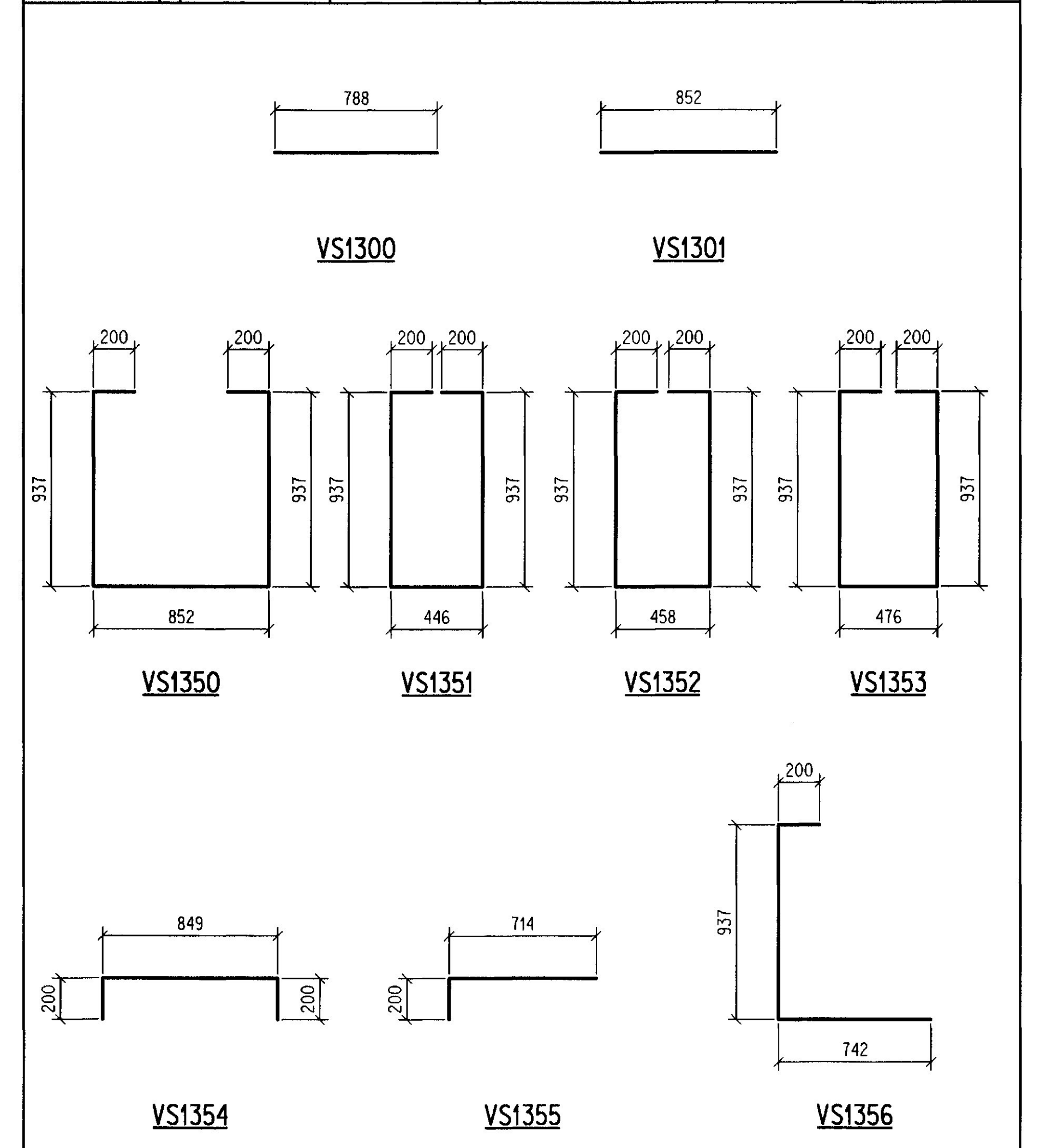
METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

F.H.W.A. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-7956/00IX	23	51

007956.00

Mark	Quantity Span 1	Quantity Span 2	Quantity Span 3	Total Quantity	Length	Loc./Descr.
VS1300	40	40	40	120	788	Diaphragms
VS1301	32	32	32	96	852	Diaphragms
VS1350	240	268	244	752	3128	Span (Epoxy Coated)
VS1351	8	0	0	8	2722	Ends (Epoxy Coated)
VS1352	0	8	0	8	2734	Ends (Epoxy Coated)
VS1353	0	0	8	8	2752	Ends (Epoxy Coated)
VS1354	240	268	244	752	1250	Span
VS1355	144	144	144	432	914	Ends
VS1356	144	144	144	432	1880	Ends (Epoxy Coated)



BRIDGE NO. 2927

First digit(s) following the letter of the mark indicates size of the bar.
All dimensions are out to out of reinforcing bar.
Bending details and hooks shall conform to the recommendations of the current revision of ACI Standard 318.
Reinforcing Bar: ASTM A615/A615M Grade 420.

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY
SUPERSTRUCTURE DETAILS

PROJECT DESIGN ENGINEER	BY	DATE
DESIGN-DETAILED	CER	5/00
CHECKED		
REVISIONS		
FIELD CHANGES		

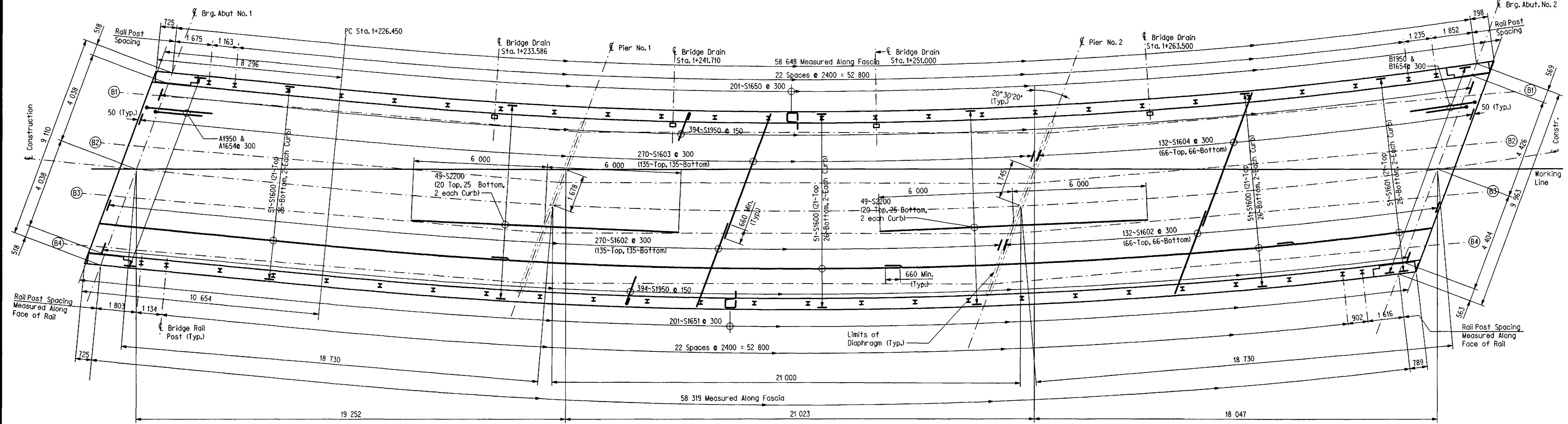
PLANS

Username: Kevin McLoggan Date: 30 MAY 2000

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

FAIRWAY REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-7956001X	24	51

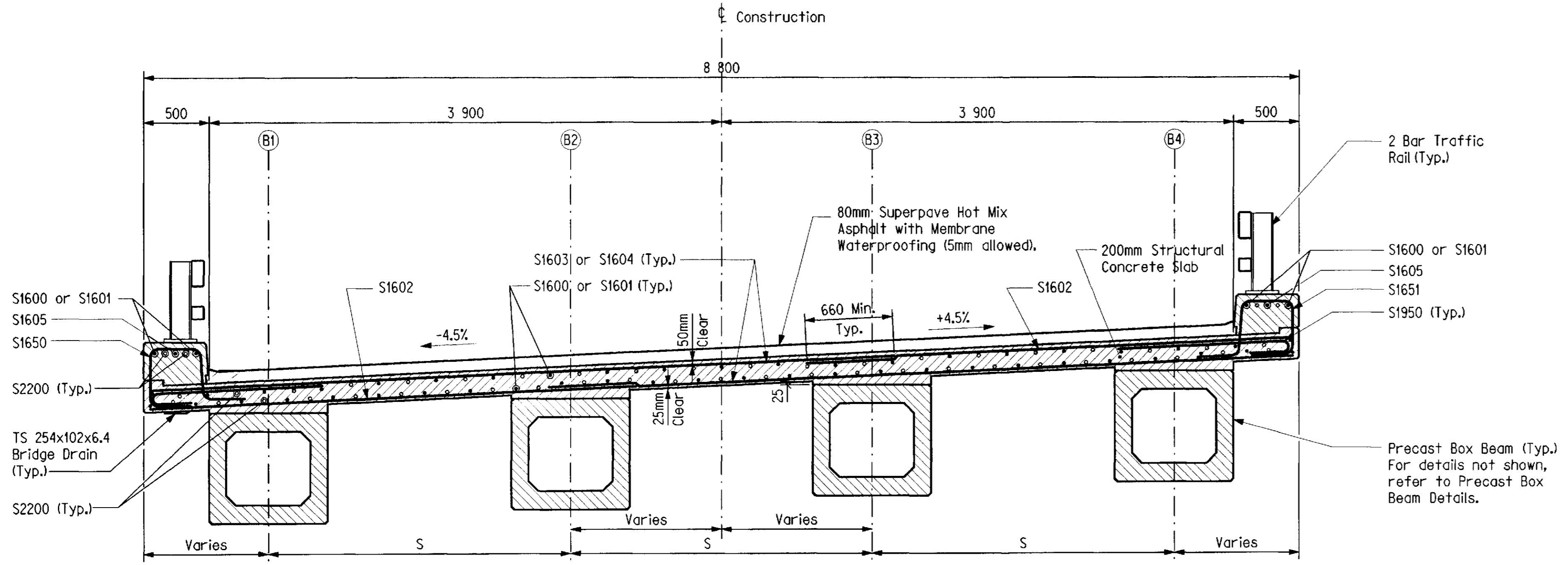


SUPERSTRUCTURE PLAN

Add'l Reinforcement:
 50-S1605 (1 Each Rail Post)
 87-S1650 (3 Each Rail Post, 6 Ea. Trans. Barrier, North Side)
 87-S1651 (3 Each Rail Post, 6 Ea. Trans. Barrier, South Side)
 32-S2500 (2 Each Box @ Each Abut. & Each Pier)
 3-S1606 (Diaphragm, Pier 1)
 3-S1607 (Diaphragm, Pier 2)
 20-S1951 (5 Ea. Trans. Barrier)
 8-S1952 (2 Ea. Trans. Barrier)

SUPERSTRUCTURE NOTES

- Adjust reinforcing steel to fit around drains in a manner approved by the Engineer. Do not cut transverse reinforcing bars.
- S1602, S1603, and S1604 Reinforcing Steel shall be staggered left to right and top to bottom.
- Form a 30mm V-groove on the fascias at the horizontal joint between the curb and slab.
- Reinforcing steel shall have a minimum cover of 50 mm unless otherwise indicated.
- The superstructure slab concrete shall be placed continuously and shall be kept plastic until the entire span has been placed.
- Protective coating for concrete surfaces shall be applied to the following areas:
 -All exposed surfaces of concrete curbs
 -Fascia including exterior face of exterior beam
 -All exposed surfaces of concrete transition barriers
- Concrete for the deck shall be paid for under Item 502.4221, Structural Concrete Roadway Slab on Concrete Bridge.
- The drilling of holes in the prestressed beams and the use of power actuated tools on the beams will not be permitted.
- Neoprene pads shall be either polychloroprene or natural polyisoprene of 50+5 Shore A durometer hardness, and shall conform to the requirements of Division 2, Section 18.2 of A.A.S.H.T.O. Standard Specifications for Highway Bridges. Neoprene pads will not be paid for directly, but will be considered incidental to related contract items.



TRANSVERSE SECTION

TABLE OF BOX BEAM DISTANCES

SPAN	S
1	2300
2	2240
3	2156

BRIDGE NO. 2927

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY

SUPERSTRUCTURE

PROJECT DESIGN ENGINEER	DATE
DESIGN-DETAILED	5/00
CHECKED	5/00
REVISIONS	
FIELD CHANGES	

PLANS

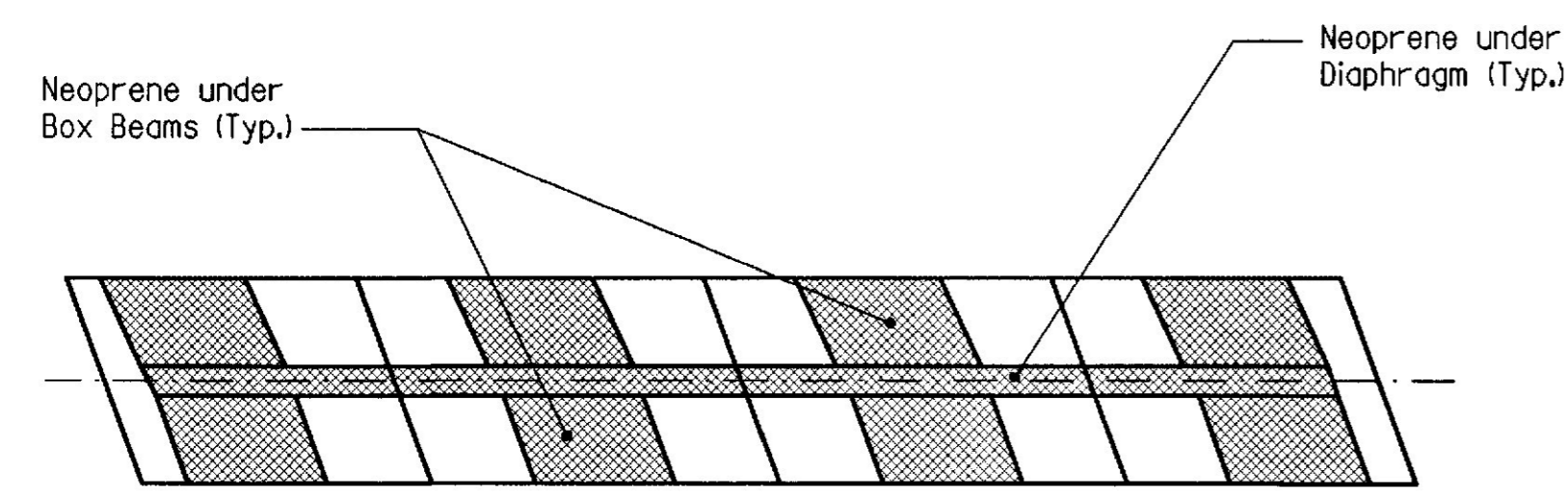
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 Division: BRIDGE
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METRIC

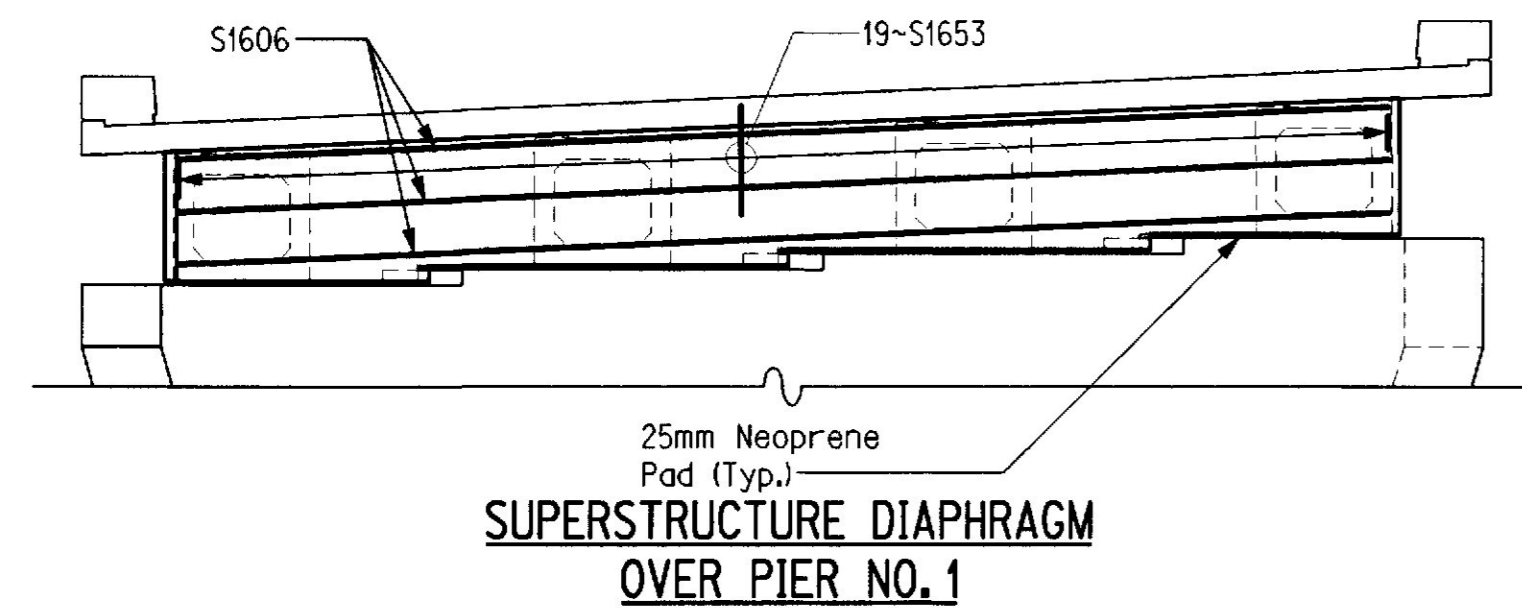
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

FHWA REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-795600X	25	51

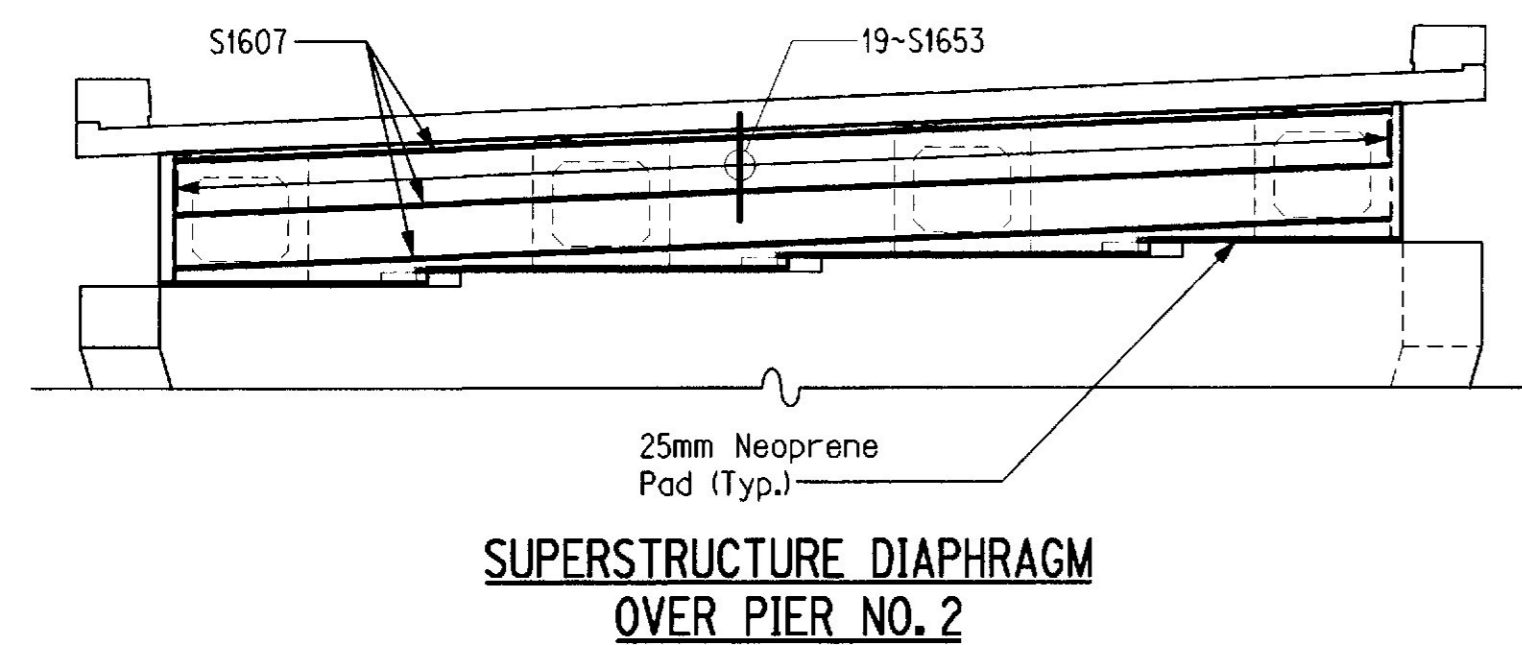
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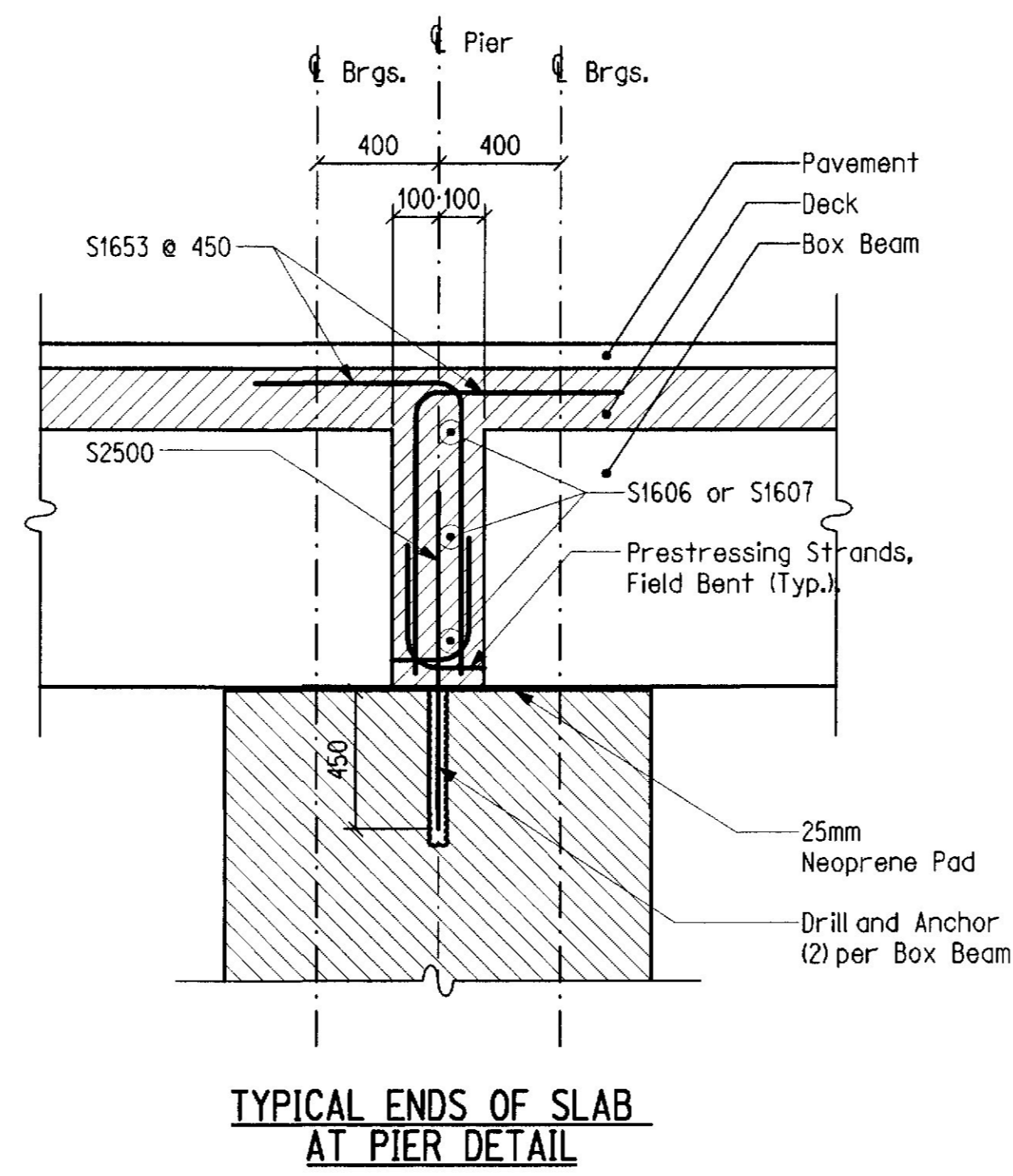
TYPICAL NEOPRENE BEARING PAD LAYOUT ON PIERS



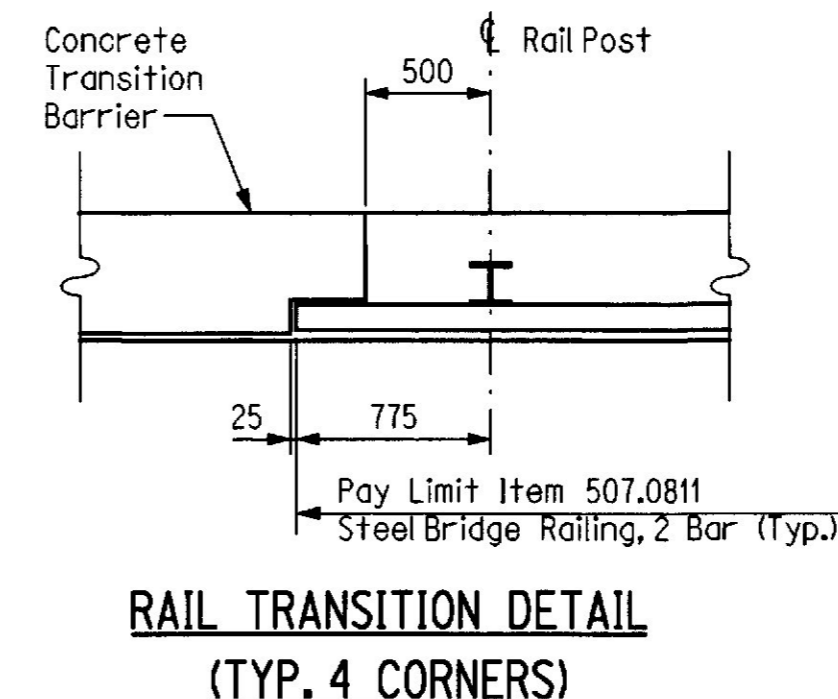
SUPERSTRUCTURE DIAPHRAGM OVER PIER NO. 1



SUPERSTRUCTURE DIAPHRAGM OVER PIER NO. 2

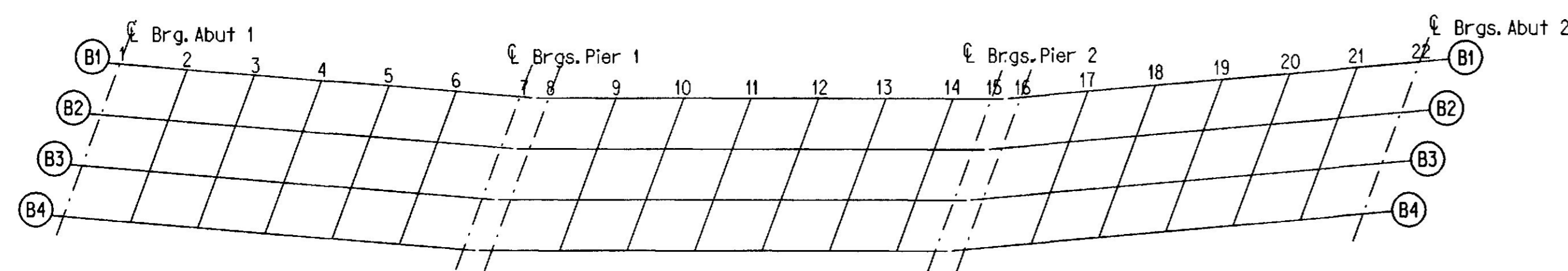


TYPICAL ENDS OF SLAB AT PIER DETAIL

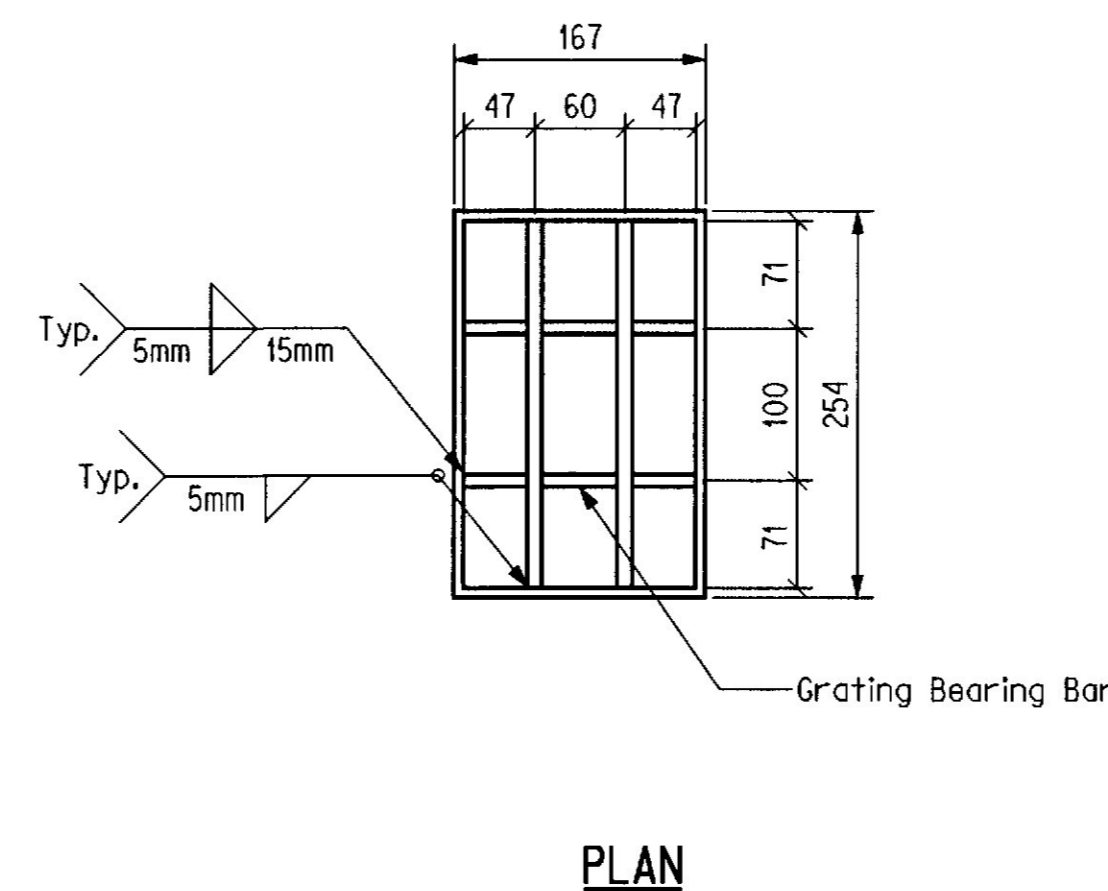


RAIL TRANSITION DETAIL (TYP. 4 CORNERS)

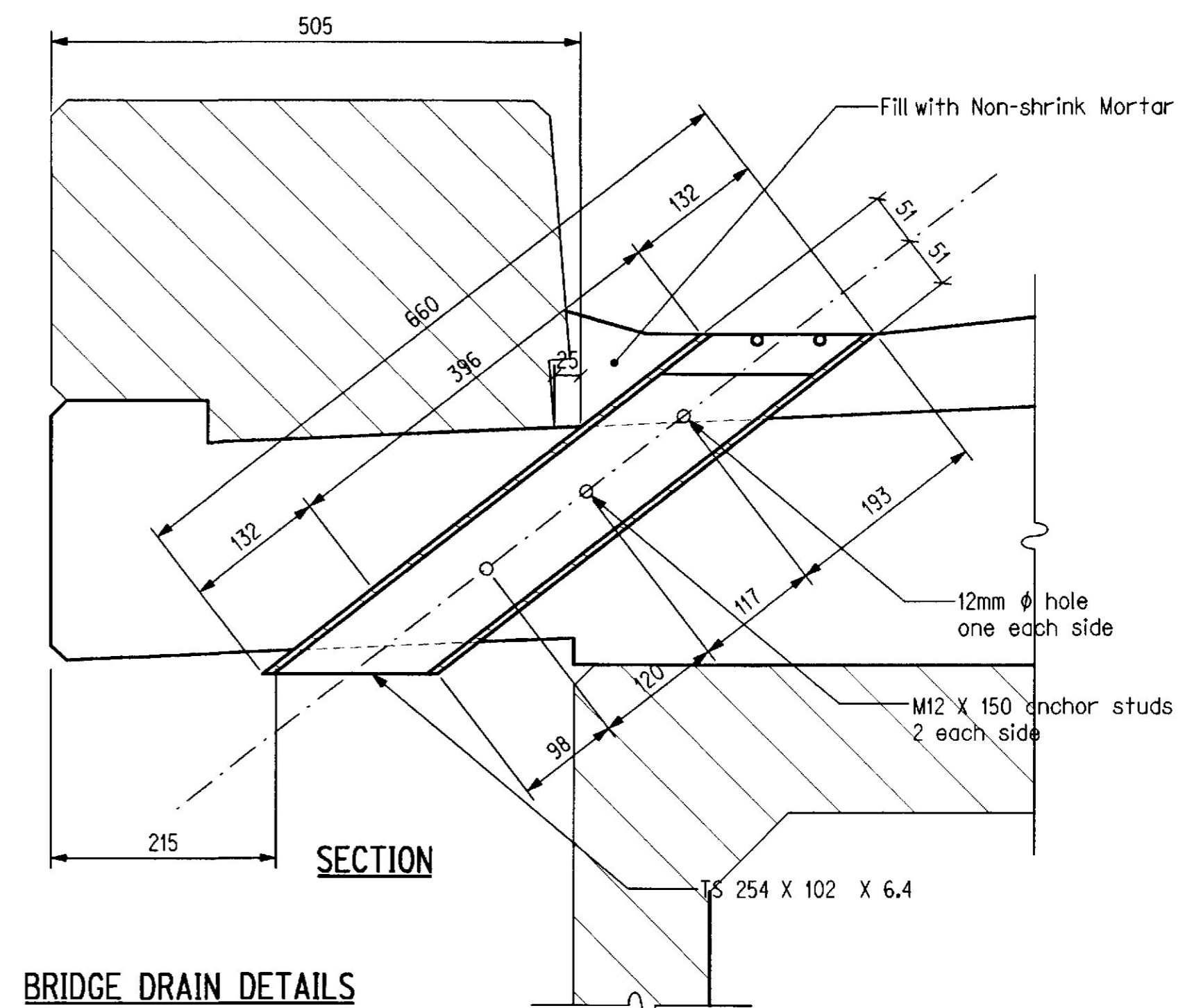
- BRIDGE DRAIN NOTES**
1. Bridge drain shall be galvanized TS 254 x 102 x 6.4 conforming to ASTM A500 Grade B.
 2. Grating shall be a commercial heavy-duty grating with 38 x 8mm bearing bars and 10mm ϕ cross bars.
 3. Taper pavement around drains.
 4. Fill void between drain and curb with non-shrink mortar. See Standard Detail 502 (5).
 5. Payment for bridge drains will be specified under Subsection 502.19 of the Standard Specifications.
 6. Locate 12mm ϕ drain hole 6mm above deck.



BOTTOM OF SLAB ELEVATION LAYOUT PLAN



PLAN



BRIDGE DRAIN DETAILS

PROJECT DESIGN ENGINEER	BY	DATE
DESIGN-DETAILED	CEP/JCC	5/00
CHECKED	RWR	5/00
REVISIONS		
FIELD CHANGES		

PLANS

Point	Abut #1		Span 1				Pier #1		Span 2						Pier #2		Span 3				Abut #2	
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22
Distance	0	3000	6000	9000	12000	15000	18315	0	3000	6000	9000	12000	15000	18000	20147	0	3000	6000	9000	12000	15000	18287
B1	4.579	4.556	4.538	4.519	4.503	4.490	4.481	4.481	4.489	4.502	4.517	4.530	4.541	4.549	4.555	4.558	4.574	4.591	4.606	4.618	4.635	4.656
B2	4.689	4.667	4.647	4.627	4.610	4.595	4.585	4.583	4.591	4.609	4.626	4.637	4.642	4.645	4.650	4.653	4.669	4.685	4.702	4.711	4.724	4.742
B3	4.799	4.777	4.755	4.735	4.716	4.700	4.687	4.687	4.692	4.702	4.715	4.727	4.750	4.740	4.745	4.748	4.763	4.779	4.792	4.803	4.814	4.829
B4	4.910	4.884	4.861	4.840	4.820	4.804	4.791	4.789	4.793	4.800	4.810	4.821	4.829	4.836	4.840	4.844	4.858	4.870	4.882	4.893	4.902	4.916

BRIDGE NO. 2927
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY
SUPERSTRUCTURE
DETAILS

REINFORCING STEEL SCHEDULE

METRIC

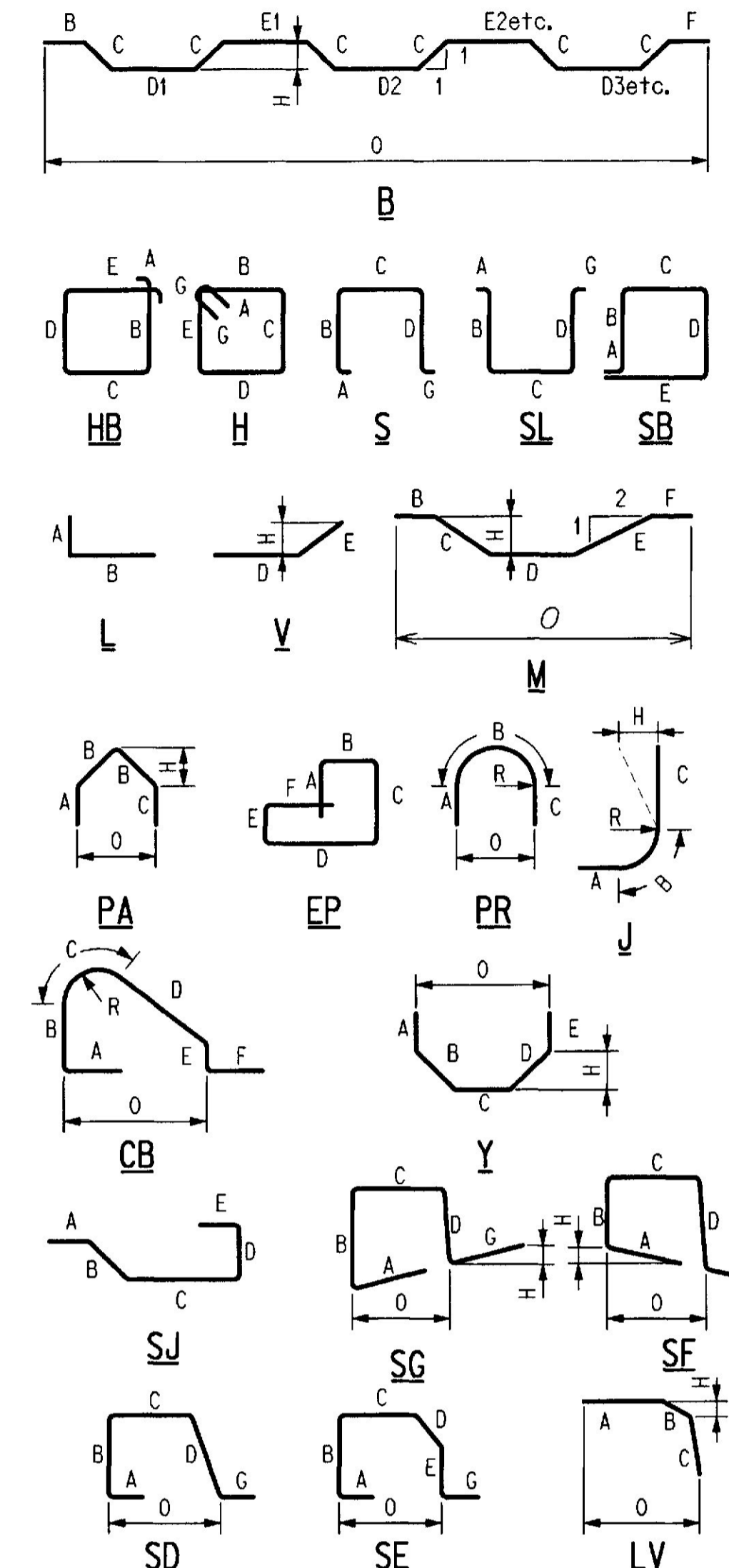
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

TAHWA REG NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-195600IX	27	51

007956.00

BENT BARS														
MARK	QTY.	LENGTH	TYPE	A	B	C	D	E	F	G	H	O	R	LOCATION
ABUTMENT NO. 2														
B1650	9	5203	S	-	2035	1133	2035	-	-	-	-	-	-	Breastwall
B1651	8	5375	S	-	2121	1133	2121	-	-	-	-	-	-	Breastwall
B1652	8	5549	S	-	2208	1133	2208	-	-	-	-	-	-	Breastwall
B1653	8	5723	S	-	2295	1133	2295	-	-	-	-	-	-	Breastwall
B1654	33	2025	L	550	1475	-	-	-	-	-	-	-	-	Breastwall & Backwall
B1655	33	1510	L	560	950	-	-	-	-	-	-	-	-	Breastwall & Backwall
B1656	2	2435	V	-	-	-	1602	833	-	-	450	-	-	North Wing
B1657	2	2385	V	-	-	-	1552	833	-	-	450	-	-	North Wing
B1658	2	2335	V	-	-	-	1502	833	-	-	450	-	-	North Wing
B1659	2	2285	V	-	-	-	1452	833	-	-	450	-	-	North Wing
B1660	2	2235	V	-	-	-	1402	833	-	-	450	-	-	North Wing
B1661	2	2185	V	-	-	-	1352	833	-	-	450	-	-	North Wing
B1662	2	2135	V	-	-	-	1302	833	-	-	450	-	-	North Wing
B1663	2	2085	V	-	-	-	1252	833	-	-	450	-	-	North Wing
B1664	2	2035	V	-	-	-	1202	833	-	-	450	-	-	North Wing
B1665	2	1985	V	-	-	-	1152	833	-	-	450	-	-	North Wing
B1666	2	1935	V	-	-	-	1102	833	-	-	450	-	-	North Wing
B1667	2	2478	V	-	-	-	1658	820	-	-	391	-	-	South Wing
B1668	2	2428	V	-	-	-	1608	820	-	-	391	-	-	South Wing
B1669	2	2378	V	-	-	-	1558	820	-	-	391	-	-	South Wing
B1670	2	2328	V	-	-	-	1508	820	-	-	391	-	-	South Wing
B1671	2	2278	V	-	-	-	1458	820	-	-	391	-	-	South Wing
B1672	2	2228	V	-	-	-	1408	820	-	-	391	-	-	South Wing
B1673	2	2178	V	-	-	-	1358	820	-	-	391	-	-	South Wing
B1674	2	2128	V	-	-	-	1308	820	-	-	391	-	-	South Wing
B1675	2	2078	V	-	-	-	1258	820	-	-	391	-	-	South Wing
B1676	2	2028	V	-	-	-	1208	820	-	-	391	-	-	South Wing
B1677	2	1978	V	-	-	-	1158	820	-	-	391	-	-	South Wing
B1678	2	1928	V	-	-	-	1108	820	-	-	391	-	-	South Wing
B1679	2	4352	LV	814	182	3356	-	-	-	-	109	1541	-	North Wing
B1680	2	4640	LV	764	287	3589	-	-	-	-	137	1605	-	South Wing
B1950	33	4730	L	3050	1680	-	-	-	-	-	-	-	-	Breastwall
B2250	1	2792	V	-	-	-	1578	1214	-	-	647	-	-	North Wing
B2251	1	2742	V	-	-	-	1528	1214	-	-	647	-	-	North Wing
B2252	1	2692	V	-	-	-	1478	1214	-	-	647	-	-	North Wing
B2253	1	2642	V	-	-	-	1428	1214	-	-	647	-	-	North Wing
B2254	1	2592	V	-	-	-	1378	1214	-	-	647	-	-	North Wing
B2255	1	2542	V	-	-	-	1328	1214	-	-	647	-	-	North Wing
B2256	1	2492	V	-	-	-	1278	1214	-	-	647	-	-	North Wing
B2257	1	2442	V	-	-	-	1228	1214	-	-	647	-	-	North Wing
B2258	1	2392	V	-	-	-	1178	1214	-	-	647	-	-	North Wing
B2259	1	2342	V	-	-	-	1128	1214	-	-	647	-	-	North Wing
B2260	1	2833	V	-	-	-	1633	1200	-	-	564	-	-	South Wing
B2261	1	2783	V	-	-	-	1583	1200	-	-	564	-	-	South Wing
B2262	1	2733	V	-	-	-	1533	1200	-	-	564	-	-	South Wing
B2263	1	2683	V	-	-	-	1483	1200	-	-	564	-	-	South Wing
B2264	1	2633	V	-	-	-	1433	1200	-	-	564	-	-	South Wing
B2265	1	2583	V	-	-	-	1383	1200	-	-	564	-	-	South Wing
B2266	1	2533	V	-	-	-	1333	1200	-	-	564	-	-	South Wing
B2267	1	2483	V	-	-	-	1283	1200	-	-	564	-	-	South Wing
B2268	1	2433	V	-	-	-	1233	1200	-	-	564	-	-	South Wing
B2269	1	2383	V	-	-	-	1183	1200	-	-	564	-	-	South Wing
B2270	1	2333	V	-	-	-	1133	1200	-	-	564	-	-	South Wing
SUPERSTRUCTURE														
S1650	288	1892	SG	310	455	379	438	-	-	310	14	414	-	North Curb
S1651	288	1849	SF	310	415	379	435	-	-	310	14	414	-	South Curb
S1653	38	1570	L	660	910	-	-	-	-	-	-	-	-	Diaphragms
S1950	402	1950	S	-	1500	140	310	-	-	-	-	-	-	Overhang
S1951	20	2784	H	200	817	375	817	375	-	200	-	-	-	Transition Barrier
S1952	8	2524	H	200	817	245	817	245	-	200	-	-	-	Transition Barrier
MARK	QTY.	LENGTH	TYPE	A	B	C	D	E	F	G	H	O	R	LOCATION

TYPE - BENDING DIAGRAMS



All dimensions are out-to-out of bar.

Bending details and hooks shall conform to the recommendations of the current revision of ACI Standard 318.

Reinforcing Bar: ASTM A615/A615M, Grade 420

GENERAL NOTES

- The first two digits following the letter(s) of the mark indicate the size of the bar:
Mark 'A1602' = bar size #16
Mark 'P2501' = bar size #25
Mark 'S1950' = bar size #19
- Each crank bar, Type B, may be replaced by two (2) straight bars (one top and one bottom) of the same bar size as the crank bar. Payment in either case shall be based on crank bars as scheduled on the plans.
- Refer to standard details for Transition Barrier Reinforcing steel not shown on this schedule. Payment for those bars shall be considered incidental to Item No. 526.34.

BRIDGE NO. 2927

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
WEST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY
REINFORCING

SHEET OF AUGUSTA, MAINE

PROJECT DESIGN ENGINEER	DATE
DESIGN-DETAILED	5/00
CHECKED	5/00
REVISIONS	
FIELD CHANGES	

PLANS

Filename: ... \027_rebar2.dgn Division: BRIDGE User: Kevin McLaggen Date: 30 MAY 2000

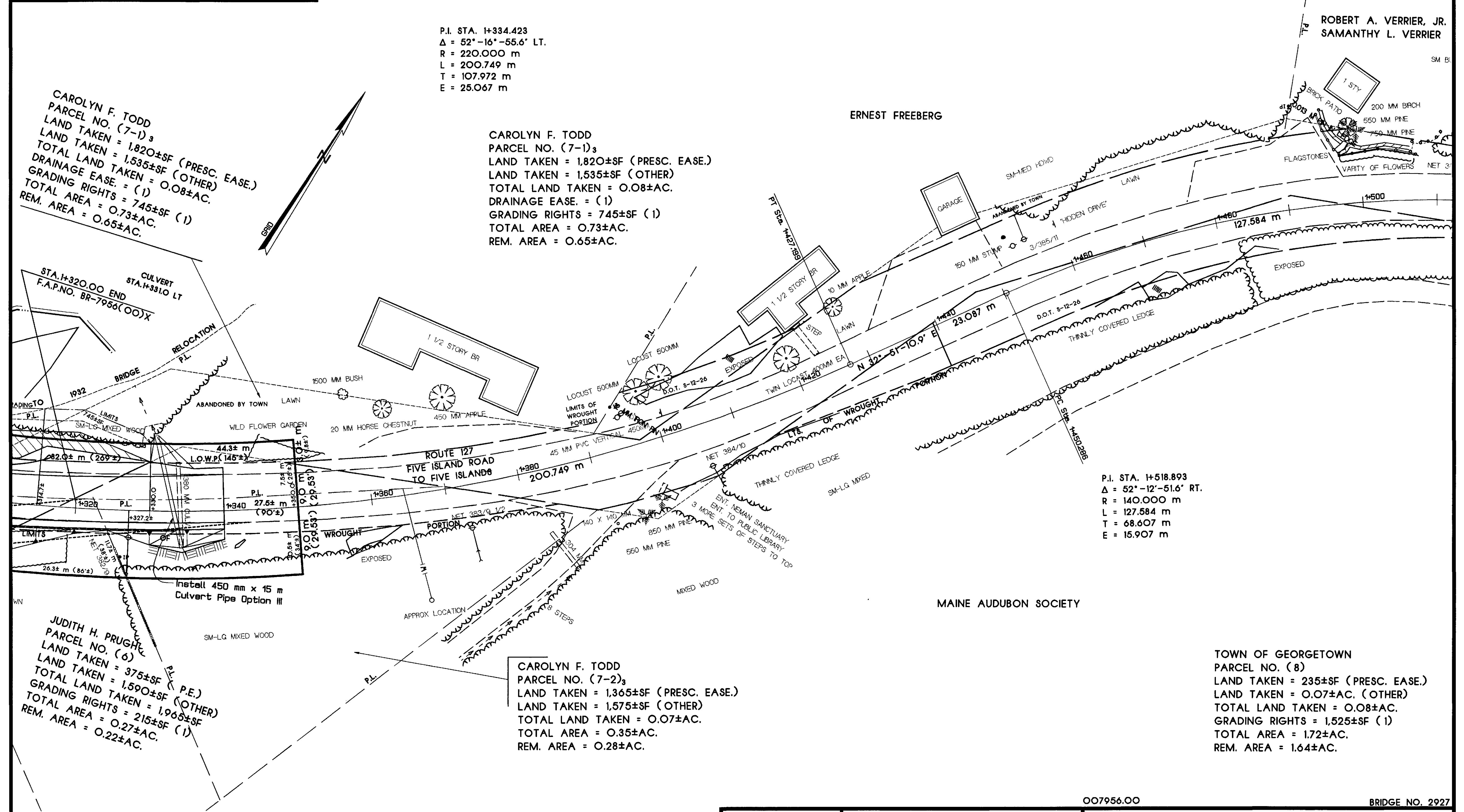
ITEM	TECH	CHECKED	REVISIONS		
			NO.	DATE	DESCRIPTION
BASE MAP					
EXIST. R/W					
PROP. LINES					
AREAS					

P.I. STA. 1+334.423
 $\Delta = 52^\circ - 16' - 55.6''$ LT.
 R = 220.000 m
 L = 200.749 m
 T = 107.972 m
 E = 25.067 m

CAROLYN F. TODD
 PARCEL NO. (7-1)₃
 LAND TAKEN = 1,820±SF (PRES. EASE.)
 LAND TAKEN = 1,535±SF (OTHER)
 TOTAL LAND TAKEN = 0.08±AC.
 DRAINAGE EASE. = (1)
 GRADING RIGHTS = 745±SF (1)
 TOTAL AREA = 0.73±AC.
 REM. AREA = 0.65±AC.

CAROLYN F. TODD
 PARCEL NO. (7-1)₃
 LAND TAKEN = 1,820±SF (PRES. EASE.)
 LAND TAKEN = 1,535±SF (OTHER)
 TOTAL LAND TAKEN = 0.08±AC.
 DRAINAGE EASE. = (1)
 GRADING RIGHTS = 745±SF (1)
 TOTAL AREA = 0.73±AC.
 REM. AREA = 0.65±AC.

P.I. STA. 1+518.893
 $\Delta = 52^\circ - 12' - 51.6''$ RT.
 R = 140.000 m
 L = 127.584 m
 T = 68.607 m
 E = 15.907 m



PLAN FILED IN PLAN BOOK		PAGE	
NO.	GRANTOR	INSTRUMENT	DATE

EXISTING R/W REFERENCES
 TOWN RECORDS
 BOOK 1 PAGE 255
 2 RODS
 10/26/1804
 (NOTE: SECTIONS OF LAYOUT NO LONGER NECESSARY
 DUE TO 1933 RELOCATION ARE CONSIDERED ABANDONED BY THE TOWN
 OF GEORGETOWN. SEE PLAN FOR LOCATIONS.)
 D.O.T. FILE NO. 5-12-26
 1933

SYMBOLS	
●	RAILROAD SPIKE
▲	CONTROL MONUMENTS
—	MAINE DEPARTMENT OF TRANSPORTATION - CENTERLINE CONTROL
—	MAINE STATE COORDINATE SYSTEM - ZONE
—	CENTERLINE CONTROL MONUMENTS
—	TRAVERSE CONTROL POINTS
—	STATION
—	NORTH
—	EAST
—	NUMBER
—	NORTH
—	EAST
—	WATER LINE
—	GAS LINE
—	ELECTRIC LINE
—	TELEPHONE LINE
—	SEWER LINE
—	PROPERTY LINE
—	LIMITS OF WROUGHT PORTION
—	EXISTING RIGHT OF WAY
—	NEW RIGHT OF WAY
—	NEW RIGHT OF WAY WITHIN EXISTING RIGHT OF WAY
—	CONTROL OF ACCESS

007956.00 BRIDGE NO. 2927

STATE OF MAINE
 DEPARTMENT OF TRANSPORTATION
 RIGHT OF WAY MAP
 STATE AID HIGHWAY NO. 1
 GEORGETOWN SAGadahoc COUNTY
 FEDERAL AID PROJECT NO. BR-7956(OO)X

DATE: DECEMBER 1999
 SCALE: 1 = 250
 SHEET NO. 2 OF 4 SHEETS
 D.O.T. FILE NO. 12-107

JOHN G. MELROSE
 COMMISSIONER

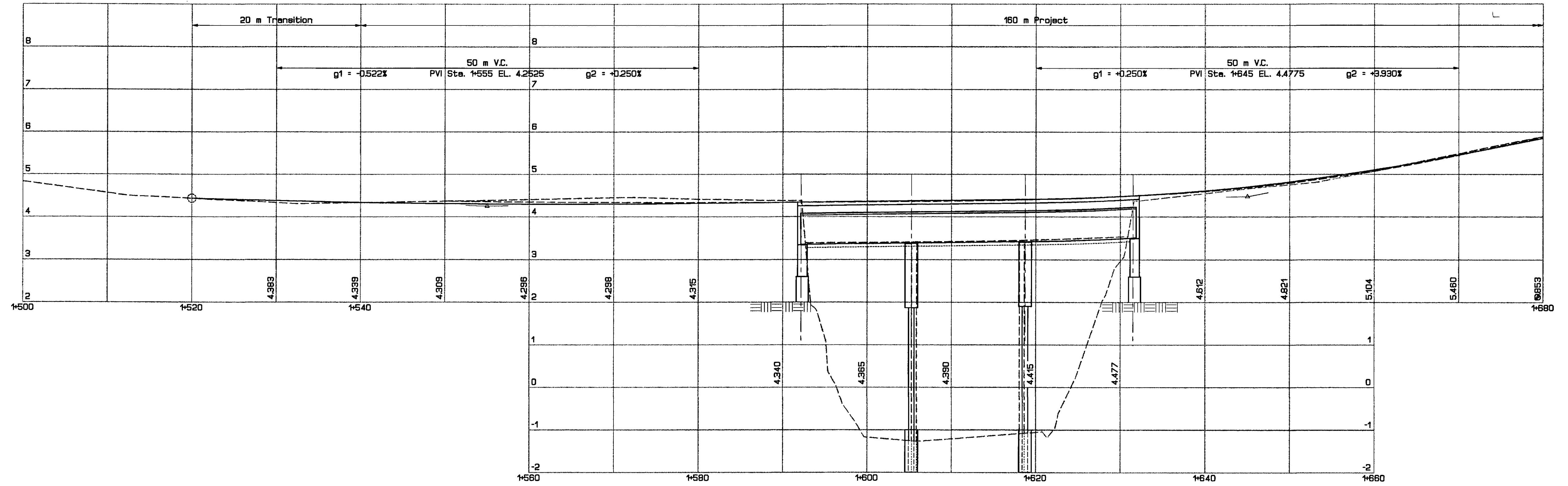
JOHN E. DORITY
 CHIEF ENGINEER

CARLSON

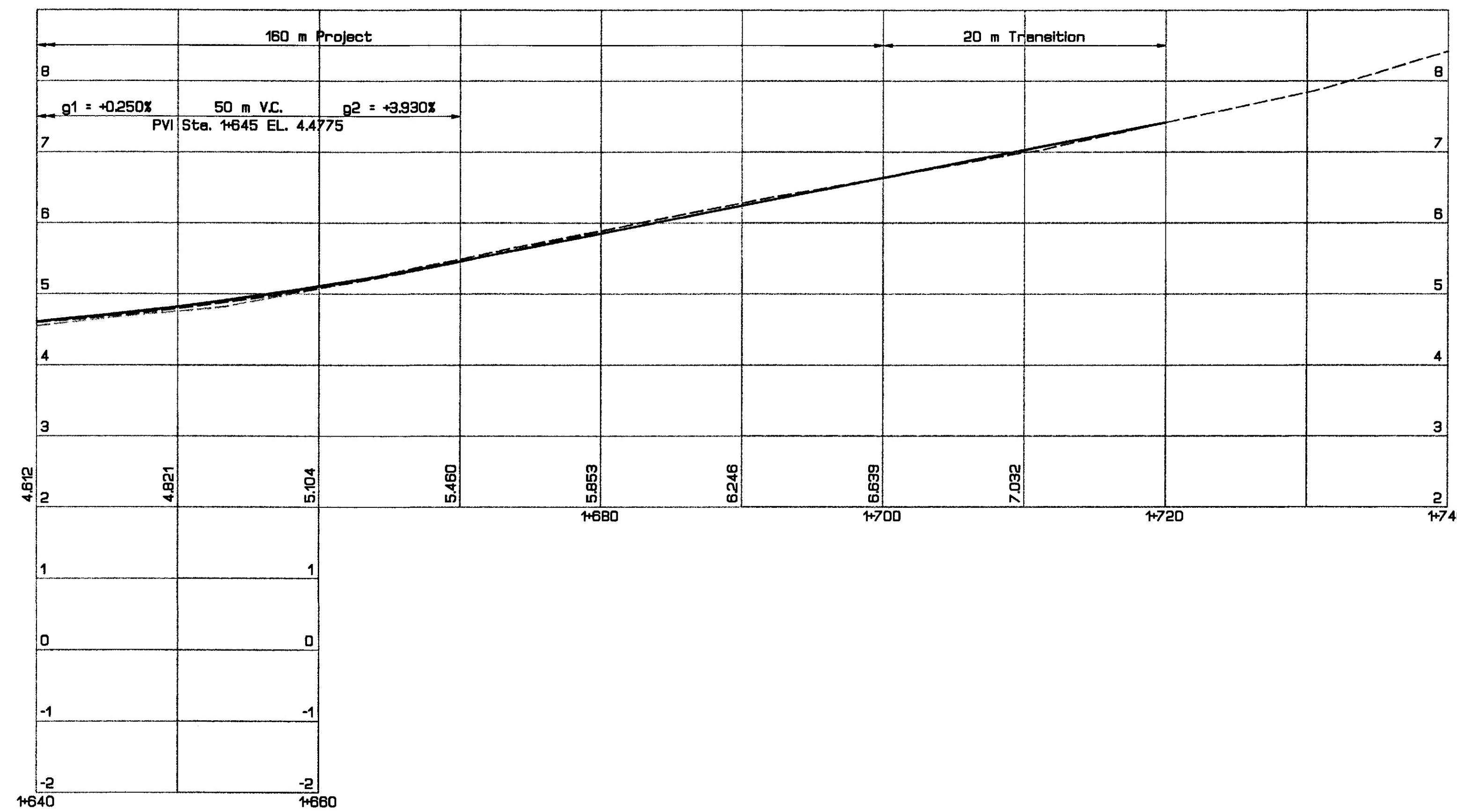
METRIC

- All dimensions are in millimeters unless otherwise noted.
- All elevations and stations are in meters.

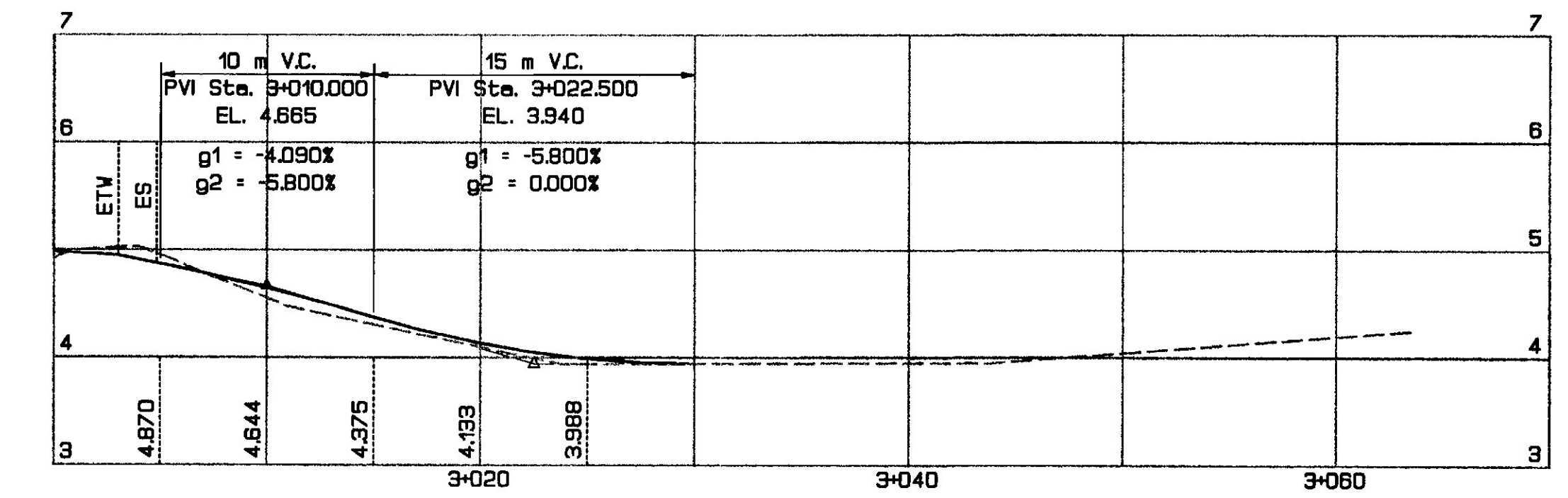
F.H.V.A. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	BR-7959(00)X	31	51



EAST BRIDGE PROFILE



EAST BRIDGE PROFILE



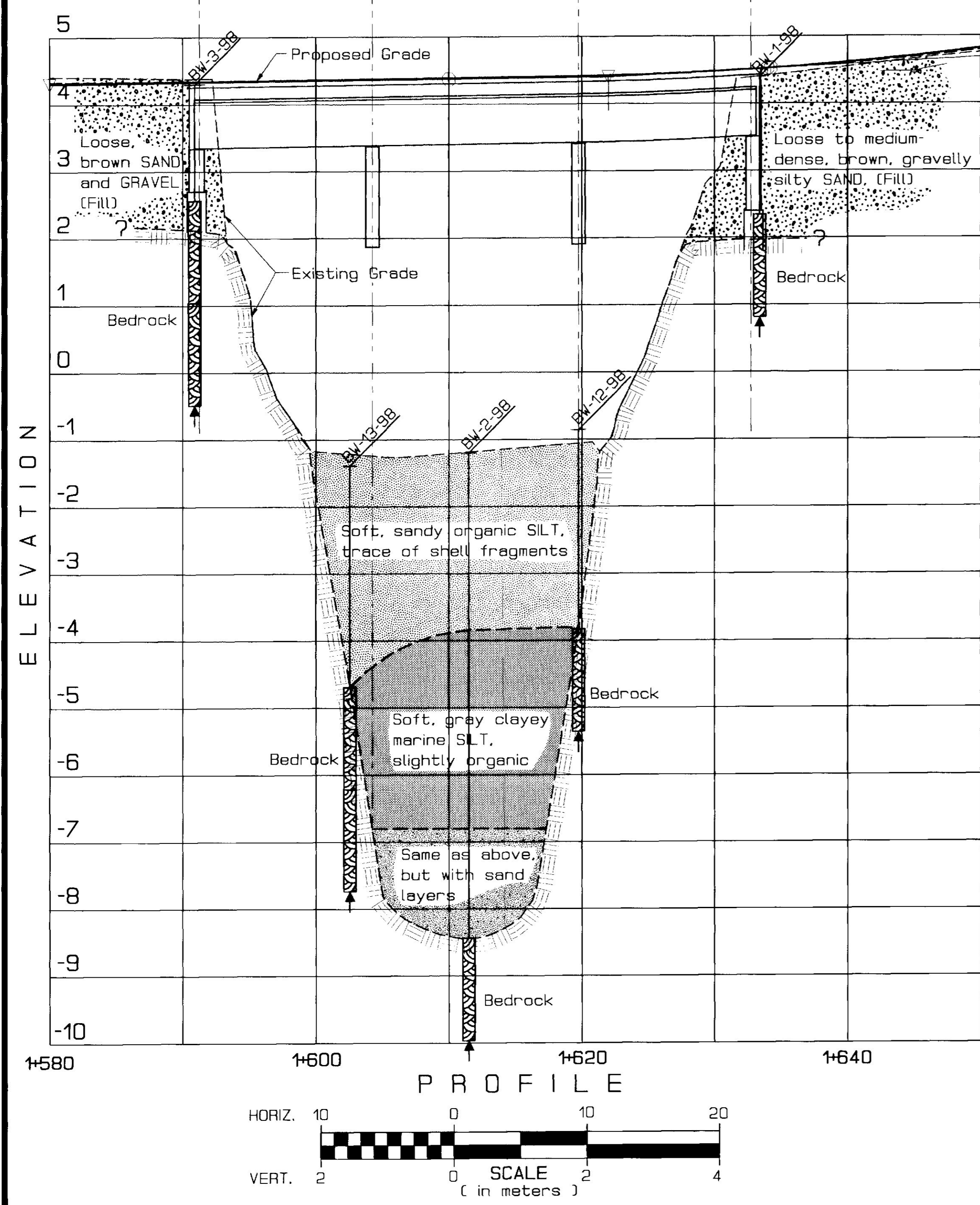
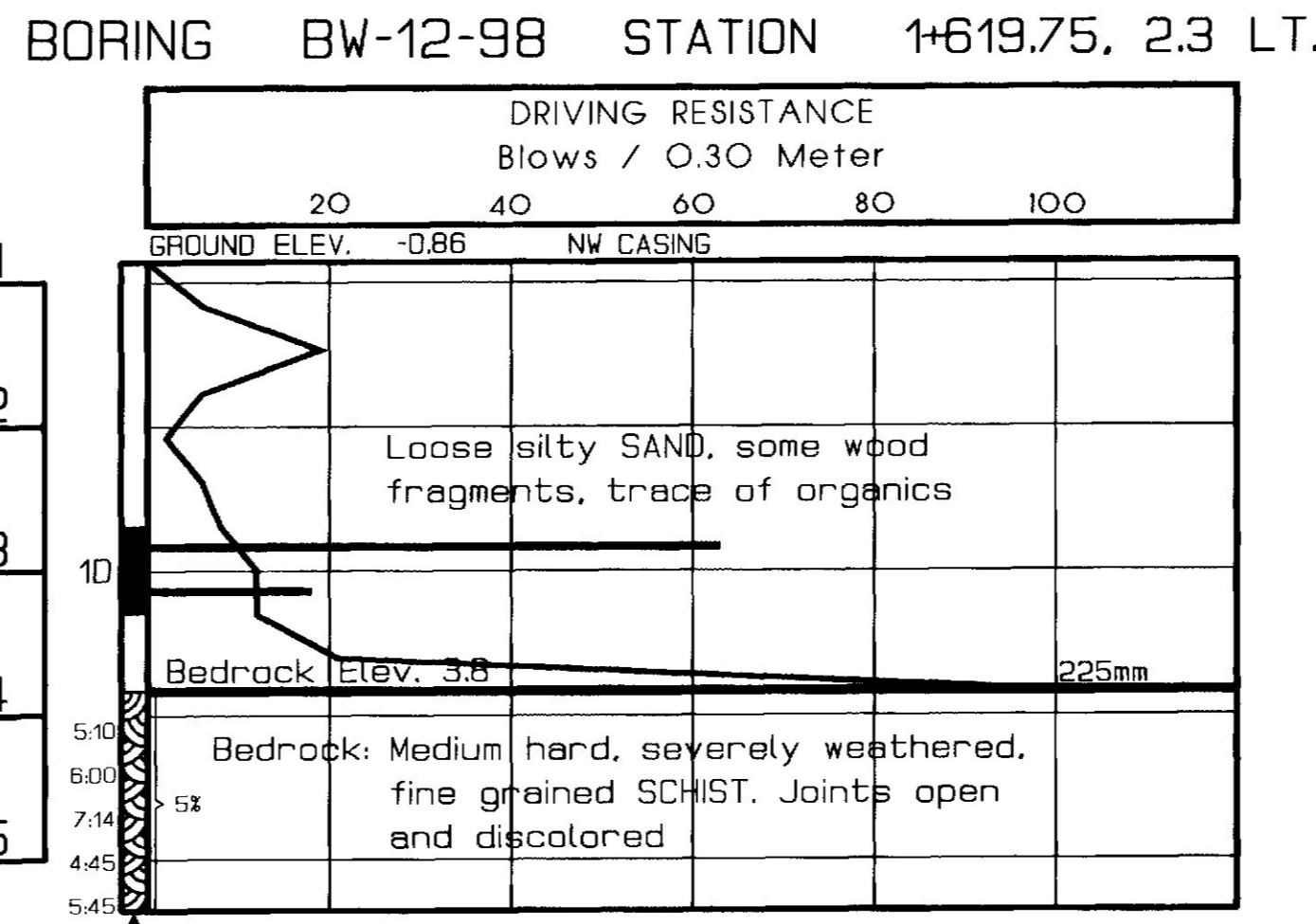
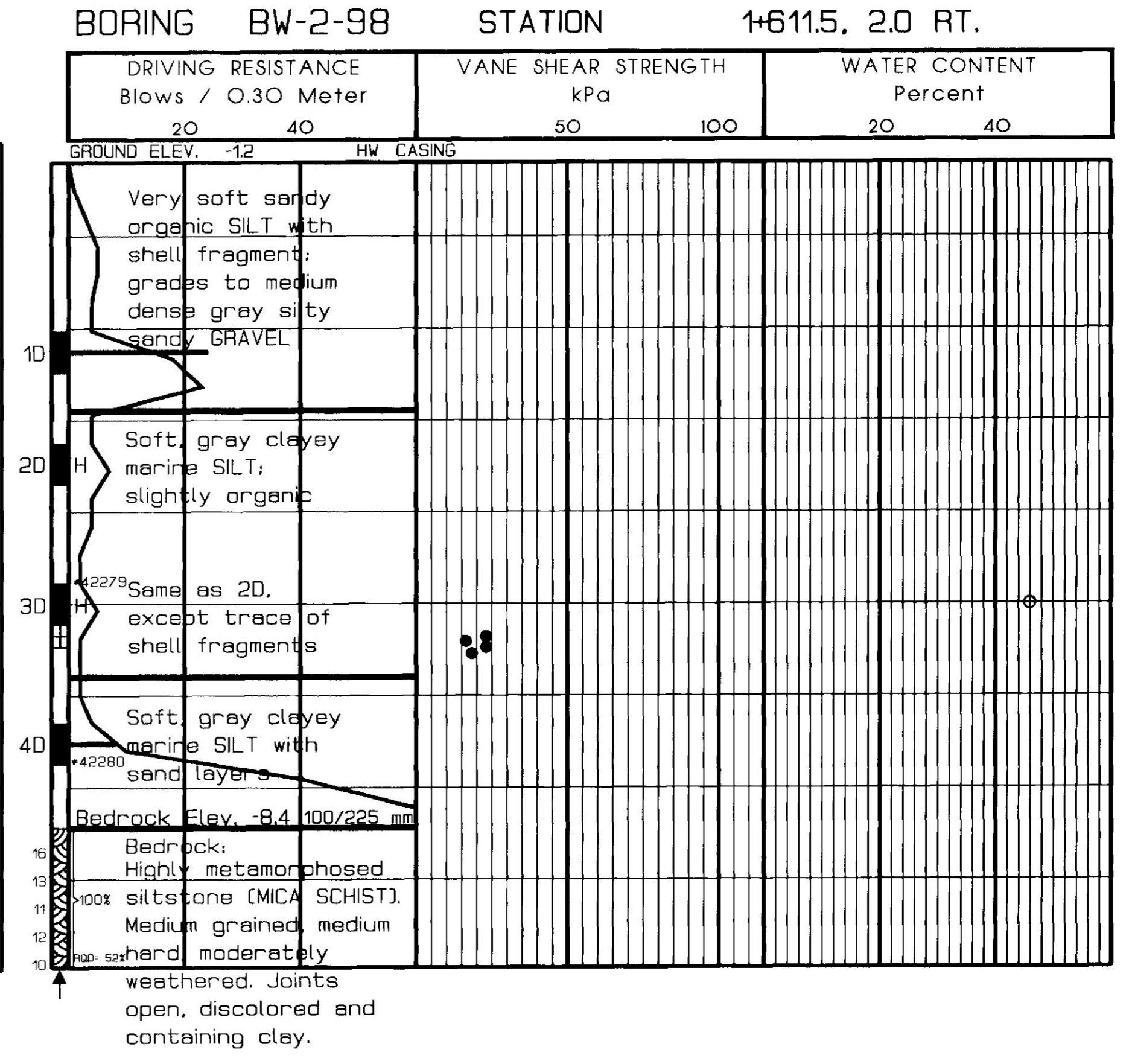
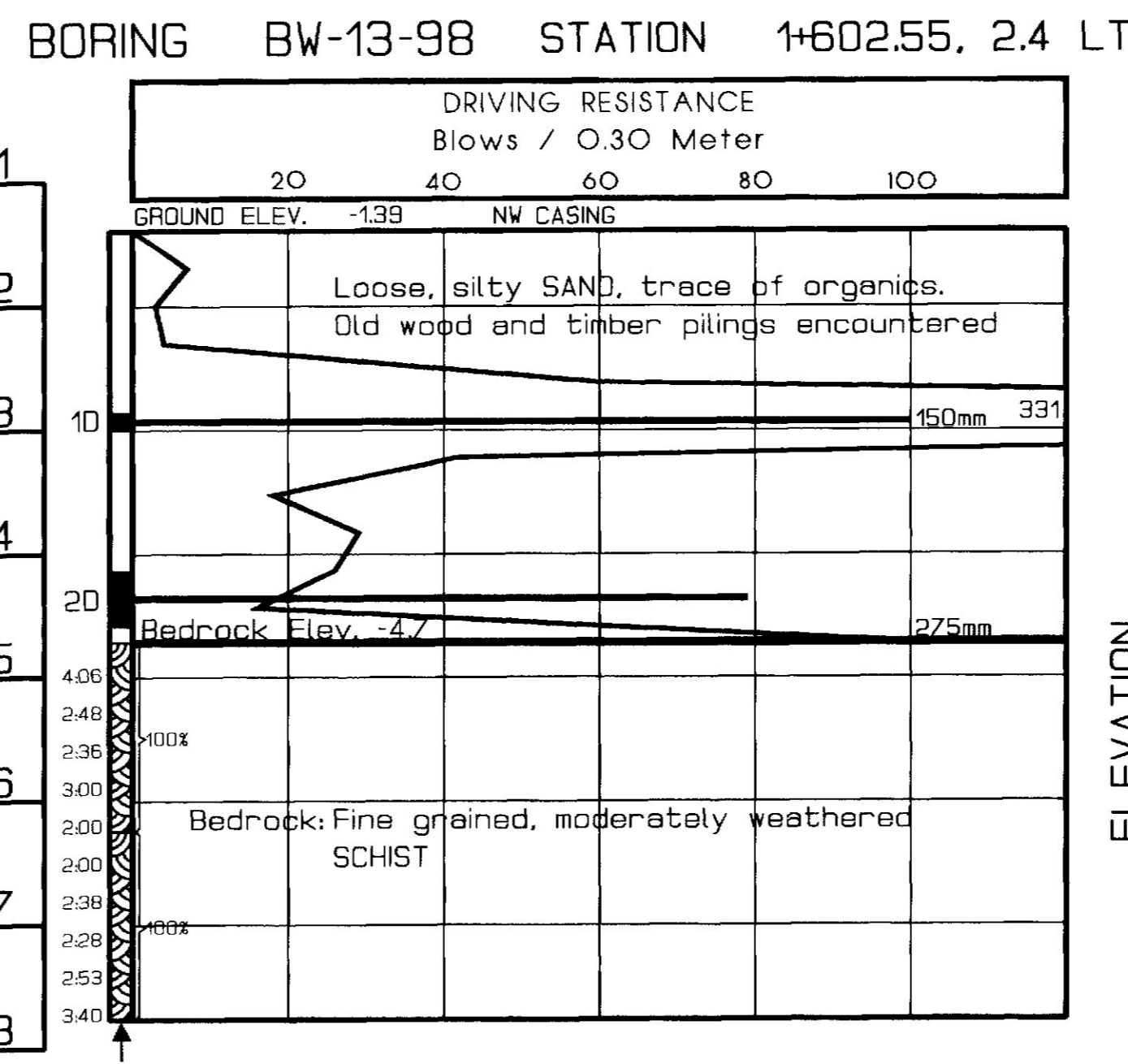
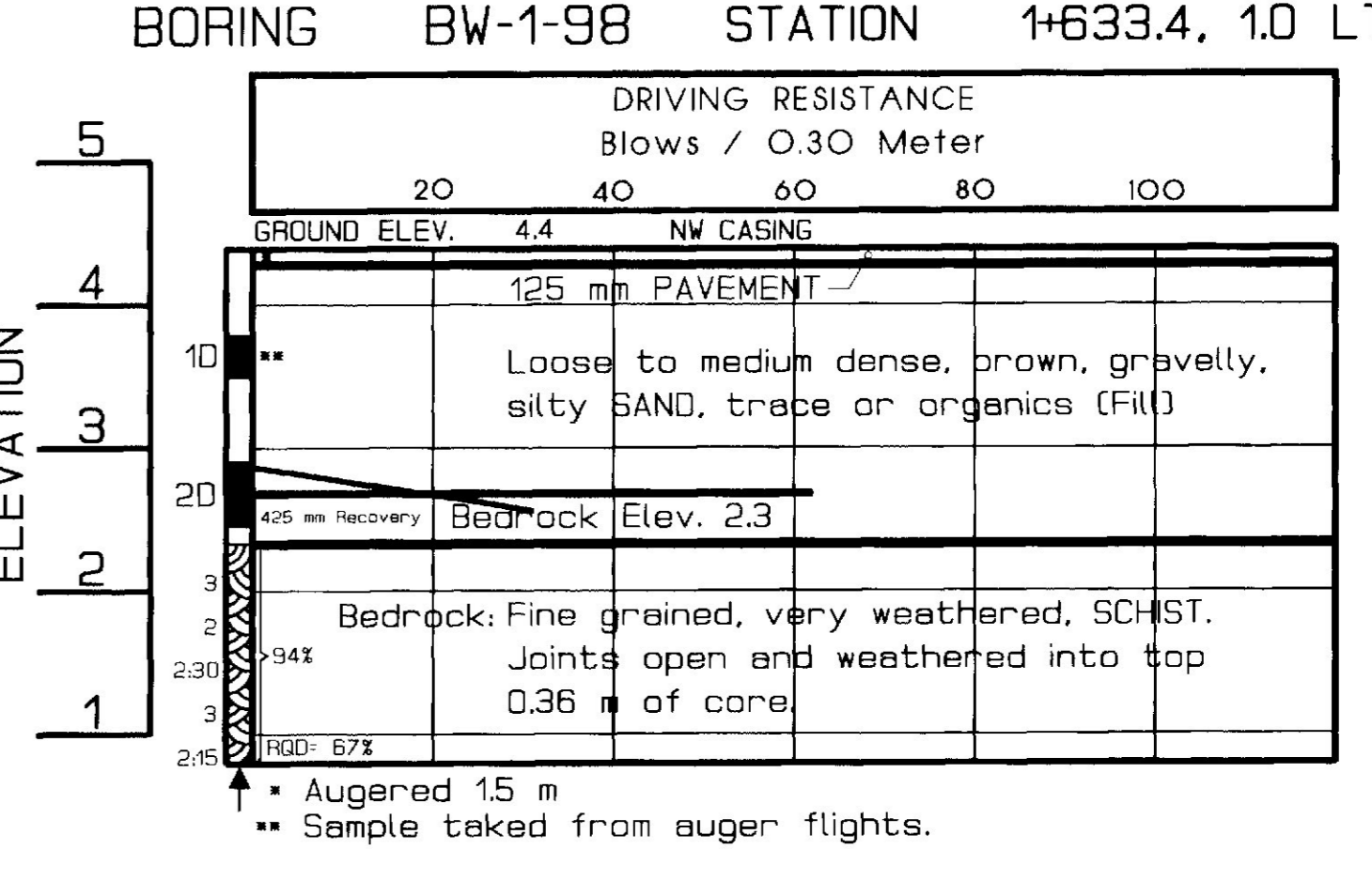
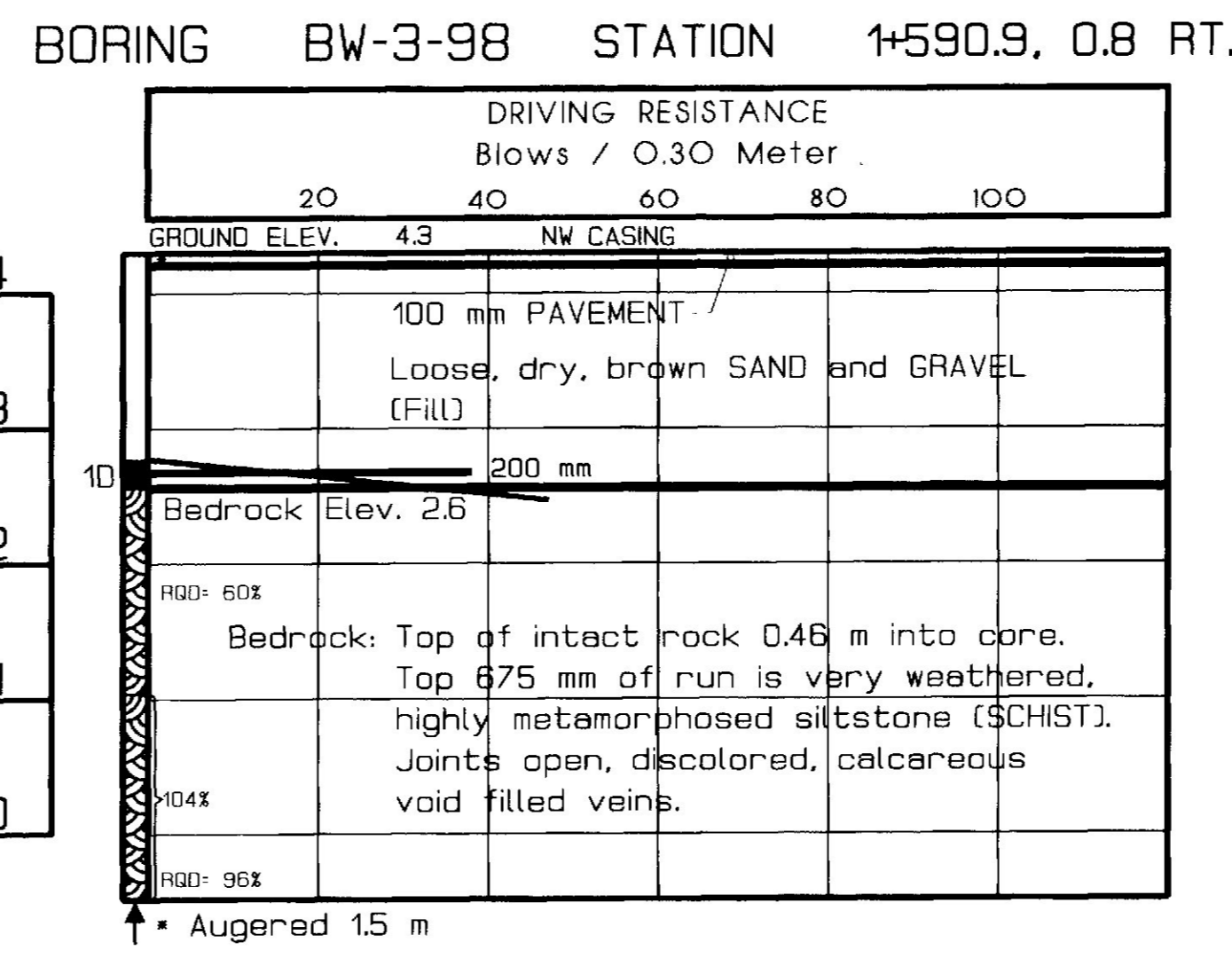
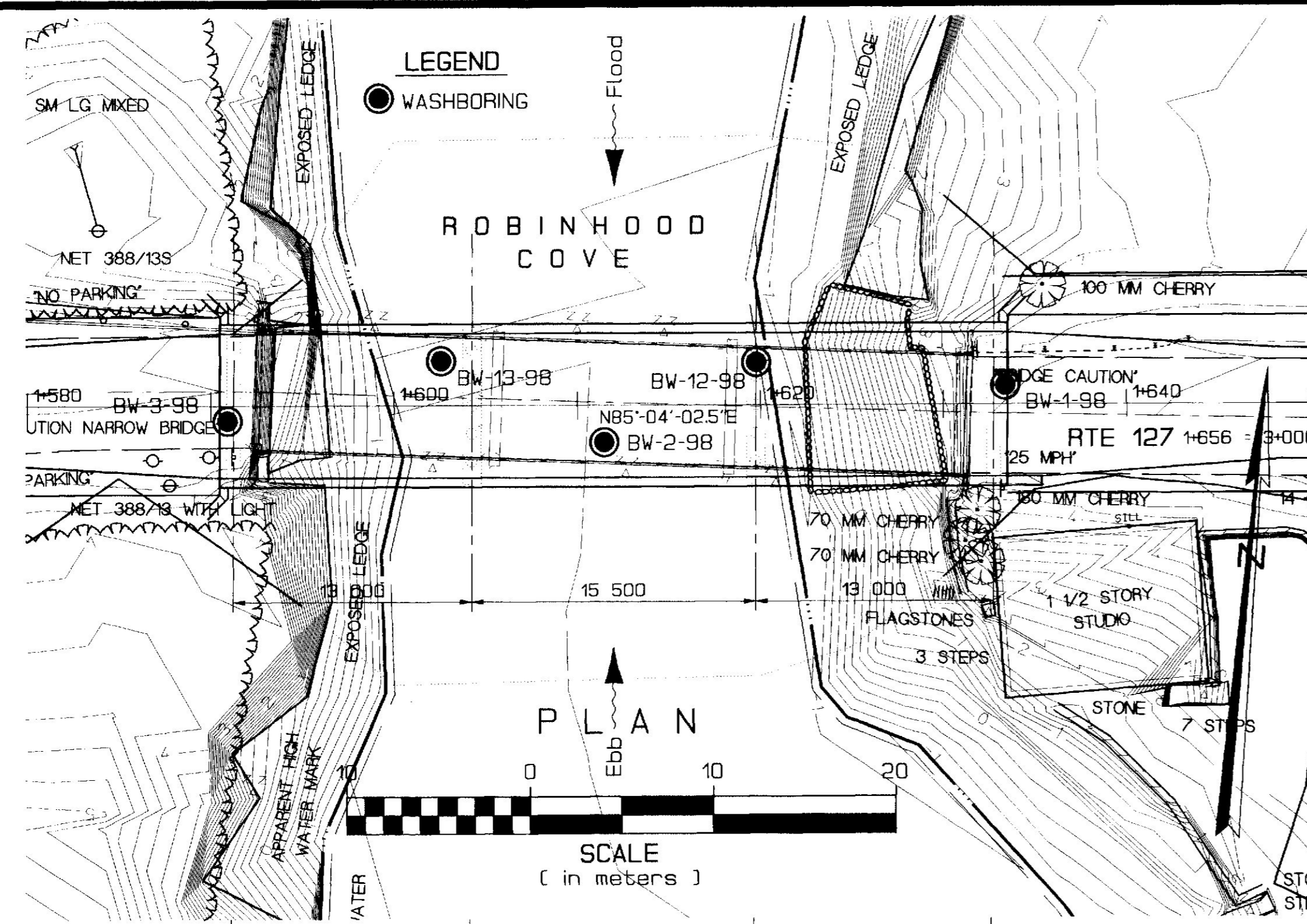
INDIAN POINT ROAD PROFILE

PROJECT DESIGN ENGINEER	BY	DATE
DESIGN-DETAILED	RVR	8/99
CHECKED	Committee	X/9X
REVISIONS	X	
FIELD CHANGES		

01MAR00-0100.10
PROF3

Bridge No. 2248
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
EAST BRIDGE
OVER
ROBINHOOD COVE
IN THE TOWN OF
GEORGETOWN
SAGADAHOC COUNTY
PROFILE

MILLPLOT File Generated 30 MAY 00 09:34:00 For 80DANNO
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PIN_DISK2:\PIN_7959.00\BRIDGE\BDRPROF.FGB1
Windows PRECAST1 PROF3 INKPROZ



- BORING NOTES**
- All samples and vane are made ahead of casing Water Elevation
 - Number of blows required to drive extra heavy casing 0.3 meters with 542 Nm of energy per blow
 - Location of sample or sample attempt
 - Number and type of dry sample
 - ID Split spoon sampler
 - IC 51 mm O.D. 10 ga. seamless tubing
 - IU 88 mm O.D. 10 ga. seamless tubing
 - W Wash sample and number
 - MD Unsuccessful sample attempt and type of sampler
 - DS Dredge Spoil Samples
 - Number of blows required to drive spoon or tubing 0.3 meters with 475 Nm of energy per blow
 - Sampling spoon or seamless tubing driven by static weight of drill rods and hammer
 - P Piston sampler
 - Field vane test
 - Bottom of boring (may not be bottom of soil strata)
 - Refusal of drill rods or casing (may not be ledge)
 - Locations cored by diamond bit and percent recovery of rock
 - Minutes/0.3 Meter Drilled
 - Natural water contents, given as percent dry weight
 - Plastic and liquid limits
 - Ignition losses are given as percent of dry weight
- SHEAR NOTES**
- Field vane shear strengths
 - Laboratory vane shear strengths
 - Shear strengths in excess of capacity of equipment
 - One half unconfined compressive strengths
- WATER CONTENT NOTES**

NOTE: Plan and Profile drawings may not be final working drawings.

STATE OF MAINE
 DEPARTMENT OF TRANSPORTATION

EAST BRIDGE
 over
 ROBINHOOD COVE
 in the town of
 GEORGETOWN
 SAGadahoc COUNTY
 FOUNDATION SURVEY EAST

SHEET OF AUGUSTA, MAINE

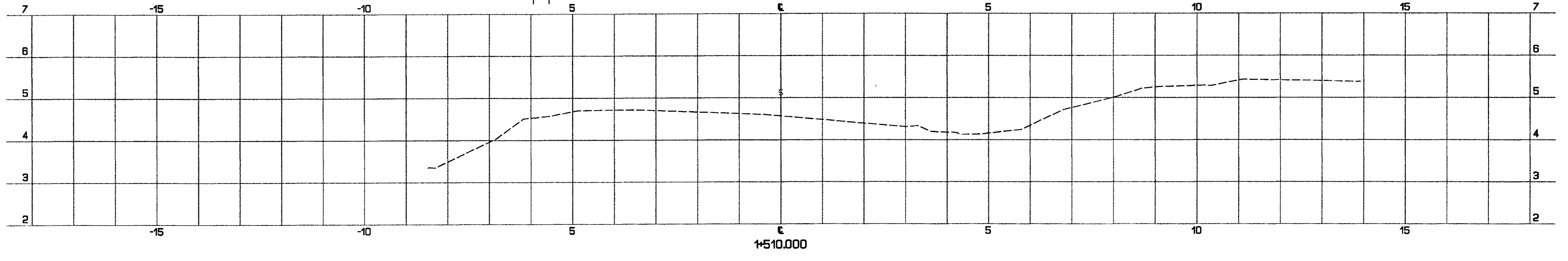
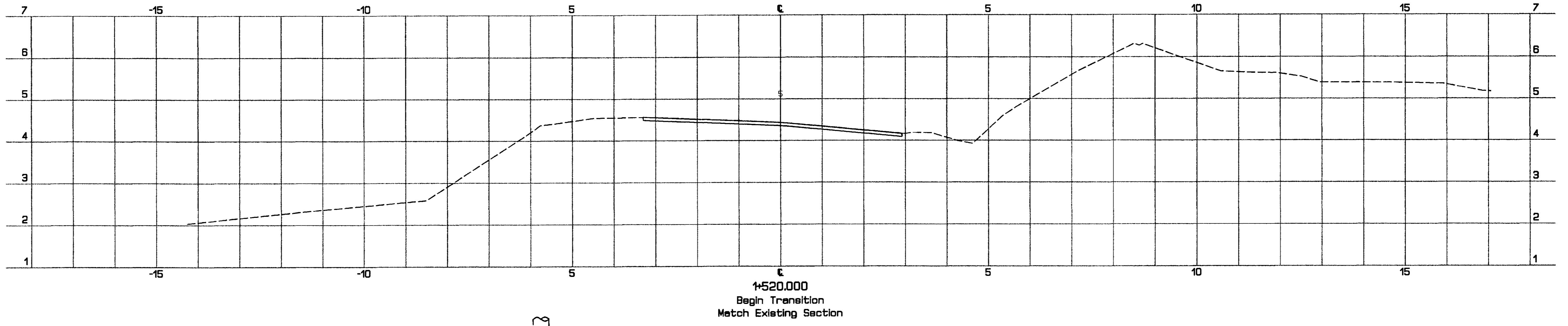
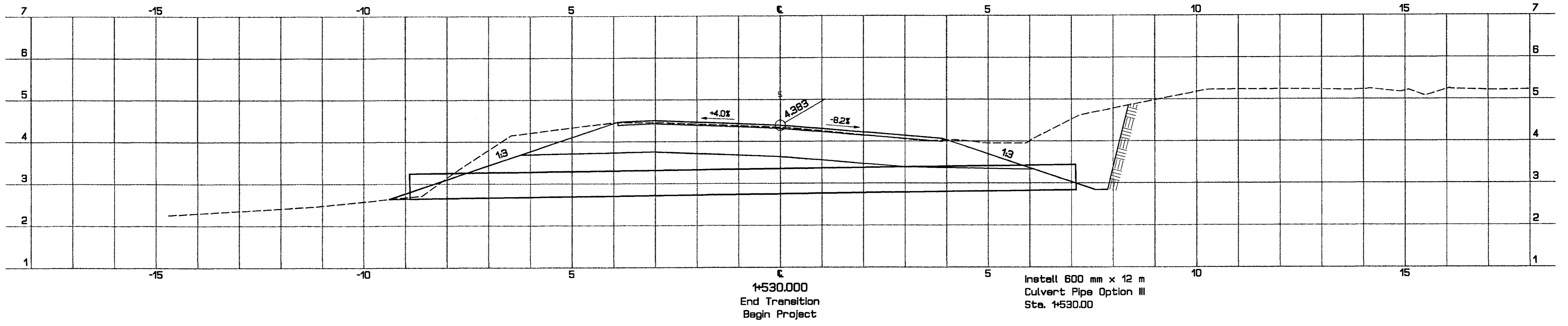
PROJECT DESIGN NUMBER: LKRUSINSKI
 BY: T. WHITE
 DATE: 20 NOV 97 - 01.00.10

DESIGN-DETAILED
 CHECKED
 REVISIONS
 FIELD CHANGES

PLANS

DATE	
BY	
DESIGNED	
PLOTTED	
TEMPLATE	
INSPK	
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DATE	
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