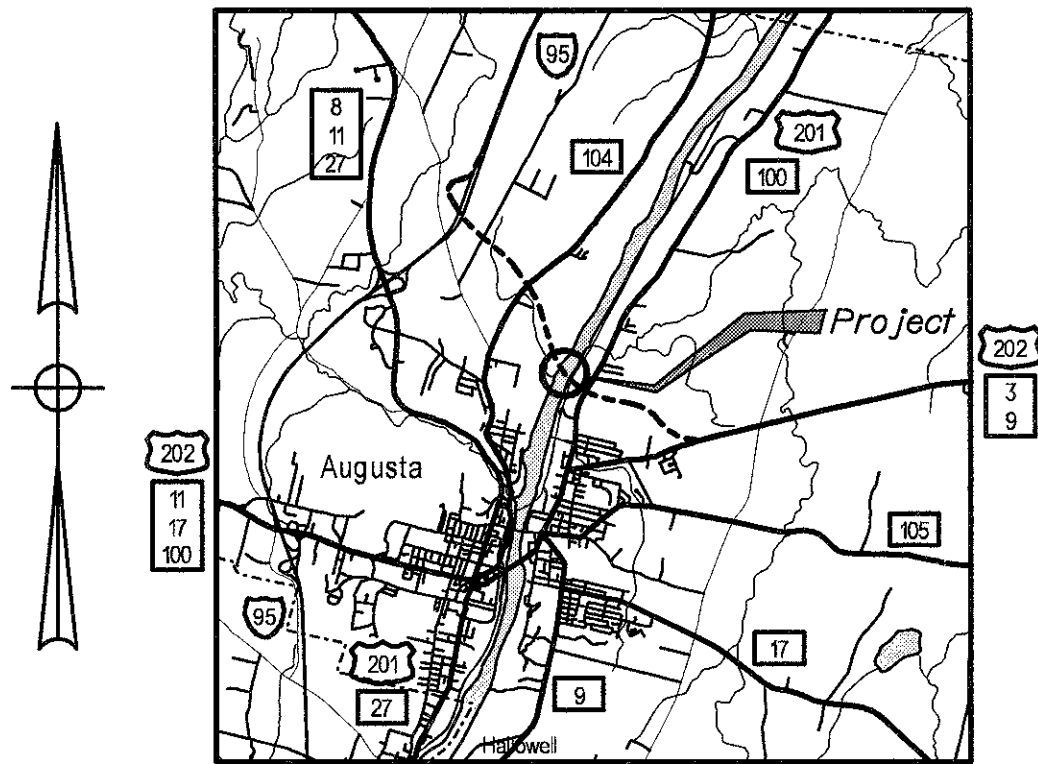


STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

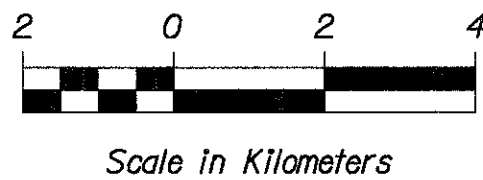


AUGUSTA  
KENNEBEC COUNTY  
STATE ROUTE 3  
OVER  
KENNEBEC RIVER

PROJECT LENGTH 0.427 km  
NEW BRIDGE CONSTRUCTION  
BRIDGE NO. 6400



LOCATION MAP



DESIGN SPECIFICATIONS

Load Factor Design per AASHTO Standard Specifications for Highway Bridges 1996 and all supplementals thereto.  
AASHTO Guide Specifications for Horizontally Curved Highway Bridges 1993.

DESIGN LOADING

Live Load ..... MS22.5

TRAFFIC DATA

Current (2005) AADT ..... 19 500  
Future (2025) AADT ..... 23 500  
Design Speed (km/h) ..... 70

HYDROLOGIC DATA

Drainage Area ..... 14 226 km<sup>2</sup>  
Ordinary High Water (Q1.1) ..... 1 760 m<sup>3</sup>/s  
10 Year Recurrence (Q10) ..... 3 740 m<sup>3</sup>/s  
50 Year Recurrence (Q50) ..... 5 495 m<sup>3</sup>/s  
Design Discharge (Q100) ..... 6 260 m<sup>3</sup>/s  
Check Discharge (Q500) ..... 8 240 m<sup>3</sup>/s  
Headwater Elevation (Q50) ..... 10.31 m  
Headwater Elevation (Q100) ..... 11.14 m  
Headwater Elevation (Q500) ..... 13.79 m  
Velocity (Q50) ..... 2.61 m/s  
Velocity (Q100) ..... 2.75 m/s  
Velocity (Q500) ..... 2.91 m/s  
Clearance (Q100) ..... 23.98 m  
Clearance (Q500) ..... 21.33 m

MATERIALS

Concrete:  
Transition Barriers, Curbs ..... Class "LP"  
Seals ..... Class "S"  
Precast ..... Class "P"  
All Other ..... Class "A"  
Reinforcing Steel ..... ASTM A 615/A 615M, Grade 420  
Structural Steel:  
Webs and Flanges at piers ..... ASTM A 709M, Grade HPS 485W  
All Other Material (unless noted) ..... ASTM A 709M, Grade 345W  
High Strength Bolts ..... ASTM A 325M, Type 3

BASIC DESIGN STRESSES

Concrete:  
Class "LP" ..... f 'c = 35 MPa  
Class "S" ..... f 'c = 20 MPa  
Class "P" ..... f 'ci = 30 MPa  
Class "A" ..... f 'c = 35 MPa  
Class "A" ..... f 'c = 30 MPa  
Reinforcing Steel ..... f y = 420 MPa  
Structural Steel:  
ASTM A 709M, Grade HPS 485W ..... F y = 485 MPa  
ASTM A 709M, Grade 345W ..... F y = 345 MPa  
ASTM A 709/A 709M, Grade 250 ..... F y = 250 MPa  
ASTM A 325M ..... F y = 170 MPa

UTILITIES

Guilford Rail System  
Verizon

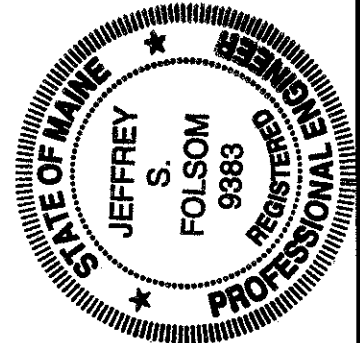
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PIN 000556.22

NH-0556(220)E

| STATE OF MAINE<br>DEPARTMENT OF TRANSPORTATION |          |
|------------------------------------------------|----------|
| APPROVED                                       | DATE     |
| COMMISSIONER: <i>Jeffrey S. Folsom</i>         | 11/25/02 |
| CHIEF ENGINEER: <i>John T. Dooly</i>           | 11/25/02 |



|                                     |                  |
|-------------------------------------|------------------|
| SIGNATURE: <i>Jeffrey S. Folsom</i> | DATE: 11/25/2002 |
| P.E. NUMBER: 9383                   |                  |

| PROJECT INFORMATION                       |                                                   |
|-------------------------------------------|---------------------------------------------------|
| PROGRAM: U & F BRIDGE PROGRAM             | DESIGNER: JEFFREY S. FOLSOM - MDOT                |
| PROJECT MANAGER: D. ANDERSON / THOMAS DOE | CONSULTANT (STRUCTURED): JEFFREY S. FOLSOM - MDOT |
| PROJECT RESIDENT: STEVEN B. PALMER - MDOT | CONTRACTOR: JUNE 30, 2004                         |

AUGUSTA  
State Route 3 over Kennebec River  
TITLE SHEET

|                                      |                 |           |           |
|--------------------------------------|-----------------|-----------|-----------|
| PROJECT DESIGN ENGINEER<br><br>PLANS | DESIGN/DETAILED | BY        | DATE      |
|                                      | CHECKED         | J. Folsom | D. Damren |
|                                      | REVISIONS       |           |           |
|                                      | FIELD CHANGES   |           |           |

| ESTIMATED QUANTITIES |                                                        |          |      |
|----------------------|--------------------------------------------------------|----------|------|
| ITEM NO.             | DESCRIPTION                                            | QUANTITY | UNIT |
| 107.51               | PROSECUTION OF WORK - INITIAL SCHEDULE                 | 1        | LS   |
| 107.52               | PROSECUTION OF WORK - MONTHLY UPDATE                   | 18       | Each |
| 203.20               | COMMON EXCAVATION                                      | 1700     | m³   |
| 203.24               | COMMON BORROW                                          | 6 750    | m³   |
| 203.246              | LOW PERMEABILITY FILL                                  | 1 350    | m³   |
| 203.25               | GRANULAR BORROW                                        | 600      | m³   |
| 203.4231             | PLACEMENT OF EMBANKMENT FILL,TIRE SHRED                | 1750     | MG   |
| 206.082              | STRUC. EARTH EXCAVATION - MAJOR STRUCTURES             | 410      | m³   |
| 206.092              | STRUC. ROCK EXCAVATION - MAJOR STRUCTURES              | 25       | m³   |
| 206.10               | STRUC. EARTH EXCAVATION - PIERS                        | 1 170    | m³   |
| 304.10               | AGGREGATE SUBBASE COURSE - GRAVEL                      | 1 400    | m³   |
| 501.231              | DYNAMIC LOADING TEST                                   | 11       | Each |
| 501.50               | STEEL H-BEAM PILES 132 kg/m, DELIVERED                 | 560      | m    |
| 501.501              | STEEL H-BEAM PILES 132 kg/m, IN PLACE                  | 560      | m    |
| 501.53               | STEEL H-BEAM PILES 108 kg/m, DELIVERED                 | 2 720    | m    |
| 501.531              | STEEL H-BEAM PILES 108 kg/m, IN PLACE                  | 2 720    | m    |
| 501.90               | PILE TIPS                                              | 128      | Each |
| 501.91               | PILE SPLICES                                           | 180      | Each |
| 501.92               | PILE DRIVING EQUIPMENT MOBILIZATION                    | 1        | LS   |
| 502.219              | STRUC. CONCRETE ABUTMENTS & RET. WALLS (825 m³)        | 1        | LS   |
| 502.23               | STRUC. CONCRETE PIERS                                  | 165      | m³   |
| 502.239              | STRUC. CONCRETE PIERS (2 905 m³)                       | 1        | LS   |
| 502.24               | STRUC. CONCRETE PIERS PLACED UNDER WATER               | 1 120    | m³   |
| 502.26               | STRUC. CONCRETE RD&SW SLAB ON STEEL BRIDGES (1 795 m³) | 1        | LS   |
| 502.31               | STRUC. CONCRETE APPROACH SLAB (36 m³)                  | 1        | LS   |
| 502.49               | STRUC. CONCRETE CURBS AND SIDEWALKS (80 m³)            | 1        | LS   |
| 503.121              | REINF. STEEL, FABRICATED & DELIVERED (383 700 kg)      | 1        | LS   |
| 503.131              | REINF. STEEL, PLACING (383 700 kg)                     | 1        | LS   |
| 504.702              | STRUC. STEEL FAB. & DELIVERED, WELDED (1 487 900 kg)   | 1        | LS   |
| 504.71               | STRUC. STEEL ERECTION (1 487 900 kg)                   | 1        | LS   |
| 505.08               | SHEAR CONNECTORS (10 932 Each)                         | 1        | LS   |
| 507.0833             | MODIFIED WYOMING STEEL BRIDGE RAIL (715 m)             | 1        | LS   |
| 511.07               | COFFERDAM: PIER NO. 2                                  | 1        | LS   |
| 511.07               | COFFERDAM: PIER NO. 3                                  | 1        | LS   |
| 512.081              | FRENCH DRAINS (38 m)                                   | 1        | LS   |
| 514.06               | CURING BOX FOR CONC CYLINDERS                          | 1        | Each |
| 515.21               | PROTECTIVE COATING FOR CONCRETE SURFACES (1 900 m²)    | 1        | LS   |
| 521.23               | EXPANSION DEVICE - FINGER JOINT - ABUTMENT NO.1        | 1        | Each |
| 521.23               | EXPANSION DEVICE - FINGER JOINT - ABUTMENT NO. 2       | 1        | Each |
| 521.32               | FABRIC TROUGH FOR FINGER JOINT                         | 2        | Each |
| 523.10               | POT BEARINGS                                           | 36       | Each |
| 526.34               | PERMANENT CONCRETE TRANSITION BARRIER                  | 4        | Each |
| 603.155              | 300 MM RCP CLASS III                                   | 71       | m    |
| 604.249              | CATCH BASIN TYPE F6-C                                  | 2        | Each |
| 605.09               | 150 MM UNDERDRAIN TYPE B                               | 35       | m    |
| 610.08               | PLAIN RIPRAP                                           | 10       | m³   |
| 610.082              | PLACING STOCKPILED PLAIN RIPRAP                        | 325      | m³   |
| 610.161              | PLACING STOCKPILED HEAVY RIPRAP                        | 420      | m³   |
| 610.18               | STONE DITCH PROTECTION                                 | 19       | m³   |
| 610.181              | PLACING STOCKPILED STONE DITCH PROTECTION              | 60       | m³   |
| 613.319              | EROSION CONTROL BLANKET                                | 60       | m²   |
| 615.07               | LOAM                                                   | 63       | m³   |
| 617.381              | EROSION CONTROL MIX                                    | 10       | m³   |
| 618.1401             | SEEDING METHOD NUMBER 2 - PLAN QUANTITY                | 2        | Unit |
| 618.1411             | SEEDING METHOD NUMBER 3 - PLAN QUANTITY                | 10       | Unit |
| 618.15               | TEMPORARY SEEDING                                      | 5        | kg   |
| 619.1201             | MULCH - PLAN QUANTITY                                  | 13       | Unit |
| 620.58               | EROSION CONTROL GEOTEXTILE                             | 75       | m²   |
| 620.60               | REINFORCEMENT GEOTEXTILE                               | 540      | m²   |
| 629.05               | HAND LABOR - STRAIGHT TIME                             | 100      | MH   |
| 631.12               | ALL PURPOSE EXCAVATOR (INCL OPERATOR)                  | 100      | Hour |
| 631.13               | BULLDOZER (INCL OPERATOR)                              | 100      | Hour |
| 631.172              | TRUCK - LARGE (INCL OPERATOR)                          | 100      | Hour |
| 631.36               | FOREMAN                                                | 100      | Hour |
| 638.01               | EMBEDDED WORK IN STRUCTURES                            | 1        | LS   |
| 639.18               | FIELD OFFICE TYPE A                                    | 1        | Each |
| 652.35               | CONSTRUCTION SIGNS                                     | 13       | m²   |
| 652.361              | MAINTENANCE OF TRAFFIC CONTROL DEVICES                 | 1        | LS   |
| 652.38               | FLAGGER                                                | 1 000    | MH   |
| 656.75               | TEMP. SOIL EROSION AND WATER POLLUTION CONTROL         | 1        | LS   |
| 659.10               | MOBILIZATION                                           | 1        | LS   |
| 660.21               | ON - THE - JOB TRAINING (BID)                          | 5 000    | MH   |
| 806.68               | UTILITY CONDUIT                                        | 1        | LS   |
| 845.101              | STRUCTURAL STEEL UTILITY SUPPORT - FURNISH             | 1        | LS   |
| 845.102              | STRUCTURAL STEEL UTILITY SUPPORT - INSTALL             | 1        | LS   |

EARTHWORK SUMMARY

COMMON EXCAVATION FOR ESTIMATE

|                                         |       |
|-----------------------------------------|-------|
| COMMON EXCAVATION (FROM CROSS SECTIONS) | 1,693 |
| GRUBBING IN FILL                        | 0     |
| EXCAVATION OF PIERS 2 & 3 (FROM MDOT)   | 1,170 |

TOTAL COMMON EXCAVATION 2,863

FILL FOR BORROW CALCULATIONS

|                                   |       |
|-----------------------------------|-------|
| COMMON FILL (FROM CROSS SECTIONS) | 6,747 |
| GRUBBING IN FILL                  | 0     |

TOTAL FILL 6,747

ROCK EXCAVATION

|                                       |   |
|---------------------------------------|---|
| ROCK EXCAVATION (FROM CROSS SECTIONS) | 0 |
|---------------------------------------|---|

TOTAL ROCK EXCAVATION 0

AVAILABLE COMMON EXCAVATION FOR BORROW CALCULATIONS

|                                     |       |
|-------------------------------------|-------|
| (1) TOTAL COMMON EXCAVATION         | 2,863 |
| DEDUCTIONS:                         |       |
| GRUBBING IN FILL                    | 0     |
| GRUBBING IN CUT                     | 0     |
| UNSUITABLE MATERIAL (WEST SIDE CUT) | 1,693 |

|                                                 |       |
|-------------------------------------------------|-------|
| (2) TOTAL DEDUCTIONS                            | 1,693 |
| TOTAL AVAILABLE COMMON EXCAVATION (1) MINUS (2) | 1,170 |
| TOTAL AVAILABLE STRUCTURAL EARTH EXCAVATION     | 0     |
| TOTAL AVAILABLE STRUCTURAL ROCK EXCAVATION      | 0     |

TOTAL AVAILABLE NON-ROCK EXCAVATION 1,170

COMPUTATION OF COMMON BORROW FOR ESTIMATE

|                                                         |              |     |       |
|---------------------------------------------------------|--------------|-----|-------|
| (3) TOTAL FILL                                          |              |     | 6,747 |
| TOTAL AVAILABLE NON-ROCK EXCAV.                         | 1,170 * 0.85 | 995 |       |
| TOTAL AVAILABLE ROCK EXCAV.                             | 0 * 1.33     | 0   |       |
| (4) TOTAL AVAILABLE EXCAVATION                          |              |     | 995   |
| TOTAL FILL MINUS TOTAL AVAILABLE EXCAVATION [(3) - (4)] |              |     | 5,752 |
| TOTAL BORROW                                            | 5,752 * 1.15 |     | 6,615 |

NOTE: THE EARTHWORK SUMMARY CALCULATIONS ASSUME NO LOAM SALVAGE AND NO ON-SITE WASTE STORAGE.

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                      |       |                |              |                 |
|----------------------|-------|----------------|--------------|-----------------|
| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                    | MAINE | NH-055612201E  | 2            | 105             |

000556.22

GENERAL NOTES

1. For easements, construction limits, and right of way lines, refer to the Right of Way Map.

2. A hydrologic report of the bridge site is available for the Contractor's reference at the temporary office of the Urban & Federal Bridge Program in Winthrop. The hydrologic report is based on the Department's interpretation of information obtained for the subject site. No assurance is given that the information or the conclusions of the report will be representative of actual conditions at the time of construction.

3. Two geotechnical reports of the bridge site (West Embankment Slope Stability Kennebec River Crossing Project, Augusta, Maine, March 29, 2002; and Phase II Subsurface Investigation for Contract 3, Kennebec River Bridge, Nov. 2002) are available for the Contractor's reference at the temporary office of the Urban & Federal Bridge Program in Winthrop. The geotechnical reports are based on the authors' interpretation of information obtained for the subject site. No assurance is given that the information or the conclusions of the reports will be representative of actual subsurface conditions at the time of construction.

4. Coordinates shown throughout these plans are given as a convenience to the Contractor and are in no way intended to relieve the Contractor's responsibilities under Section 105, General Scope of Work.

5. Earth excavation for Pier No. 1 and Pier No. 4 will be paid for under Item No. 206.082, Structural Earth Excavation, Major Structures.

6. Dredge materials shall be used as construction fill in the approach roadway embankment east of Abutment No. 2 in accordance with Special Provision, Section 203, Excavation and Embankment (Riverbed Excavation -- Dredge Materials).

7. Any necessary preparation of the previously constructed abutment foundation areas will be paid for under Item No.'s 203.25, Granular Borrow and/or 206.082, Structural Earth Excavation - Major Structures.

8. Place a 600 mm wide strip of temporary erosion control blanket on the side slopes along the top of the riprap and on the slopes along the faces of the wingwalls.

9. Any spillage of oil - based products during construction shall be promptly reported to the United States Coast Guard by calling 1-800-424-8802.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ESTIMATE & NOTES**

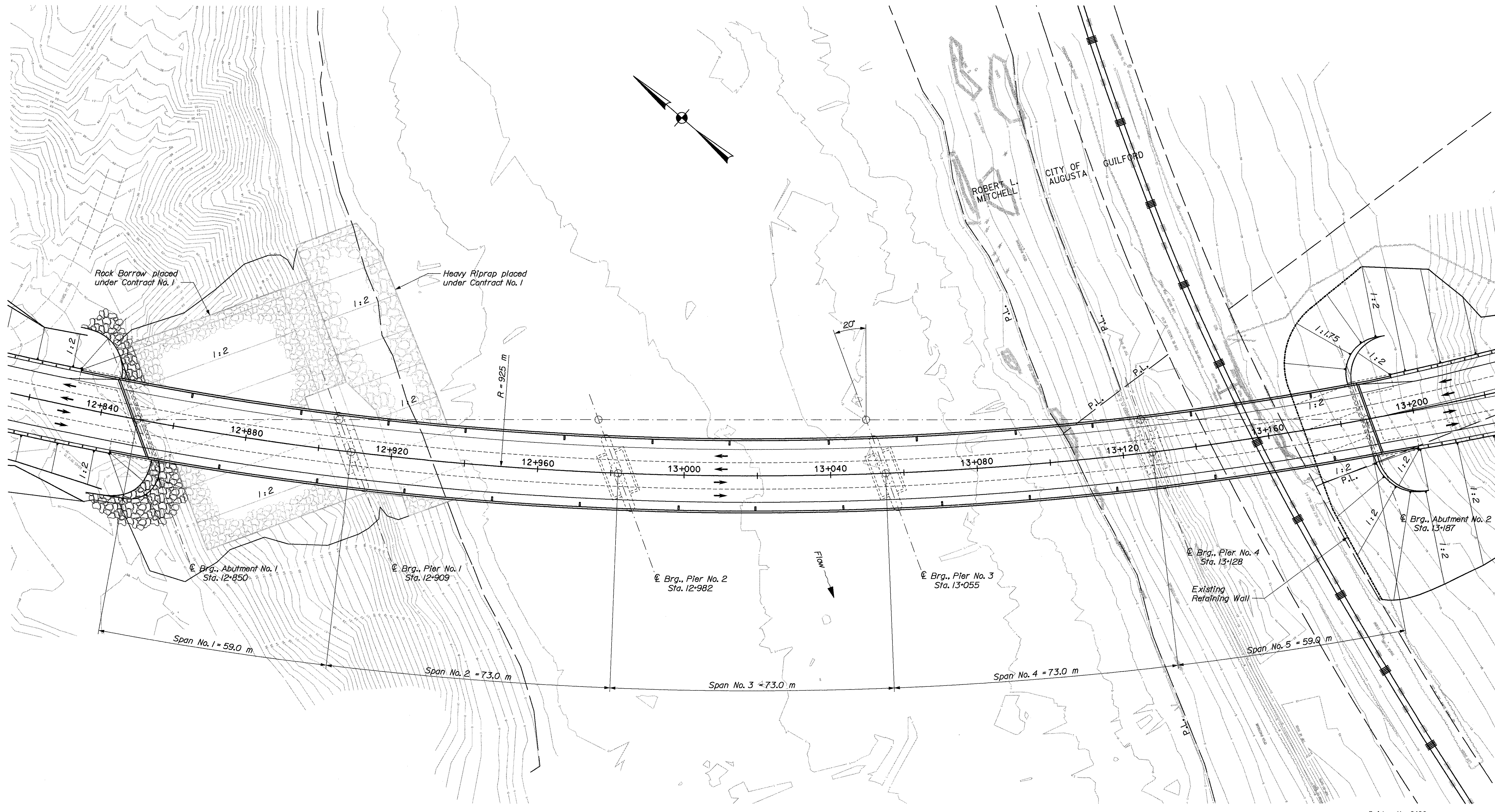
METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|-----------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 3         | 103             |
| PIN 000556.22        |       |                |           |                 |

| PROJECT DESIGN ENGINEER | XXX       | BY        | DATE      |
|-------------------------|-----------|-----------|-----------|
| DESIGN-DETAILED         | J. Folsom | D. Damren | Nov. 2002 |
| CHECKED                 | XXX       |           | XXX       |
| REVISIONS               | XXX       |           | XXX       |
| FIELD CHANGES           | XXX       |           | XXX       |

PLANS



Bridge No. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**GENERAL PLAN**

SHEET 3 OF 103  
AUGUSTA, MAINE  
December 2002

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ... \004\_profile.dgn

|                         |           |
|-------------------------|-----------|
| PROJECT DESIGN ENGINEER | DATE      |
| DESIGN DETAIL           | Nov. 2002 |
| CHECKED                 |           |
| REVISIONS               |           |
| FIELD CHANGES           |           |

|           |      |
|-----------|------|
| BY        | DATE |
| J. Folsom |      |
| D. Damren |      |

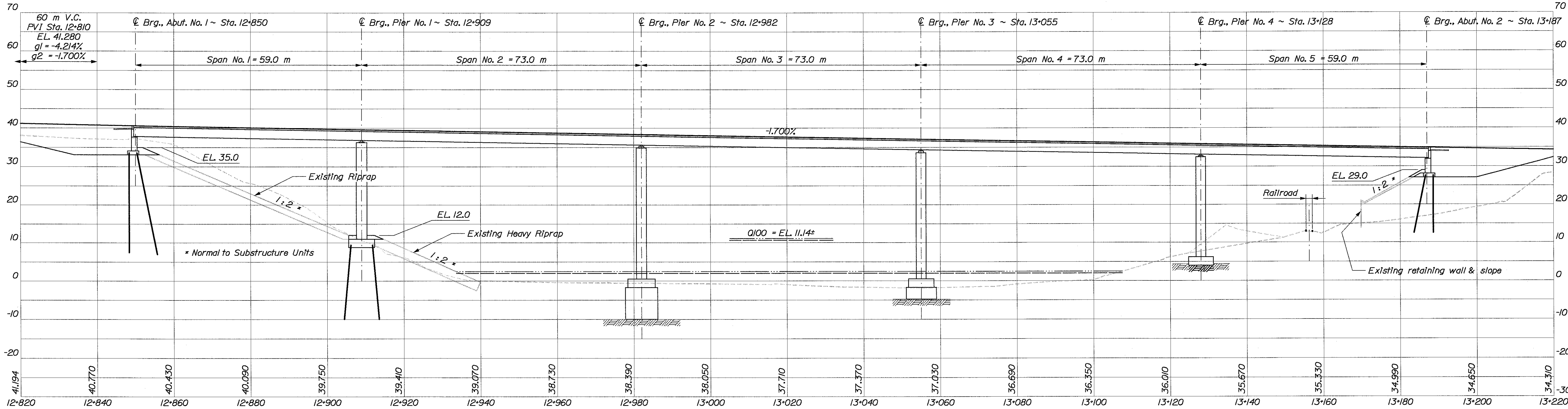
|       |
|-------|
| PLANS |
|-------|

METRIC

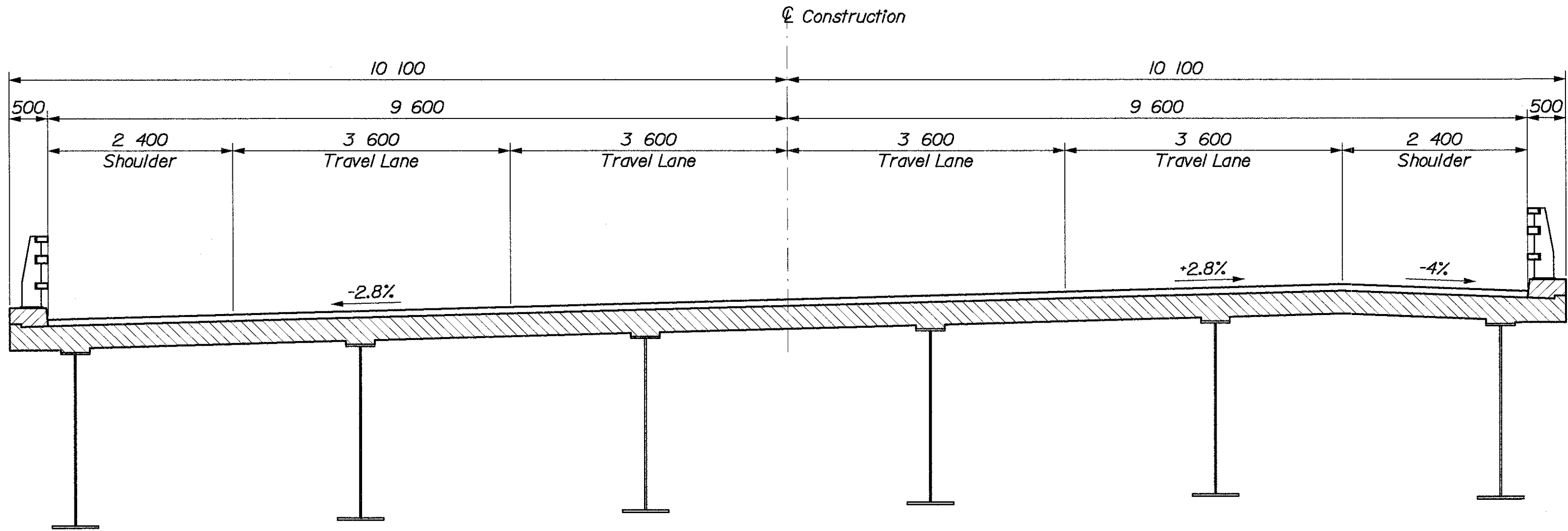
1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| FHWA REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|---------------|-------|----------------|-----------|--------------|
| 1             | MAINE | NH-0556(220)E  | 4         | 103          |

PIN 000556.22



PROFILE



TRANSVERSE SECTION

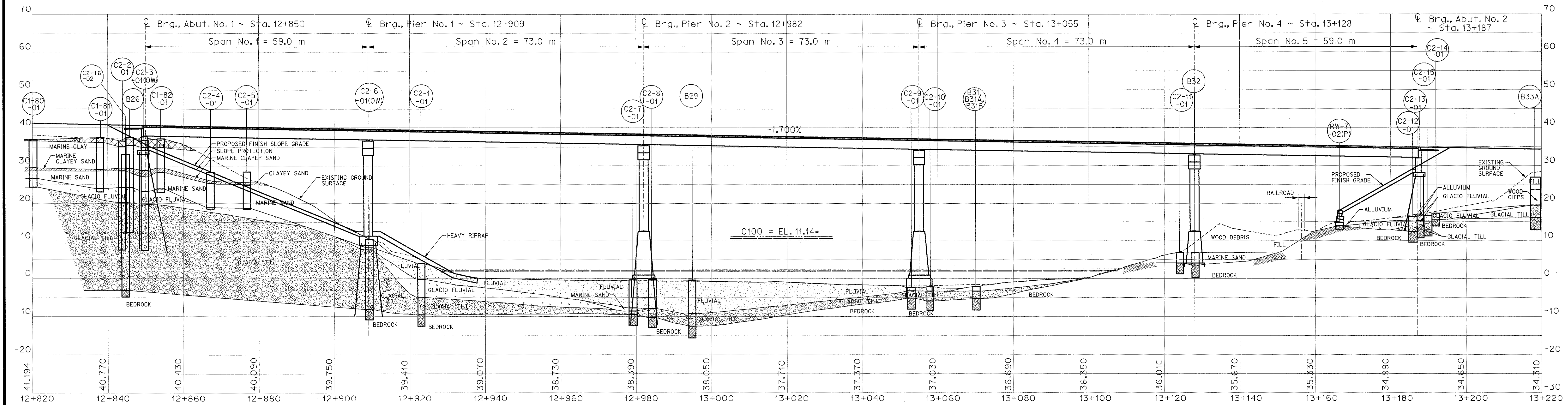
Bridge No. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
BRIDGE PROFILE

SHEET 4 OF 103  
AUGUSTA, MAINE  
December 2002

Filename: i:\jobs\34612\Bridge\CADD\Design\FoundationSurvey&Boring\planprofile~rev.dgn

METRIC

- PROFILE KEY/GEOLOGIC  
UNITS
- MARINE SAND- MARINE SANDS AND SILTY SANDS, PRIMARILY NON-PLASTIC
- MARINE CLAY- PRIMARILY SILT AND CLAY (AND VARYING QUANTITIES OF SAND AT SOME LOCATIONS) HAVING SOME DEGREE OF PLASTICITY
- MARINE CLAYEY SAND- MIXTURE OF SAND AND CLAY, OFTEN LAYERED
- ALLUVIUM-SURFICIAL SILTY SAND DEPOSIT
- FLUVIAL DEPOSITS- PRIMARILY SAND AND GRAVEL (TRANSPORTED BY A RIVER)
- GLACIOFLUVIAL DEPOSITS- PRIMARILY SAND AND GRAVEL (TYPICALLY DERIVED FROM GLACIAL MELTWATER, MAY ALSO INCLUDE OTHER GLACIAL DEPOSITIONAL FEATURES)
- GLACIAL TILL DEPOSITS- HETEROGENEOUS MIX OF SAND, SILT, AND GRAVEL
- FILL-REWORKED NATURAL SOILS, MISCELLANEOUS DEBRIS FILL, WOOD CHIPS, WOOD DEBRIS, ETC.
- BEDROCK SURFACE- INTERPOLATED OR EXTRAPOLATED FROM BEDROCK CORES, REFUSALS OR JUDGEMENT

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

---

# FOUNDATION SURVEY

AUGUSTA  
THIRD BRIDGE



10 0 10 20

SCALE  
(in meters)

AUGUSTA, MAINE

SHEET 1 OF 1

AUGUSTA 29 SPUR ROUTE D

Date: \$DATE\$

Filename: \$FILE\$.

|                         |      |      |       |
|-------------------------|------|------|-------|
| PROJECT DESIGN ENGINEER | BKL  | DATE | 08/02 |
| CHECKED                 | ML\$ | DATE | 08/02 |
| DESIGN-DETAILED         | UWW  |      |       |
| REVISIONS               |      |      |       |
| FIELD CHANGES           |      |      |       |

PLANS

|                                                                                                                                                                                                                |                                 |                                    |                      |
|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------|------------------------------------|----------------------|
| Haley & Aldrich, Inc.                                                                                                                                                                                          |                                 | Project: Kennebec River Crossing   | Boring No.: C1-80-01 |
| Soil/Bore Exploration Log                                                                                                                                                                                      |                                 | Location: Augusta, Maine           | PIN: 556.21          |
| METRIC UNITS                                                                                                                                                                                                   |                                 | File No. 80785-001                 |                      |
| Drillers: Mark Manning                                                                                                                                                                                         | Elevation (m): 37.02            | Auger ID/OD: 5.35 cm / 14.51 cm    |                      |
| Operator: Maine Test Boring                                                                                                                                                                                    | Datum: NVD                      | Sampler: 3.48 cm Split Spoon       |                      |
| Logged By: Tim Erickson                                                                                                                                                                                        | Rig Type: Mobile B33 Bombardier | Hammer Wt./Fall: 140 lbs / 76.2 cm |                      |
| Date Start/Finish: 8/22/01 / 8/22/01                                                                                                                                                                           | Drilling Method: HSA Spin       | Core Barrel: ---                   |                      |
| Boring Location: Station 12 + 830.0 L                                                                                                                                                                          | Coating ID/OD: ---              | Water Level: Dry                   |                      |
| Definitions:<br>S = Split Spoon Sample<br>SP = Standardized Split Spoon Sample attempt<br>U = Thin Soil Tube Sample<br>H = Rock Core Sample<br>V = In Situ Vane Shear Test<br>SS = Solid Stem Auger            |                                 |                                    |                      |
| Abbreviations:<br>WC = water content, percent<br>LL = Liquid Limit<br>PL = Plasticity Limit<br>PI = Plasticity Index<br>L = Lab Test Shear Strength (kPa)<br>SS = weight of 64 kg hammer<br>SS = weight of rod |                                 |                                    |                      |
| Sample Information                                                                                                                                                                                             |                                 |                                    |                      |
| Depth (m)                                                                                                                                                                                                      | Sample No.                      | Penetration (cm)                   | Sample Depth (m)     |
| 0                                                                                                                                                                                                              | D1                              | 61.0/43.2 0.00 - 0.61              | 4/7/4/5              |
| 1                                                                                                                                                                                                              |                                 |                                    |                      |
| 2                                                                                                                                                                                                              |                                 |                                    |                      |
| 3                                                                                                                                                                                                              |                                 |                                    |                      |
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| 15                                                                                                                                                                                                             |                                 |                                    |                      |
| Visual Description and Remarks                                                                                                                                                                                 |                                 |                                    |                      |
| Light-brown, dry, medium dense, gravelly SAND, poorly-graded (FILL-SP)                                                                                                                                         |                                 |                                    |                      |
| Yellow-brown, moist, stiff, CLAY, trace silt, moderately plastic, mottled (MARINE DEPOSIT-CL)                                                                                                                  |                                 |                                    |                      |
| Gray, moist, medium stiff CLAY, plastic (MARINE DEPOSIT-CL)                                                                                                                                                    |                                 |                                    |                      |
| Notes: Shear vane test taken at 3.05 m                                                                                                                                                                         |                                 |                                    |                      |
| Gray, moist, medium stiff CLAY, plastic (MARINE DEPOSIT-CL)                                                                                                                                                    |                                 |                                    |                      |
| Brown-gray, moist, stiff, sandy CLAY, trace silt, moderately plastic (MARINE DEPOSIT-CL)                                                                                                                       |                                 |                                    |                      |
| Gray-brown, moist, very stiff, sandy SILT, trace clay, stratified (MARINE DEPOSIT-ML)                                                                                                                          |                                 |                                    |                      |
| Remarks:<br>Shear vane: 55 mm x 110 mm field vane                                                                                                                                                              |                                 |                                    |                      |
| Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                                          |                                 |                                    |                      |
| * Water level readings have been made at time and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.                   |                                 |                                    |                      |
| Page 1 of 2<br>Boring No.: C1-80-01                                                                                                                                                                            |                                 |                                    |                      |

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|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------|------------------------------------|----------------------|
| Haley & Aldrich, Inc.                                                                                                                                                                                          |                                 | Project: Kennebec River Crossing   | Boring No.: C1-80-01 |
| Soil/Bore Exploration Log                                                                                                                                                                                      |                                 | Location: Augusta, Maine           | PIN: 556.21          |
| METRIC UNITS                                                                                                                                                                                                   |                                 | File No. 80785-001                 |                      |
| Drillers: Mark Manning                                                                                                                                                                                         | Elevation (m): 37.02            | Auger ID/OD: 5.35 cm / 14.51 cm    |                      |
| Operator: Maine Test Boring                                                                                                                                                                                    | Datum: NVD                      | Sampler: 3.48 cm Split Spoon       |                      |
| Logged By: Tim Erickson                                                                                                                                                                                        | Rig Type: Mobile B33 Bombardier | Hammer Wt./Fall: 140 lbs / 76.2 cm |                      |
| Date Start/Finish: 8/22/01 / 8/22/01                                                                                                                                                                           | Drilling Method: HSA Spin       | Core Barrel: ---                   |                      |
| Boring Location: Station 12 + 830.0 L                                                                                                                                                                          | Coating ID/OD: ---              | Water Level: Dry                   |                      |
| Definitions:<br>S = Split Spoon Sample<br>SP = Standardized Split Spoon Sample attempt<br>U = Thin Soil Tube Sample<br>H = Rock Core Sample<br>V = In Situ Vane Shear Test<br>SS = Solid Stem Auger            |                                 |                                    |                      |
| Abbreviations:<br>WC = water content, percent<br>LL = Liquid Limit<br>PL = Plasticity Limit<br>PI = Plasticity Index<br>L = Lab Test Shear Strength (kPa)<br>SS = weight of 64 kg hammer<br>SS = weight of rod |                                 |                                    |                      |
| Sample Information                                                                                                                                                                                             |                                 |                                    |                      |
| Depth (m)                                                                                                                                                                                                      | Sample No.                      | Penetration (cm)                   | Sample Depth (m)     |
| 8                                                                                                                                                                                                              |                                 |                                    |                      |
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| Visual Description and Remarks                                                                                                                                                                                 |                                 |                                    |                      |
| Light-brown, damp, dense, SAND, little silt, poorly-graded, stratified (MARINE DEPOSIT-ML)                                                                                                                     |                                 |                                    |                      |
| Light-brown, moist, very dense, gravelly SAND, trace silt, poorly-graded (GLACIAL STREAM DEPOSIT-ML)                                                                                                           |                                 |                                    |                      |
| Brown, dry to damp, very dense, gravelly SAND, trace silt, well-graded (GLACIAL STREAM DEPOSIT-SP)                                                                                                             |                                 |                                    |                      |
| Bottom of Exploration at 12.34 m below ground surface.                                                                                                                                                         |                                 |                                    |                      |
| Remarks:<br>Shear vane: 55 mm x 110 mm field vane                                                                                                                                                              |                                 |                                    |                      |
| Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                                          |                                 |                                    |                      |
| * Water level readings have been made at time and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.                   |                                 |                                    |                      |
| Page 2 of 2<br>Boring No.: C1-80-01                                                                                                                                                                            |                                 |                                    |                      |

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| Haley & Aldrich, Inc.                                                                                                                                                                                          |                                 | Project: Kennebec River Crossing   | Boring No.: C1-81-01 |
| Soil/Bore Exploration Log                                                                                                                                                                                      |                                 | Location: Augusta, Maine           | PIN: 556.21          |
| METRIC UNITS                                                                                                                                                                                                   |                                 | File No. 80785-001                 |                      |
| Drillers: Darrel McKean                                                                                                                                                                                        | Elevation (m): 37.20            | Auger ID/OD: 5.35 cm / 14.51 cm    |                      |
| Operator: Maine Test Boring                                                                                                                                                                                    | Datum: NVD                      | Sampler: 3.48 cm Split Spoon       |                      |
| Logged By: Tim Erickson                                                                                                                                                                                        | Rig Type: Mobile B33 Bombardier | Hammer Wt./Fall: 140 lbs / 76.2 cm |                      |
| Date Start/Finish: 8/22/01                                                                                                                                                                                     | Drilling Method: HSA Spin       | Core Barrel: ---                   |                      |
| Boring Location: Station 12 + 838.10 S L                                                                                                                                                                       | Coating ID/OD: ---              | Water Level: Dry                   |                      |
| Definitions:<br>S = Split Spoon Sample<br>SP = Standardized Split Spoon Sample attempt<br>U = Thin Soil Tube Sample<br>H = Rock Core Sample<br>V = In Situ Vane Shear Test<br>SS = Solid Stem Auger            |                                 |                                    |                      |
| Abbreviations:<br>WC = water content, percent<br>LL = Liquid Limit<br>PL = Plasticity Limit<br>PI = Plasticity Index<br>L = Lab Test Shear Strength (kPa)<br>SS = weight of 64 kg hammer<br>SS = weight of rod |                                 |                                    |                      |
| Sample Information                                                                                                                                                                                             |                                 |                                    |                      |
| Depth (m)                                                                                                                                                                                                      | Sample No.                      | Penetration (cm)                   | Sample Depth (m)     |
| 0                                                                                                                                                                                                              | D1                              | 61.0/43.2 0.00 - 0.61              | 3/6/7/5              |
| 1                                                                                                                                                                                                              |                                 |                                    |                      |
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| 15                                                                                                                                                                                                             |                                 |                                    |                      |
| Visual Description and Remarks                                                                                                                                                                                 |                                 |                                    |                      |
| Light-brown, dry, medium dense, SAND, some gravel, trace silt, well-graded (FILL-SP)                                                                                                                           |                                 |                                    |                      |
| Gray, moist, medium stiff, CLAY, trace silt, uniform, moderately plastic (MARINE DEPOSIT-CL)                                                                                                                   |                                 |                                    |                      |
| Notes: Shear vane attempt at 3.05 m no penetration                                                                                                                                                             |                                 |                                    |                      |
| Notes: From 3.05-3.11 m is a fine sand layer                                                                                                                                                                   |                                 |                                    |                      |
| Gray, moist, medium stiff, CLAY, trace silt, fine sand, moderately plastic (MARINE DEPOSIT-CL)                                                                                                                 |                                 |                                    |                      |
| Notes: Shear vane taken at 4.88 m                                                                                                                                                                              |                                 |                                    |                      |
| Gray, moist, medium stiff, CLAY, trace silt, fine sand, high plasticity (MARINE DEPOSIT-CL)                                                                                                                    |                                 |                                    |                      |
| Gray, moist, medium stiff CLAY, trace silt, fine sand, moderately plastic (MARINE DEPOSIT-CL)                                                                                                                  |                                 |                                    |                      |
| Bottom of Exploration at 14.33 m below ground surface.                                                                                                                                                         |                                 |                                    |                      |
| Remarks:<br>Shear Vane: 55 mm x 110 mm field vane                                                                                                                                                              |                                 |                                    |                      |
| Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                                          |                                 |                                    |                      |
| * Water level readings have been made at time and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.                   |                                 |                                    |                      |
| Page 1 of 2<br>Boring No.: C1-81-01                                                                                                                                                                            |                                 |                                    |                      |

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|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------|------------------------------------|----------------------|
| Haley & Aldrich, Inc.                                                                                                                                                                                          |                                 | Project: Kennebec River Crossing   | Boring No.: C1-81-01 |
| Soil/Bore Exploration Log                                                                                                                                                                                      |                                 | Location: Augusta, Maine           | PIN: 556.21          |
| METRIC UNITS                                                                                                                                                                                                   |                                 | File No. 80785-001                 |                      |
| Drillers: Darrel McKean                                                                                                                                                                                        | Elevation (m): 37.20            | Auger ID/OD: 5.35 cm / 14.51 cm    |                      |
| Operator: Maine Test Boring                                                                                                                                                                                    | Datum: NVD                      | Sampler: 3.48 cm Split Spoon       |                      |
| Logged By: Tim Erickson                                                                                                                                                                                        | Rig Type: Mobile B33 Bombardier | Hammer Wt./Fall: 140 lbs / 76.2 cm |                      |
| Date Start/Finish: 8/22/01                                                                                                                                                                                     | Drilling Method: HSA Spin       | Core Barrel: ---                   |                      |
| Boring Location: Station 12 + 838.10 S L                                                                                                                                                                       | Coating ID/OD: ---              | Water Level: Dry                   |                      |
| Definitions:<br>S = Split Spoon Sample<br>SP = Standardized Split Spoon Sample attempt<br>U = Thin Soil Tube Sample<br>H = Rock Core Sample<br>V = In Situ Vane Shear Test<br>SS = Solid Stem Auger            |                                 |                                    |                      |
| Abbreviations:<br>WC = water content, percent<br>LL = Liquid Limit<br>PL = Plasticity Limit<br>PI = Plasticity Index<br>L = Lab Test Shear Strength (kPa)<br>SS = weight of 64 kg hammer<br>SS = weight of rod |                                 |                                    |                      |
| Sample Information                                                                                                                                                                                             |                                 |                                    |                      |
| Depth (m)                                                                                                                                                                                                      | Sample No.                      | Penetration (cm)                   | Sample Depth (m)     |
| 8                                                                                                                                                                                                              |                                 |                                    |                      |
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| Visual Description and Remarks                                                                                                                                                                                 |                                 |                                    |                      |
| Gray-brown, damp, medium dense, clayey SAND, little silt (MARINE DEPOSIT-SC)                                                                                                                                   |                                 |                                    |                      |
| Gray-brown, damp, medium dense, SAND, little clay, trace silt, poorly-graded, slightly stratified (MARINE DEPOSIT-SP)                                                                                          |                                 |                                    |                      |
| Gray-brown, damp, dense, SAND, little clay, trace silt, poorly-graded, stratified (MARINE DEPOSIT-SP)                                                                                                          |                                 |                                    |                      |
| Gray-brown, damp, medium dense, SAND, little clay, trace silt, poorly-graded, slightly stratified (MARINE DEPOSIT-SP)                                                                                          |                                 |                                    |                      |
| Brown, damp, very dense, gravelly SAND, trace silt, well-graded (GLACIAL STREAM DEPOSIT-SP)                                                                                                                    |                                 |                                    |                      |
| Bottom of Exploration at 14.33 m below ground surface.                                                                                                                                                         |                                 |                                    |                      |
| Remarks:<br>Shear Vane: 55 mm x 110 mm field vane                                                                                                                                                              |                                 |                                    |                      |
| Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                                          |                                 |                                    |                      |
| * Water level readings have been made at time and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.                   |                                 |                                    |                      |
| Page 2 of 2<br>Boring No.: C1-81-01                                                                                                                                                                            |                                 |                                    |                      |

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| Haley & Aldrich, Inc.                                                                                                                                                                                          |                                 | Project: Kennebec River Crossing   | Boring No.: C1-82-01 |
| Soil/Bore Exploration Log                                                                                                                                                                                      |                                 | Location: Augusta, Maine           | PIN: 556.21          |
| METRIC UNITS                                                                                                                                                                                                   |                                 | File No. 80785-001                 |                      |
| Drillers: Darrel McKean                                                                                                                                                                                        | Elevation (m): 37.20            | Auger ID/OD: 5.35 cm / 14.51 cm    |                      |
| Operator: Maine Test Boring                                                                                                                                                                                    | Datum: NVD                      | Sampler: 3.48 cm Split Spoon       |                      |
| Logged By: Tim Erickson                                                                                                                                                                                        | Rig Type: Mobile B33 Bombardier | Hammer Wt./Fall: 140 lbs / 76.2 cm |                      |
| Date Start/Finish: 8/22/01 / 8/22/01                                                                                                                                                                           | Drilling Method: HSA Spin       | Core Barrel: ---                   |                      |
| Boring Location: Station 12 + 854.10 R                                                                                                                                                                         | Coating ID/OD: ---              | Water Level: Dry                   |                      |
| Definitions:<br>S = Split Spoon Sample<br>SP = Standardized Split Spoon Sample attempt<br>U = Thin Soil Tube Sample<br>H = Rock Core Sample<br>V = In Situ Vane Shear Test<br>SS = Solid Stem Auger            |                                 |                                    |                      |
| Abbreviations:<br>WC = water content, percent<br>LL = Liquid Limit<br>PL = Plasticity Limit<br>PI = Plasticity Index<br>L = Lab Test Shear Strength (kPa)<br>SS = weight of 64 kg hammer<br>SS = weight of rod |                                 |                                    |                      |
| Sample Information                                                                                                                                                                                             |                                 |                                    |                      |
| Depth (m)                                                                                                                                                                                                      | Sample No.                      | Penetration (cm)                   | Sample Depth (m)     |
| 0                                                                                                                                                                                                              | D1                              | 61.0/43.2 0.00 - 0.61              | 12/26/23/20          |
| 1                                                                                                                                                                                                              |                                 |                                    |                      |
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| 15                                                                                                                                                                                                             |                                 |                                    |                      |
| Visual Description and Remarks                                                                                                                                                                                 |                                 |                                    |                      |
| TOPSOIL                                                                                                                                                                                                        |                                 |                                    |                      |
| Light-brown, dry, hard, SILT, little fine sand, 0.03 plastic (MARINE DEPOSIT-ML)                                                                                                                               |                                 |                                    |                      |
| Light-brown, dry, dense, SAND, trace gravel, silt, well-graded (MARINE DEPOSIT-SP)                                                                                                                             |                                 |                                    |                      |
| Brown, damp, medium dense, SAND, little silt, trace gravel, well-graded (MARINE DEPOSIT-SP)                                                                                                                    |                                 |                                    |                      |
| Notes: Considerable amount of silt found in spoon tip and indicates a strata change                                                                                                                            |                                 |                                    |                      |
| Brown-gray, damp, hard CLAY, little silt, moderately plastic, mottled (MARINE DEPOSIT-CL)                                                                                                                      |                                 |                                    |                      |
| Olive, damp, stiff, CLAY, trace silt, moderately plastic, mottled (MARINE DEPOSIT-CL)                                                                                                                          |                                 |                                    |                      |
| Gray, moist, stiff CLAY, trace silt, moderately plastic (MARINE DEPOSIT-CL)                                                                                                                                    |                                 |                                    |                      |
| Notes: Attempted vane shear test at 1.52 m, unable to turn                                                                                                                                                     |                                 |                                    |                      |
| Gray-brown, damp, very stiff, sandy CLAY, trace silt, moderately plastic, sand layers are 0.6-1.0 m thick (MARINE DEPOSIT-CL)                                                                                  |                                 |                                    |                      |
| Remarks:<br>Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                              |                                 |                                    |                      |
| * Water level readings have been made at time and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.                   |                                 |                                    |                      |
| Page 1 of 2<br>Boring No.: C1-82-01                                                                                                                                                                            |                                 |                                    |                      |

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| Haley & Aldrich, Inc.                                                                                                                                                                                          |                                 | Project: Kennebec River Crossing   | Boring No.: C1-82-01 |
| Soil/Bore Exploration Log                                                                                                                                                                                      |                                 | Location: Augusta, Maine           | PIN: 556.21          |
| METRIC UNITS                                                                                                                                                                                                   |                                 | File No. 80785-001                 |                      |
| Drillers: Darrel McKean                                                                                                                                                                                        | Elevation (m): 37.20            | Auger ID/OD: 5.35 cm / 14.51 cm    |                      |
| Operator: Maine Test Boring                                                                                                                                                                                    | Datum: NVD                      | Sampler: 3.48 cm Split Spoon       |                      |
| Logged By: Tim Erickson                                                                                                                                                                                        | Rig Type: Mobile B33 Bombardier | Hammer Wt./Fall: 140 lbs / 76.2 cm |                      |
| Date Start/Finish: 8/22/01 / 8/22/01                                                                                                                                                                           | Drilling Method: HSA Spin       | Core Barrel: ---                   |                      |
| Boring Location: Station 12 + 854.10 R                                                                                                                                                                         | Coating ID/OD: ---              | Water Level: Dry                   |                      |
| Definitions:<br>S = Split Spoon Sample<br>SP = Standardized Split Spoon Sample attempt<br>U = Thin Soil Tube Sample<br>H = Rock Core Sample<br>V = In Situ Vane Shear Test<br>SS = Solid Stem Auger            |                                 |                                    |                      |
| Abbreviations:<br>WC = water content, percent<br>LL = Liquid Limit<br>PL = Plasticity Limit<br>PI = Plasticity Index<br>L = Lab Test Shear Strength (kPa)<br>SS = weight of 64 kg hammer<br>SS = weight of rod |                                 |                                    |                      |
| Sample Information                                                                                                                                                                                             |                                 |                                    |                      |
| Depth (m)                                                                                                                                                                                                      | Sample No.                      | Penetration (cm)                   | Sample Depth (m)     |
| 9                                                                                                                                                                                                              |                                 |                                    |                      |
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| 14                                                                                                                                                                                                             |                                 |                                    |                      |
| 15                                                                                                                                                                                                             |                                 |                                    |                      |
| Visual Description and Remarks                                                                                                                                                                                 |                                 |                                    |                      |
| Light-brown, moist, dense, SAND, little clay, trace silt, poorly-graded (MARINE DEPOSIT-SP)                                                                                                                    |                                 |                                    |                      |
| Light-brown, moist, dense, SAND, trace clay, silt, poorly-graded (MARINE DEPOSIT-SP)                                                                                                                           |                                 |                                    |                      |
| Light-brown, moist, very dense, SAND, trace silt, well-graded (MARINE DEPOSIT-SP)                                                                                                                              |                                 |                                    |                      |
| Light-brown, damp, very dense, SAND, little silt, trace silt, well-graded (GLACIAL STREAM DEPOSIT-SP)                                                                                                          |                                 |                                    |                      |
| Bottom of Exploration at 14.33 m below ground surface.                                                                                                                                                         |                                 |                                    |                      |
| Remarks:<br>Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                              |                                 |                                    |                      |
| * Water level readings have been made at time and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.                   |                                 |                                    |                      |
| Page 2 of 2<br>Boring No.: C1-82-01                                                                                                                                                                            |                                 |                                    |                      |

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

BORING LOGS  
AUGUSTA

State Route 3 over Kennebec River

Date: \$DATE\$

Filename: \$FILES\$

|                         |     |      |       |
|-------------------------|-----|------|-------|
| PROJECT DESIGN ENGINEER | BAL | DATE | 08/02 |
| DESIGN-DETAILED         | BKL | M/S  | 08/02 |
| CHECKED                 | JWW |      |       |
| REVISIONS               |     |      |       |
| FIELD CHANGES           |     |      |       |

PLANS

|                                                                                                                                                                                                |                         |                                                          |                  |                                         |                                |
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| Haley & Aldrich, Inc.                                                                                                                                                                          |                         | Project: Kennebec River Crossing                         |                  | Boring No.: C2-1-01                     |                                |
| Soil/Litho/Exploration Log                                                                                                                                                                     |                         | Location: Augusta, Maine                                 |                  | File No. 80785-004                      |                                |
| METRIC UNITS                                                                                                                                                                                   |                         | P/N: 556.21                                              |                  |                                         |                                |
| Driller:                                                                                                                                                                                       | G. Rudnik               | Elevation (m):                                           | 4.02             | Auger ID/OD:                            | — / 121.92 mm                  |
| Operator:                                                                                                                                                                                      | Maine Test Borings      | Datum:                                                   | NVD              | Sampler:                                | 3.49 on Split Spoon            |
| Logged By:                                                                                                                                                                                     | W. Rubik                | Rig Type:                                                | CME 45 Skid      | Hammer Wt./Fall:                        | 623 N / 76.2 cm                |
| Date Start/Finish:                                                                                                                                                                             | 8/24/01 / 8/25/01       | Drilling Method:                                         | HSA Spn          | Core Barrel:                            | —                              |
| Boring Location:                                                                                                                                                                               | Station 12 + 823, 0.5 R | Coating ID/OD:                                           | —                | Water Level:                            | 3.17 m                         |
| Soil/Notes                                                                                                                                                                                     |                         | Soil/Notes                                               |                  | Soil/Notes                              |                                |
| 1 = Standard Pen. Sample                                                                                                                                                                       |                         | 1 <sub>u</sub> = Unified Field Test Shear Strength (psi) |                  | 1 <sub>u</sub> = water content, percent |                                |
| 2 = Standard Pen. Sample                                                                                                                                                                       |                         | 2 <sub>u</sub> = Pocket Torque Shear Strength (psi)      |                  | L = Liquid Limit                        |                                |
| 3 = Thin Wall Tube Sample                                                                                                                                                                      |                         | 3 <sub>u</sub> = Standard Compression Strength (psi)     |                  | P <sub>u</sub> = Plasticity Index       |                                |
| 4 = Rock Core Sample                                                                                                                                                                           |                         | 4 <sub>u</sub> = Lab Value Shear Strength (psi)          |                  | PI = Plasticity Index                   |                                |
| 5 = In-situ Test Sample                                                                                                                                                                        |                         | 5 <sub>u</sub> = Weight of Soil Sample                   |                  | S = Soil Test Results                   |                                |
| 6 = Split Tension Test                                                                                                                                                                         |                         | 6 <sub>u</sub> = Weight of Core                          |                  | C = Compression Test                    |                                |
| Sample Information                                                                                                                                                                             |                         |                                                          |                  |                                         |                                |
| Depth (m)                                                                                                                                                                                      | Sample No.              | Penetration (cm)                                         | Sample Depth (m) | Blow Count (150 mm)                     | Visual Description and Remarks |
| 0                                                                                                                                                                                              | D1                      | 61.0/25.6                                                | 0.00 - 0.61      | 1/25/8                                  | PROBE                          |
| 0.61                                                                                                                                                                                           |                         |                                                          |                  |                                         | AUGER                          |
| 1                                                                                                                                                                                              |                         |                                                          |                  |                                         |                                |
| 1.22                                                                                                                                                                                           | D2                      | 61.0/45.7                                                | 0.62 - 2.13      | 17/16/10/10                             | 26 28                          |
| 2.13                                                                                                                                                                                           |                         |                                                          |                  |                                         |                                |
| 2.74                                                                                                                                                                                           | D3                      | 61.0/26.1                                                | 2.14 - 3.66      | 23/13/11/10                             | 24 17                          |
| 3.66                                                                                                                                                                                           |                         |                                                          |                  |                                         |                                |
| 4.27                                                                                                                                                                                           | D4                      | 61.0/25.4                                                | 3.67 - 5.18      | 20/12/10/10                             | 22 21                          |
| 5.18                                                                                                                                                                                           |                         |                                                          |                  |                                         |                                |
| 5.79                                                                                                                                                                                           | D5                      | 61.0/20.5                                                | 5.19 - 6.71      | 17/14/13/14                             | 27 24                          |
| 6.71                                                                                                                                                                                           |                         |                                                          |                  |                                         |                                |
| 7.32                                                                                                                                                                                           | D6                      | 61.0/17.6                                                | 6.72 - 8.23      | 26/27/28/30                             | 35 21                          |
| 8.23                                                                                                                                                                                           |                         |                                                          |                  |                                         |                                |
| Remarks:                                                                                                                                                                                       |                         |                                                          |                  |                                         |                                |
| Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                          |                         |                                                          |                  |                                         |                                |
| * Soiler level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                         |                                                          |                  |                                         |                                |
| Page 1 of 3                                                                                                                                                                                    |                         |                                                          |                  |                                         |                                |
| Boring No.: C2-1-01                                                                                                                                                                            |                         |                                                          |                  |                                         |                                |

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| Haley & Aldrich, Inc.                     |                         | Project: Kennebec River Crossing                           |                  | Boring No.: C2-1-01                     |                                |
| Soil/Book Exploration Log                 |                         | Location: Augusta, Maine                                   |                  | File No. 80785-004                      |                                |
| METRIC UNITS                              |                         | P/N: 556.21                                                |                  |                                         |                                |
| Driller:                                  | G. Rudnik               | Elevation (m):                                             | 4.02             | Auger ID/OD:                            | — / 121.92 on                  |
| Operator:                                 | Maine Test Borings      | Datum:                                                     | NVD              | Sampler:                                | 3.49 on Split Spoon            |
| Logged By:                                | W. Rubik                | Rig Type:                                                  | CME 45 Skid      | Hammer Wt./Fall:                        | 623 N / 76.2 cm                |
| Date Start/Finish:                        | 8/24/01 / 8/25/01       | Drilling Method:                                           | HSA Spn          | Core Barrel:                            | —                              |
| Boring Location:                          | Station 12 + 823, 0.5 R | Coating ID/OD:                                             | —                | Water Level:                            | 3.17 m                         |
| Soil Information                          |                         | Soil Conditions                                            |                  | Soil Conditions                         |                                |
| P = Split Spoon Sample                    |                         | S <sub>u</sub> = In Situ Field Vane Shear Strength (kPa)   |                  | S <sub>u</sub> = water content, percent |                                |
| AD = Shearless Split Spoon Sample attempt |                         | T <sub>u</sub> = Pocket Torque Shear Strength (kPa)        |                  | L <sub>c</sub> = Liquid Limit           |                                |
| R = Thin Wall Tube Sample                 |                         | A <sub>g</sub> = Unconsolidated Compressive Strength (kPa) |                  | P <sub>t</sub> = Plasticity Index       |                                |
| B = Bulk Core Sample                      |                         | L <sub>u</sub> = Lab Vane Shear Strength (kPa)             |                  | S = Swell Free Settlement               |                                |
| I = In Situ Vane Shear Test               |                         | W <sub>u</sub> = Weight of 1 kg Water                      |                  | C = Consolidation Test                  |                                |
| S <sub>u</sub> = Split Spoon Sample       |                         |                                                            |                  |                                         |                                |
| Visual Description and Remarks            |                         |                                                            |                  |                                         |                                |
| Depth (m)                                 | Sample No.              | Pen/Ref (cm)                                               | Sample Depth (m) | Blow (150 mm) (blows/30 cm)             | Visual Description and Remarks |
| 8                                         | D7                      | 61.0/25.4                                                  | 8.24 - 9.80      | 41/25/24/28                             | 49 36                          |
|                                           |                         |                                                            |                  |                                         | 8.89                           |
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| Haley & Aldrich, Inc.                                                                                                                                                                          |                          | Project: Kennebec River Crossing                                                                                                                                                                                                                                               |                  | Boring No.: C2-1-01                                                                                                                                                                        |                                                        |                  |  |
| Soil/Book Exploration Log                                                                                                                                                                      |                          | Location: Augusta, Maine                                                                                                                                                                                                                                                       |                  | File No. 80785-004                                                                                                                                                                         |                                                        |                  |  |
| METRIC UNITS                                                                                                                                                                                   |                          | P/N: 556.21                                                                                                                                                                                                                                                                    |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Driller: G. Rudnik                                                                                                                                                                             | Elevation (m): 4.02      | Auger ID/OD: — / 121.92 cm                                                                                                                                                                                                                                                     |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Operator: Maine Test Borings                                                                                                                                                                   | Datum: NVD               | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                                                                   |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Logged By: W. Rubik                                                                                                                                                                            | Rig Type: CME 45 Skid    | Hammer Wt./Fall: 623 N / 76.2 cm                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Date Start/Finish: 8/24/01 / 8/25/01                                                                                                                                                           | Drilling Method: HSA Spn | Core Barrel: —                                                                                                                                                                                                                                                                 |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Boring Location: Station 12 + 823, 0.5 R                                                                                                                                                       | Coating ID/OD: —         | Water Level: 3.17 m                                                                                                                                                                                                                                                            |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Soil Information                                                                                                                                                                               |                          | Soil Information                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                            |                                                        | Soil Information |  |
| S <sub>u</sub> = Split Spoon Sample<br>SD = Unconsolidated Split Spoon Sample<br>C = Thin Wall Tube Sample<br>H = Hand Core Sample<br>T = Direct Vane Shear Test<br>SS = Split Shear Auger     |                          | S <sub>u</sub> = Split Field Vane Shear Strength (kPa)<br>T <sub>v</sub> = Pocket Torque Shear Strength (kPa)<br>Q <sub>u</sub> = Unconfined Compressive Strength (Pa)<br>K <sub>sp</sub> = Lab Vane Shear Strength (kPa)<br>WS = Weight of Soil Sample<br>WS = Weight of Soil |                  | S <sub>u</sub> = Split Spoon Sample<br>SD = Unconsolidated Split Spoon Sample<br>C = Thin Wall Tube Sample<br>H = Hand Core Sample<br>T = Direct Vane Shear Test<br>SS = Split Shear Auger |                                                        |                  |  |
| Soil Description and Remarks                                                                                                                                                                   |                          | Soil Description and Remarks                                                                                                                                                                                                                                                   |                  | Soil Description and Remarks                                                                                                                                                               |                                                        |                  |  |
| Depth (m)                                                                                                                                                                                      | Sample No.               | Penetration (cm)                                                                                                                                                                                                                                                               | Sample Depth (m) | Blow (150 mm) (blows/30 cm)                                                                                                                                                                | Visual Description and Remarks                         |                  |  |
| 16                                                                                                                                                                                             |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            | SOILIST-<br>Recovery 85%                               |                  |  |
| 17                                                                                                                                                                                             |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            | Unable to dislodge and retrieve all of core rod.       |                  |  |
| 18                                                                                                                                                                                             |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            | Bottom of Exploration at 16.86 m below ground surface. |                  |  |
| 19                                                                                                                                                                                             |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            |                                                        |                  |  |
| 20                                                                                                                                                                                             |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            |                                                        |                  |  |
| 21                                                                                                                                                                                             |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            |                                                        |                  |  |
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| 23                                                                                                                                                                                             |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Remarks:                                                                                                                                                                                       |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                          |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            |                                                        |                  |  |
| * Soiler level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                          |                                                                                                                                                                                                                                                                                |                  |                                                                                                                                                                                            |                                                        |                  |  |
| Page 3 of 3                                                                                                                                                                                    |                          |                                                                                                                                                                                                                                                                                |                  | Boring No.: C2-1-01                                                                                                                                                                        |                                                        |                  |  |

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| Haley & Aldrich, Inc.                   |  |                                                    |  | Project: Kennebec River Crossing        |  | Boring No.: C2-2-01 |  |
| Soil/Book Exploration Log               |  |                                                    |  | Location: Augusta, Maine                |  | File No. 80785-004  |  |
| METRIC UNITS                            |  |                                                    |  | P/N: 556.21                             |  |                     |  |
| Driller: G. Rudnik                      |  | Elevation (m): 35.40                               |  | Auger ID/OD:                            |  |                     |  |
| Operator: Maine Test Borings            |  | Datum: NVD                                         |  | Sampler: 3.49 on Split Spoon            |  |                     |  |
| Logged By: T. Erickson                  |  | Rig Type: Mobile B33 Bombardier                    |  | Hammer Wt./Fall: 623 kg / 7.62 cm       |  |                     |  |
| Date Start/Finish: 1/4/02 / 1/7/02      |  | Drilling Method: Probe Auger/ Hammer Coasting (NW) |  | Core Barrel:                            |  |                     |  |
| Boring Location: Station 12 + 844, 5 L  |  | Coating ID/OD: 7.62 cm (NW)                        |  | Water Level(s): Dry                     |  |                     |  |
| Definitions                             |  |                                                    |  | Definitions                             |  |                     |  |
| S <sub>u</sub> = Saturated Soil Sample  |  |                                                    |  | W <sub>c</sub> = water content, percent |  |                     |  |
| σ <sub>v</sub> = measured soil stress   |  |                                                    |  | L = Liquid Limit                        |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | P <sub>L</sub> = Plastic Limit          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | PI = Plasticity Index                   |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
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| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
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| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>v</sub> = vertical soil stress   |  |                                                    |  | e = void ratio                          |  |                     |  |
| σ <sub>h</sub> = horizontal soil stress |  |                                                    |  | e = void ratio                          |  |                     |  |
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|--------------------------------------------|-----------------------|------------------|------------------------------------------------------------|----------------------------------------------|--------------------------------------------------------------------------------------------|------------------------------------------------------|
| Haley & Aldrich, Inc.                      |                       |                  | Project: Kennebec River Crossing                           |                                              | Boring No.: C2-2-01                                                                        |                                                      |
| Soil/Book Exploration Log                  |                       |                  | Location: Augusta, Maine                                   |                                              | P/N: 556.21                                                                                |                                                      |
| METRIC UNITS                               |                       |                  | File No. 80785-004                                         |                                              |                                                                                            |                                                      |
| Driller:                                   | G. Rudnik             |                  | Elevation (m):                                             | 35.40                                        | Auger ID/OD:                                                                               |                                                      |
| Operator:                                  | Maine Test Borings    |                  | Datum:                                                     | NVD                                          | Sampler:                                                                                   | 3.49 on Split Spoon                                  |
| Logged By:                                 | T. Erickson           |                  | Rig Type:                                                  | Mobile B33 Bombardier                        | Hammer Wt./Fall:                                                                           | 623 N / 7.62 cm                                      |
| Date Start/Finish:                         | 1/4/02 / 1/7/02       |                  | Drilling Method:                                           | Probe Auger/Hammer Coasting (NW)             | Core Barrel:                                                                               |                                                      |
| Boring Location:                           | Station 12 + 844, 5 L |                  | Coating ID/OD:                                             | 7.62 cm (NW)                                 | Water Level:                                                                               | Dry                                                  |
| Soil/Notes:                                |                       |                  | Soil/Notes:                                                |                                              | Soil/Notes:                                                                                |                                                      |
| a = Split Spoon Sample                     |                       |                  | S <sub>u</sub> = Saturated Field Vane Shear Strength (kPa) |                                              | BB = water content, percent                                                                |                                                      |
| AB = Thompson's Split Spoon Sample attempt |                       |                  | T <sub>u</sub> = Pocket Torque Shear Strength (kPa)        |                                              | LL = Liquid Limit                                                                          |                                                      |
| L = Split Spoon Tube Sample                |                       |                  | A <sub>v</sub> = Measured Compression Strength (kPa)       |                                              | PL = Plasticity Limit                                                                      |                                                      |
| V = Vane Shear Sample                      |                       |                  | T <sub>u</sub> = Lab Vane Shear Strength (kPa)             |                                              | PI = Plasticity Index                                                                      |                                                      |
| T = Torque Shear Shear Test                |                       |                  | BB = Moisture of 40 No. 100                                |                                              | G = Grain Size Analysis                                                                    |                                                      |
| SA = Split 1/2 Core Auger                  |                       |                  | WB = Weight of core                                        |                                              | C = Coarse Fraction Test                                                                   |                                                      |
| Sample Information                         |                       |                  |                                                            |                                              |                                                                                            |                                                      |
| Depth (m)                                  | Sample No.            | Penetration (cm) | Sample Depth (m)                                           | Blow Count (150 mm) (blows/30 cm) or MOU (3) | Visual Description and Remarks                                                             | Laboratory Testing Results/ASD/DT and Unified Class. |
|                                            |                       |                  |                                                            |                                              |                                                                                            |                                                      |
| 8                                          |                       |                  |                                                            |                                              | Light gray, fine sand, sandy                                                               |                                                      |
|                                            |                       |                  |                                                            |                                              | -MARINE DEPOSIT-(CL)                                                                       |                                                      |
| 9                                          |                       |                  |                                                            |                                              | Light brown, dry, dense, SAND with trace silt, poorly-sorted, stratified                   |                                                      |
|                                            |                       |                  |                                                            |                                              | -MARINE DEPOSIT-(SP)                                                                       |                                                      |
| 10                                         |                       |                  |                                                            |                                              |                                                                                            |                                                      |
|                                            |                       |                  |                                                            |                                              |                                                                                            |                                                      |
| 11                                         |                       |                  |                                                            |                                              | Light brown, dry, dense, SAND with trace silt, poorly-sorted, stratified                   |                                                      |
|                                            |                       |                  |                                                            |                                              | -MARINE DEPOSIT-(SP)                                                                       |                                                      |
| 12                                         |                       |                  |                                                            |                                              |                                                                                            |                                                      |
|                                            |                       |                  |                                                            |                                              |                                                                                            |                                                      |
| 13                                         |                       |                  |                                                            |                                              | Light brown, moist, medium dense, silt/sand, poorly-sorted, stratified                     |                                                      |
|                                            |                       |                  |                                                            |                                              | -MARINE DEPOSIT-(GM)                                                                       |                                                      |
| 14                                         |                       |                  |                                                            |                                              |                                                                                            |                                                      |
|                                            |                       |                  |                                                            |                                              |                                                                                            |                                                      |
| 15                                         |                       |                  |                                                            |                                              | Brown, moist, very dense, SAND with some gravel and little silt, well-sorted, no structure |                                                      |
|                                            |                       |                  |                                                            |                                              | -GLACIO FLUVIAL DEPOSIT-(SW)                                                               |                                                      |
| 16                                         |                       |                  |                                                            |                                              |                                                                                            |                                                      |
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| 17                                         |                       |                  |                                                            |                                              |                                                                                            |                                                      |
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| Haley & Aldrich, Inc.                                                                                                                                                                          |                                                   | Project: Kennebec River Crossing        |                                        | Boring No.: C2-2-01                                |                  |
| Soil/Book Exploration Log                                                                                                                                                                      |                                                   | Location: Augusta, Maine                |                                        | P/N: 556.21                                        |                  |
| METRIC UNITS                                                                                                                                                                                   |                                                   | File No. 80785-004                      |                                        |                                                    |                  |
| Driller: G. Rudnik                                                                                                                                                                             | Elevation (m): 35.40                              | Auger ID/OD:                            |                                        |                                                    |                  |
| Operator: Maine Test Borings                                                                                                                                                                   | Datum: NVD                                        | Sampler:                                |                                        | 3.49 on Split Spoon                                |                  |
| Logged By: T. Erickson                                                                                                                                                                         | Rig Type: Mobile B33 Bombardier                   | Hammer Wt./Fall: 623 N / 7.62 cm        |                                        |                                                    |                  |
| Date Start/Finish: 1/4/02 / 1/7/02                                                                                                                                                             | Drilling Method: Probe Auger/Hammer Coasting (NW) | Core Barrel:                            |                                        |                                                    |                  |
| Boring Location: Station 12 + 844, 5 L                                                                                                                                                         | Coating ID/OD: 7.62 cm (NW)                       | Water Level:                            |                                        | Dry                                                |                  |
| Soil Information                                                                                                                                                                               |                                                   | Soil Information:                       |                                        |                                                    |                  |
| S <sub>u</sub> = Static Field Vane Shear Strength (kPa)                                                                                                                                        |                                                   | W <sub>c</sub> = water content, percent |                                        |                                                    |                  |
| S <sub>t</sub> = Pocket Torque Shear Strength (kPa)                                                                                                                                            |                                                   | L = Liquid Limit                        |                                        |                                                    |                  |
| R <sub>u</sub> = Unconfined Compressive Strength (kPa)                                                                                                                                         |                                                   | P <sub>L</sub> = Plasticity Limit       |                                        |                                                    |                  |
| T <sub>u</sub> = Lab Vane Shear Strength (kPa)                                                                                                                                                 |                                                   | PI = Plasticity Index                   |                                        |                                                    |                  |
| W <sub>L</sub> = Weight of 30 gm. water                                                                                                                                                        |                                                   | S = Soil Size Analysis                  |                                        |                                                    |                  |
| W <sub>t</sub> = Weight of soil                                                                                                                                                                |                                                   | S = Soil Classification                 |                                        |                                                    |                  |
| Sample Information                                                                                                                                                                             |                                                   | Visual Description and Remarks          |                                        | Laboratory Testing Results/ASTM and Unified Class. |                  |
| Sample No.                                                                                                                                                                                     | Penetration (cm)                                  | Sample Depth (m)                        | Blow (150 mm) (blows/30 cm) or Mod (%) | Penetration (cm)                                   | Sample Depth (m) |
| 16                                                                                                                                                                                             |                                                   |                                         |                                        | 55                                                 |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 66                                                 |                  |
| 17                                                                                                                                                                                             | D12                                               | 61.0/25.6                               | 16.78 - 17.31                          | 22/16/16/22                                        | 32 43            |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 43                                                 |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 89                                                 |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 99                                                 |                  |
| 18                                                                                                                                                                                             |                                                   |                                         |                                        | 205                                                | 16.68            |
|                                                                                                                                                                                                |                                                   |                                         |                                        |                                                    | 17.92            |
|                                                                                                                                                                                                | D13                                               | 61.0/25.4                               | 17.32 - 18.60                          | 92/63/51/38                                        | 114 101          |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 96                                                 |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 99                                                 |                  |
| 19                                                                                                                                                                                             |                                                   |                                         |                                        | 125                                                |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 105                                                |                  |
|                                                                                                                                                                                                | D14                                               | 61.0/20.3                               | 18.61 - 20.48                          | 33/23/28/37                                        | 51 57            |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 71                                                 |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 73                                                 |                  |
| 21                                                                                                                                                                                             |                                                   |                                         |                                        | 107                                                |                  |
|                                                                                                                                                                                                | D15                                               | 61.0/25.4                               | 20.49 - 21.66                          | 67/50/57/53                                        | 107 86           |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 129                                                |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 111                                                |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 115                                                |                  |
| 22                                                                                                                                                                                             |                                                   |                                         |                                        | 171                                                |                  |
|                                                                                                                                                                                                | D16                                               | 61.0/27.8                               | 21.67 - 22.41                          | 67/42/46/42                                        | 87 52            |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 96                                                 |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 118                                                |                  |
|                                                                                                                                                                                                |                                                   |                                         |                                        | 110                                                |                  |
| Remarks:                                                                                                                                                                                       |                                                   |                                         |                                        |                                                    |                  |
| Stratification lines represent approximate boundaries between soil types; transitions may be gradual.                                                                                          |                                                   |                                         |                                        |                                                    |                  |
| * Soiler level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                                                   |                                         |                                        |                                                    |                  |
| Page 3 of 4                                                                                                                                                                                    |                                                   |                                         |                                        | Boring No.: C2-2-01                                |                  |

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| Haley & Aldrich, Inc.              |                       | Project: Kennebec River Crossing                       |                                   | Boring No.: C2-2-01                     |                     |
| Soil/Book Exploration Log          |                       | Location: Augusta, Maine                               |                                   | File No. 80785-004                      |                     |
| METRIC UNITS                       |                       | P/N: 556.21                                            |                                   |                                         |                     |
| Driller:                           | G. Rudnik             | Elevation (m):                                         | 35.40                             | Auger ID/OD:                            |                     |
| Operator:                          | Maine Test Borings    | Datum:                                                 | NVD                               | Sampler:                                | 3.49 on Split Spoon |
| Logged By:                         | T. Erickson           | Rig Type:                                              | Mobile B33 Bombardier             | Hammer Wt./Fall:                        | 623 N / 7.62 cm     |
| Date Start/Finish:                 | 1/4/02 / 1/7/02       | Drilling Method:                                       | Probe Auger/ Hammer Coasting (NW) | Core Barrel:                            |                     |
| Boring Location:                   | Station 12 + 844, 5 L | Coating ID/OD:                                         | 7.62 cm (NW)                      | Water Level:                            | Dry                 |
| Soil Information                   |                       | Soil Notes                                             |                                   | Soil Notes                              |                     |
| S = Split Spoon Sample             |                       | S <sub>u</sub> = Split Field Vane Shear Strength (kPa) |                                   | S <sub>u</sub> = water content, percent |                     |
| MB = Horizontal Split Spoon Sample |                       | T <sub>u</sub> = Pocket Torque Shear Strength (kPa)    |                                   | L = Liquid Limit                        |                     |
| S = Thin Wall Tube Sample          |                       | C = Unconsolidated Compressive Strength (kPa)          |                                   | P <sub>L</sub> = Plasticity Limit       |                     |
| P = Rock Core Sample               |                       | T <sub>u</sub> (kPa) = Lab Vane Shear Strength (kPa)   |                                   | PI = Plasticity Index                   |                     |
| S = Split Spoon Sample             |                       | W <sub>t</sub> = Weight of Soil Sample                 |                                   | S = Swell Potential Test                |                     |
| MB = Split Spoon Sample            |                       | W <sub>t</sub> = Weight of Soil Sample                 |                                   |                                         |                     |
| Visual Description and Remarks     |                       |                                                        |                                   |                                         |                     |
| Depth (m)                          | Sample No.            | Penetration (cm)                                       | Sample Depth (m)                  | Blow Count (150 mm)                     | Soil Description    |
| 24.03                              | D17                   | 61.0/20.3                                              | 24.38 - 24.86                     | 43/28/26/32                             | 54 52               |
| 24.86                              |                       |                                                        |                                   |                                         | 128                 |
| 25.47                              | D18                   | 61.0/22.9                                              | 24.87 - 26.52                     | 46/44/40/33                             | 84 70               |
| 26.52                              |                       |                                                        |                                   |                                         | 95                  |
| 27.13                              | D19                   | 61.0/22.9                                              | 26.53 - 28.04                     | 39/20/25/33                             | 65 61               |
| 28.04                              |                       |                                                        |                                   |                                         | 103                 |
| 28.65                              | D20                   | 61.0/17.8                                              | 28.05 - 29.57                     | 72/37/37/46                             | 134                 |
| 29.57                              |                       |                                                        |                                   |                                         | 137                 |
| 30.18                              |                       |                                                        |                                   |                                         | 139                 |
| 30.79                              |                       |                                                        |                                   |                                         | 141                 |
| 31.40                              |                       |                                                        |                                   |                                         | 143                 |
| 32.01                              |                       |                                                        |                                   |                                         | 145                 |
| 32.62                              |                       |                                                        |                                   |                                         | 147                 |
| 33.23                              |                       |                                                        |                                   |                                         | 149                 |
| 33.84                              |                       |                                                        |                                   |                                         | 151                 |
| 34.45                              |                       |                                                        |                                   |                                         | 153                 |
| 35.06                              |                       |                                                        |                                   |                                         | 155                 |
| 35.67                              |                       |                                                        |                                   |                                         | 157                 |
| 36.28                              |                       |                                                        |                                   |                                         | 159                 |
| 36.89                              |                       |                                                        |                                   |                                         | 161                 |
| 37.50                              |                       |                                                        |                                   |                                         | 163                 |
| 38.11                              |                       |                                                        |                                   |                                         | 165                 |
| 38.72                              |                       |                                                        |                                   |                                         | 167                 |
| 39.33                              |                       |                                                        |                                   |                                         | 169                 |
| 39.94                              |                       |                                                        |                                   |                                         | 171                 |
| 40.55                              |                       |                                                        |                                   |                                         | 173                 |
| 41.16                              |                       |                                                        |                                   |                                         | 175                 |
| 41.77                              |                       |                                                        |                                   |                                         | 177                 |
| 42.38                              |                       |                                                        |                                   |                                         | 179                 |
| 42.99                              |                       |                                                        |                                   |                                         | 181                 |
| 43.60                              |                       |                                                        |                                   |                                         | 183                 |
| 44.21                              |                       |                                                        |                                   |                                         | 185                 |
| 44.82                              |                       |                                                        |                                   |                                         | 187                 |
| 45.43                              |                       |                                                        |                                   |                                         | 189                 |
| 46.04                              |                       |                                                        |                                   |                                         | 191                 |
| 46.65                              |                       |                                                        |                                   |                                         | 193                 |
| 47.26                              |                       |                                                        |                                   |                                         | 195                 |
| 47.87                              |                       |                                                        |                                   |                                         | 197                 |
| 48.48                              |                       |                                                        |                                   |                                         | 199                 |
| 49.09                              |                       |                                                        |                                   |                                         | 201                 |
| 49.70                              |                       |                                                        |                                   |                                         | 203                 |
| 50.31                              |                       |                                                        |                                   |                                         | 205                 |
| 50.92                              |                       |                                                        |                                   |                                         | 207                 |
| 51.53                              |                       |                                                        |                                   |                                         | 209                 |
| 52.14                              |                       |                                                        |                                   |                                         | 211                 |
| 52.75                              |                       |                                                        |                                   |                                         | 213                 |
| 53.36                              |                       |                                                        |                                   |                                         | 215                 |
| 53.97                              |                       |                                                        |                                   |                                         | 217                 |
| 54.58                              |                       |                                                        |                                   |                                         | 219                 |
| 55.19                              |                       |                                                        |                                   |                                         | 221                 |
| 55.80                              |                       |                                                        |                                   |                                         | 223                 |
| 56.41                              |                       |                                                        |                                   |                                         | 225                 |
| 57.02                              |                       |                                                        |                                   |                                         | 227                 |
| 57.63                              |                       |                                                        |                                   |                                         | 229                 |
| 58.24                              |                       |                                                        |                                   |                                         | 231                 |
| 58.85                              |                       |                                                        |                                   |                                         | 233                 |
| 59.46                              |                       |                                                        |                                   |                                         | 235                 |
| 60.07                              |                       |                                                        |                                   |                                         | 237                 |
| 60.68                              |                       |                                                        |                                   |                                         | 239                 |
| 61.29                              |                       |                                                        |                                   |                                         | 241                 |
| 61.90                              |                       |                                                        |                                   |                                         | 243                 |
| 62.51                              |                       |                                                        |                                   |                                         | 245                 |
| 63.12                              |                       |                                                        |                                   |                                         | 247                 |
| 63.73                              |                       |                                                        |                                   |                                         | 249                 |
| 64.34                              |                       |                                                        |                                   |                                         | 251                 |
| 64.95                              |                       |                                                        |                                   |                                         | 253                 |
| 65.56                              |                       |                                                        |                                   |                                         | 255                 |
| 66.17                              |                       |                                                        |                                   |                                         | 257                 |
| 66.78                              |                       |                                                        |                                   |                                         | 259                 |
| 67.39                              |                       |                                                        |                                   |                                         | 261                 |
| 68.00                              |                       |                                                        |                                   |                                         | 263                 |
| 68.61                              |                       |                                                        |                                   |                                         | 265                 |
| 69.22                              |                       |                                                        |                                   |                                         | 267                 |
| 69.83                              |                       |                                                        |                                   |                                         | 269                 |
| 70.44                              |                       |                                                        |                                   |                                         | 271                 |
| 71.05                              |                       |                                                        |                                   |                                         | 273                 |
| 71.66                              |                       |                                                        |                                   |                                         | 275                 |
| 72.27                              |                       |                                                        |                                   |                                         | 277                 |
| 72.88                              |                       |                                                        |                                   |                                         | 279                 |
| 73.49                              |                       |                                                        |                                   |                                         | 281                 |
| 74.10                              |                       |                                                        |                                   |                                         | 283                 |
| 74.71                              |                       |                                                        |                                   |                                         | 285                 |
| 75.32                              |                       |                                                        |                                   |                                         | 287                 |
| 75.93                              |                       |                                                        |                                   |                                         | 289                 |
| 76.54                              |                       |                                                        |                                   |                                         | 291                 |
| 77.15                              |                       |                                                        |                                   |                                         | 293                 |
| 77.76                              |                       |                                                        |                                   |                                         | 295                 |
| 78.37                              |                       |                                                        |                                   |                                         | 297                 |
| 78.98                              |                       |                                                        |                                   |                                         | 299                 |
| 79.59                              |                       |                                                        |                                   |                                         | 301                 |
| 80.20                              |                       |                                                        |                                   |                                         | 303                 |
| 80.81                              |                       |                                                        |                                   |                                         | 305                 |
| 81.42                              |                       |                                                        |                                   |                                         | 307                 |
| 82.03                              |                       |                                                        |                                   |                                         | 309                 |
| 82.64                              |                       |                                                        |                                   |                                         | 311                 |
| 83.25                              |                       |                                                        |                                   |                                         | 313                 |
| 83.86                              |                       |                                                        |                                   |                                         | 315                 |
| 84.47                              |                       |                                                        |                                   |                                         | 317                 |
| 85.08                              |                       |                                                        |                                   |                                         | 319                 |
| 85.69                              |                       |                                                        |                                   |                                         | 321                 |
| 86.30                              |                       |                                                        |                                   |                                         | 323                 |
| 86.91                              |                       |                                                        |                                   |                                         | 325                 |
| 87.52                              |                       |                                                        |                                   |                                         | 327                 |
| 88.13                              |                       |                                                        |                                   |                                         | 329                 |
| 88.74                              |                       |                                                        |                                   |                                         | 331                 |
| 89.35                              |                       |                                                        |                                   |                                         | 333                 |
| 89.96                              |                       |                                                        |                                   |                                         | 335                 |
| 90.57                              |                       |                                                        |                                   |                                         | 337                 |
| 91.18                              |                       |                                                        |                                   |                                         | 339                 |
| 91.79                              |                       |                                                        |                                   |                                         | 341                 |
| 92.40                              |                       |                                                        |                                   |                                         | 343                 |
| 93.01                              |                       |                                                        |                                   |                                         | 345                 |
| 93.62                              |                       |                                                        |                                   |                                         | 347                 |
| 94.23                              |                       |                                                        |                                   |                                         | 349                 |
| 94.84                              |                       |                                                        |                                   |                                         | 351                 |
| 95.45                              |                       |                                                        |                                   |                                         | 353                 |
| 96.06                              |                       |                                                        |                                   |                                         | 355                 |
| 96.67                              |                       |                                                        |                                   |                                         | 357                 |
| 97.28                              |                       |                                                        |                                   |                                         | 359                 |
| 97.89                              |                       |                                                        |                                   |                                         | 361                 |
| 98.50                              |                       |                                                        |                                   |                                         | 363                 |
| 99.11                              |                       |                                                        |                                   |                                         | 365                 |
| 99.72                              |                       |                                                        |                                   |                                         | 367                 |
| 100.33                             |                       |                                                        |                                   |                                         | 369                 |
| 100.94                             |                       |                                                        |                                   |                                         | 371                 |
| 101.55                             |                       |                                                        |                                   |                                         | 373                 |
| 102.16                             |                       |                                                        |                                   |                                         | 375                 |
| 102.77                             |                       |                                                        |                                   |                                         | 377                 |
| 103.38                             |                       |                                                        |                                   |                                         | 379                 |
| 103.99                             |                       |                                                        |                                   |                                         | 381                 |
| 104.60                             |                       |                                                        |                                   |                                         | 383                 |
| 105.21                             |                       |                                                        |                                   |                                         | 385                 |
| 105.82                             |                       |                                                        |                                   |                                         | 387                 |
| 106.43                             |                       |                                                        |                                   |                                         | 389                 |
| 107.04                             |                       |                                                        |                                   |                                         | 391                 |
| 107.65                             |                       |                                                        |                                   |                                         | 393                 |
| 108.26                             |                       |                                                        |                                   |                                         | 395                 |
| 108.87                             |                       |                                                        |                                   |                                         | 397                 |
| 109.48                             |                       |                                                        |                                   |                                         | 399                 |
| 110.09                             |                       |                                                        |                                   |                                         | 401                 |
| 110.70                             |                       |                                                        |                                   |                                         | 403                 |
| 111.31                             |                       |                                                        |                                   |                                         | 405                 |
| 111.92                             |                       |                                                        |                                   |                                         | 407                 |
| 112.53                             |                       |                                                        |                                   |                                         | 409                 |
| 113.14                             |                       |                                                        |                                   |                                         | 411                 |
| 113.75                             |                       |                                                        |                                   |                                         | 413                 |
| 114.36                             |                       |                                                        |                                   |                                         | 415                 |
| 114.97                             |                       |                                                        |                                   |                                         | 417                 |
| 115.58                             |                       |                                                        |                                   |                                         | 419                 |
| 116.19                             |                       |                                                        |                                   |                                         | 421                 |
| 116.80                             |                       |                                                        |                                   |                                         | 423                 |
| 117.41                             |                       |                                                        |                                   |                                         | 425                 |
| 118.02                             |                       |                                                        |                                   |                                         | 427                 |
| 118.63                             |                       |                                                        |                                   |                                         | 429                 |
| 119.24                             |                       |                                                        |                                   |                                         | 431                 |
| 119.85                             |                       |                                                        |                                   |                                         | 433                 |
| 120.46                             |                       |                                                        |                                   |                                         | 435                 |
| 121.07                             |                       |                                                        |                                   |                                         | 437                 |
| 121.68                             |                       |                                                        |                                   |                                         | 439                 |
| 122.29                             |                       |                                                        |                                   |                                         | 441                 |
| 122.90                             |                       |                                                        |                                   |                                         | 443                 |
| 123.51                             |                       |                                                        |                                   |                                         | 445                 |
| 124.12                             |                       |                                                        |                                   |                                         | 447                 |
| 124.73                             |                       |                                                        |                                   |                                         | 449                 |
| 125.34                             |                       |                                                        |                                   |                                         | 451                 |
| 125.95                             |                       |                                                        |                                   |                                         | 453                 |
| 126.56                             |                       |                                                        |                                   |                                         | 455                 |
| 127.17                             |                       |                                                        |                                   |                                         | 457                 |
| 127.78                             |                       |                                                        |                                   |                                         | 459                 |
| 128.39                             |                       |                                                        |                                   |                                         | 461                 |
| 129.00                             |                       |                                                        |                                   |                                         | 463                 |
| 129.61                             |                       |                                                        |                                   |                                         | 465                 |
| 130.22                             |                       |                                                        |                                   |                                         | 467                 |
| 130.83                             |                       |                                                        |                                   |                                         | 469                 |
| 131.44                             |                       |                                                        |                                   |                                         | 471                 |
| 132.05                             |                       |                                                        |                                   |                                         | 473                 |
| 132.66                             |                       |                                                        |                                   |                                         | 475                 |
| 133.27                             |                       |                                                        |                                   |                                         | 477                 |
| 133.88                             |                       |                                                        |                                   |                                         | 479                 |
| 134.49                             |                       |                                                        |                                   |                                         | 481                 |
| 135.10                             |                       |                                                        |                                   |                                         | 483                 |
| 135.71                             |                       |                                                        |                                   |                                         | 485                 |
| 136.32                             |                       |                                                        |                                   |                                         | 487                 |
| 136.93                             |                       |                                                        |                                   |                                         | 489                 |
| 137.54                             |                       |                                                        |                                   |                                         | 491                 |
| 138.15                             |                       |                                                        |                                   |                                         | 493                 |
| 138.76                             |                       |                                                        |                                   |                                         | 495                 |
| 139.37                             |                       |                                                        |                                   |                                         | 497                 |
| 140.00                             |                       |                                                        |                                   |                                         | 499                 |
| 140.61                             |                       |                                                        |                                   |                                         | 501                 |
| 141.22                             |                       |                                                        |                                   |                                         | 503                 |
| 141.83                             |                       |                                                        |                                   |                                         | 505                 |
| 142.44                             |                       |                                                        |                                   |                                         | 507                 |
| 143.05                             |                       |                                                        |                                   |                                         | 509                 |
| 143.66                             |                       |                                                        |                                   |                                         | 511                 |
| 144.27                             |                       |                                                        |                                   |                                         | 513                 |
| 144.88                             |                       |                                                        |                                   |                                         | 515                 |
| 145.49                             |                       |                                                        |                                   |                                         | 517                 |
| 146.10                             |                       |                                                        |                                   |                                         | 519                 |
| 146.71                             |                       |                                                        |                                   |                                         | 521                 |
| 147.32                             |                       |                                                        |                                   |                                         | 523                 |
| 147.93                             |                       |                                                        |                                   |                                         | 525                 |
| 148.54                             |                       |                                                        |                                   |                                         | 527                 |
| 149.15                             |                       |                                                        |                                   |                                         | 529                 |
| 149.76                             |                       |                                                        |                                   |                                         | 531                 |
| 150.37                             |                       |                                                        |                                   |                                         | 533                 |
| 150.98                             |                       |                                                        |                                   |                                         | 535                 |
| 151.59                             |                       |                                                        |                                   |                                         | 537                 |
| 152.20                             |                       |                                                        |                                   |                                         | 539                 |
| 152.81                             |                       |                                                        |                                   |                                         | 541                 |
| 153.42                             |                       |                                                        |                                   |                                         | 543                 |
| 154.03                             |                       |                                                        |                                   |                                         | 545                 |
| 154.64                             |                       |                                                        |                                   |                                         | 547                 |
| 155.25                             |                       |                                                        |                                   |                                         | 549                 |
| 155.86                             |                       |                                                        |                                   |                                         | 551                 |
| 156.47                             |                       |                                                        |                                   |                                         | 553                 |
| 157.08                             |                       |                                                        |                                   |                                         | 555                 |
| 157.69                             |                       |                                                        |                                   |                                         | 557                 |
| 158.30                             |                       |                                                        |                                   |                                         | 559                 |
| 158.91                             |                       |                                                        |                                   |                                         | 561                 |
| 159.52                             |                       |                                                        |                                   |                                         | 563                 |
| 160.13                             |                       |                                                        |                                   |                                         | 565                 |
| 160.74                             |                       |                                                        |                                   |                                         | 567                 |
| 161.35                             |                       |                                                        |                                   |                                         | 569                 |
| 161.96                             |                       |                                                        |                                   |                                         | 571                 |
| 162.57                             |                       |                                                        |                                   |                                         | 573                 |
| 163.18                             |                       |                                                        |                                   |                                         | 575                 |
| 163.79                             |                       |                                                        |                                   |                                         | 577                 |
| 164.40                             |                       |                                                        |                                   |                                         | 579                 |
| 165.01                             |                       |                                                        |                                   |                                         | 581                 |
| 165.62                             |                       |                                                        |                                   |                                         | 583                 |
| 166.23                             |                       |                                                        |                                   |                                         | 585                 |
| 166.84                             |                       |                                                        |                                   |                                         | 587                 |
| 167.45                             |                       |                                                        |                                   |                                         | 589                 |
| 168.06                             |                       |                                                        |                                   |                                         | 591                 |
| 168.67                             |                       |                                                        |                                   |                                         | 593                 |
| 169.28                             |                       |                                                        |                                   |                                         | 595                 |
| 169.89                             |                       |                                                        |                                   |                                         | 597                 |
| 170.50                             |                       |                                                        |                                   |                                         | 599                 |
| 171.11                             |                       |                                                        |                                   |                                         | 601                 |
| 171.72                             |                       |                                                        |                                   |                                         | 603                 |
| 172.33                             |                       |                                                        |                                   |                                         | 605                 |
| 172.94                             |                       |                                                        |                                   |                                         | 607                 |
| 173.55                             |                       |                                                        |                                   |                                         | 609                 |
| 174.16                             |                       |                                                        |                                   |                                         | 611                 |
| 174.77                             |                       |                                                        |                                   |                                         | 613                 |
| 175.38                             |                       |                                                        |                                   |                                         | 615                 |
| 175.99                             |                       |                                                        |                                   |                                         | 617                 |
| 176.60                             |                       |                                                        |                                   |                                         | 619                 |
| 177.21                             |                       |                                                        |                                   |                                         | 621                 |
| 177.82                             |                       |                                                        |                                   |                                         | 623                 |
| 178.43                             |                       |                                                        |                                   |                                         | 625                 |
| 179.04                             |                       |                                                        |                                   |                                         | 627                 |
| 179.65                             |                       |                                                        |                                   |                                         | 629                 |
| 180.26                             |                       |                                                        |                                   |                                         | 631                 |
| 180.87                             |                       |                                                        |                                   |                                         | 633                 |
| 181.48                             |                       |                                                        |                                   |                                         | 635                 |
| 182.09                             |                       |                                                        |                                   |                                         | 637                 |
| 182.70                             |                       |                                                        |                                   |                                         | 639                 |
| 183.31                             |                       |                                                        |                                   |                                         | 641                 |
| 183.92                             |                       |                                                        |                                   |                                         | 643                 |
| 184.53                             |                       |                                                        |                                   |                                         | 645                 |
| 185.14                             |                       |                                                        |                                   |                                         | 647                 |
| 185.75                             |                       |                                                        |                                   |                                         | 649                 |
| 186.36                             |                       |                                                        |                                   |                                         | 651                 |
| 186.97                             |                       |                                                        |                                   |                                         | 653                 |
| 187.58                             |                       |                                                        |                                   |                                         | 655                 |
| 188.19                             |                       |                                                        |                                   |                                         | 657                 |
| 188.80                             |                       |                                                        |                                   |                                         | 659                 |
| 189.41                             |                       |                                                        |                                   |                                         | 661                 |
| 190.02                             |                       |                                                        |                                   |                                         | 663                 |
| 190.63                             |                       |                                                        |                                   |                                         | 665                 |
| 191.24                             |                       |                                                        |                                   |                                         | 667                 |
| 191.85                             |                       |                                                        |                                   |                                         | 669                 |
| 192.46                             |                       |                                                        |                                   |                                         | 671                 |
| 193.07                             |                       |                                                        |                                   |                                         | 673                 |
| 193.68                             |                       |                                                        |                                   |                                         | 675                 |
| 194.29                             |                       |                                                        |                                   |                                         | 677                 |
| 194.90                             |                       |                                                        |                                   |                                         | 679                 |
| 195.51                             |                       |                                                        |                                   |                                         | 681                 |
| 196.12                             |                       |                                                        |                                   |                                         | 683                 |
| 196.73                             |                       |                                                        |                                   |                                         | 685                 |
| 197.34                             |                       |                                                        |                                   |                                         | 687                 |
| 197.95                             |                       |                                                        |                                   |                                         | 689                 |
| 198.56                             |                       |                                                        |                                   |                                         | 691                 |
| 199.17                             |                       |                                                        |                                   |                                         | 693                 |
| 199.78                             |                       |                                                        |                                   |                                         | 695                 |
| 200.39                             |                       |                                                        |                                   |                                         | 697                 |
| 201.00                             |                       |                                                        |                                   |                                         | 699                 |
| 201.61                             |                       |                                                        |                                   |                                         | 701                 |
| 202.22                             |                       |                                                        |                                   |                                         | 703                 |
| 202.83                             |                       |                                                        |                                   |                                         | 705                 |
| 203.44                             |                       |                                                        |                                   |                                         | 707                 |
| 204.05                             |                       |                                                        |                                   |                                         | 709</               |

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

BORING LOGS

AUGUSTA

Date: \$DATE\$

Filename: \$FILE\$

|                         |     |      |       |
|-------------------------|-----|------|-------|
| PROJECT DESIGN ENGINEER | BKL | DATE | 08/02 |
| DESIGN-DETAILED         | BKL | BY   | M.S.  |
| CHECKED                 | JWW |      | 08/02 |
| REVISIONS               |     |      |       |
| FIELD CHANGES           |     |      |       |

PLANS

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| Boring Location: Station 12 + 850.5 R                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  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| Description:<br>1 - Split Spoon Sample<br>2 - 100 mm Split Spoon Sample<br>3 - 100 mm Split Spoon Sample<br>4 - 100 mm Split Spoon Sample<br>5 - 100 mm Split Spoon Sample<br>6 - 100 mm Split Spoon Sample<br>7 - 100 mm Split Spoon Sample<br>8 - 100 mm Split Spoon Sample<br>9 - 100 mm Split Spoon Sample<br>10 - 100 mm Split Spoon Sample<br>11 - 100 mm Split Spoon Sample<br>12 - 100 mm Split Spoon Sample<br>13 - 100 mm Split Spoon Sample<br>14 - 100 mm Split Spoon Sample<br>15 - 100 mm Split Spoon Sample<br>16 - 100 mm Split Spoon Sample<br>17 - 100 mm Split Spoon Sample<br>18 - 100 mm Split Spoon Sample<br>19 - 100 mm Split Spoon Sample<br>20 - 100 mm Split Spoon Sample<br>21 - 100 mm Split Spoon Sample<br>22 - 100 mm Split Spoon Sample<br>23 - 100 mm Split Spoon Sample<br>24 - 100 mm Split Spoon Sample<br>25 - 100 mm Split Spoon Sample<br>26 - 100 mm Split Spoon Sample<br>27 - 100 mm Split Spoon Sample<br>28 - 100 mm Split Spoon Sample<br>29 - 100 mm Split Spoon Sample<br>30 - 100 mm Split Spoon Sample<br>31 - 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| Haley & Aldrich, Inc.                                                                                                                                                                |            |                             |                    |                                                              |                      | Project: Kerebas River Crossing                 |               | Boring No.: C2-6-01 (CW)                                                                                                                                                                                        |                                                       |
|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------|-----------------------------|--------------------|--------------------------------------------------------------|----------------------|-------------------------------------------------|---------------|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------|
| Soil/Rock Exploration Log<br>METRIC UNITS                                                                                                                                            |            |                             |                    |                                                              |                      | Locations: Augusta, Maine<br>File No. 80765-004 |               | PIN: 556.21                                                                                                                                                                                                     |                                                       |
| Driller: G.Rudnicki                                                                                                                                                                  |            | Elevation (m): 10.56        |                    | Date: NWD                                                    |                      | Auger ID/OD:                                    |               |                                                                                                                                                                                                                 |                                                       |
| Operator: Melvin Tait Boring                                                                                                                                                         |            | Datum:                      |                    | Notes:                                                       |                      | Scribers:                                       |               | 3.49 cm Split Spoon                                                                                                                                                                                             |                                                       |
| Copper Pipe: 7.62 mm                                                                                                                                                                 |            | Rig Type: CM 45 Solid       |                    | Drilling Method: Hammer Coaling                              |                      | Hammer Jolt Fall:                               |               | 423 N / 7.62 m                                                                                                                                                                                                  |                                                       |
| Data Start/Finish: 12-15-01/12-22-01                                                                                                                                                 |            | Drilling Method:            |                    | Core Barrel:                                                 |                      | NO                                              |               |                                                                                                                                                                                                                 |                                                       |
| Boring Location: Station 12 + 90N, CL                                                                                                                                                |            | Coating ID/OD: 7.62 cm (M8) |                    | Water Level/Hr:                                              |                      | 10.24 m                                         |               |                                                                                                                                                                                                                 |                                                       |
| <b>Definitions:</b>                                                                                                                                                                  |            |                             |                    |                                                              |                      | <b>Definitions:</b>                             |               |                                                                                                                                                                                                                 |                                                       |
| S = Split Spoon Sample                                                                                                                                                               |            |                             |                    |                                                              |                      | W = water content, percent                      |               |                                                                                                                                                                                                                 |                                                       |
| H = Unconsolidated Split Spoon Sample strength                                                                                                                                       |            |                             |                    |                                                              |                      | LL = Liquid Limit                               |               |                                                                                                                                                                                                                 |                                                       |
| P = Plasticity Index                                                                                                                                                                 |            |                             |                    |                                                              |                      | PL = Plastic Limit                              |               |                                                                                                                                                                                                                 |                                                       |
| R = Rock Core Sample                                                                                                                                                                 |            |                             |                    |                                                              |                      | PI = Plasticity Index                           |               |                                                                                                                                                                                                                 |                                                       |
| T = Torque Tube Shear Test                                                                                                                                                           |            |                             |                    |                                                              |                      | d = depth Station Test                          |               |                                                                                                                                                                                                                 |                                                       |
| SW = Field Shear Apparatus                                                                                                                                                           |            |                             |                    |                                                              |                      | w = weight of soil                              |               |                                                                                                                                                                                                                 |                                                       |
| <b>Sample Information</b>                                                                                                                                                            |            |                             |                    |                                                              |                      |                                                 |               |                                                                                                                                                                                                                 |                                                       |
| Depth (m)                                                                                                                                                                            | Sample No. | Piercing (cm)               | Sample length (cm) | Blows (500 gmb) or Strength (kg/cm <sup>2</sup> ) or RQD (%) | Penetration (mm/sec) | Seismic Blow Rate                               | Elevation (m) | Visual Description and Remarks                                                                                                                                                                                  | Laboratory Testing Results, AASHTO and Unified Class. |
| 16                                                                                                                                                                                   |            |                             |                    |                                                              | 48                   |                                                 |               |                                                                                                                                                                                                                 |                                                       |
|                                                                                                                                                                                      | D12        | 61.0/15.2                   | 18.76 - 17.37      | 18/15/16/21                                                  | 38                   |                                                 |               | Brown, wet, dense, GRAVEL with some silt and little sand, poorly-sorted, loosely bonded -GLACIAL TILL DEPOSIT-(G)                                                                                               |                                                       |
| 17                                                                                                                                                                                   |            |                             |                    |                                                              | 54                   |                                                 |               |                                                                                                                                                                                                                 |                                                       |
|                                                                                                                                                                                      |            |                             |                    |                                                              | 55                   |                                                 |               |                                                                                                                                                                                                                 |                                                       |
|                                                                                                                                                                                      |            |                             |                    |                                                              | 91                   |                                                 |               |                                                                                                                                                                                                                 |                                                       |
| 18                                                                                                                                                                                   |            |                             |                    |                                                              | 83                   | -7.48                                           |               | Brown, wet, dense, SAND with some silt, well-sorted, non-striated, GLACIAL TILL DEPOSIT-(G)                                                                                                                     | 18-04                                                 |
|                                                                                                                                                                                      | D13        | 48.3/12.7                   | 18.09 - 16.77      | 25/44/34/50(2.5)                                             | 78                   |                                                 |               |                                                                                                                                                                                                                 |                                                       |
|                                                                                                                                                                                      | R1         | 100.0/25.4                  | 19.78 - 19.87      | RSD = 4X%                                                    | 100(17)              | -0.29 - 9.21                                    |               | Gray, wet, very dense, GRAVEL with silt and sand, poorly-sorted, well-sorted -GLACIAL TILL DEPOSIT-(G)                                                                                                          | 18-65                                                 |
|                                                                                                                                                                                      |            |                             |                    |                                                              |                      |                                                 |               | Gray, ophanitic, metamorphic, moderately hard, fresh to slightly jointed dipping at horizontal to steep angles, very close to moderately close, open, gray and shaly fracturing, RQD poor. Recovery 100% SLATE. | 18-70                                                 |
|                                                                                                                                                                                      |            |                             |                    |                                                              |                      |                                                 |               | Gray, ophanitic, metamorphic, moderately hard, fresh to very slightly jointed at moderate angles, close to moderately close, tight, no infillings RQD excellent. Recovery 100% -SLATE-                          |                                                       |
| 20                                                                                                                                                                                   | R2         | 157.0/39.6                  | 16.7 - 31.45       | RSD = 97%                                                    |                      |                                                 |               |                                                                                                                                                                                                                 |                                                       |
| 21                                                                                                                                                                                   |            |                             |                    |                                                              |                      |                                                 |               |                                                                                                                                                                                                                 |                                                       |
|                                                                                                                                                                                      |            |                             |                    |                                                              |                      | 10.80                                           |               | Bottom of Exploration at 21.46 m below ground surface.                                                                                                                                                          | 21-46                                                 |
| 22                                                                                                                                                                                   |            |                             |                    |                                                              |                      |                                                 |               |                                                                                                                                                                                                                 |                                                       |
| Remarks:                                                                                                                                                                             |            |                             |                    |                                                              |                      |                                                 |               |                                                                                                                                                                                                                 |                                                       |
| Stratification lines represent approximate boundaries between all types transitions may be gradual.                                                                                  |            |                             |                    |                                                              |                      |                                                 |               |                                                                                                                                                                                                                 |                                                       |
| * Some down readings have been made at times and under conditions stated. Boundary fluctuations may occur due to conditions other than those present on the Test material work note. |            |                             |                    |                                                              |                      |                                                 |               |                                                                                                                                                                                                                 |                                                       |

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 Boring No.: C2-6-01 (CW)

|                                                |
|------------------------------------------------|
| STATE OF MAINE<br>DEPARTMENT OF TRANSPORTATION |
| <b>BORING LOG</b>                              |
| AUGUSTA                                        |
| State Route 3 over Kennebec River              |

| Haley & Aldrich, Inc.                                                                                                                               |            |                      |                   | Projects: Karamba River Crossing                      |                  | Boring No.:                |               | C2-6-01                                          |                                                                                                                          |
|-----------------------------------------------------------------------------------------------------------------------------------------------------|------------|----------------------|-------------------|-------------------------------------------------------|------------------|----------------------------|---------------|--------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------|
| Soil/Block Exploration Log<br>METRIC LIMITS                                                                                                         |            |                      |                   | Location: Augusta, Maine                              |                  | PIN:                       |               | 556.21                                           |                                                                                                                          |
| Driller:                                                                                                                                            |            | O. Rudnicki          |                   | Elevation (m):                                        |                  | Auger ID (m):              |               |                                                  |                                                                                                                          |
| Operator:                                                                                                                                           |            | Maine Test Boring    |                   | Datum:                                                |                  | Sampers:                   |               | 3.49 on Split Spoon                              |                                                                                                                          |
| Logged By:                                                                                                                                          |            | T. D. Latham         |                   | Wig Type:                                             |                  | Sampler SP/SP/11:          |               | 425 W / 7.62 on                                  |                                                                                                                          |
| Data Start/Finish:                                                                                                                                  |            | 12-6-01/12-6-01      |                   | Drilling Method:                                      |                  | Sampler W/ Coating         |               | Core Bore(s):                                    |                                                                                                                          |
| Boring Location:                                                                                                                                    |            | Station 12 + 584.7 R |                   | Coating ID (m):                                       |                  | Water Level (m):           |               | 2.29 m                                           |                                                                                                                          |
| Definition:                                                                                                                                         |            |                      |                   | Definition:                                           |                  |                            |               |                                                  |                                                                                                                          |
| N = Split Spoon Sample                                                                                                                              |            |                      |                   | N = Test Field Free Shear Strength (kPa)              |                  | N = Water content, percent |               |                                                  |                                                                                                                          |
| N = Unsuccessful Split Spoon Sample attempt                                                                                                         |            |                      |                   | T <sub>u</sub> = Pocket Pressure Shear Strength (kPa) |                  | LL = Liquid Limit          |               |                                                  |                                                                                                                          |
| N = 750 ml Tube Sample                                                                                                                              |            |                      |                   | Q = Unconfined Compressive Strength (kPa)             |                  | PL = Plastic Limit         |               |                                                  |                                                                                                                          |
| N = 200 Core Sample                                                                                                                                 |            |                      |                   | N <sub>60</sub> = Lab Free Shear Strength (kPa)       |                  | P = Plasticity Index       |               |                                                  |                                                                                                                          |
| N = Direct Shear Box Test                                                                                                                           |            |                      |                   | W = Weight of Soil Sample                             |                  | L = Shrinkage              |               |                                                  |                                                                                                                          |
| N = Split Spoon Auger                                                                                                                               |            |                      |                   | W = Weight of Soil                                    |                  | S = Shrinkage              |               |                                                  |                                                                                                                          |
| Sample Information                                                                                                                                  |            |                      |                   | Visual Description and Remarks                        |                  |                            |               | Laboratory Testing Results/ASTM or Sifted Close. |                                                                                                                          |
| Depth (m)                                                                                                                                           | Sample No. | Penetration (cm)     | Sample Depth (cm) | Time (1:00 min)                                       | Penetration (cm) | Sample Depth (cm)          | Elevation (m) | Length (m)                                       |                                                                                                                          |
| 01                                                                                                                                                  | 61-0/9-1   | 0.00 - 0.61          | 23/17/12/14       | 29                                                    |                  |                            |               |                                                  | Brown, wet, medium dense, gravelly SAND, poorly-graded - FLUVIAL DEPOSIT (-SP)                                           |
| 1                                                                                                                                                   | 02         | 61-0/12-7            | 1.31 - 1.92       | 14/11/10/9                                            | 9                |                            |               |                                                  | Notes Rollerbit used through boulder from 1.22-1.31 m                                                                    |
| 2                                                                                                                                                   |            |                      |                   |                                                       | 14               |                            |               |                                                  | Brown, wet, medium dense, GRAVEL with some sand and little silt, poorly-graded, no structure - FLUVIAL DEPOSIT (-SP)     |
|                                                                                                                                                     |            |                      |                   |                                                       | 17               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 14               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 13               |                            |               |                                                  | Notes Wash water indicates a change of 2.41 m - 2.41                                                                     |
| 3                                                                                                                                                   | 03         | 61-0/27.9            | 1.4 - 3.39        | 7/7/7/7                                               | 14               | 7                          |               |                                                  | Brown, wet, medium dense, SAND with some silt, trace fine gravel, well-graded, no structure - FLUVIAL DEPOSIT (-SP)      |
| 4                                                                                                                                                   |            |                      |                   |                                                       | 11               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 13               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 11               |                            |               |                                                  |                                                                                                                          |
| 5                                                                                                                                                   | 04         | 61-0/27.9            | 4.27 - 4.88       | 7/8/7/11                                              | 17               | 9                          |               |                                                  | Light gray brown, medium dense, wet SAND with some silt, poorly-graded, no structure - FLUVIAL DEPOSIT (-SP)             |
| 6                                                                                                                                                   |            |                      |                   |                                                       | 19               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 20               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 20               |                            |               |                                                  |                                                                                                                          |
| 7                                                                                                                                                   | 06         | 61-0/5-1             | 5.79 - 6.40       | 8/11/11/12                                            | 22               | 10                         |               |                                                  | Brown, wet, medium dense, SAND with some silt, gravel and fine gravel; well-graded, no structure - FLUVIAL DEPOSIT (-SP) |
|                                                                                                                                                     |            |                      |                   |                                                       | 14               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 31               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 44               |                            |               |                                                  |                                                                                                                          |
|                                                                                                                                                     |            |                      |                   |                                                       | 45               |                            |               |                                                  | -7.60                                                                                                                    |
| 8                                                                                                                                                   | 07         | 61-0/12-7            | 7.32 - 7.92       | 35/25/32/50                                           | 57               | 22                         |               |                                                  | Notes Wash water indicates a strata change of 7.13                                                                       |
|                                                                                                                                                     |            |                      |                   |                                                       | 45               |                            |               |                                                  | -7.13                                                                                                                    |
|                                                                                                                                                     |            |                      |                   |                                                       | 28               |                            |               |                                                  | Gray, very dense, wet, sandy GRAVEL with little silt, poorly-graded, very loosely bonded                                 |
|                                                                                                                                                     |            |                      |                   |                                                       | 28               |                            |               |                                                  | 7.32                                                                                                                     |
|                                                                                                                                                     |            |                      |                   |                                                       |                  |                            |               |                                                  | 35.40 m, TAIL DEPOSIT (-SP)                                                                                              |
| Remarks:                                                                                                                                            |            |                      |                   |                                                       |                  |                            |               |                                                  |                                                                                                                          |
| Stratification lines represent approximate boundaries between soil types. Transitions may be gradual.                                               |            |                      |                   |                                                       |                  |                            |               |                                                  |                                                                                                                          |
| N = Test Field Free Shear Strength (kPa) or T <sub>u</sub> = Pocket Pressure Shear Strength (kPa). Rollerbit used through boulder from 1.22-1.31 m. |            |                      |                   |                                                       |                  |                            |               |                                                  |                                                                                                                          |
| Boring No.:                                                                                                                                         |            |                      |                   |                                                       |                  |                            |               |                                                  |                                                                                                                          |
| C2-6-01                                                                                                                                             |            |                      |                   |                                                       |                  |                            |               |                                                  |                                                                                                                          |

|                                                |
|------------------------------------------------|
| STATE OF MAINE<br>DEPARTMENT OF TRANSPORTATION |
| <b>BORING LOGS</b>                             |
| AUGUSTA                                        |
| State Route 3 over Kennebec River              |

Date: 11/20/2017

| PROJECT | DESIGN ENGINEER | DATE     | BY     | DATE     |
|---------|-----------------|----------|--------|----------|
| PLANS   | DESIGN-DETAILED | 08/02/17 | M.S.   | 08/02/17 |
|         | CHECKED         |          | J.W.V. |          |
|         | REVISIONS       |          |        |          |
|         | FIELD CHANGES   |          |        |          |

|                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
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| Haley & Aldrich, Inc.<br>Soil/Book Exploration Log<br>METRIC UNITS                                                                                                                            |                                                                                                                                                                                                                               | Project: Kennebec River Crossing<br>Location: Augusta, Maine<br>File No. 80765-004                                                                                                                                            | Boring No.: C2-11-01<br>PIN: 556.21                                                                                                                                                                                           |
| Driller: G. Rudolph                                                                                                                                                                           | Elevation (m): 7.27                                                                                                                                                                                                           | Auger ID/OD: 17.27                                                                                                                                                                                                            | Operator: Main Test Borings                                                                                                                                                                                                   |
| Operator: Main Test Borings                                                                                                                                                                   | Datum: NVD                                                                                                                                                                                                                    | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                  | Logged By: T. Erickson                                                                                                                                                                                                        |
| Logged By: T. Erickson                                                                                                                                                                        | Rig Type: Hammer Wt./Fall: 623 N / 7.62 m                                                                                                                                                                                     | Core Barrel: NO                                                                                                                                                                                                               | Date Start/Finish: 12-11-01/12-11-01                                                                                                                                                                                          |
| Date Start/Finish: 12-11-01/12-11-01                                                                                                                                                          | Drilling Method: Split/Hammer NW Casing                                                                                                                                                                                       | Water Level: NO                                                                                                                                                                                                               | Boring Location: Station 13 + 124. CL                                                                                                                                                                                         |
| Boring Location: Station 13 + 124. CL                                                                                                                                                         | Casing ID/OD: 7.62 cm (NW)                                                                                                                                                                                                    | Water Level: Dry                                                                                                                                                                                                              | Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                                 |
| Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test |
| Sample Information                                                                                                                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Depth (m)                                                                                                                                                                                     | Sample No.                                                                                                                                                                                                                    | Pen/Sec (cm)                                                                                                                                                                                                                  | Sample Depth (m)                                                                                                                                                                                                              |
| 0                                                                                                                                                                                             | D1                                                                                                                                                                                                                            | 61.0/22.8                                                                                                                                                                                                                     | 0.00 - 0.61                                                                                                                                                                                                                   |
| 1                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 2                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 3                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 4                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 5                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 6                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 7                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Visual Description and Remarks                                                                                                                                                                |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, brown, damp, loose, SAND with trace silt, poorly-graded, stratified in some spots. Notes: Casing is hammered down with 140 lb hammer.                                                   |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, damp, loose, SAND with possible trace silt, poorly-graded. -MARINE DEPOSIT-(SP)                                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, granitic to fine grained, metamorphic, moderately hard, fresh to slightly jointed, dipping at steep angles, very close to open, no infilling. -SCHIST-                                  |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, granitic to fine grained, metamorphic, moderately hard, fresh to slightly jointed, dipping at steep angles, very close to open, no infilling. -SCHIST-                                  |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Bottom of Exploration at 5.82 m below ground surface.                                                                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Remarks:                                                                                                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Stratification lines represent approximate boundaries between soil types. Transitions may be gradual.                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| * Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Page 1 of 1<br>Boring No.: C2-11-01                                                                                                                                                           |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |

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| Haley & Aldrich, Inc.<br>Soil/Book Exploration Log<br>METRIC UNITS                                                                                                                            |                                                                                                                                                                                                                               | Project: Kennebec River Crossing<br>Location: Augusta, Maine<br>File No. 80765-004                                                                                                                                            | Boring No.: C2-11A-01<br>PIN: 556.21                                                                                                                                                                                          |
| Driller: G. Rudolph                                                                                                                                                                           | Elevation (m): 7.27                                                                                                                                                                                                           | Auger ID/OD: 17.27                                                                                                                                                                                                            | Operator: Main Test Borings                                                                                                                                                                                                   |
| Operator: Main Test Borings                                                                                                                                                                   | Datum: NVD                                                                                                                                                                                                                    | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                  | Logged By: T. Erickson                                                                                                                                                                                                        |
| Logged By: T. Erickson                                                                                                                                                                        | Rig Type: Hammer Wt./Fall: 623 N / 7.62 m                                                                                                                                                                                     | Core Barrel: NO                                                                                                                                                                                                               | Date Start/Finish: 12-12-01/12-12-01                                                                                                                                                                                          |
| Date Start/Finish: 12-12-01/12-12-01                                                                                                                                                          | Drilling Method: Split/Hammer NW Casing                                                                                                                                                                                       | Water Level: NO                                                                                                                                                                                                               | Boring Location: Station 13 + 122. 15 L                                                                                                                                                                                       |
| Boring Location: Station 13 + 122. 15 L                                                                                                                                                       | Casing ID/OD: 7.62 cm (NW)                                                                                                                                                                                                    | Water Level: Dry                                                                                                                                                                                                              | Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                                 |
| Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test |
| Sample Information                                                                                                                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Depth (m)                                                                                                                                                                                     | Sample No.                                                                                                                                                                                                                    | Pen/Sec (cm)                                                                                                                                                                                                                  | Sample Depth (m)                                                                                                                                                                                                              |
| 0                                                                                                                                                                                             | D1                                                                                                                                                                                                                            | 61.0/49.6                                                                                                                                                                                                                     | 0.00 - 0.61                                                                                                                                                                                                                   |
| 1                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 2                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 3                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 4                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 5                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 6                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 7                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Visual Description and Remarks                                                                                                                                                                |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, brown, dry, loose, SAND with little organic matter and trace silt, poorly-graded, some stratification. Notes: Organic disappear at 0.57 m.                                              |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, brown, dry, medium dense, SAND with trace silt, poorly-graded, no structure, color change to light brown at 1.58 m. -MARINE DEPOSIT-(SP)                                                |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Notes: 140 lb hammer used to pound down casing. Notes: Top of rock at 3.02 m, hammer used to pound through soft rock from 3.02-3.05 m.                                                        |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, granitic to fine grained, metamorphic, moderately hard, fresh to slightly jointed, dipping at steep angles, very close to open, no infilling. -SCHIST-                                  |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, granitic to fine grained, metamorphic, moderately hard, fresh to slightly jointed, dipping at steep angles, very close to open, no infilling. -SCHIST-                                  |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Bottom of Exploration at 6.04 m below ground surface.                                                                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Remarks:                                                                                                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Stratification lines represent approximate boundaries between soil types. Transitions may be gradual.                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| * Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Page 1 of 1<br>Boring No.: C2-11A-01                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |

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| Haley & Aldrich, Inc.<br>Soil/Book Exploration Log<br>METRIC UNITS                                                                                                                            |                                                                                                                                                                                                                               | Project: Kennebec River Crossing<br>Location: Augusta, Maine<br>File No. 80765-004                                                                                                                                            | Boring No.: C2-12-01<br>PIN: 556.21                                                                                                                                                                                           |
| Driller: G. Rudolph                                                                                                                                                                           | Elevation (m): 11.09                                                                                                                                                                                                          | Auger ID/OD: 17.27                                                                                                                                                                                                            | Operator: Main Test Borings                                                                                                                                                                                                   |
| Operator: Main Test Borings                                                                                                                                                                   | Datum: NVD                                                                                                                                                                                                                    | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                  | Logged By: T. Erickson                                                                                                                                                                                                        |
| Logged By: T. Erickson                                                                                                                                                                        | Rig Type: Hammer Wt./Fall: 623 N / 7.62 m                                                                                                                                                                                     | Core Barrel: NO                                                                                                                                                                                                               | Date Start/Finish: 1/9/02 / 1/10/02                                                                                                                                                                                           |
| Date Start/Finish: 1/9/02 / 1/10/02                                                                                                                                                           | Drilling Method: Hammer Casing                                                                                                                                                                                                | Water Level: NO                                                                                                                                                                                                               | Boring Location: Station 13 + 187. 6 L                                                                                                                                                                                        |
| Boring Location: Station 13 + 187. 6 L                                                                                                                                                        | Casing ID/OD: 7.62 cm (NW)                                                                                                                                                                                                    | Water Level: 2.23 m                                                                                                                                                                                                           | Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                                 |
| Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test |
| Sample Information                                                                                                                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Depth (m)                                                                                                                                                                                     | Sample No.                                                                                                                                                                                                                    | Pen/Sec (cm)                                                                                                                                                                                                                  | Sample Depth (m)                                                                                                                                                                                                              |
| 0                                                                                                                                                                                             | D1                                                                                                                                                                                                                            | 61.0/35.6                                                                                                                                                                                                                     | 0.00 - 0.61                                                                                                                                                                                                                   |
| 1                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 2                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 3                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 4                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 5                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 6                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 7                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Visual Description and Remarks                                                                                                                                                                |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Brown silty SAND with trace roots, poorly-graded. -TOPSOIL-                                                                                                                                   |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Yellow brown, dry, loose silty SAND, poorly-graded. -ALLUVIAL FLUVIAL DEPOSIT-(SM)                                                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Yellow brown and brown dry, very dense SAND with some silt and gravel, poorly graded. -GLACIO FLUVIAL DEPOSIT-(SM)                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Notes: Roller bit used from 3.14-3.17 m. Notes: Weathered bedrock fragments in till appears to be -SCHIST-                                                                                    |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Brown, damp, very dense, sandy GRAVEL with trace silt, poorly-graded. -GLACIAL TILL-(GP)                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, granitic to fine grained, metamorphic, moderately hard, fresh to slightly jointed, dipping at steep angles, very close to open, minor silt infilling. -SCHIST-                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, granitic to fine grained, metamorphic, moderately hard, fresh to slightly jointed, dipping at steep angles, very close to open, minor silt infilling. -SCHIST-                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Bottom of Exploration at 6.22 m below ground surface. 3.17 m because the rollerbit was used before casing was advanced.                                                                       |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Remarks:                                                                                                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Stratification lines represent approximate boundaries between soil types. Transitions may be gradual.                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| * Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Page 1 of 1<br>Boring No.: C2-12-01                                                                                                                                                           |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |

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| Haley & Aldrich, Inc.<br>Soil/Book Exploration Log<br>METRIC UNITS                                                                                                                                                         |                                                                                                                                                                                                                               | Project: Kennebec River Crossing<br>Location: Augusta, Maine<br>File No. 80765-004                                                                                                                                            | Boring No.: C2-13-01<br>PIN: 556.21                                                                                                                                                                                           |
| Driller: G. Rudolph                                                                                                                                                                                                        | Elevation (m): 15.90                                                                                                                                                                                                          | Auger ID/OD: 17.27                                                                                                                                                                                                            | Operator: Main Test Borings                                                                                                                                                                                                   |
| Operator: Main Test Borings                                                                                                                                                                                                | Datum: NVD                                                                                                                                                                                                                    | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                  | Logged By: T. Erickson                                                                                                                                                                                                        |
| Logged By: T. Erickson                                                                                                                                                                                                     | Rig Type: Hammer Wt./Fall: 623 N / 7.62 m                                                                                                                                                                                     | Core Barrel: NO                                                                                                                                                                                                               | Date Start/Finish: 1/9/02 / 1/10/02                                                                                                                                                                                           |
| Date Start/Finish: 1/9/02 / 1/10/02                                                                                                                                                                                        | Drilling Method: Hammer Casing                                                                                                                                                                                                | Water Level: NO                                                                                                                                                                                                               | Boring Location: Station 13 + 187. 6 R                                                                                                                                                                                        |
| Boring Location: Station 13 + 187. 6 R                                                                                                                                                                                     | Casing ID/OD: 7.62 cm (NW)                                                                                                                                                                                                    | Water Level: 5.06 m                                                                                                                                                                                                           | Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                                 |
| Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                              | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test |
| Sample Information                                                                                                                                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Depth (m)                                                                                                                                                                                                                  | Sample No.                                                                                                                                                                                                                    | Pen/Sec (cm)                                                                                                                                                                                                                  | Sample Depth (m)                                                                                                                                                                                                              |
| 0                                                                                                                                                                                                                          | D1                                                                                                                                                                                                                            | 61.0/33.0                                                                                                                                                                                                                     | 0.00 - 0.61                                                                                                                                                                                                                   |
| 1                                                                                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 2                                                                                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 3                                                                                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 4                                                                                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 5                                                                                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 6                                                                                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 7                                                                                                                                                                                                                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Visual Description and Remarks                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Brown silty SAND, trace roots, poorly-graded. -TOPSOIL-                                                                                                                                                                    |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Yellow brown, loose, dry, silty SAND, poorly-graded. -ALLUVIAL FLUVIAL DEPOSIT-(SM)                                                                                                                                        |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Light brown, dry, medium dense, SAND with some gravel and little silt, poorly-graded. -GLACIO FLUVIAL DEPOSIT-(SP)                                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Brown, damp, very dense, sandy GRAVEL with little silt, poorly-graded. -GLACIAL TILL-(GP)                                                                                                                                  |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Notes: Roller bit used from 4.05-4.21 m. White to light gray, granitic to fine grained, dipping at horizontal to vertical angles, very close to moderately close, tight to open, very silty brown clay infilling. -SCHIST- |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| White to light gray, granitic to fine grained, dipping at horizontal to vertical angles, very close to moderately close, tight to open, minor silt infilling. -SCHIST-                                                     |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Bottom of Exploration at 1.25 m below ground surface.                                                                                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Remarks:                                                                                                                                                                                                                   |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Stratification lines represent approximate boundaries between soil types. Transitions may be gradual.                                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| * Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made.                              |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Page 1 of 1<br>Boring No.: C2-13-01                                                                                                                                                                                        |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |

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| Haley & Aldrich, Inc.<br>Soil/Book Exploration Log<br>METRIC UNITS                                                                                                                            |                                                                                                                                                                                                                               | Project: Kennebec River Crossing<br>Location: Augusta, Maine<br>File No. 80765-004                                                                                                                                            | Boring No.: C2-14-01<br>PIN: 556.21                                                                                                                                                                                           |
| Driller: G. Rudolph                                                                                                                                                                           | Elevation (m): 17.40                                                                                                                                                                                                          | Auger ID/OD: 17.27                                                                                                                                                                                                            | Operator: Main Test Borings                                                                                                                                                                                                   |
| Operator: Main Test Borings                                                                                                                                                                   | Datum: NVD                                                                                                                                                                                                                    | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                  | Logged By: T. Erickson                                                                                                                                                                                                        |
| Logged By: T. Erickson                                                                                                                                                                        | Rig Type: Hammer Wt./Fall: 623 N / 7.62 m                                                                                                                                                                                     | Core Barrel: NO                                                                                                                                                                                                               | Date Start/Finish: 1/10/02 / 1/10/02                                                                                                                                                                                          |
| Date Start/Finish: 1/10/02 / 1/10/02                                                                                                                                                          | Drilling Method: Hammer Casing                                                                                                                                                                                                | Water Level: NO                                                                                                                                                                                                               | Boring Location: Station 13 + 190. 10 L                                                                                                                                                                                       |
| Boring Location: Station 13 + 190. 10 L                                                                                                                                                       | Casing ID/OD: 7.62 cm (NW)                                                                                                                                                                                                    | Water Level: 2.29 m                                                                                                                                                                                                           | Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                                 |
| Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test |
| Sample Information                                                                                                                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Depth (m)                                                                                                                                                                                     | Sample No.                                                                                                                                                                                                                    | Pen/Sec (cm)                                                                                                                                                                                                                  | Sample Depth (m)                                                                                                                                                                                                              |
| 0                                                                                                                                                                                             | D1                                                                                                                                                                                                                            | 61.0/35.6                                                                                                                                                                                                                     | 0.00 - 0.61                                                                                                                                                                                                                   |
| 1                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 2                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 3                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 4                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 5                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 6                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 7                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Visual Description and Remarks                                                                                                                                                                |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Brown silty SAND, trace roots, poorly-graded. -TOPSOIL-(SM)                                                                                                                                   |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Yellow brown, dry, loose silty SAND, poorly-graded. -GLACIO FLUVIAL DEPOSIT-(SM)                                                                                                              |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Notes: Glacio fluvial deposits from 1.62-1.86 m contain fine to coarse gravel in about 25% of the string.                                                                                     |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Brown and gray, dry, very dense, GRAVEL with some sand and little silt, poorly-graded. -GLACIO FLUVIAL DEPOSIT-(GP)                                                                           |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, granitic to fine grained, metamorphic, moderately hard, fresh to slightly jointed, dipping at steep angles, very close to open, minor silt infilling. -SCHIST-                          |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Bottom of Exploration at 3.51 m below ground surface.                                                                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Remarks:                                                                                                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Stratification lines represent approximate boundaries between soil types. Transitions may be gradual.                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| * Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Page 1 of 1<br>Boring No.: C2-14-01                                                                                                                                                           |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |

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| Haley & Aldrich, Inc.<br>Soil/Book Exploration Log<br>METRIC UNITS                                                                                                                            |                                                                                                                                                                                                                               | Project: Kennebec River Crossing<br>Location: Augusta, Maine<br>File No. 80765-004                                                                                                                                            | Boring No.: C2-15-01<br>PIN: 556.21                                                                                                                                                                                           |
| Driller: G. Rudolph                                                                                                                                                                           | Elevation (m): 17.00                                                                                                                                                                                                          | Auger ID/OD: 17.27                                                                                                                                                                                                            | Operator: Main Test Borings                                                                                                                                                                                                   |
| Operator: Main Test Borings                                                                                                                                                                   | Datum: NVD                                                                                                                                                                                                                    | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                  | Logged By: T. Erickson                                                                                                                                                                                                        |
| Logged By: T. Erickson                                                                                                                                                                        | Rig Type: Hammer Wt./Fall: 623 N / 7.62 m                                                                                                                                                                                     | Core Barrel: NO                                                                                                                                                                                                               | Date Start/Finish: 1/9/02 / 1/10/02                                                                                                                                                                                           |
| Date Start/Finish: 1/9/02 / 1/10/02                                                                                                                                                           | Drilling Method: Hammer Casing                                                                                                                                                                                                | Water Level: NO                                                                                                                                                                                                               | Boring Location: Station 13 + 190. 10 R                                                                                                                                                                                       |
| Boring Location: Station 13 + 190. 10 R                                                                                                                                                       | Casing ID/OD: 7.62 cm (NW)                                                                                                                                                                                                    | Water Level: 4.57 m                                                                                                                                                                                                           | Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                                 |
| Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test |
| Sample Information                                                                                                                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Depth (m)                                                                                                                                                                                     | Sample No.                                                                                                                                                                                                                    | Pen/Sec (cm)                                                                                                                                                                                                                  | Sample Depth (m)                                                                                                                                                                                                              |
| 0                                                                                                                                                                                             | D1                                                                                                                                                                                                                            | 61.0/35.6                                                                                                                                                                                                                     | 0.00 - 0.61                                                                                                                                                                                                                   |
| 1                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 2                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 3                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 4                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 5                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 6                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 7                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Visual Description and Remarks                                                                                                                                                                |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| -TOPSOIL-                                                                                                                                                                                     |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Tan, loose, dry, SAND with some silt and trace roots, poorly-graded. -GLACIO FLUVIAL DEPOSIT-(SM)                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Gray, brown, damp, very dense, GRAVEL with some sand, poorly-graded. -GLACIAL TILL-(GP)                                                                                                       |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Notes: Cored from 1.64-3.05 m. Cored through boulder from 1.64-1.98 m.                                                                                                                        |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Yellow brown dry, very dense, sandy GRAVEL with little silt, poorly-graded. -GLACIAL TILL-(GP)                                                                                                |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Notes: Roller bit used from 12.8-13.2 ft.                                                                                                                                                     |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| White, granitic to fine grained, hard, fresh to moderate, joints dipping at horizontal to vertical angles, very close to close, tight to open, some light brown clay in filling. -SCHIST-     |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Bottom of Exploration at 5.55 m below ground surface.                                                                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Remarks:                                                                                                                                                                                      |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Stratification lines represent approximate boundaries between soil types. Transitions may be gradual.                                                                                         |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| * Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Page 1 of 1<br>Boring No.: C2-15-01                                                                                                                                                           |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |

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| Haley & Aldrich, Inc.<br>Soil/Book Exploration Log<br>METRIC UNITS                                                                                                                            |                                                                                                                                                                                                                               | Project: Kennebec River Crossing<br>Location: Augusta, Maine<br>File No. 80765-004                                                                                                                                            | Boring No.: C2-16-01<br>PIN: 556.21                                                                                                                                                                                           |
| Driller: G. Rudolph                                                                                                                                                                           | Elevation (m): 33.00                                                                                                                                                                                                          | Auger ID/OD: 17.27                                                                                                                                                                                                            | Operator: Main Test Borings                                                                                                                                                                                                   |
| Operator: Main Test Borings                                                                                                                                                                   | Datum: NVD                                                                                                                                                                                                                    | Sampler: 3.49 on Split Spoon                                                                                                                                                                                                  | Logged By: T. Erickson                                                                                                                                                                                                        |
| Logged By: T. Erickson                                                                                                                                                                        | Rig Type: Hammer Wt./Fall: 623 N / 7.62 m                                                                                                                                                                                     | Core Barrel: NO                                                                                                                                                                                                               | Date Start/Finish: 10/24/02 / 11/01/02                                                                                                                                                                                        |
| Date Start/Finish: 10/24/02 / 11/01/02                                                                                                                                                        | Drilling Method: Hammer Casing                                                                                                                                                                                                | Water Level: NO                                                                                                                                                                                                               | Boring Location: Station 12 + 845. CL                                                                                                                                                                                         |
| Boring Location: Station 12 + 845. CL                                                                                                                                                         | Casing ID/OD: 7.62 cm (NW)                                                                                                                                                                                                    | Water Level: 4.57 m                                                                                                                                                                                                           | Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger                                 |
| Definition: 1 = Split Spoon Sample<br>2 = Unsuccessful Split Spoon Sample attempt<br>3 = Thin Wall Tube Sample<br>4 = Rock Core Sample<br>5 = In Situ Vane Shear Test<br>6 = Solid Stem Auger | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test | Definition: 1 = In Situ Field Vane Shear Strength (kPa)<br>2 = Liquid Limit<br>3 = Plasticity Index<br>4 = Moisture Content (%)<br>5 = In Situ Vane Shear Strength (kPa)<br>6 = Grain Size Analysis<br>7 = Consolidation Test |
| Sample Information                                                                                                                                                                            |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| Depth (m)                                                                                                                                                                                     | Sample No.                                                                                                                                                                                                                    | Pen/Sec (cm)                                                                                                                                                                                                                  | Sample Depth (m)                                                                                                                                                                                                              |
| 0                                                                                                                                                                                             | D1                                                                                                                                                                                                                            | 61.0/35.6                                                                                                                                                                                                                     | 0.00 - 0.61                                                                                                                                                                                                                   |
| 1                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 2                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 3                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 4                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 5                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               |
| 6                                                                                                                                                                                             |                                                                                                                                                                                                                               |                                                                                                                                                                                                                               | </                                                                                                                                                                                                                            |

|       |                         |     |     |       |
|-------|-------------------------|-----|-----|-------|
| PLANS | PROJECT DESIGN ENGINEER | BKL | BY  | DATE  |
|       | DESIGN-DETAILED         | BKL | MLS | 08/02 |
|       | CHECKED                 | JWW |     | 08/02 |
|       | REVISIONS               |     |     |       |
|       | FIELD CHANGES           |     |     |       |

|                                                                                                                |      |                        |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|----------------------------------------------------------------------------------------------------------------|------|------------------------|-------------------------|-----------------------------|------------------------------------------------------------------------------------------------------|--------------------------------------|---------------------------------------------------------------------------------------------------------------------|------------------------------------------------------------------------|----------------------------|
| Soun Portland, Maine                                                                                           |      | TEST BORING REPORT     |                         |                             |                                                                                                      | BORING NO.                           |                                                                                                                     | B26                                                                    |                            |
| PROJECT: Konahelec River Crossing Project<br>LOCATION: Augusta, Maine<br>CLIENT: Maine Dept. of Transportation |      |                        |                         |                             |                                                                                                      | CONTRACTOR: Maine Test Borings, Inc. |                                                                                                                     | FILE NO. B0765-006<br>SHEET NO. 1 OF 3<br>LOCATION Sta 2+640.15<br>TEL |                            |
| ITEM                                                                                                           |      | CASING                 | DIVE SAMPLER            | CORE BARREL                 | DRILLING EQUIPMENT & PROCEDURES                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| TYPE                                                                                                           |      | HW                     | SS                      |                             | RO TYPE: Model B53 Bombardier<br>BT TYPE: Raber 34<br>DIAL: W&D<br>OTHER: Solid stem auger<br>6-12dm |                                      | ELEVATION 26.97<br>NOV 0<br>START May 11, 2000<br>FINISH May 11, 2000<br>DRILLER M. Goring<br>HSA, REP S. Wlechniak |                                                                        |                            |
| INSIDE DIAMETER (CM)                                                                                           |      | 8                      | 3                       |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| HAMMER WEIGHT (KG)                                                                                             |      | 136                    | 64                      | -                           |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| HAMMER FALL (CM)                                                                                               |      | 61                     | 75                      |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| DEPTH (M)                                                                                                      |      | CASING BLOWS PER 0.30M | SAMPLER BLOWS PER 0.30M | SAMPLE NUMBER & REC (CM)    | SAMPLE DEPTH (M)                                                                                     | CLAY / DEPTH (M)                     | VISUAL DESCRIPTION AND REMARKS                                                                                      |                                                                        |                            |
| 0.0                                                                                                            |      | 1                      | S1                      | 0.00                        |                                                                                                      |                                      | Medium - dense brown coarse to fine SAND.<br>Trace silt                                                             |                                                                        |                            |
|                                                                                                                |      | 5                      | 30                      | 0.61                        |                                                                                                      |                                      | -GLACIOFLUVIAL DEPOSIT-                                                                                             |                                                                        |                            |
|                                                                                                                |      | 7                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 9                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| 1.0                                                                                                            |      |                        |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 2                      | S2                      | 1.92                        | 95.30                                                                                                |                                      | Medium - stiff, brown-gray clayey SILT                                                                              |                                                                        |                            |
|                                                                                                                |      | 2                      | 61                      | 2.13                        | 1.77                                                                                                 |                                      | -MAYNE DEPOSIT-                                                                                                     |                                                                        |                            |
| 2.0                                                                                                            |      | 3                      | 4                       |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 2                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 5                      |                         |                             | 35.53                                                                                                | 2.94                                 |                                                                                                                     |                                                                        |                            |
| 3.0                                                                                                            |      | 7                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 3                      | W01                     | S3                          | 3.06                                                                                                 |                                      | Medium - stiff, gray silty CLAY                                                                                     |                                                                        |                            |
|                                                                                                                |      | 2                      |                         | 61                          | 3.66                                                                                                 |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 4                      | 3                       |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| 4.0                                                                                                            |      | 5                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 10                     |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 12                     | W01                     | S4                          | 4.57                                                                                                 |                                      | Medium - stiff, gray silty CLAY                                                                                     |                                                                        |                            |
| 5.0                                                                                                            |      | 12                     | W01                     | 61                          | 5.18                                                                                                 |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 3                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 5                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| 6.0                                                                                                            |      | 14                     |                         | S5                          | 6.10                                                                                                 |                                      | Medium - stiff, gray silty CLAY, frequent fine sand partings                                                        |                                                                        |                            |
|                                                                                                                |      | 2                      |                         | 61                          | 6.71                                                                                                 |                                      | -MAYNE DEPOSIT-                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 3                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 5                      |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| 7.0                                                                                                            |      | 17                     |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
|                                                                                                                |      | 19                     |                         |                             |                                                                                                      |                                      |                                                                                                                     |                                                                        |                            |
| WATER LEVEL DATA                                                                                               |      |                        |                         |                             |                                                                                                      |                                      | SAMPLE IDENTIFICATION                                                                                               |                                                                        | SUMMARY                    |
| DATE                                                                                                           | TIME | ELAPSED TIME (HR)      | BOTTOM OF CASING        | DEPTH (M) TO BOTTOM OF HOLE | WATER                                                                                                | O<br>I<br>U<br>S                     | OPEN END ROD<br>3" IN WALL TUBE<br>UNSTIRRED SAMPLE<br>SPUT SPOON                                                   | OVERBURDEN (M)<br>HOLE CORED (M)<br>UNSTIRRED SAMPLE<br>HSA            | 25.30<br>175<br>METRIC LOG |
| 5/2/00                                                                                                         | 2:00 | 11.0                   | 19.29                   | 19.29                       | 17.63                                                                                                |                                      |                                                                                                                     |                                                                        |                            |

South Portland,  
Maine

# TEST BORING REPORT

BORING NO. 826  
FILE NO. 80785-000  
SHEET NO. 2 OF 3

| DEPTH<br>(M) | CASING<br>BLOWS<br>PER 0.30M | SAMPLER<br>BLOWS<br>PER 0.30M | SAMPLE<br>NUMBERS &<br>REC. (CM) | SAMPLE<br>DEPTH<br>(M) | ELEV./<br>DEPTH<br>(M) | VISUAL DESCRIPTION AND REMARKS                                                                                            |
|--------------|------------------------------|-------------------------------|----------------------------------|------------------------|------------------------|---------------------------------------------------------------------------------------------------------------------------|
| 8.0          | 21                           | 3                             | 56                               | 7.62                   | 29.20                  | Medium stiff, gray silty CLAY, frequent fine sand partings                                                                |
|              |                              | 5                             | 61                               | 8.23                   | 7.77                   |                                                                                                                           |
|              |                              | 9                             |                                  |                        |                        | -MARINE DEPOSIT-                                                                                                          |
|              | 39                           | 10                            |                                  |                        |                        | Medium dense, gray silty CLAY seams interbedded with brown fine SAND layers and seams                                     |
|              | 52                           |                               |                                  |                        |                        | -MARINE DEPOSIT-                                                                                                          |
| 9.0          | 73                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 29                           | 8                             | 57                               | 9.14                   |                        | Dense, brown, tan, gray, orange finely stratified silty CLAY, fine SAND, silty fine sand laminae, partings and seams      |
|              | 21                           | 12                            | 36                               | 9.75                   |                        |                                                                                                                           |
|              | 51                           | 28                            |                                  |                        |                        |                                                                                                                           |
|              | 66                           | 46                            |                                  |                        |                        |                                                                                                                           |
| 10.0         | 75                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 86                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 105                          |                               |                                  |                        |                        |                                                                                                                           |
|              | 51                           | 29                            | 58                               | 10.67                  |                        | Very dense, brown, tan, gray, orange finely stratified silty CLAY, fine SAND, silty fine sand laminae, partings and seams |
|              | 60                           | 40                            | 33                               | 11.28                  |                        |                                                                                                                           |
|              | 63                           | 36                            |                                  |                        |                        |                                                                                                                           |
|              | 66                           |                               |                                  |                        |                        |                                                                                                                           |
| 12.0         | 71                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 37                           | 13                            | 59                               | 12.13                  |                        | Very dense, brown, orange, gray finely stratified fine SAND                                                               |
|              | 63                           | 16                            | 28                               | 12.80                  |                        |                                                                                                                           |
|              |                              | 52                            |                                  |                        |                        |                                                                                                                           |
|              |                              | 58                            |                                  |                        |                        |                                                                                                                           |
| 13.0         | 118                          |                               |                                  |                        | 25.67                  |                                                                                                                           |
|              | 151                          |                               |                                  |                        | 12.11                  |                                                                                                                           |
|              | 267                          |                               |                                  |                        |                        |                                                                                                                           |
|              | 50                           | 60                            | 510                              | 13.72                  |                        | Very dense, brown coarser to fine SAND, trace silt                                                                        |
| 14.0         | 36                           | 136                           | 23                               | 14.02                  |                        |                                                                                                                           |
|              | 27                           |                               |                                  |                        |                        | -GLACIAL TILL-                                                                                                            |
|              | 26                           |                               |                                  |                        |                        |                                                                                                                           |
| 15.0         | 30                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 48                           | 41                            | 511                              | 15.24                  |                        | Very dense, brown/orange coarser to fine SAND, some gravel, trace silt                                                    |
|              | 30                           | 50/-4                         | 18                               | 15.67                  |                        |                                                                                                                           |
|              | 46                           |                               |                                  |                        |                        |                                                                                                                           |
| 16.0         | 28                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 51                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 46                           |                               |                                  |                        |                        |                                                                                                                           |
| 17.0         | 55                           | 81                            | 512                              | 16.76                  |                        | Very dense, brown/orange coarser to fine SAND, some gravel, trace silt                                                    |
|              |                              | 205/-3                        | 10                               | 17.01                  |                        |                                                                                                                           |
|              | 55                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 46                           |                               |                                  |                        |                        |                                                                                                                           |
|              | 32                           |                               |                                  |                        |                        |                                                                                                                           |
| 18.0         | 27                           |                               |                                  |                        |                        |                                                                                                                           |

H&A

METRIC LOG

South Portland,  
Maine

# TEST BORING REPORT

BORING NO. 826  
FILE NO. 80785-000  
SHEET NO. 3 OF 3

| DEPTH<br>(M) | CASING<br>BLOW<br>PER 0.30M | SAMPLER<br>BLOW<br>PER 0.15M | SAMPLE<br>NUMBERS &<br>REC. (CM) | SAMPLE<br>DEPTH<br>(M) | ELEV./<br>DEPTH<br>(M) | VISUAL DESCRIPTION AND REMARKS                                                      |
|--------------|-----------------------------|------------------------------|----------------------------------|------------------------|------------------------|-------------------------------------------------------------------------------------|
| 19.0         | 48                          | 21                           | 513                              | 18.29                  | 51.2                   | Very dense, brown gravelly coarse to fine<br>SAND, trace silt<br><br>-GLACIAL TILL- |
|              | 46                          | 95                           | 10                               | 18.75                  | 51.2                   |                                                                                     |
|              |                             | 62                           |                                  |                        | 51.2                   |                                                                                     |
|              | 45                          |                              |                                  |                        | 51.2                   |                                                                                     |
|              | 56                          |                              |                                  |                        | 51.2                   |                                                                                     |
| 20.0         | 71                          |                              |                                  |                        | 51.2                   | Very dense, brown gravelly coarse to fine<br>SAND                                   |
|              | 69                          | 514                          | 19.81                            |                        | 51.2                   |                                                                                     |
|              | 91                          | 94                           | 10                               | 20.12                  | 51.2                   |                                                                                     |
|              | 118                         |                              |                                  |                        | 51.2                   |                                                                                     |
|              | 166                         |                              |                                  |                        | 51.2                   |                                                                                     |
| 21.0         | 243                         |                              |                                  |                        | 51.2                   | Very dense, brown gravelly coarse to fine<br>SAND, trace silt<br><br>-GLACIAL TILL- |
|              | 65                          |                              |                                  |                        | 51.2                   |                                                                                     |
|              | 61                          | 515                          | 21.64                            |                        | 51.2                   |                                                                                     |
|              | 52                          | 97                           | 13                               | 22.10                  | 51.2                   |                                                                                     |
|              | 110                         |                              |                                  |                        | 51.2                   |                                                                                     |
| 23.0         | 130                         |                              |                                  |                        | 51.2                   | Very dense, brown gravelly coarse to fine<br>SAND, trace silt<br><br>-GLACIAL TILL- |
|              | 48                          | 516                          | 22.86                            |                        | 51.2                   |                                                                                     |
|              | 75/-4                       | 10                           | 23.13                            |                        | 51.2                   |                                                                                     |
|              | 35                          |                              |                                  |                        | 51.2                   |                                                                                     |
|              | 63                          |                              |                                  |                        | 51.2                   |                                                                                     |
| 24.0         | 89                          |                              |                                  |                        | 51.2                   | Very dense, brown gravelly coarse to fine<br>SAND, trace silt<br><br>-GLACIAL TILL- |
|              | 109                         |                              |                                  |                        | 51.2                   |                                                                                     |
|              | 45                          | 517                          | 24.38                            |                        | 51.2                   |                                                                                     |
|              | 80                          | 15                           | 24.84                            |                        | 51.2                   |                                                                                     |
|              | 10                          |                              |                                  |                        | 51.2                   |                                                                                     |
| 25.0         |                             |                              |                                  |                        | 11.67<br>25.30         | Washed ahead of casing to 25.3m.<br>Bottom of Exploration at 25.3m.<br>No refusal   |

H&A

METRIC LOG

|                                                                                                              |                       |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|--------------------------------------------------------------------------------------------------------------|-----------------------|------------------------|----------------------------|-----------------------------|---------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------------------|--|
| South Portland, Maine                                                                                        |                       | TEST BORING REPORT     |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | B27                                                              |  |
| PROJECT: Kennebec River Crossing Project<br>LOCATION: Augusta Marine<br>CLIENT: Maine Dept of Transportation |                       |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | FILE NO. 60706-006<br>SHEET NO. 1 OF 2<br>LOCATION: Sta. 2+95.0L |  |
| CONTRACTOR: Maine Test Borings, Inc.                                                                         |                       |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| ITEM                                                                                                         |                       | CASING                 | DRIVE SAMPLER              | CORE BARREL                 | DRILLING EQUIPMENT & PROCEDURES |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | ELEVATION 95.69                                                  |  |
| TYPE                                                                                                         |                       | MM                     | SS                         |                             | RIG TYPE CME 45                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | DATE/MAY 22, 2000                                                |  |
| INSIDE DIAMETER (CM)                                                                                         |                       | 8                      | 2                          |                             | BIT TYPE Fuller bit             |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | FINISH May 23, 2000                                              |  |
| HAMMER WEIGHT (KG)                                                                                           |                       | 136                    | 64                         | --                          | DRILL M.O.D.                    |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | DRILLER G. Linderoth                                             |  |
| HAMMER FALL (CM)                                                                                             |                       | 61                     | 76                         | --                          | OTHER --                        |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | HSA REP M. Conner                                                |  |
| DEPTH (M)                                                                                                    | CASING SLOW PER 0.30M | SAMPLER SLOW PER 0.30M | SAMPLER NUMBER & REC.(CM)  | SAMPLE DEPTH (M)            | RLYV/ DEPTH (M)                 | VISUAL DESCRIPTION AND REMARKS                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                |                                                                  |  |
| 0.0                                                                                                          | 1                     | 1                      | 59A                        | 0.30                        | 0.54                            | Very loose, brown loamy fine sandy SLT.<br>(No organics)<br>(No nodules)<br>Very loose, brown fine SAND, trace silt<br>--FLUMAL DEPOSIT--<br>Medium dense, yellow-brown coarse to fine SAND, trace gravel, silt<br>Medium dense, yellow-brown coarse to fine SAND, trace gravel, silt<br>Medium dense, yellow-brown coarse to fine SAND, trace gravel, silt<br>Dense, light brown gravelly/gruilly coarse to fine SAND, trace silt<br>--FLUMAL DEPOSIT--<br>Very dense, orange-brown gravelly coarse to fine SAND, trace silt<br>How: Casing refusal at 6.7m, trace bit ahead to 6.95m. Core to 7.0m.<br>Cobbles and Boulders |                                                                  |  |
|                                                                                                              | 1                     | 1                      | 18                         | 0.61                        | 0.15                            |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 4                     | 1                      |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| 1.0                                                                                                          |                       |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 17                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 19                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 8                     | 52                     |                            | 1.52                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 8                     | 8                      | 23                         | 2.13                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| 2.0                                                                                                          | 21                    | 9                      |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 44                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| 3.0                                                                                                          | 51                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 16                    | 53                     |                            | 3.06                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 20                    | 11                     | 26                         | 3.66                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 12                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| 4.0                                                                                                          | 47                    |                        |                            |                             | 0.28                            |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 40                    |                        |                            |                             | 3.88                            |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 57                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 53                    | 25                     | 54                         | 4.52                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 53                    | 22                     | 18                         | 5.18                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| 5.0                                                                                                          | 62                    | 27                     |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 98                    | 24                     |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 80                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| 6.0                                                                                                          | 130                   |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 66                    | 46                     | 50                         | 6.10                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 200                   | 42                     | 20                         | 6.64                        |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 250                   | 502                    |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| 7.0                                                                                                          | 66                    |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
|                                                                                                              | 77                    |                        |                            |                             | 8.47                            |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               |                                                                  |  |
| WATER LEVEL DATA                                                                                             |                       |                        |                            |                             |                                 | SAMPLE IDENTIFICATION                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                         |                                                                  |  |
| DATE                                                                                                         | TIME                  | ELAPSED TIME (H:M)     | DEPTH (M) TO TOP OF CASING | DEPTH (M) TO BOTTOM OF HOLE | WATER                           | SUMMARY                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       |                                                                  |  |
| 5/26/00                                                                                                      | 1745                  | 0.25                   | 4.57                       | 5.15                        | 4.05                            | O <input type="checkbox"/> OPEN END ROD<br>U <input type="checkbox"/> UNH. WALL TUBE<br>S <input type="checkbox"/> SPLIT SPOON                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                | OVERBURDEN (M)<br>ROCK CORED (M)<br>SAMPLES                      |  |
|                                                                                                              |                       |                        |                            |                             |                                 |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               | 14.33<br>106                                                     |  |
|                                                                                                              |                       |                        |                            |                             |                                 | HSA METRIC LOG                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                |                                                                  |  |

South Portland,

TEST BORE REPORT

BORING NO. 927

FILE NO. 80765-000

SHEET NO. 2 OF 2

| DEPTH<br>(M) | CASING<br>BLOWS<br>PER 0.30M | SAMPLER<br>BLOWS<br>PER 0.15M | SAMPLE<br>NUMBER &<br>REC.(CM) | SAMPLE<br>DEPTH<br>(M) | ELEV./<br>DEPTH<br>(M) | VISUAL DESCRIPTION AND REMARKS               |
|--------------|------------------------------|-------------------------------|--------------------------------|------------------------|------------------------|----------------------------------------------|
| 8.0          | 18                           | 26                            | 56                             | 7.62                   | 51                     | Very dense, gray-brown/orange-brown gravelly |
|              |                              | 33                            | 25                             | 8.23                   | 51                     | coarse to fine SAND, little silt             |
|              | 18                           | 27                            |                                |                        | 51                     | -GLACIAL TILL-                               |
|              | 25                           |                               |                                |                        | 51                     |                                              |
| 9.0          | 39                           |                               |                                |                        | 51                     |                                              |
|              | 92                           |                               |                                |                        | 51                     |                                              |
|              | 17                           | 45                            | 57                             | 8.14                   | 51                     | Very dense, gray-brown gravelly coarse to    |
|              |                              | 48                            | 20                             | 8.75                   | 51                     | fine SAND, trace silt                        |
| 10.0         | 39                           | 38                            |                                |                        | 51                     |                                              |
|              |                              | 42                            |                                |                        | 51                     |                                              |
|              | 94                           |                               |                                |                        | 51                     |                                              |
|              | 105                          |                               |                                |                        | 51                     |                                              |
| 11.0         | 85                           |                               |                                |                        | 51                     |                                              |
|              | 13                           | 32                            | 58                             | 10.67                  | 51                     | Very dense, gray-brown gravelly coarse to    |
|              |                              | 52                            | 23                             | 11.28                  | 51                     | fine SAND, trace silt                        |
|              | 92                           | 28                            |                                |                        | 51                     |                                              |
| 12.0         |                              | 22                            |                                |                        | 51                     |                                              |
|              | 106                          |                               |                                |                        | 51                     |                                              |
|              | 62                           |                               |                                |                        | 51                     |                                              |
|              | 21                           | 26                            | 59                             | 12.19                  | 51                     | Very dense, gray-brown/orange-brown gravelly |
| 13.0         |                              | 52                            | 10                             | 12.80                  | 51                     | coarse to fine SAND, trace silt              |
|              | 36                           | 57                            |                                |                        | 51                     | -GLACIAL TILL-                               |
|              |                              | 50                            |                                |                        | 51                     |                                              |
|              | 84                           |                               |                                |                        | 51                     |                                              |
| 14.0         |                              |                               |                                |                        | 51                     |                                              |
|              | 71                           |                               |                                |                        | 51                     |                                              |
|              | 38                           |                               | 510                            | 13.72                  | 51                     | Very dense, gray-brown gravelly coarse to    |
|              | 129                          | 23                            |                                | 14.33                  | 51                     | fine SAND, trace silt, with 10cm. layer      |
|              | 58                           |                               |                                |                        | 51                     | coarse sand, fine gravel                     |
|              | 43                           |                               |                                |                        | 51                     |                                              |
|              |                              |                               |                                |                        | 1.77                   |                                              |
|              |                              |                               |                                | 14.33                  |                        | Bottom of Exploration at 14.33m.             |

H&A

METRIC LOG

| South Portland, Maine                    |                       | TEST BORING LOG                      |                               |                                 |                                                                                                      | BORING NO. B28         |            |  |
|------------------------------------------|-----------------------|--------------------------------------|-------------------------------|---------------------------------|------------------------------------------------------------------------------------------------------|------------------------|------------|--|
| PROJECT: Kennebec River Crossing Project |                       |                                      |                               |                                 |                                                                                                      | FILE NO. 80765-000     |            |  |
| LOCATION: Augusta, Maine                 |                       |                                      |                               |                                 |                                                                                                      | SHEET NO. 1 OF 3       |            |  |
| CLIENT: Maine Dept. of Transportation    |                       | CONTRACTOR: Maine Test Borings, Inc. |                               |                                 |                                                                                                      | LOCATION Sta. 2+992.19 |            |  |
| ITEM                                     | CASING                | DRIVE SAMPLER                        | CORE BARREL                   | DRILLING EQUIPMENT & PROCEDURES |                                                                                                      | ELEVATION -0.52        |            |  |
| TYPE                                     | NW                    | SS                                   | NX                            | RIG TYPE CME 45                 |                                                                                                      | DATUM NGVD             |            |  |
| INSIDE DIAMETER (CM)                     | 8                     | 3                                    | S                             | BIT TYPE Roller bit             |                                                                                                      | START June 1, 2000     |            |  |
| HAMMER WEIGHT (KG)                       | 136                   | 64                                   | -                             | DRILL MUD                       |                                                                                                      | FINISH June 4, 2000    |            |  |
| HAMMER FALL (CM)                         | 61                    | 76                                   | -                             | OTHER -                         |                                                                                                      | DRILLER G. Lefortone   |            |  |
|                                          |                       |                                      |                               |                                 |                                                                                                      | HSA REP A. Mercadante  |            |  |
| DEPTH (M)                                | CASING BLOW PER 0.30M | SAMPLER BLOW NUMBER & REC. (CM)      | SAMPLE DEPTH (M)              | ELEV./DEPTH (M)                 | VISUAL DESCRIPTION AND REMARKS                                                                       |                        |            |  |
| 0.0                                      |                       |                                      |                               |                                 | No recovery; possibly pushing on boulder                                                             |                        |            |  |
|                                          | 28                    | 34                                   | 51                            | 0.00                            |                                                                                                      |                        |            |  |
|                                          |                       | 35                                   | 0                             | 0.61                            |                                                                                                      |                        |            |  |
|                                          | 62                    | 16                                   |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 70                    | 18                                   | 56                            | 0.61                            | Very dense, gray-brown gravelly coarse to fine SAND, trace silt                                      |                        |            |  |
| 1.0                                      | 43                    |                                      | 10                            | 1.22                            | -FLUVIAL DEPOSIT-                                                                                    |                        |            |  |
|                                          | 44                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 24                    | 5                                    | 53                            | 1.52                            | Medium dense, brown coarse to fine SAND, little fine gravel, trace silt, coarse gravel               |                        |            |  |
| 2.0                                      | 25                    | 9                                    | 25                            | 2.13                            |                                                                                                      |                        |            |  |
|                                          |                       | 10                                   |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 24                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
| 3.0                                      | 28                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 28                    | 9                                    | 54                            | 3.06                            | Medium dense, brown medium to fine SAND, little coarse sand, fine gravel, trace silt                 |                        |            |  |
|                                          |                       | 10                                   | 36                            | 3.66                            |                                                                                                      |                        |            |  |
|                                          | 14                    | 8                                    |                               |                                 |                                                                                                      |                        |            |  |
| 4.0                                      | 18                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 30                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 33                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 40                    | 9                                    | 55                            | 4.57                            | Medium dense, brown medium to fine SAND, little coarse sand, trace silt, frequent seams of fine sand |                        |            |  |
| 5.0                                      | 16                    | 10                                   | 36                            | 5.18                            |                                                                                                      |                        |            |  |
|                                          |                       | 8                                    |                               |                                 | -FLUVIAL DEPOSIT-                                                                                    |                        |            |  |
|                                          | 27                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 53                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
| 6.0                                      | 45                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 57                    | 13                                   | 56                            | 6.10                            | Medium dense, brown coarse to fine SAND, trace silt, fine gravel                                     |                        |            |  |
|                                          |                       | 10                                   | 25                            | 6.71                            |                                                                                                      |                        |            |  |
|                                          | 37                    | 8                                    |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 48                    | 12                                   |                               |                                 |                                                                                                      |                        |            |  |
| 7.0                                      |                       |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 55                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          | 67                    |                                      |                               |                                 |                                                                                                      |                        |            |  |
|                                          |                       |                                      |                               | -6.14                           |                                                                                                      |                        |            |  |
| WATER LEVEL DATA                         |                       |                                      |                               |                                 | SAMPLE IDENTIFICATION                                                                                |                        | SUMMARY    |  |
| DATE                                     | TIME                  | ELAPSED TIME (HR)                    | DEPTH (M) TO BOTTOM OF CASING | DEPTH (M) TO WATER              | 0                                                                                                    | OVERBURNED (M)         | 10.67      |  |
|                                          |                       |                                      |                               |                                 | 1 <th>ROCK CONED (M)</th> <th>2.93</th>                                                              | ROCK CONED (M)         | 2.93       |  |
|                                          |                       |                                      |                               |                                 | U <th>UNDISTURBED SAMPLE</th> <th>85.20</th>                                                         | UNDISTURBED SAMPLE     | 85.20      |  |
|                                          |                       |                                      |                               |                                 | S <th>SPLIT SPOON</th> <th></th>                                                                     | SPLIT SPOON            |            |  |
|                                          |                       |                                      |                               |                                 |                                                                                                      | HAA                    | METRIC LOG |  |

South Portland,

Maine

TEST BORING REPORT

BORING NO. 828

FILE NO. 80765-000

SHEET NO. 2 OF 3

| DEPTH<br>(M) | CASING<br>E.L.O.C.<br>PER D.30MFR 0.15M | SAMPLER<br>E.L.O.S.<br>REC.(CM) | SAMPLE<br>NUMBER | SAMPLE<br>DEPTH<br>(M) | ELEV./<br>DEPTH<br>(M) | VISUAL DESCRIPTION AND REMARKS                                                                                      |
|--------------|-----------------------------------------|---------------------------------|------------------|------------------------|------------------------|---------------------------------------------------------------------------------------------------------------------|
| 8.0          |                                         | 15                              | S7               | 7.62                   | 5.7                    | Very dense, gray medium to fine silty SAND,<br>little coarse sand, fine gravel, moderately<br>well bonded lit-silty |
|              |                                         | 14                              | Z5               | 8.23                   | 5.8                    |                                                                                                                     |
|              |                                         | 15                              |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    | -GLACIAL TILL-                                                                                                      |
|              |                                         |                                 |                  |                        | 5.9                    | Note: Spin casing to 9.08m.                                                                                         |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         | 94                              | S8               | 9.14                   | 5.7                    | Very dense, gray gravelly coarse to fine<br>SAND, little silt                                                       |
|              |                                         | 90                              | Z0               | 9.45                   | 5.8                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    | -GLACIAL TILL-                                                                                                      |
|              |                                         |                                 |                  |                        | 5.9                    | -10.82<br>Begin NX core at 10.42m.                                                                                  |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    |                                                                                                                     |
|              |                                         |                                 |                  |                        | 5.9                    | -11.15<br>Cored through cobble and Till                                                                             |
|              |                                         |                                 |                  |                        | 5.9                    | -11.15<br>Top of Bedrock at 10.67m.                                                                                 |
|              |                                         |                                 |                  |                        | 5.9                    | -10.67<br>See Core Logging Report                                                                                   |

H&A

METRIC LOG

# BORING LOGS

## AUGUSTA

State Route 3 over Kennebec River

Date: \$DATE\$

Filename: \$FILE\$.

|                         |       |
|-------------------------|-------|
| PROJECT DESIGN ENGINEER | DATE  |
| DESIGN-DETAILED         | 08/02 |
| CHECKED                 | 08/02 |
| REVISIONS               |       |
| FIELD CHANGES           |       |
| PLANS                   |       |

| South Portland, Maine |                            |               |                  |  |             | CORE BORING REPORT |                                |  |  |  |  | BORING NO. B28 |  | FILE NO. 80765-000 |  | SHEET NO. 3 OF 3 |  |
|-----------------------|----------------------------|---------------|------------------|--|-------------|--------------------|--------------------------------|--|--|--|--|----------------|--|--------------------|--|------------------|--|
| DEPTH (M)             | DRILLING RATE (MIN./0.30M) | RUN DEPTH (M) | RECOVERY/ROD M % |  | WEATH-ERING | ELEV./DEPTH (M)    | VISUAL DESCRIPTION AND REMARKS |  |  |  |  |                |  |                    |  |                  |  |
| 8.0                   |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
| 9.0                   |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
| 10.0                  |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
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|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |  |             |                    |                                |  |  |  |  |                |  |                    |  |                  |  |

| South Portland, Maine                                                      |  |                        |  |                         | TEST BORING REPORT |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                | BORING NO. B29          |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|----------------------------------------------------------------------------|--|------------------------|--|-------------------------|--------------------|----------------------------------------|--|---------------------------------|--|-----------------|--|-------------------------------------------------------------------------------------------------------------------|---------------------|----------------|-------------------------|-------|--|--|--|--|--|--|--|------------|--|--|--|--|--|--|--|--|--|--|--|
| PROJECT: Kennebec River Crossing Project                                   |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                | FILE NO. 80765-000      |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| LOCATION: Augusta, Maine                                                   |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                | SHEET NO. 1 OF 3        |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| CLIENT: Maine Dept. of Transportation CONTRACTOR: Maine Test Borings, Inc. |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                | LOCATION Sta. 2+692. 9L |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| ITEM                                                                       |  | CASING                 |  | DRIVE SAMPLER           |                    | CORE BARREL                            |  | DRILLING EQUIPMENT & PROCEDURES |  |                 |  |                                                                                                                   | ELEVATION           |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| TYPE                                                                       |  | NW                     |  | SS                      |                    | NX                                     |  | RIG TYPE CME 45                 |  |                 |  |                                                                                                                   | -0.82               |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| INSIDE DIAMETER (CM)                                                       |  | 8                      |  | 3                       |                    | 5                                      |  | BIT TYPE Roller bit             |  |                 |  |                                                                                                                   | DATUM NGVD          |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| HAMMER WEIGHT (KG)                                                         |  | 136                    |  | 64                      |                    | —                                      |  | DRILL MUD OTHER —               |  |                 |  |                                                                                                                   | START May 25, 2000  |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| HAMMER FALL (CM)                                                           |  | 61                     |  | 76                      |                    | —                                      |  |                                 |  |                 |  |                                                                                                                   | FINISH May 31, 2000 |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   | DRILLER G. Lidstone |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   | H&A REP M. Cronan   |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| DEPTH (M)                                                                  |  | CASING BLOWS PER 0.30M |  | SAMPLER BLOWS PER 0.15M |                    | SAMPLE NUMBER & REC. (CM)              |  | SAMPLE DEPTH (M)                |  | ELEV./DEPTH (M) |  | VISUAL DESCRIPTION AND REMARKS                                                                                    |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 0.0                                                                        |  | 48                     |  | 22                      |                    | 51                                     |  | 15                              |  | 0.00            |  | Very dense, gray-yellow-brown gravelly coarse to fine SAND, trace silt<br><br>—FLUVIAL DEPOSIT—                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 60                     |  | 42                      |                    | 58                                     |  | 0.61                            |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 49                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 15                     |  | 24                      |                    | NR                                     |  | 0.91                            |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 1.0                                                                        |  | 8                      |  | 14                      |                    | 0                                      |  | 1.52                            |  |                 |  | No recovery                                                                                                       |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 11                     |  | 11                      |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 2.0                                                                        |  | 16                     |  |                         |                    |                                        |  |                                 |  |                 |  | Medium dense, gray coarse to fine SAND, little fine gravel, trace silt                                            |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 7                      |  | 13                      |                    | 52                                     |  | 2.44                            |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 10                     |  | 20                      |                    | 3.05                                   |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 3.0                                                                        |  | 13                     |  | 12                      |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 24                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 32                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 28                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 4.0                                                                        |  | 8                      |  | 11                      |                    | 53                                     |  | 3.96                            |  |                 |  | Medium dense, gray coarse to fine SAND, little gravel, trace silt                                                 |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 12                     |  | 11                      |                    | 30                                     |  | 4.57                            |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 5.0                                                                        |  | 22                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 24                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 5                      |  | 8                       |                    | 54                                     |  | 5.49                            |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 16                     |  | 10                      |                    | 56                                     |  | 6.10                            |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 6.0                                                                        |  | 9                      |  | 17                      |                    |                                        |  |                                 |  |                 |  | Medium dense, gray-brown coarse to fine SAND, trace gravel, silt<br><br>—FLUVIAL DEPOSIT—                         |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 25                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 33                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 34                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| 7.0                                                                        |  | 17                     |  | 16                      |                    | 55                                     |  | 7.01                            |  |                 |  | Medium dense, gray-brown medium to fine SAND, trace coarse sand, silt, with frequent gray fine sand silt partings |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 16                     |  | 15                      |                    | 15                                     |  | 7.62                            |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  | 24                     |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  |                                                                                                                   |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
| WATER LEVEL DATA                                                           |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  | SAMPLE IDENTIFICATION                                                                                             |                     |                |                         |       |  |  |  |  |  |  |  | SUMMARY    |  |  |  |  |  |  |  |  |  |  |  |
| DATE                                                                       |  | TIME                   |  | ELAPSED TIME (HR)       |                    | DEPTH (M) TO: BOTTOM OF CASING OF HOLE |  | WATER                           |  | O               |  | OPEN END ROD                                                                                                      |                     | OVERBURDEN (M) |                         | 11.31 |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  | T               |  | THIN WALL TUBE                                                                                                    |                     | ROCK CORED (M) |                         | 2.74  |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  | S               |  | UNDISTURBED SAMPLE                                                                                                |                     | SAMPLES        |                         | 75.30 |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  | SPLIT SPOON                                                                                                       |                     |                |                         |       |  |  |  |  |  |  |  |            |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                                        |  |                                 |  |                 |  | H&A                                                                                                               |                     |                |                         |       |  |  |  |  |  |  |  | METRIC LOG |  |  |  |  |  |  |  |  |  |  |  |

| South Portland, Maine |                        |                         |                           |                  | TEST BORING REPORT |                                |  |  |  |  |  |  |  |  | BORING NO. B29 |  | FILE NO. 80765-000 |  | SHEET NO. 2 OF 3 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |    |
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| DEPTH (M)             | CASING BLOWS PER 0.30M | SAMPLER BLOWS PER 0.15M | SAMPLE NUMBER & REC. (CM) | SAMPLE DEPTH (M) | ELEV. / DEPTH (M)  | VISUAL DESCRIPTION AND REMARKS |  |  |  |  |  |  |  |  |                |  |                    |  |                  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |    |
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| South Portland, Maine |                            |               |                  |             |                 | CORE BORING REPORT             |  |  |  |  |  | BORING NO. B29 |  | FILE NO. 80765-000 |  | SHEET NO. 3 OF 3 |  |
|-----------------------|----------------------------|---------------|------------------|-------------|-----------------|--------------------------------|--|--|--|--|--|----------------|--|--------------------|--|------------------|--|
| DEPTH (M)             | DRILLING RATE (MIN./0.30M) | RUN DEPTH (M) | RECOVERY/ROD (M) | WEATH-ERING | ELEV./DEPTH (M) | VISUAL DESCRIPTION AND REMARKS |  |  |  |  |  |                |  |                    |  |                  |  |
| 9.0                   |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 10.0                  |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 11.0                  |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |             | -12.1           |                                |  |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |             | 11.3            |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 6                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 12.0                  |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 5                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 5                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 4                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 13.0                  |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 4                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 4                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 4                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 14.0                  |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
| 3                     |                            |               |                  |             |                 |                                |  |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |             | -15.3           |                                |  |  |  |  |  |                |  |                    |  |                  |  |
|                       |                            |               |                  |             | 14.5            |                                |  |  |  |  |  |                |  |                    |  |                  |  |
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| South Portland, Maine                                                      |  |                        |  |                         | TEST BORING REPORT |                           |  |                                 |  |                                                                                                                                                                                                                                         |  |                                |  |  | BORING NO. B30 |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
|----------------------------------------------------------------------------|--|------------------------|--|-------------------------|--------------------|---------------------------|--|---------------------------------|--|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--|--------------------------------|--|--|----------------|--|--|---------------------|--|--|--|--|--|--|--|--|--|--|--|
| PROJECT: Kennebec River Crossing Project                                   |  |                        |  |                         |                    |                           |  |                                 |  | FILE NO. 80765-000                                                                                                                                                                                                                      |  |                                |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
| LOCATION: Augusta, Maine                                                   |  |                        |  |                         |                    |                           |  |                                 |  | SHEET NO. 1 OF 2                                                                                                                                                                                                                        |  |                                |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
| CLIENT: Maine Dept. of Transportation CONTRACTOR: Maine Test Borings, Inc. |  |                        |  |                         |                    |                           |  |                                 |  | LOCATION Sta. 3+080.6R                                                                                                                                                                                                                  |  |                                |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
| ITEM                                                                       |  | CASING                 |  | DRIVE SAMPLER           |                    | CORE BARREL               |  | DRILLING EQUIPMENT & PROCEDURES |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  | ELEVATION -1.37     |  |  |  |  |  |  |  |  |  |  |  |
| TYPE                                                                       |  | NW                     |  | SS                      |                    | NX                        |  | RIG TYPE CME 45                 |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  | DATUM NGVD          |  |  |  |  |  |  |  |  |  |  |  |
| INSIDE DIAMETER (CM)                                                       |  | 8                      |  | 3                       |                    | 5                         |  | BIT TYPE Roller bit             |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  | START June 6, 2000  |  |  |  |  |  |  |  |  |  |  |  |
| HAMMER WEIGHT (KG)                                                         |  | 136                    |  | 64                      |                    | -                         |  | DRILL MUD OTHER -               |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  | FINISH June 7, 2000 |  |  |  |  |  |  |  |  |  |  |  |
| HAMMER FALL (CM)                                                           |  | 61                     |  | 76                      |                    | -                         |  |                                 |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  | DRILLER G. Rudnicki |  |  |  |  |  |  |  |  |  |  |  |
|                                                                            |  |                        |  |                         |                    |                           |  |                                 |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  | H&A REP B. Lawrence |  |  |  |  |  |  |  |  |  |  |  |
| DEPTH (M)                                                                  |  | CASING BLOWS PER 0.30M |  | SAMPLER BLOWS PER 0.15M |                    | SAMPLE NUMBER & REC. (CM) |  | SAMPLE DEPTH (M)                |  | ELEV./DEPTH (M)                                                                                                                                                                                                                         |  | VISUAL DESCRIPTION AND REMARKS |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
| 0.0                                                                        |  | 14 50/1                |  | 51 18                   |                    | 0.06 0.18                 |  | -1.43 0.06 -1.55 0.18           |  | GRAVEL<br>Very dense, gray fine sandy SILT, trace clay, trace gravel, little coarse to medium sand<br><br>-GLACIAL TILL-<br>Top of Bedrock at 0.18m.<br>Spn casing to 0.67m.<br>Advanced roller bit to 0.67m.<br>See Core Boring Report |  |                                |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
| 1.0                                                                        |  |                        |  |                         |                    |                           |  |                                 |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
| 2.0                                                                        |  |                        |  |                         |                    |                           |  |                                 |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
| 3.0                                                                        |  |                        |  |                         |                    |                           |  |                                 |  |                                                                                                                                                                                                                                         |  |                                |  |  |                |  |  |                     |  |  |  |  |  |  |  |  |  |  |  |
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Date: \$DATE\$

Filename: \$FILE\$.

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| PROJECT DESIGN ENGINEER | DATE  |
| CHECKED                 | 08/02 |
| DESIGN-DETAILED         | 08/02 |
| REVISIONS               |       |
| FIELD CHANGES           |       |

PLANS

| South Portland, Maine                                                                                                                              |                        | TEST BORING REPORT                |                           |                                                                                                                         |                 | BORING NO. B31A                                                      |  |
|----------------------------------------------------------------------------------------------------------------------------------------------------|------------------------|-----------------------------------|---------------------------|-------------------------------------------------------------------------------------------------------------------------|-----------------|----------------------------------------------------------------------|--|
| PROJECT: Kennebec River Crossing Project<br>LOCATION: Augusta, Maine<br>CLIENT: Maine Dept. of Transportation CONTRACTOR: Maine Test Borings, Inc. |                        |                                   |                           |                                                                                                                         |                 | FILE NO. 80765-000<br>SHEET NO. 1 OF 2<br>LOCATION Sta. 3+067.10L    |  |
| ITEM                                                                                                                                               | CASING                 | DRIVE SAMPLER                     | CORE BARREL               | DRILLING EQUIPMENT & PROCEDURES                                                                                         |                 |                                                                      |  |
| TYPE                                                                                                                                               | NW                     | SS                                | NX                        | RIG TYPE CME 45                                                                                                         |                 |                                                                      |  |
| INSIDE DIAMETER (CM)                                                                                                                               | 8                      | 3                                 | 5                         | BIT TYPE Roller bit                                                                                                     |                 |                                                                      |  |
| HAMMER WEIGHT (KG)                                                                                                                                 | 136                    | 64                                | -                         | DRILL MUD                                                                                                               |                 |                                                                      |  |
| HAMMER FALL (CM)                                                                                                                                   | 61                     | 76                                | -                         | OTHER -                                                                                                                 |                 |                                                                      |  |
|                                                                                                                                                    |                        |                                   |                           | ELEVATION -1.46<br>DATUM NGVD<br>START June 5, 2000<br>FINISH June 5, 2000<br>DRILLER G. Rudolph<br>HAA REP B. Lawrence |                 |                                                                      |  |
| DEPTH (M)                                                                                                                                          | CASING BLOWS PER 0.30M | SAMPLER BLOWS PER 0.15M REC. (CM) | SAMPLE NUMBER & REC. (CM) | SAMPLE DEPTH (M)                                                                                                        | ELEV./DEPTH (M) | VISUAL DESCRIPTION AND REMARKS                                       |  |
| 0.0                                                                                                                                                |                        |                                   |                           |                                                                                                                         |                 | See B31 Test Boring Report for overburden soil descriptions 0-3.04m. |  |
| 1.0                                                                                                                                                |                        |                                   |                           |                                                                                                                         |                 |                                                                      |  |
| 2.0                                                                                                                                                |                        |                                   |                           |                                                                                                                         |                 |                                                                      |  |
| 3.0                                                                                                                                                |                        |                                   |                           |                                                                                                                         |                 |                                                                      |  |
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| South Portland, Maine |                          | CORE BORING REPORT          |                          |                      |                    | BORING NO. B31A<br>FILE NO. 80765-000<br>SHEET NO. 2 OF 2 |                                                                                                                                                                                                                                                                   |
|-----------------------|--------------------------|-----------------------------|--------------------------|----------------------|--------------------|-----------------------------------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| DEPTH (M)             | DRILLING RATE MIN./0.30M | RUN DEPTH (M)               | RECOVERY/ROD M %         |                      | WEATH-ERING        | ELEV./DEPTH (M)                                           | VISUAL DESCRIPTION AND REMARKS                                                                                                                                                                                                                                    |
| 0.0                   |                          |                             |                          |                      |                    |                                                           |                                                                                                                                                                                                                                                                   |
| 1.0                   |                          |                             |                          |                      |                    |                                                           |                                                                                                                                                                                                                                                                   |
| 2.0                   |                          |                             |                          |                      |                    |                                                           |                                                                                                                                                                                                                                                                   |
| 3.0                   |                          |                             |                          |                      |                    |                                                           |                                                                                                                                                                                                                                                                   |
| 4.0                   | 2<br>1                   | C1 3.4<br>3.7<br>3.7<br>5.2 | 0.3<br>0.0<br>1.0<br>1.0 | 100<br>0<br>88<br>88 | Fr.-Sl.<br>Fr.-Sl. | -4.8<br>3.4                                               | Top of Bedrock at 3.38m.<br>Begin rock core.<br>C1: Hard to moderately hard, fresh to slightly weathered, fine grained to ophanitic muscovite schist. Primary joint set is high angle, moderately spaced, rough, undulating, discolored, tight.<br>C2: Same as C1 |
| 5.0                   | 1                        | C3 5.2<br>6.4               | 1.2<br>1.2               | 100<br>100           | Fr.-Sl.            |                                                           | C3: Hard to moderately hard, fresh to slightly weathered fine grained to ophanitic muscovite schist. Primary joint set is moderately dipping, close to moderately spaced, rough, undulating, fresh to slightly discolored, tight.                                 |
| 6.0                   | 1                        |                             |                          |                      |                    | -7.8<br>6.4                                               | Bottom of Exploration at 6.43m.                                                                                                                                                                                                                                   |

METRIC

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| South Portland, Maine                                                                                                                              |                        | TEST BORING REPORT                |                                        |                  |                                                                                                                                                                         | BORING NO. B31B                                                                            |                |
| PROJECT: Kennebec River Crossing Project<br>LOCATION: Augusta, Maine<br>CLIENT: Maine Dept. of Transportation CONTRACTOR: Maine Test Borings, Inc. |                        |                                   |                                        |                  |                                                                                                                                                                         | FILE NO. 80765-000<br>SHEET NO. 1 OF 1<br>LOCATION Sta. 3+067.10L                          |                |
| ITEM                                                                                                                                               |                        | CASING                            | DRIVE SAMPLER                          | CORE BARREL      | DRILLING EQUIPMENT & PROCEDURES                                                                                                                                         | ELEVATION -1.46                                                                            |                |
| TYPE                                                                                                                                               |                        | NW                                | SS                                     |                  | RIG TYPE CME 45                                                                                                                                                         | DATUM NGVD                                                                                 |                |
| INSIDE DIAMETER (CM)                                                                                                                               |                        | 8                                 | 3                                      |                  | BIT TYPE Roller bit                                                                                                                                                     | START June 5, 2000                                                                         |                |
| HAMMER WEIGHT (KG)                                                                                                                                 |                        | 136                               | 64                                     | -                | DRILL MUD                                                                                                                                                               | FINISH June 5, 2000                                                                        |                |
| HAMMER FALL (CM)                                                                                                                                   |                        | 61                                | 76                                     | -                | OTHER -                                                                                                                                                                 | DRILLER G. Rudolph<br>HAA REP B. Lawrence                                                  |                |
| DEPTH (M)                                                                                                                                          | CASING BLOWS PER 0.30M | SAMPLER BLOWS PER 0.15M REC. (CM) | SAMPLE NUMBER & REC. (CM)              | SAMPLE DEPTH (M) | ELEV./ DEPTH (M)                                                                                                                                                        | VISUAL DESCRIPTION AND REMARKS                                                             |                |
| 0.0                                                                                                                                                |                        | 1 51<br>2 25<br>10 10<br>10 5     | 0.00<br>0.61                           |                  |                                                                                                                                                                         | Medium dense, brown-gray medium to fine SAND, trace silt                                   |                |
| 1.0                                                                                                                                                |                        | 5 52<br>5 25<br>16 13             | 0.61<br>1.22                           |                  |                                                                                                                                                                         | Medium dense, brown-gray fine SAND, little silt, little gravel                             |                |
|                                                                                                                                                    |                        |                                   |                                        | -2.68<br>1.22    |                                                                                                                                                                         | Bottom of Exploration at 1.22m.<br>Notes These samples taken for chemical testing purposes |                |
| WATER LEVEL DATA                                                                                                                                   |                        | SAMPLE IDENTIFICATION             |                                        |                  | SUMMARY                                                                                                                                                                 |                                                                                            |                |
| DATE                                                                                                                                               | TIME                   | ELAPSED TIME (HR)                 | DEPTH (M) TO: BOTTOM OF CASING OF HOLE | WATER            | <input type="checkbox"/> OPEN END ROD<br><input type="checkbox"/> THIN WALL TUBE<br><input type="checkbox"/> UNDISTURBED SAMPLE<br><input type="checkbox"/> SPLIT SPOON | OVERBURDEN (M)<br>ROCK CORED (M)<br>SAMPLES                                                | 1.22<br><br>2S |
| HAA                                                                                                                                                |                        |                                   |                                        |                  |                                                                                                                                                                         | METRIC LOG                                                                                 |                |

| South Portland, Maine                                                      |                                            | TEST BORING REPORT               |                  |                  |                                                                                                           | BORING NO. B32     |                       |
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| PROJECT: Kennebec River Crossing Project                                   |                                            | LOCATION: Augusta, Maine         |                  |                  |                                                                                                           | FILE NO. 80765-000 |                       |
| CLIENT: Maine Dept. of Transportation CONTRACTOR: Maine Test Borings, Inc. |                                            |                                  |                  |                  |                                                                                                           | SHEET NO. 1 OF 2   |                       |
| LOCATION: Sta. 3+135.5L                                                    |                                            |                                  |                  |                  |                                                                                                           |                    |                       |
| ITEM                                                                       |                                            | CASING                           | DRIVE SAMPLER    | CORE BARREL      | DRILLING EQUIPMENT & PROCEDURES                                                                           |                    | ELEVATION 7.13        |
| TYPE                                                                       |                                            | NW                               | SS               | NX               | RIG TYPE CME 45                                                                                           |                    | DATUM NGVD            |
| INSIDE DIAMETER (CM)                                                       |                                            | 8                                | 3                | 5                | BIT TYPE Roller bit                                                                                       |                    | START May 18, 2000    |
| HAMMER WEIGHT (KG)                                                         |                                            | 136                              | 64               | -                | DRILL MUD                                                                                                 |                    | FINISH May 19, 2000   |
| HAMMER FALL (CM)                                                           |                                            | 61                               | 76               | -                | OTHER - Solid stem auger                                                                                  |                    | DRILLER G. Lidenstone |
|                                                                            |                                            |                                  |                  |                  | HAA REP M. Cronan                                                                                         |                    |                       |
| DEPTH (M)                                                                  | CASING BLOWS PER 0.30M PER 0.15M REC. (CM) | SAMPLER BLOWS NUMBER & REC. (CM) | SAMPLE DEPTH (M) | ELEV./ DEPTH (M) | VISUAL DESCRIPTION AND REMARKS                                                                            |                    |                       |
| 0.0                                                                        |                                            | 1 51 0.00                        |                  |                  | Very loose, to loose, gray-brown medium to fine SAND, trace silt, organics, with frequent silt seams      |                    |                       |
|                                                                            |                                            | 2 20 0.61                        |                  |                  |                                                                                                           |                    |                       |
|                                                                            |                                            | 3 2                              |                  |                  | -ALLUVIAL DEPOSIT-                                                                                        |                    |                       |
|                                                                            |                                            | 4 4                              |                  |                  |                                                                                                           |                    |                       |
| 1.0                                                                        |                                            |                                  |                  |                  | Loose, gray-brown medium to fine SAND, trace silt with occasional silt partings                           |                    |                       |
|                                                                            |                                            | 5 3 52 1.52                      |                  |                  |                                                                                                           |                    |                       |
|                                                                            |                                            | 4 4 53 2.13                      |                  |                  |                                                                                                           |                    |                       |
|                                                                            |                                            | 5 6                              |                  |                  |                                                                                                           |                    |                       |
| 2.0                                                                        |                                            |                                  |                  |                  |                                                                                                           |                    |                       |
|                                                                            |                                            | 12 12                            |                  |                  |                                                                                                           |                    |                       |
|                                                                            |                                            | 13 12                            |                  |                  |                                                                                                           |                    |                       |
|                                                                            |                                            | 14 12                            |                  |                  |                                                                                                           |                    |                       |
| 3.0                                                                        |                                            |                                  |                  |                  | Medium dense, gray-brown coarse to fine SAND, trace gravel, silt with occasional fine sandy silt partings |                    |                       |
|                                                                            |                                            | 7 53 3.05                        | 3.93             |                  |                                                                                                           |                    |                       |
|                                                                            | 50/.03                                     | 15 3.23                          | 3.20             |                  | Top of bedrock at 3.20m.<br>Roller bit to 3.47m.<br>See Core Boring Report                                |                    |                       |
|                                                                            |                                            |                                  |                  |                  |                                                                                                           |                    |                       |
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| South Portland, Maine |                              |               |                  |            | CORE BORING REPORT |                  | BORING NO. B32<br>FILE NO. 80765-000<br>SHEET NO. 2 OF 2                                                                                                                                                                                                                                                                 |  |
|-----------------------|------------------------------|---------------|------------------|------------|--------------------|------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--|
| DEPTH (M)             | DRILLING RATE MIN./0.30M (M) | RUN DEPTH (M) | RECOVERY/ROD M % |            | WEATH-ERING        | ELEV./ DEPTH (M) | VISUAL DESCRIPTION AND REMARKS                                                                                                                                                                                                                                                                                           |  |
| 2.0                   |                              |               |                  |            |                    |                  |                                                                                                                                                                                                                                                                                                                          |  |
| 3.0                   |                              |               |                  |            |                    |                  |                                                                                                                                                                                                                                                                                                                          |  |
| 4                     | C1                           | 3.5<br>5.0    | 1.5<br>1.5       | 100<br>97  | Sl.                | 3.7<br>3.5       | Top of Bedrock at 3.20m.<br>Advanced roller bit to 3.47m.<br>Begin NX rock core.<br>C1s Hard, slightly to moderately weathered, gray-white fine grained SCHIST. Foliation dipping at high angles, extremely close. Primary joint set dipping at low and high angles, close to moderate, undulating, rough, fresh, tight. |  |
| 4.0                   | 5                            |               |                  |            |                    |                  |                                                                                                                                                                                                                                                                                                                          |  |
|                       | 5                            |               |                  |            |                    |                  |                                                                                                                                                                                                                                                                                                                          |  |
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| 5.0                   | 6                            | C2            | 5.0<br>6.5       | 1.5<br>1.5 | 100<br>100         | Fr.              | C2: Hard, fresh, gray-white fine grained SCHIST. Foliation dipping at high angles, extremely close. Primary joint set dipping at low to high angles, close to moderate, undulating, rough, discolored, orange-brown, tight to open.                                                                                      |  |
|                       | 5                            |               |                  |            |                    |                  |                                                                                                                                                                                                                                                                                                                          |  |
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| 6.0                   | 5                            |               |                  |            |                    |                  |                                                                                                                                                                                                                                                                                                                          |  |
|                       | 5                            |               |                  |            |                    |                  |                                                                                                                                                                                                                                                                                                                          |  |
|                       |                              |               |                  |            |                    | 0.6<br>6.5       | Bottom of Exploration at 6.52m.                                                                                                                                                                                                                                                                                          |  |
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| South Portland, Maine                                                                                                                              |                        | TEST BORING REPORT                |                           |                  |                                                                     | BORING NO. B33A                                                                                            |                    |                |       |
|----------------------------------------------------------------------------------------------------------------------------------------------------|------------------------|-----------------------------------|---------------------------|------------------|---------------------------------------------------------------------|------------------------------------------------------------------------------------------------------------|--------------------|----------------|-------|
| PROJECT: Kennebec River Crossing Project<br>LOCATION: Augusta, Maine<br>CLIENT: Maine Dept. of Transportation CONTRACTOR: Maine Test Borings, Inc. |                        |                                   |                           |                  |                                                                     | FILE NO. 80765-000<br>SHEET NO. 1 OF 3<br>LOCATION Sta. 3+225.3R                                           |                    |                |       |
| ITEM                                                                                                                                               |                        | CASING                            | DRIVE SAMPLER             | CORE BARREL      | DRILLING EQUIPMENT & PROCEDURES                                     |                                                                                                            |                    |                |       |
| TYPE                                                                                                                                               |                        | NW                                | SS                        | NX               | RIG TYPE Mobil B33 Bombardier                                       | ELEVATION 26.28                                                                                            |                    |                |       |
| INSIDE DIAMETER (CM)                                                                                                                               |                        | 10                                | 3                         | 5                | BIT TYPE Roller bit                                                 | DATUM NGVD                                                                                                 |                    |                |       |
| HAMMER WEIGHT (KG)                                                                                                                                 |                        | 136                               | 64                        | -                | DRILL MUD                                                           | START May 4, 2000                                                                                          |                    |                |       |
| HAMMER FALL (CM)                                                                                                                                   |                        | 61                                | 76                        | -                | OTHER - Solid stem auger<br>0-1.52m: HW casing from<br>3.05-10.45m. | FINISH May 4, 2000<br>DRILLER M. Porter<br>HAA REP S. Wierosinski                                          |                    |                |       |
| DEPTH (M)                                                                                                                                          | CASING BLOWS PER 0.30M | SAMPLER BLOWS PER 0.15M REC. (CM) | SAMPLE NUMBER & REC. (CM) | SAMPLE DEPTH (M) | ELEV./DEPTH (M)                                                     | VISUAL DESCRIPTION AND REMARKS                                                                             |                    |                |       |
| 0.0                                                                                                                                                |                        |                                   |                           |                  |                                                                     | Brown, silty SAND, trace gravel, cobbles and boulders or surface                                           |                    |                |       |
| 1.0                                                                                                                                                |                        | NR                                | 51<br>13                  | 0.61<br>0.98     |                                                                     | -FILL-<br>Loose, brown coarse to fine sandy SILT, trace gravel, organics coarse gravel lodged in spoon tip |                    |                |       |
| 2.0                                                                                                                                                |                        |                                   | 52<br>38                  | 1.52<br>2.13     |                                                                     | Medium dense, gray-brown fine sandy SILT, trace coarse to medium sand, gravel                              |                    |                |       |
| 3.0                                                                                                                                                |                        |                                   |                           |                  |                                                                     | -FILL-                                                                                                     |                    |                |       |
| 4.0                                                                                                                                                |                        |                                   |                           |                  |                                                                     | -WOOD CHIPS-                                                                                               |                    |                |       |
| 5.0                                                                                                                                                |                        | 8<br>4<br>5<br>7                  | NR<br>0                   | 3.05<br>3.66     | 23.23<br>3.05                                                       | No recovery                                                                                                |                    |                |       |
| 6.0                                                                                                                                                |                        |                                   |                           |                  |                                                                     | Dark brown WOOD CHIPS<br>Cored through probable boulders 4.88-6.10m.                                       |                    |                |       |
| 7.0                                                                                                                                                |                        |                                   |                           |                  |                                                                     | Dark brown WOOD CHIPS<br>-WOOD CHIPS-                                                                      |                    |                |       |
| 14                                                                                                                                                 |                        |                                   |                           |                  | 15.03                                                               | Drilling action suggests gravelly material                                                                 |                    |                |       |
| 17                                                                                                                                                 |                        |                                   |                           |                  | 7.25                                                                |                                                                                                            |                    |                |       |
|                                                                                                                                                    |                        |                                   |                           |                  | 18.66                                                               |                                                                                                            |                    |                |       |
| WATER LEVEL DATA                                                                                                                                   |                        |                                   |                           |                  |                                                                     | SAMPLE IDENTIFICATION                                                                                      |                    | SUMMARY        |       |
| DATE                                                                                                                                               | TIME                   | ELAPSED TIME (HR)                 | BOTTOM OF CASING OF HOLE  | BOTTOM OF HOLE   | WATER                                                               | O                                                                                                          | OPEN END ROD       | OVERBURDEN (M) | 10.45 |
| 5/4/00                                                                                                                                             | 1745                   | 0.25                              | 10.30                     | 13.50            | 6.34                                                                | T <th>THIN WALL TUBE</th> <th>ROCK CORED (M)</th> <th>3.05</th>                                            | THIN WALL TUBE     | ROCK CORED (M) | 3.05  |
| 5/5/00                                                                                                                                             | 0600                   | 12.0                              | 10.30                     | 13.50            | 9.57                                                                | U <th>UNDISTURBED SAMPLE</th> <th>SAMPLES</th> <th>65.2C</th>                                              | UNDISTURBED SAMPLE | SAMPLES        | 65.2C |
| 5/5/00                                                                                                                                             | 0630                   | 12.5                              |                           | 11.40            | 7.99                                                                | S <th>SPLIT SPOON</th> <td></td> <td></td>                                                                 | SPLIT SPOON        |                |       |
|                                                                                                                                                    |                        |                                   |                           |                  |                                                                     | HAA                                                                                                        |                    | METRIC LOG     |       |

| South Portland, Maine |                        |                         |                           | TEST BORING REPORT |                 | BORING NO. B33A<br>FILE NO. 80765-000<br>SHEET NO. 2 OF 3               |  |
|-----------------------|------------------------|-------------------------|---------------------------|--------------------|-----------------|-------------------------------------------------------------------------|--|
| DEPTH (M)             | CASING BLOWS PER 0.30M | SAMPLER BLOWS PER 0.15M | SAMPLE NUMBER & REC. (CM) | SAMPLE DEPTH (M)   | ELEV./DEPTH (M) | VISUAL DESCRIPTION AND REMARKS                                          |  |
| 8.0                   | 18                     | 6                       | SSA/SB                    | 7.62               | 7.82            | Medium dense, gray fine sandy SILT                                      |  |
|                       | 5                      | 5                       | SB                        | 8.14               | 8.14            |                                                                         |  |
|                       | 36                     |                         |                           |                    | 18.21           | -GLACIAL TILL-                                                          |  |
|                       | 50/2                   |                         |                           |                    | 8.08            | Very dense, brown-gray mottled coarse to fine sandy SILT, little gravel |  |
| 9.0                   | 85                     |                         |                           |                    | 8.19            |                                                                         |  |
|                       | 86                     |                         |                           |                    | 8.29            |                                                                         |  |
|                       |                        |                         |                           |                    | 8.39            |                                                                         |  |
|                       | 180                    |                         |                           |                    | 17.11           |                                                                         |  |
| 10.0                  | 165                    |                         |                           |                    | 9.17            | Core run recovery 9.17-9.88m. Cobbles and gravel                        |  |
|                       | 165                    |                         |                           |                    | 9.17            |                                                                         |  |
|                       | 77                     | 24                      | SSA/SB                    | 9.88               | 16.41           | Dense, brown silty GRAVEL                                               |  |
|                       | 107/3                  | 20                      | 46                        | 10.30              | 9.88            |                                                                         |  |
|                       | 50/4                   |                         |                           |                    | 16.13           |                                                                         |  |
|                       |                        |                         |                           |                    | 16.15           | Very dense, gray-green-brown silty fine SAND                            |  |
|                       |                        |                         |                           |                    | 16.23           | (color changes exist in fine partings)                                  |  |
|                       |                        |                         |                           |                    | 10.45           | Top of Bedrock at 10.45m.<br>See Core Boring Report                     |  |

HAA

METRIC LOG

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

# BORING LOGS

AUGUSTA

State Route 3 over Kennebec River

Filename: \$FILE\$

| PROJECT DESIGN ENGINEER | BKL | BY   | DATE  |
|-------------------------|-----|------|-------|
| DESIGN-DETAILED         | BKL | ML/S | 08/02 |
| CHECKED                 | JWW |      | 08/02 |
| REVISONS                |     |      |       |
| FIELD CHANGES           |     |      |       |

| South Portland,<br>Maine |                                    |                     |                     |     |                 | CORE BORING REPORT     |                                                                                                                                                                                                                                                                                 | BORING NO. B33A<br>FILE NO. 80785-000<br>SHEET NO. 3 OF 5 |  |
|--------------------------|------------------------------------|---------------------|---------------------|-----|-----------------|------------------------|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-----------------------------------------------------------|--|
| DEPTH<br>(M)             | DRILLING<br>RATE<br>MIN./0.30MINO. | RUN<br>DEPTH<br>(M) | RECOVERY/RSD<br>M % |     | WEATH-<br>ERING | ELEV./<br>DEPTH<br>(M) | VISUAL DESCRIPTION<br>AND REMARKS                                                                                                                                                                                                                                               |                                                           |  |
| 9.0                      |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 10.0                     |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 2                        | C1                                 | 10.5                | 1.4                 | 92  | Fr.-Sl.         | 15.8                   | Top of Bedrock at 10.45m.<br>Begin NX rock core                                                                                                                                                                                                                                 |                                                           |  |
| 2                        |                                    | 12.0                | 1.1                 | 75  |                 | 10.5                   | C1: Medium hard, fresh to slightly weathered, gray<br>fine grained to granitic, massive SLIST with<br>extremely thin foliations throughout. Primary joint<br>set is moderately dipping, close to moderately<br>close, rough, stepped and planar, fresh to<br>discolored, tight. |                                                           |  |
| 11.0                     |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 2                        |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 2                        |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 3                        |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 12.0                     |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 2                        | C2                                 | 12.0                | 1.6                 | 100 | Fr.-Sl.         |                        | C2: Same as C1                                                                                                                                                                                                                                                                  |                                                           |  |
| 3                        |                                    | 13.5                | 1.1                 | 74  |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 2                        |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 5                        |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 13.0                     |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
| 3                        |                                    |                     |                     |     |                 |                        |                                                                                                                                                                                                                                                                                 |                                                           |  |
|                          |                                    |                     |                     |     |                 | 12.8                   |                                                                                                                                                                                                                                                                                 |                                                           |  |
|                          |                                    |                     |                     |     |                 | 13.5                   | Bottom of Exploration at 13.50m.                                                                                                                                                                                                                                                |                                                           |  |

METRIC

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-00556(22)E  | 15           | 105             |

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

# BORING LOGS

AUGUSTA

State Route 3 over Kennebec River



| PROJECT DESIGN ENGINEER |  | BKL | BY    | DATE  |
|-------------------------|--|-----|-------|-------|
| DESIGN-DETAILED         |  | BKL | M.L.S | 06/02 |
| CHECKED                 |  | JWW |       | 06/02 |
| REVISIONS               |  |     |       |       |
| FIELD CHANGES           |  |     |       |       |

**Haley & Aldrich, Inc.**  
 Soil/Bolt/Excavation Log  
 METRIC UNITS

Project: Kershaw River Bridge, Reinstating  
 Location: Augusta, Maine  
 File No. 82765-004

Boring No.: **RW-8-02**  
 PIN: \_\_\_\_\_

|                                            |                        |                                                        |                         |                                         |                         |
|--------------------------------------------|------------------------|--------------------------------------------------------|-------------------------|-----------------------------------------|-------------------------|
| Driller:                                   | G. Rudnicki            | Elevation (m):                                         | 14.17                   | Auger ID/OD:                            | Auger ID/OD Split Spoon |
| Operator:                                  | Maline Teast Borlino   | Depth:                                                 | N/D/O                   |                                         | Sampler:                |
| Logged by:                                 | B. Lawrence            | Rip Type:                                              | Mod/Tie B52 Sandbarrier |                                         | Humus Wt./Roll:         |
| Date Start/Finish:                         | 6/2/02 / 8/2/02        | Drilling Method:                                       | SSA                     |                                         | Cone Barrel:            |
| Boring Location:                           | Station 13 + 169, 10 R | Coating ID/OD:                                         |                         |                                         | Water Level/H:          |
| Definitions:                               |                        | Soil Moisture:                                         |                         | Soil Moisture:                          |                         |
| S = Split Spoon Sample                     |                        | S <sub>u</sub> = Netto Field Vane Shear Strength (kPa) |                         | W <sub>c</sub> = water content, percent |                         |
| SS = (Standard) Split Spoon Sample attempt |                        | T <sub>v</sub> = Plastic Vane Shear Strength (kPa)     |                         | L = Liquid Limit                        |                         |
| S <sub>1</sub> = Split Spoon Sample        |                        | Q <sub>u</sub> = Unconfined Compressive Strength (kPa) |                         | P <sub>L</sub> = Plasticity Limit       |                         |
| S <sub>2</sub> = Rock Core Sample          |                        | T <sub>90</sub> = 100 Vane Shear Strength (kPa)        |                         | PI = Plasticity Index                   |                         |
| S <sub>3</sub> = In-situ Vane Shear Test   |                        | W <sub>u</sub> = weight of air to water                |                         | S = Grain Size Analysis                 |                         |
| S <sub>4</sub> = Solid Vane Auger          |                        | W <sub>200</sub> = weight of no. 200                   |                         | C = Consolidation Test                  |                         |

| Sample Information |            |                  |                  |                |              |              | Visual Description and Remarks                                      | Laboratory Testing Results/ASTM and Unified Classification |
|--------------------|------------|------------------|------------------|----------------|--------------|--------------|---------------------------------------------------------------------|------------------------------------------------------------|
| Depth (m)          | Sample No. | Penetration (cm) | Sample Depth (m) | Soil Size (mm) | Interval (m) | Coating (mm) |                                                                     |                                                            |
| 0                  | D1         | 61.0/40.6        | 0.00 - 0.61      | 1/1/1/1        | 2            | 14.11        | Very loose dark brown silty fine SAND, roots, moist (SM)            | -TOPSOIL-<br>0.06                                          |
|                    | D2         | 61.0/30.8        | 0.61 - 1.22      | 2/2/2/2        | 4            |              | Very loose reddish-brown silty fine SAND, trace roots, damp (SM)    |                                                            |
|                    |            |                  |                  |                |              |              | -ALLUVIUM DEPOSIT-                                                  |                                                            |
| 1                  | D3         | 61.0/30.8        | 1.52 - 1.52      | 50/0           | 50           | 12.64        | Very loose reddish-brown silty fine SAND, trace roots, damp (SM)    | 1.52                                                       |
| 2                  |            |                  |                  |                |              |              | weathered BEDROCK (Loaf)                                            |                                                            |
| 3                  |            |                  |                  |                |              | 11.18        | Bottom of Excavation at 2.93 m below ground surface. Auger refusal! | 2.93                                                       |
| 4                  |            |                  |                  |                |              |              |                                                                     |                                                            |
| 5                  |            |                  |                  |                |              |              |                                                                     |                                                            |
| 6                  |            |                  |                  |                |              |              |                                                                     |                                                            |
| 7                  |            |                  |                  |                |              |              |                                                                     |                                                            |

Remarks:

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.

\* Water level readings have been made at 15 min and 45 min after insertion. Intermediate fluctuations may occur due to conditions other than those present at the time measurements were made.

Page 1 of 1

Boring No.: **RW-8-02**

Haley & Aldrich, Inc.

Sol/Walk Exploration Log

METRIC UNITS

Project: Kames River Bridge, Retaining Wall

Location: Augusta, Maine

File No. 8226-004

Boring No.: RW-9-02 (P)

PIN: \_\_\_\_\_

|                                        |                                |                        |
|----------------------------------------|--------------------------------|------------------------|
| Drillers: G. Rudolph                   | Elevation (m): 13.43           | Auger 10/00            |
| Operator: Melina Test Borings          | Soiltype: NND0                 | Sampler: _____         |
| Logged By: B. Lawrence                 | Wig Type: Melina B32 Subsoiler | Humus %/Falls: _____   |
| Date Start/Finish: 8/2/02 / 8/2/02     | Drilling Method: SSA           | Cone Barrel: _____     |
| Boring Location: Station 13 + 173.20 R | Coating 10/00                  | Water Level: _____ Dry |

Soil/Type:

S = Split-Spoon Sample

SB = Standard Split-Spoon Sample

U = Thin Wall Tube Sample

P = Push Core Sample

T = Jacking Core Sample

SA = Split-Spoon Auger

Soil/Type:

Su = Split Field Vane Shear Strength (kPa)

Tu = Pocket Torque Vane Shear Strength (kPa)

Su = Unconfined Compressive Strength (kPa)

Su = Lab Vane Shear Strength (kPa)

Wt = weight of 1 kg hammer

Wt = weight of 1 kg hammer

Soil/Type:

S = Split-Spoon Sample

SB = Standard Split-Spoon Sample

U = Thin Wall Tube Sample

P = Push Core Sample

T = Jacking Core Sample

SA = Split-Spoon Auger

Soil/Type:

Su = Split Field Vane Shear Strength (kPa)

Tu = Pocket Torque Vane Shear Strength (kPa)

Su = Unconfined Compressive Strength (kPa)

Su = Lab Vane Shear Strength (kPa)

Wt = weight of 1 kg hammer

Wt = weight of 1 kg hammer

| Depth (m) | Sample Information |                   |                  |                    |                    | Visual Description and Remarks                        | Laboratory Testing Results/ASTM and Unified Class. |
|-----------|--------------------|-------------------|------------------|--------------------|--------------------|-------------------------------------------------------|----------------------------------------------------|
|           | Sample No.         | Penetration (kPa) | Sample Depth (m) | Soil Type (150 mm) | Soil Type (150 mm) |                                                       |                                                    |
| 0         |                    |                   |                  |                    |                    | Auger cuttings Indioles                               |                                                    |
| 1         |                    |                   |                  |                    |                    | Reddish-brown fine SAND, some silt, damp              |                                                    |
| 2         |                    |                   |                  |                    |                    | Auger cuttings Indioles gravelly soil                 | 2.10                                               |
| 3         |                    |                   |                  |                    |                    | PROBABLY GLACIOFLUVIAL DEPOSIT                        |                                                    |
| 4         |                    |                   |                  |                    |                    | Weathered BEDROCK (hard)                              | 3.69                                               |
| 5         |                    |                   |                  |                    |                    | Bottom of Exploration at 3.61 m below ground surface. |                                                    |
| 6         |                    |                   |                  |                    |                    | Auger "retail"                                        |                                                    |
| 7         |                    |                   |                  |                    |                    |                                                       |                                                    |

Remarks:

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.

\* Water level readings were based on 1 ft and 6 inch water conditions entered. Transducer fluctuations may occur due to conditions other than those present on the 100 mm water level test.

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Boring No.: RW-9-02 (P)

| <b>Haley &amp; Aldrich, Inc.</b><br>Soil/Borehole Exploration Log<br>METRIC UNITS                                                                                                                                          |            |                                                                                                                                                                                                                                                                                                                               |                  | Project: Karambo River Bridge - Retaining Wall<br>Location: Augusta, Maine<br>File No. 80785-004                                                                                   |                                 |                      |                  | Boring No.: <u>          </u><br>RW-10-02                                                       |                                                  |
|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------|------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------|----------------------|------------------|-------------------------------------------------------------------------------------------------|--------------------------------------------------|
| Driller: G. Rudnicki                                                                                                                                                                                                       |            | Elevation (m): 13.61                                                                                                                                                                                                                                                                                                          | Auger ID/OD:     |                                                                                                                                                                                    | Age: /ID/OD                     |                      |                  |                                                                                                 |                                                  |
| Operator: Melina Teet Borjine                                                                                                                                                                                              |            | Datum: NVD                                                                                                                                                                                                                                                                                                                    |                  |                                                                                                                                                                                    | Sampler: 3.5 m Split Spoon      |                      |                  |                                                                                                 |                                                  |
| Logged By: G. Lawrence                                                                                                                                                                                                     |            | Pig Type: Metric BS Standard                                                                                                                                                                                                                                                                                                  |                  |                                                                                                                                                                                    | Water Wt./Fall: 523 N / 76.5 cm |                      |                  |                                                                                                 |                                                  |
| Date Start/Finish: 8/2/05 / 8/2/05                                                                                                                                                                                         |            | Drilling Method: SSA                                                                                                                                                                                                                                                                                                          |                  |                                                                                                                                                                                    | Core Barrel: 1                  |                      |                  |                                                                                                 |                                                  |
| Boring Location: Station 13 + 176.29 R                                                                                                                                                                                     |            | Casing ID/OD:                                                                                                                                                                                                                                                                                                                 |                  |                                                                                                                                                                                    | Water Level: % Dry              |                      |                  |                                                                                                 |                                                  |
| Definitions:<br>S = Split Stem Sample<br>M = Manufacturer's Split Stem Sample atrest<br>T = Thin Wall Tube Sample<br>A = Rock Core Sample<br>S <sub>1</sub> = Single Stem Shear Test<br>SS <sub>1</sub> = Split Stem Auger |            | Definitions:<br>S <sub>p</sub> = Static Field Test Shear Strength (kPa)<br>S <sub>r</sub> = Robot Triaxial Shear Strength (kPa)<br>C <sub>s</sub> = Unconfined Compression Strength (kPa)<br>S <sub>u</sub> (L) = Lab Vane Shear Strength (kPa)<br>W <sub>t</sub> = Weight of Wet Soil<br>W <sub>d</sub> = Weight of Dry Soil |                  | Definitions:<br>BC = water content, percent<br>L = Liquid Limit<br>P <sub>L</sub> = Plasticity Index<br>PI = Plasticity Zone<br>S = grain size Analysis<br>S = Classification Test |                                 |                      |                  |                                                                                                 |                                                  |
| Sample Information                                                                                                                                                                                                         |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 |                      |                  |                                                                                                 |                                                  |
| Depth (m)                                                                                                                                                                                                                  | Sample No. | Penetration (cm)                                                                                                                                                                                                                                                                                                              | Sample Depth (m) | Notes (100 feet or less depth or 300 ft.)                                                                                                                                          | Interval (m)                    | Casing Diameter (in) | Penetration (lb) | Visual Description and Remarks                                                                  | Laboratory Testing Results/AASHTO Unified Class. |
| 0                                                                                                                                                                                                                          | D1         | 61.0/50.8                                                                                                                                                                                                                                                                                                                     | 0.00 - 0.61      | 2/1/2/2                                                                                                                                                                            | 3                               | 13.53                |                  | Very loose dark brown silty fine SAND, roots, moist (SM)                                        | -TOPSOIL-                                        |
| 1                                                                                                                                                                                                                          |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 |                      |                  | Very loose reddish-brown fine SAND, some silt, damp (SM)                                        | -0.09                                            |
| 2                                                                                                                                                                                                                          | D2         | 61.0/50.8                                                                                                                                                                                                                                                                                                                     | 0.52 - 2.13      | 3/2/1/7                                                                                                                                                                            | 7                               |                      |                  | Loose light brown fine SAND, little silt, trace medium sand, moist (SM)                         |                                                  |
| 3                                                                                                                                                                                                                          |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 | 11.23                |                  | Moist less fines and slightly larger particles sizes in spoon below 5.0 m<br>-ALLUVIAL DEPOSIT- | -2.38                                            |
| 4                                                                                                                                                                                                                          |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 |                      |                  | Dense reddish-brown coarse to fine SAND, little gravel, trace silt, wet (SP)                    |                                                  |
| 5                                                                                                                                                                                                                          | D3         | 61.0/61.0                                                                                                                                                                                                                                                                                                                     | 3.06 - 5.66      | 19/25/24/23                                                                                                                                                                        | 45                              |                      |                  | -GLACIAL FLUVIAL DEPOSIT-                                                                       |                                                  |
| 6                                                                                                                                                                                                                          |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 | 9.49                 |                  | Very dense red-gray mortared coarse to fine sand GRAVEL, trace silt, wet (GP)                   | -4.11                                            |
| 7                                                                                                                                                                                                                          | D4         | 48.7/45.7                                                                                                                                                                                                                                                                                                                     | 4.57 - 6.03      | 36/35/36                                                                                                                                                                           | 140                             |                      |                  | -GLACIAL TILL DEPOSIT-                                                                          |                                                  |
| 8                                                                                                                                                                                                                          |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 | 8.67                 |                  | Weathered BEDROCK (hard)                                                                        | -4.94                                            |
| 9                                                                                                                                                                                                                          |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 |                      |                  | Bottom of Exploration at 6.12 m below ground surface.<br>Auger Refused!                         | -5.12                                            |
| Remarks:                                                                                                                                                                                                                   |            |                                                                                                                                                                                                                                                                                                                               |                  |                                                                                                                                                                                    |                                 |                      |                  |                                                                                                 |                                                  |

Stratification lines represent approximate boundaries between soil types; transitions may be gradual.  
  
 \* Water level readings have been made at 15 min and color readings are noted. Drunker's fluctuations may occur due to conditions after that time interval or 15 min and/or weather work done.

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 Boring No.: RW-10-02

| Haley & Aldrich, Inc.                              |  |                                                         |  | Project: Keweenaw River Bridge, Retaining Wall |  | Boring No.: RW-11-02 |  |
|----------------------------------------------------|--|---------------------------------------------------------|--|------------------------------------------------|--|----------------------|--|
| Soil/Soak Exploration Log                          |  |                                                         |  | Location: Aqueduct, Maine                      |  | P.I.N.: _____        |  |
| METRIC UNITS                                       |  |                                                         |  | File No. 80785-004                             |  |                      |  |
| Driller: G. Rudnicki                               |  | Elevation (m): 13.77                                    |  | Auger 10/08                                    |  |                      |  |
| Operator: Melvin Test Borings                      |  | Datum: NAD83                                            |  | Soils: 1                                       |  |                      |  |
| Logged By: B. Lawrence                             |  | Rig Type: Machine B33 Sanderizer                        |  | Header: Wt./Feet/1                             |  |                      |  |
| Date started/finish: 8/2/02 / 8/2/02               |  | Drilling Method: SSA                                    |  | Core barrel:                                   |  |                      |  |
| Boring Location: Station 13 + 179.39 R             |  | Casing 10/08                                            |  | Water Level: 0                                 |  | Dry                  |  |
| Soils/Notes:                                       |  | Soils/Notes:                                            |  | Soils/Notes:                                   |  |                      |  |
| a = Soil 1/2 Spoon Sample                          |  | a <sub>1</sub> = Field Field Vane Shear Strength (kPa)  |  | Soils/Notes:                                   |  |                      |  |
| a <sub>2</sub> = Undisturbed Soil 1/2 Spoon Sample |  | a <sub>2</sub> = Undisturbed Shear Strength (kPa)       |  | W = water content, percent                     |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u = Unconfined Compressive Strength (kPa)               |  | L = Liquid Limit                               |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>1</sub> = Unconfined Compressive Strength (kPa)  |  | P = Plasticity Limit                           |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>2</sub> = Unconfined Compressive Strength (kPa)  |  | I = Shrinkage Index                            |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>3</sub> = Unconfined Compressive Strength (kPa)  |  | S = Swell Index                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>4</sub> = Unconfined Compressive Strength (kPa)  |  | C = Consolidation Test                         |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>5</sub> = Unconfined Compressive Strength (kPa)  |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>6</sub> = Unconfined Compressive Strength (kPa)  |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>7</sub> = Unconfined Compressive Strength (kPa)  |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>8</sub> = Unconfined Compressive Strength (kPa)  |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>9</sub> = Unconfined Compressive Strength (kPa)  |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>10</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>11</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>12</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>13</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>14</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>15</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>16</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>17</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>18</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>19</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>20</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>21</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>22</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>23</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>24</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>25</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>26</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>27</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>28</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>29</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>30</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>31</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>32</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>33</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>34</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>35</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>36</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>37</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>38</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>39</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>40</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>41</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>42</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>43</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>44</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>45</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>46</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>47</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>48</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>49</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |
| v = Thin Wall Tube Sample                          |  | u <sub>50</sub> = Unconfined Compressive Strength (kPa) |  |                                                |  |                      |  |

| Sample Information |                  |                 |                     |                     |                     |                     |                     |                     |                     | Visual Description and Remarks |  | Laboratory Testing Results and Unified Class. |  |                      |                      |                      |                      |                      |                      |                      |                      |                      |                      |                      |                      |                      |                      |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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| Sample No.         | Penetration (cm) | Grain Size (mm) | Blow Count (100 mm) | Blow Count (150 mm) | Blow Count (300 mm) | Blow Count (450 mm) | Blow Count (600 mm) | Blow Count (750 mm) | Blow Count (900 mm) |                                |  |                                               |  | Blow Count (1050 mm) | Blow Count (1200 mm) | Blow Count (1350 mm) | Blow Count (1500 mm) | Blow Count (1650 mm) | Blow Count (1800 mm) | Blow Count (1950 mm) | Blow Count (2100 mm) | Blow Count (2250 mm) | Blow Count (2400 mm) | Blow Count (2550 mm) | Blow Count (2700 mm) | Blow Count (2850 mm) | Blow Count (3000 mm) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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Haley & Aldrich, Inc.

Project: Karamba River Bridge, Retaining Wall  
Location: Aguapito, Maine  
File No: 00785-S04

Boring No.: RW-12-02

RW-12-02

Soil/Slope Exploration Log

METRIC UNITS

PIN: \_\_\_\_\_

|                                               |                      |                  |                       |                                                        |                    |
|-----------------------------------------------|----------------------|------------------|-----------------------|--------------------------------------------------------|--------------------|
| Driller:                                      | G. Rudnicki          | Elevation (m):   | 13.55                 | Auger ID/OD:                                           |                    |
| Spectator:                                    | Maine Test Borings   | Datum:           | NVD                   | Sampler:                                               | 3.5 on Split Spoon |
| Lugged Bit:                                   | R. Lawrence          | Rtc Type:        | Mobile BSS Bandaroter | Water Wt./Flow:                                        | 823 N / 76.2 cm    |
| Date Start/Finish:                            | 8/2/02 / 8/2/02      | Drilling Method: | SEA                   | Core Barrel:                                           |                    |
| Boring Location:                              | Station 13 + 183.4 R | Casing ID/OD:    |                       | Water Levels:                                          | 5.0 m              |
| <b>Definitions</b>                            |                      |                  |                       | <b>Definitions</b>                                     |                    |
| B = Bore Spec Sample                          |                      |                  |                       | SE = Static Field Vane Shear Strength (kPa)            |                    |
| BD = Standardized Split Spoon Sample Interval |                      |                  |                       | TV = Pocket Torque Shear Strength (kPa)                |                    |
| L = Hole Wall Tube Sample                     |                      |                  |                       | C <sub>u</sub> = Unconfined Compressive Strength (kPa) |                    |
| S = Bulk Core Sample                          |                      |                  |                       | SLV (k) = Lab. Vane Shear Strength (kPa)               |                    |
| W = In-situ Vane Shear Test                   |                      |                  |                       | SW = Weight of dry soil sample                         |                    |
| SA = Solid Free Auger                         |                      |                  |                       | SD = weight of dry soil                                |                    |
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| SD = weight of dry soil                       |                      |                  |                       | SD = weight of dry soil                                |                    |
| SD = weight of dry soil                       |                      |                  |                       | SD = weight of dry soil                                |                    |
| SD = weight of dry soil                       |                      |                  |                       | SD = weight of dry soil                                |                    |
| SD = weight of dry soil                       |                      |                  |                       | SD = weight of dry soil                                |                    |
| SD = weight of dry soil                       |                      |                  |                       | SD = weight of dry soil                                |                    |
| SD = weight of dry soil                       |                      |                  |                       |                                                        |                    |



- 
- The diagram illustrates a cross-section of a highway embankment with the following components and dimensions:
- Horizontal Dimensions (Top):** 2400 SHOULDER, 3600 TRAVEL LANE, 3600 TRAVEL LANE, 3600 TRAVEL LANE, 3600 TRAVEL LANE, 2400 SHOULDER, 600, 900.
  - Vertical Layers (from top to bottom):**
    - AGGREGATE SUBBASE COURSE-GRAVEL
    - AGGREGATE SUBBASE COURSE-GRAVEL (OVER TIRE SHRED SECTIONS ONLY, ALL OTHER AREAS COMMON BORROW).
    - SUBGRADE
    - LOW PERMEABILITY FILL
    - VARIES
    - 50 LOAM AND SEED
    - TIRE SHREDS - TYPE B (2.0m thick)
    - TIRE SHREDS - TYPE B (2.4m thick)
    - LOW PERMEABILITY FILL
    - GEOTEXTILE FABRIC WRAPPED ALL AROUND TIRE SHREDS (722.02 CLASS A). PAYMENT SHALL BE INCIDENTAL TO ITEM 203.4231, PLACEMENT OF EMBANKMENT FILL, TIRE SHREDS - TYPE B.
    - HIGH STRENGTH REINFORCEMENT GEOTEXTILE (SEE NOTE 8)
    - AGG. SUBBASE COURSE-GRAVEL (703.06(b), TYPE D)
  - Dimensions and Slopes:**
    - Profile Grade slopes: -4%, -2.8%, -2.8%, -2.8%, -4%, -2%.
    - Internal layer slopes: -4%, -4%, -4%, -4%, -4%, -4%.
    - Right side slope: 2:1 MIN.
    - Left side slope: 1:2.
    - Vertical thicknesses: 0.55m, 1.6m, 0.6m, 2.0m, 1.0m, 2.4m, 0.3m, 0.6m, 0.075m, 0.3m.
  - Structural Features:**
    - DRAINAGE GEOCOMPOSITE
    - PROPOSED WINGWALL AND FOOTING (SEE STRUCTURAL DRAWINGS)
    - 150mm UNDERDRAIN TYPE B (INVERT = PROFILE - 8.425 M)
  - Other Labels:** EXISTING GROUND, LOW PERMEABILITY FILL.

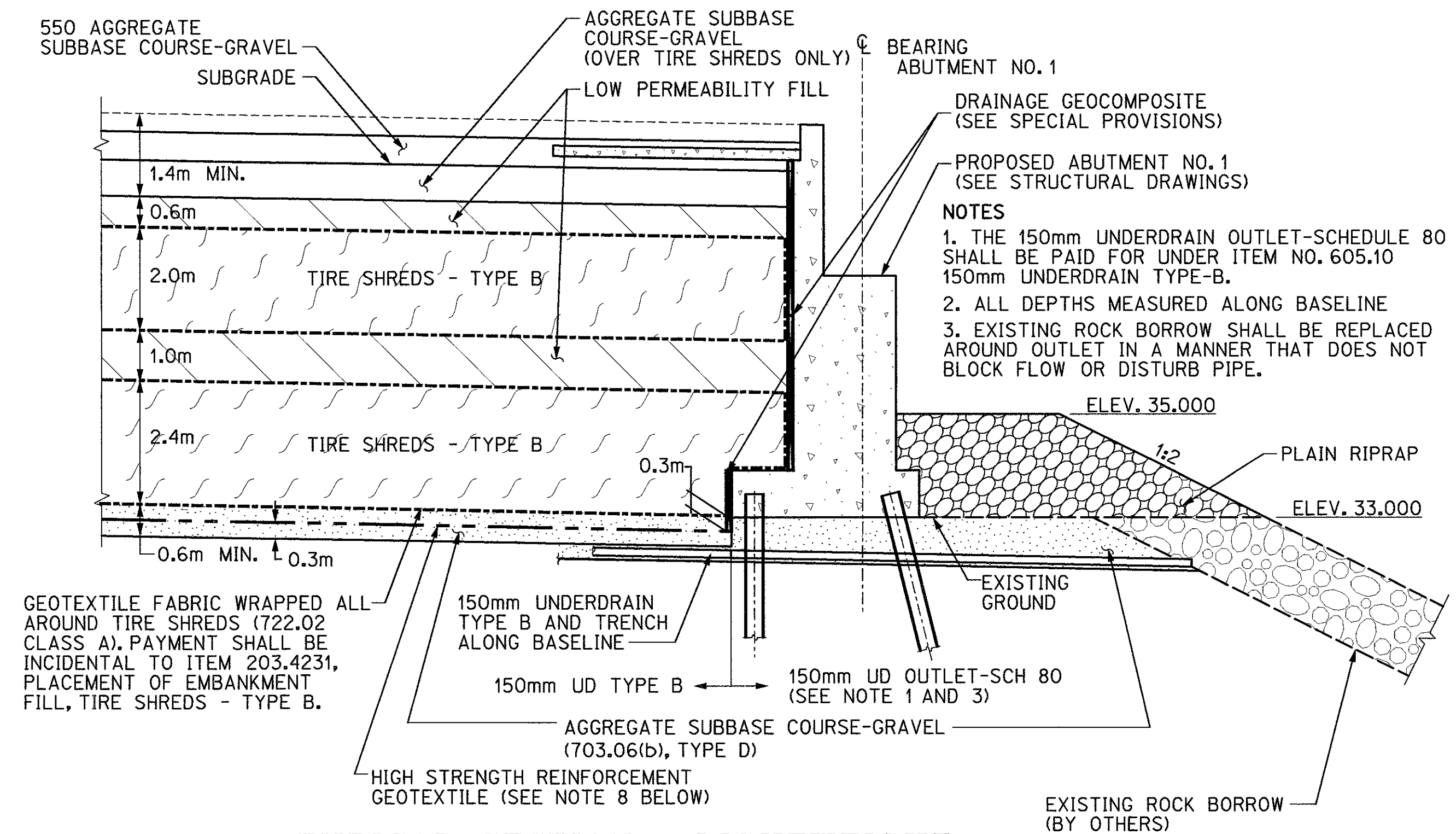
**TYPICAL SECTION - LIGHTWEIGHT FILL  
PERPENDICULAR TO BASELINE**  
NTS

- NOTES:
1. TIRE SHREDS - TYPE B SHALL BE PLACED IN ACCORDANCE WITH SPECIAL PROVISIONS.
  2. LOW PERMEABILITY FILL SHALL MEET THE MATERIAL REQUIREMENTS FOR COMMON BORROW (703.18) WITH THE EXCEPTION THAT A MINIMUM OF 30 PERCENT BY WEIGHT OF THE MATERIAL SHALL PASS THE NO.200 SIEVE, AND THE MAXIMUM PARTICLE SIZE SHALL NOT EXCEED 75mm. MATERIALS SHALL BE PLACED AND COMPACTED IN ACCORDANCE WITH SPECIAL PROVISION 203.
  3. OVERBUILD TIRE CELL LAYERS 10 PERCENT TO ACCOUNT FOR SHRED COMPRESSION.
  4. SEE PROFILE FOR TIRE SHRED CELL LENGTHS MEASURED ALONG BASELINE.
  5. 150-MM UNDERDRAIN, HIGH STRENGTH GEOTEXTILE REINFORCEMENT AND 600 MM SUBBASE COURSE GRAVEL LAYER SHALL NOT BE INSTALLED UNTIL PILE INSTALLATION AND ABUTMENT CONSTRUCTION IS COMPLETE. 150-MM UNDERDRAIN OUTLET AND ASSOCIATED 600 MM SUBBASE COURSE GRAVEL LAYER SHALL BE INSTALLED AFTER THE INSTALLATION OF THE PILES AND PRIOR TO THE INSTALLATION OF THE ABUTMENT.
  6. INSTALLATION OF DRAINAGE STRUCTURES IN THE TIRE SHRED APPROACH SECTION, AND ANY TRENCHING FOR UTILITY CONDUITS, SHALL BE DONE AFTER THE COMPLETION OF THE 10% TIRE SHRED COMPRESSION PERIOD.
  7. GEOCOMPOSITE DRAIN STRIPS SHALL BE INSTALLED IN ACCORDANCE WITH SPECIAL PROVISION 203, DELIVERY AND PLACEMENT OF TIRE SHREDS AS LIGHTWEIGHT FILL, AND SHALL BE CONSIDERED INCIDENTAL TO ITEM 203.4231.
  8. HIGH STRENGTH REINFORCEMENT GEOTEXTILE SHALL BE AS SPECIFIED IN SPECIAL PROVISION - SECTION 620 GEOTEXTILES (HIGH STRENGTH REINFORCEMENT GEOTEXTILE). HIGH STRENGTH REINFORCEMENT GEOTEXTILE SHALL BE PLACED WITH THE MACHINE DIRECTION PARALLEL TO THE ROADWAY BASELINE. EACH STRIP SHALL BE PROVIDED IN A SINGLE CONTINUOUS LENGTH (NO SEAMS ALLOWED). ADJACENT STRIPS SHALL BE JOINED BY OVERLAPPING A MINIMUM OF 600 MM.

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 18           | 105             |
| 000556.22            |       |                |              |                 |

| STATION                                           | RCP CULVERT PIPE |        |       | UNDERDRAIN |        |               | CATCH BASIN |
|---------------------------------------------------|------------------|--------|-------|------------|--------|---------------|-------------|
|                                                   |                  |        |       | TYPE B     |        | SCH 80 OUTLET |             |
|                                                   | SIZE             | LENGTH | CLASS | SIZE       | LENGTH | LENGTH        | F6-C        |
| STA. 12+803, 9.3m LT. TO STA. 12+831.5, 9.15m LT. | 300mm            | 28.8m  | III   |            |        |               | 1           |
| STA. 12+803, 9.3m RT. TO STA. 12+843, 9.15m RT.   | 300mm            | 41.6m  | III   |            |        |               | 1           |
| STA. 12+821.8 TO STA. 12+847.1                    |                  |        |       | 150mm      | 25.3m  |               |             |
| STA. 12+847.1 TO STA. 12+856.7                    |                  |        |       |            |        | 9.6m          |             |



TYPICAL SECTION - LIGHTWEIGHT  
FILL AT ABUTMENT NO.1  
NTS

| SHEET         | DESCRIPTION                                    |
|---------------|------------------------------------------------|
| 1             | TYPICAL SECTION, DETAILS, AND DRAINAGE SUMMARY |
| WEST APPROACH |                                                |
| 2             | PLAN                                           |
| 3             | PROFILE                                        |
| 4-5           | CROSS SECTIONS                                 |
| EAST APPROACH |                                                |
| 6             | PLAN                                           |
| 7             | PROFILE                                        |
| 8-9           | CROSS SECTIONS                                 |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA

# KENNEBEC COUNTY HIGHWAY APPROACH PLANS

TYPICAL SECTION, DETAILS,  
AND DRAINAGE SUMMARY

File: \\jobs\34612\Bridge\34612\Design\Sheets\abutmentplan.dgn

Date: 11/25/02

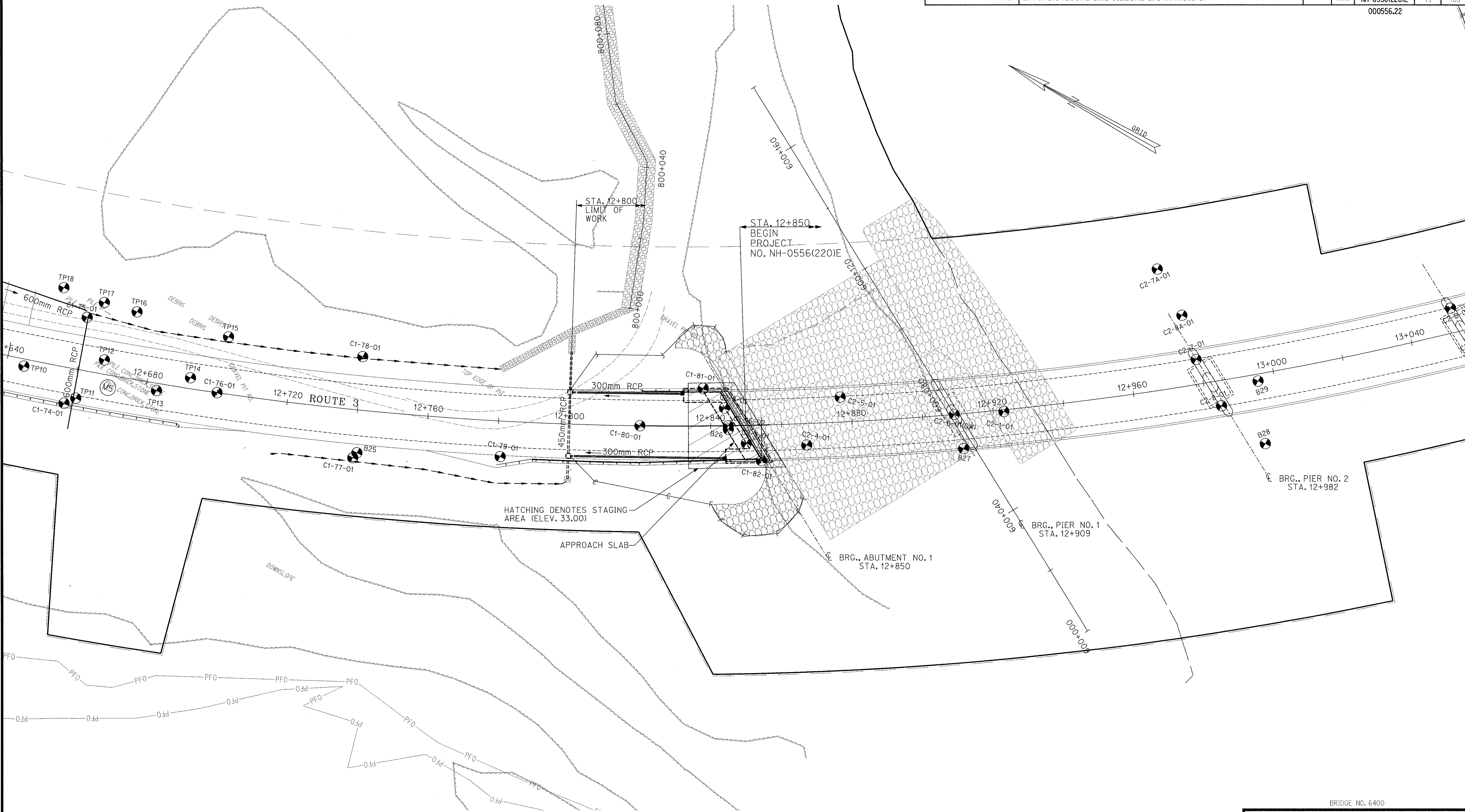
METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| FHWA<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|------------------|-------|----------------|--------------|-----------------|
| 1                | MAINE | NH-0556(220)E  | 19           | 105             |

000556.22

| PROJECT DESIGN ENGINEER | BY      | DATE |
|-------------------------|---------|------|
| PLANS                   | LEF, WR |      |
| DESIGN-DETAILED         | JMW     |      |
| CHECKED                 |         |      |
| REVISIONS               |         |      |
| FIELD CHANGES           |         |      |



CONSTRUCTION BASELINE

M5  
PI STA = 13+174.259  
Δ = 62°-21'-08.0" LT.  
R = 925.000 m  
L = 1006.633 m  
T = 559.673 m  
E = 156.138 m

NOTE:  
1. FUTURE GUARD RAIL SHOWN FOR INFORMATIONAL PURPOSES ONLY

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

WEST APPROACH PLAN

HIGHWAY APPROACH PLANS

SHEET 2 OF 9

AUGUSTA, MAINE

PLAN  
STA. 12+440 TO STA. 12+860

Date: 11/25/02

Filename: j:\jobs\34612\Bridge\CAADD\Design\Sheets\abutment\profile.dgn

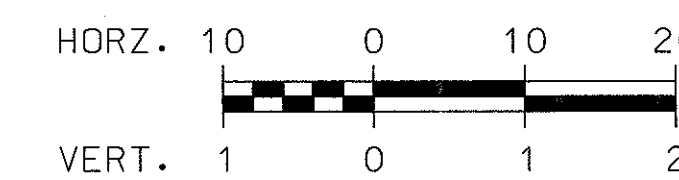
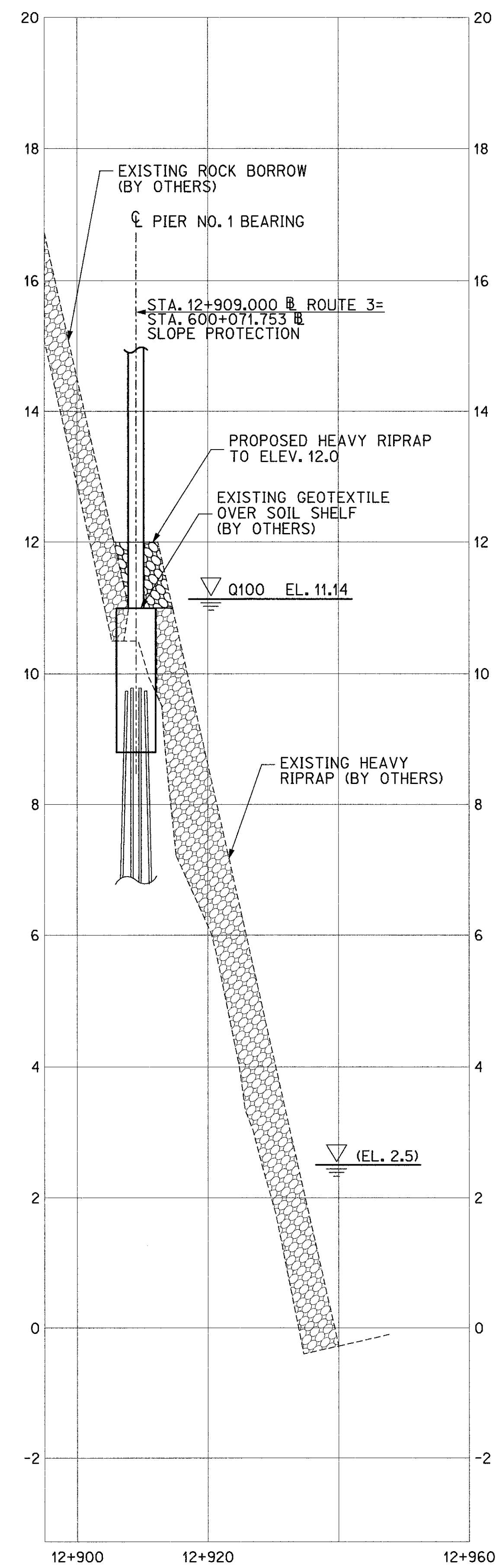
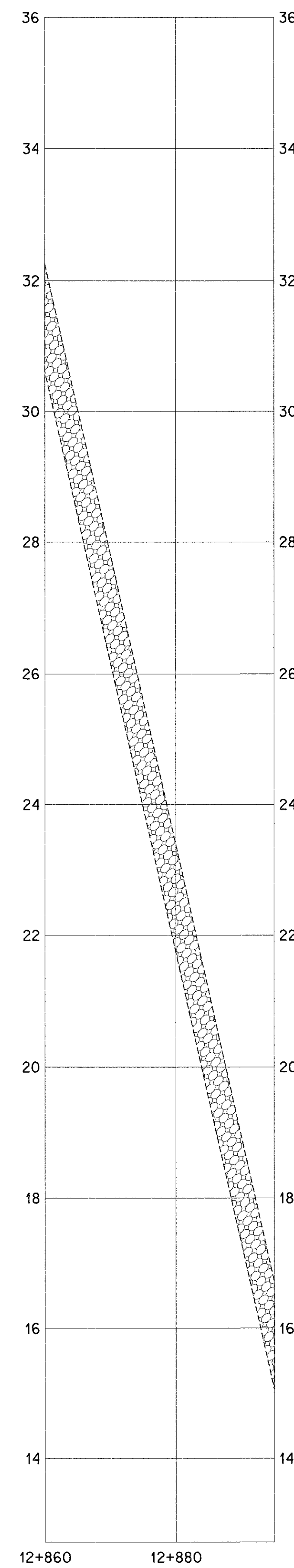
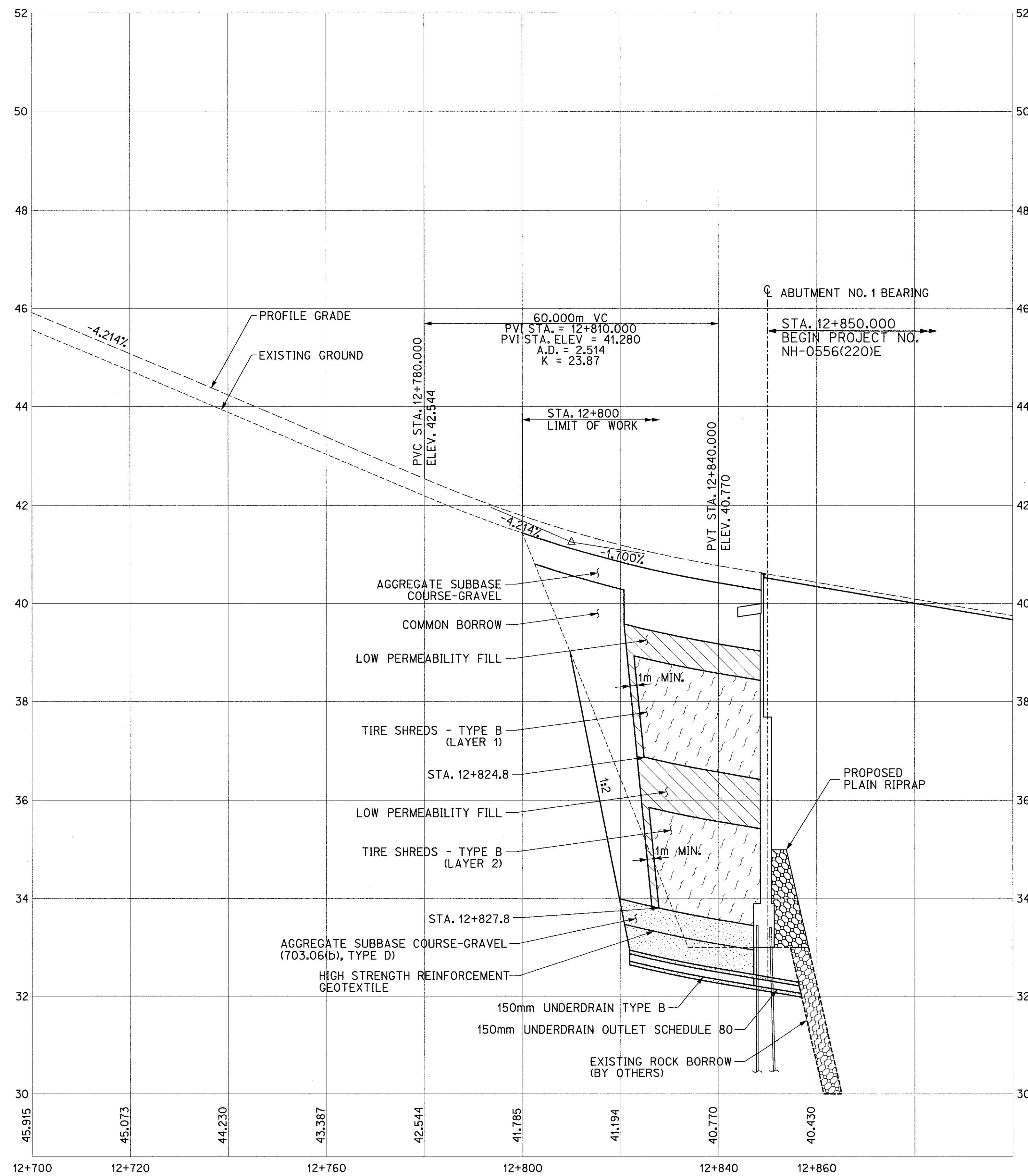
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| DESIGN-DETAILED         | LEF | 12/01 | 01/02 |
| CHECKED                 | DDS |       |       |
| REVISIONS               |     |       |       |
| FIELD CHANGES           |     |       |       |

PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                  |       |                |              |                 |
|------------------|-------|----------------|--------------|-----------------|
| FHWA<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                | MAINE | NH-0556(220)E  | 20           | 103             |



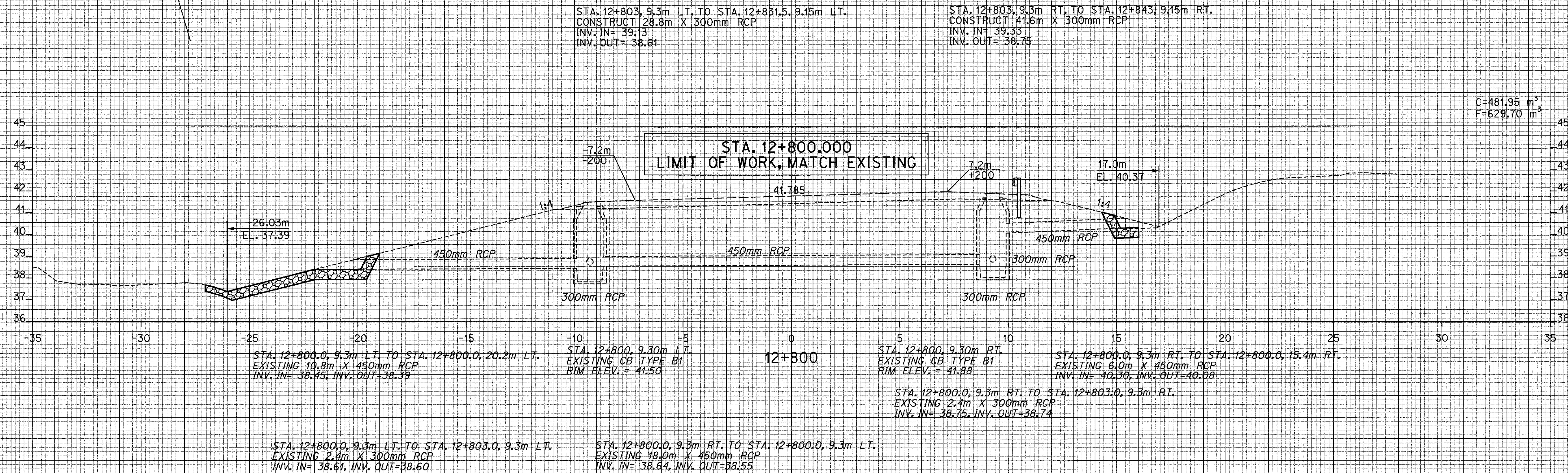
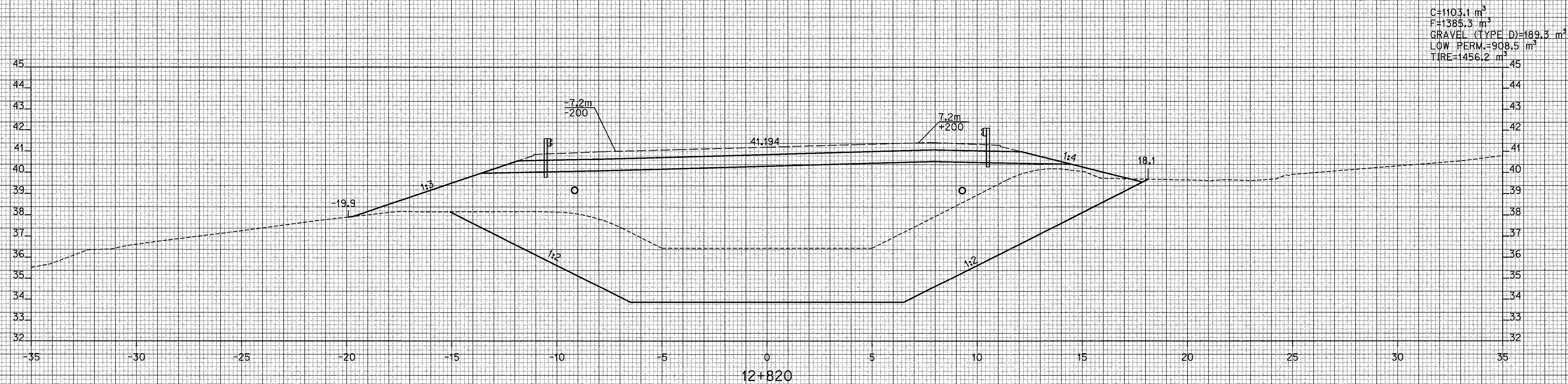
WEST APPROACH PROFILE

Date: 11/25/02

Filename: j:\jobs\34612\Bridge\CADD\Design\Sheets\abutment\ksec.dgn

| PROJECT DESIGN ENGINEER | DDSS/AHM | BY | DATE  |
|-------------------------|----------|----|-------|
| DESIGN-DETAILED         | LEF      |    | 09/02 |
| CHECKED                 | DGE      |    | 09/02 |
| REVISIONS               |          |    |       |
| FIELD CHANGES           |          |    |       |

PLANS



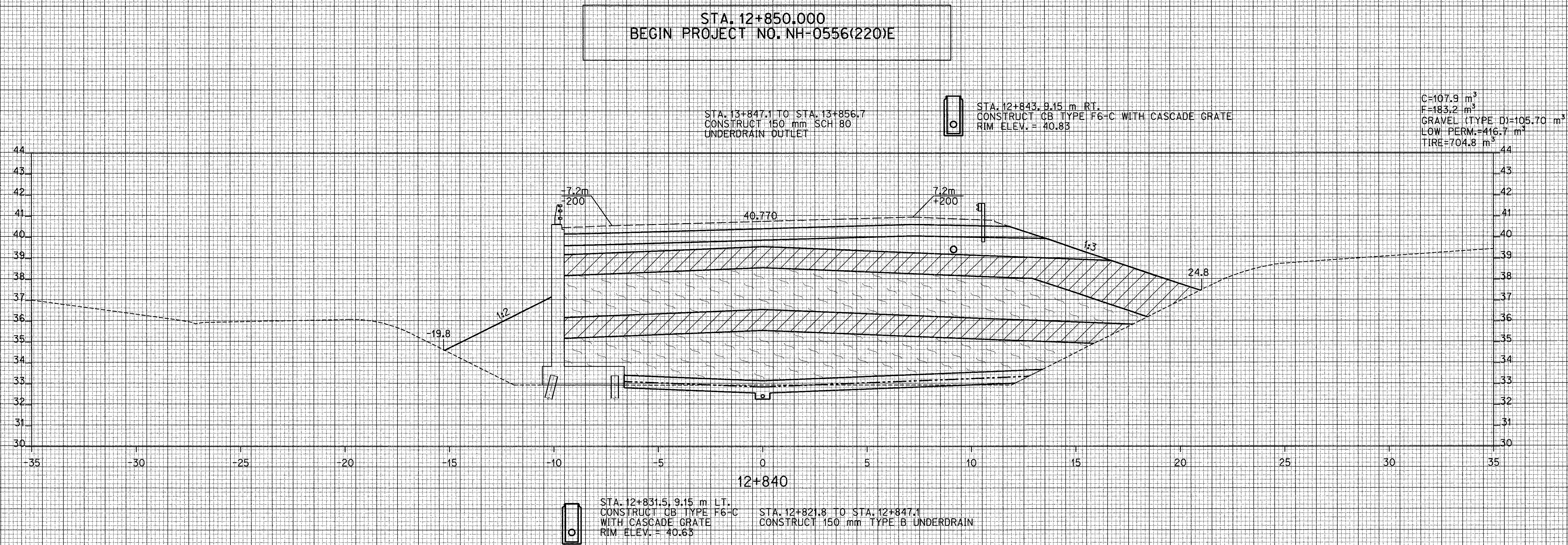
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| PROJECT DESIGN ENGINEER | DATE  |
| DESIGN DETAILED         | 09/02 |
| CHECKED                 | 09/02 |
| REVISIONS               |       |
| FIELD CHANGES           |       |

PLANS

|    |      |
|----|------|
| BY | LEFT |
|    | DOE  |



WEST SIDE APPROACH CROSS SECTIONS

CROSS SECTIONS

File: j:\jobs\34612\Bridg\CAD\Design\Sheets\abutment2\plan.dgn

Date: 11/25/02

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN/DETAILED         | J. Folsom |      |
| CHECKED                 | D. Darrin |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS

BASELINE CURVE DATA

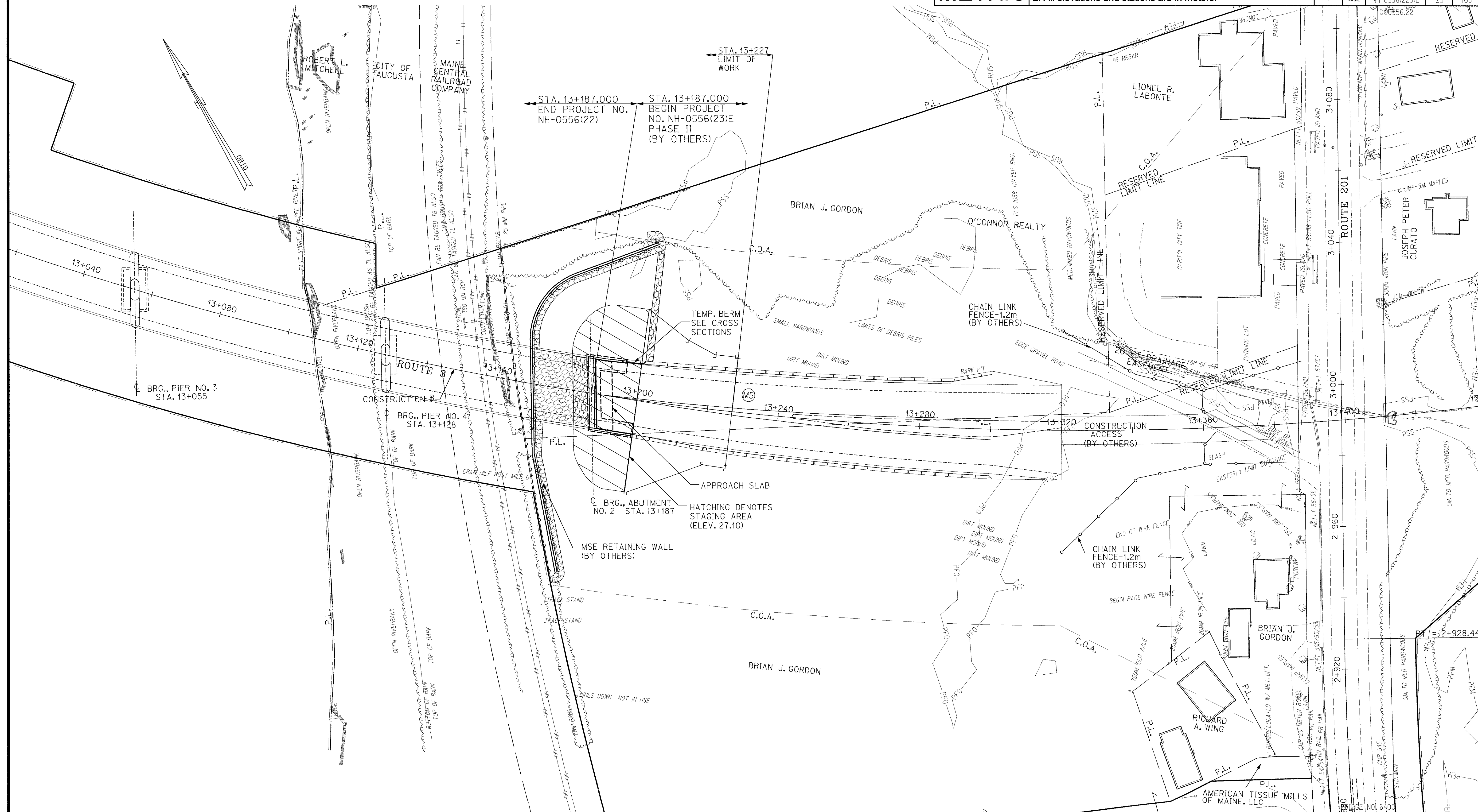
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| M5 | PI STA = 13+174.259<br>$\Delta = 62^{\circ}-21'-08.0''$ LT.<br>R = 925.000 m<br>L = 1006.633 m<br>T = 559.673 m<br>E = 156.138 m |
|----|----------------------------------------------------------------------------------------------------------------------------------|

|                                |     |
|--------------------------------|-----|
| STONE DITCH PROTECTION         | M3  |
| STA 13+182 TO 13+187           | 111 |
| STA 13+201, 11.1 to 42.2 M LT. | 28  |

- NOTES:
- ROADWAY IS IN FULL SUPERELEVATION AT BEGINNING OF THIS PROJECT.
  - FUTURE GUARD RAIL AND CURBING SHOWN FOR INFORMATIONAL PURPOSES ONLY.

METRIC 1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(22)01E | 23        | 105          |



EAST APPROACH PLAN

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

HIGHWAY APPROACH PLANS

Date: 11/25/02

Filename: j:\jobs\34612\Bridge\CADD\Design\Sheets\abutment2\profile.dgn

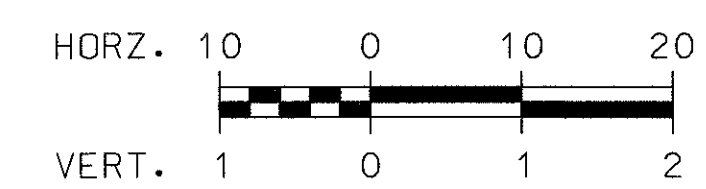
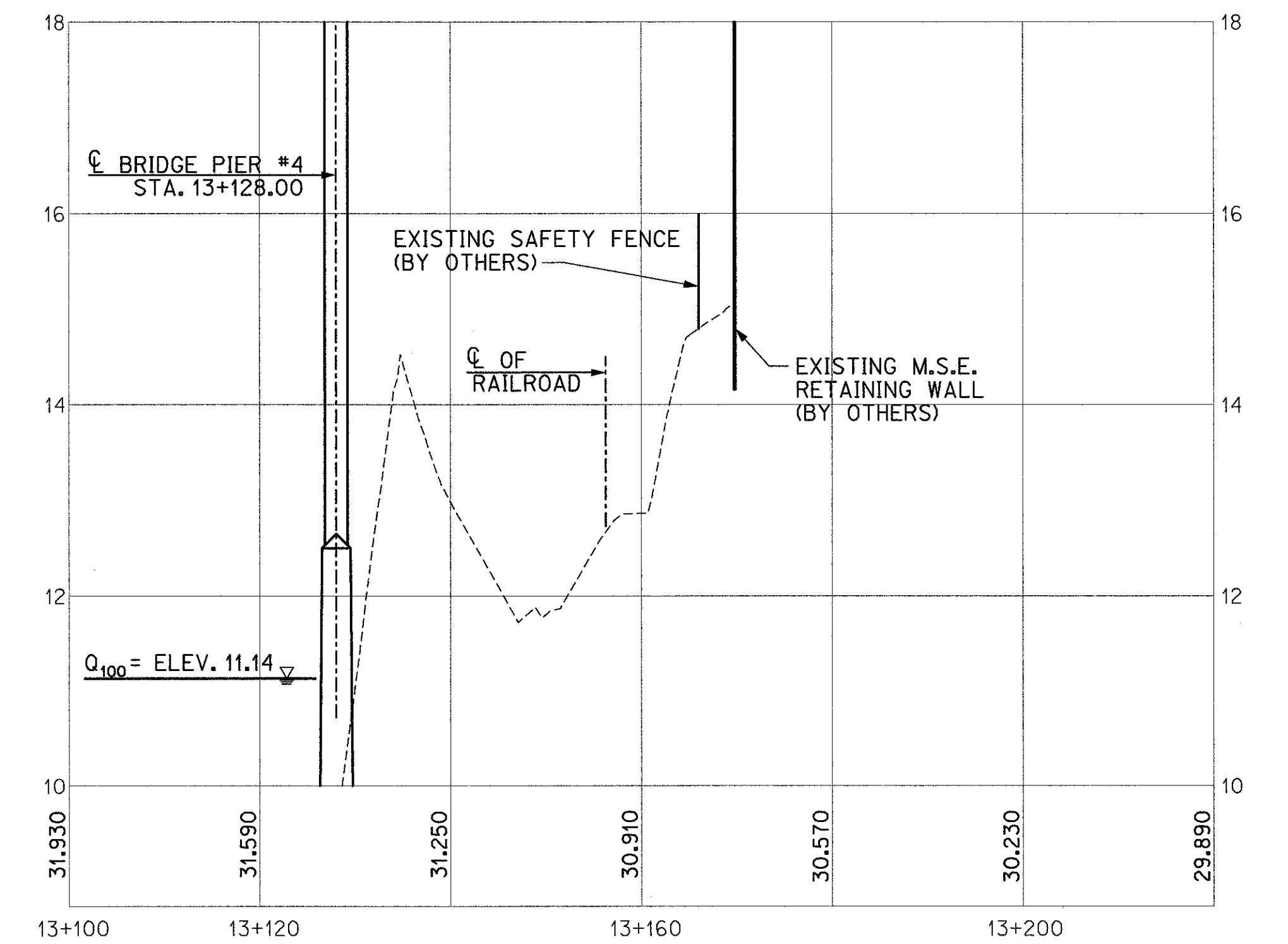
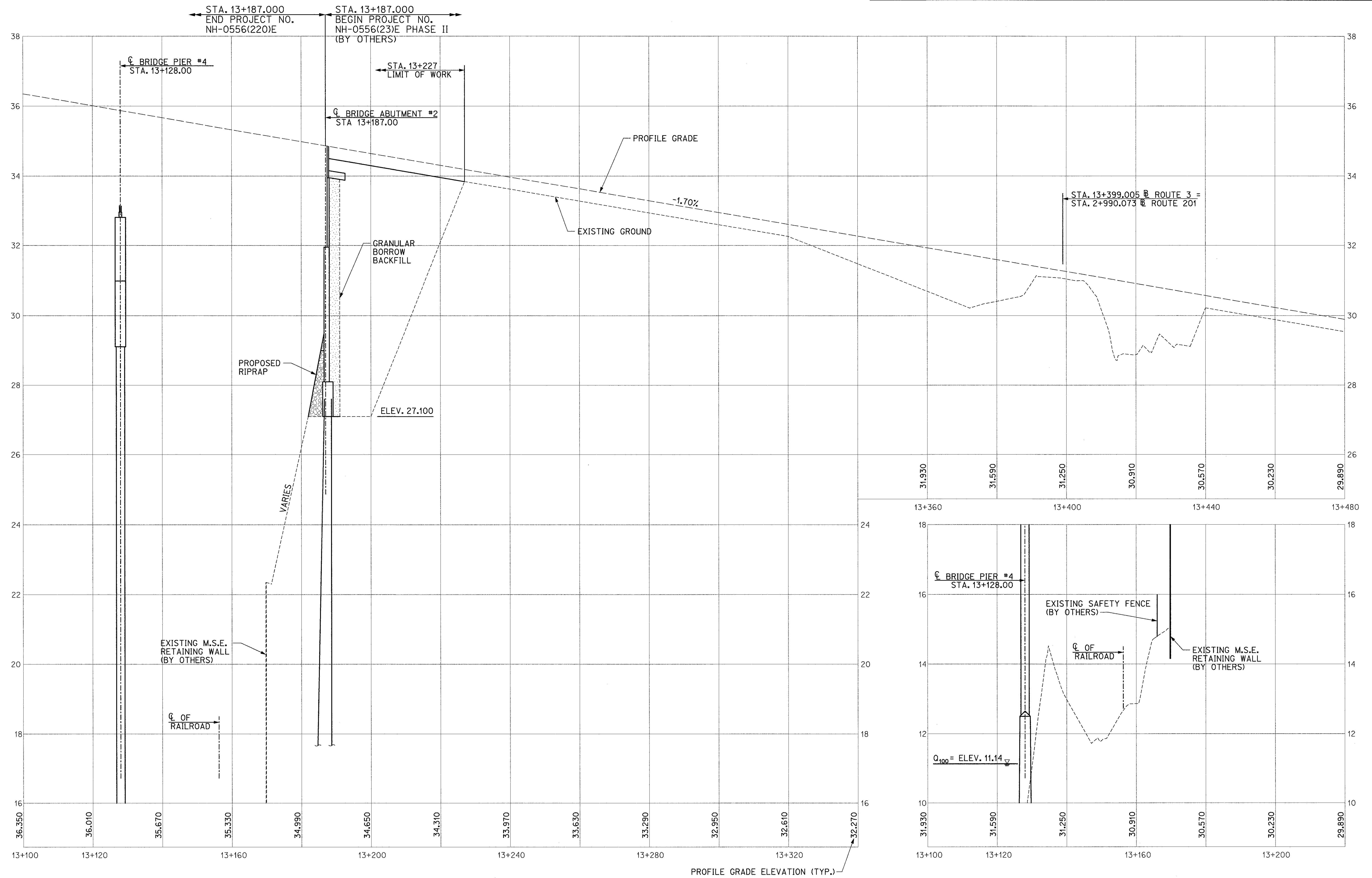
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|-------------------------|---------|----|-------|
| PROJECT DESIGN ENGINEER | DOS/JHM | BY | DATE  |
| DESIGN-DETAILED         | LIF     |    | 09/02 |
| CHECKED                 | DCL     |    | 09/02 |
| REVISIONS               |         |    |       |
| FIELD CHANGES           |         |    |       |

PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

|                  |       |                |              |                 |
|------------------|-------|----------------|--------------|-----------------|
| FHWA<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                | MAINE | NH-0556(220)E  | 24           | 103             |



EAST APPROACH PROFILE

Date:11/25/02

F:\jobs\j\jobs\34612\Bridge\CADD\Design\Sheets\cbutment2\sec.dgn

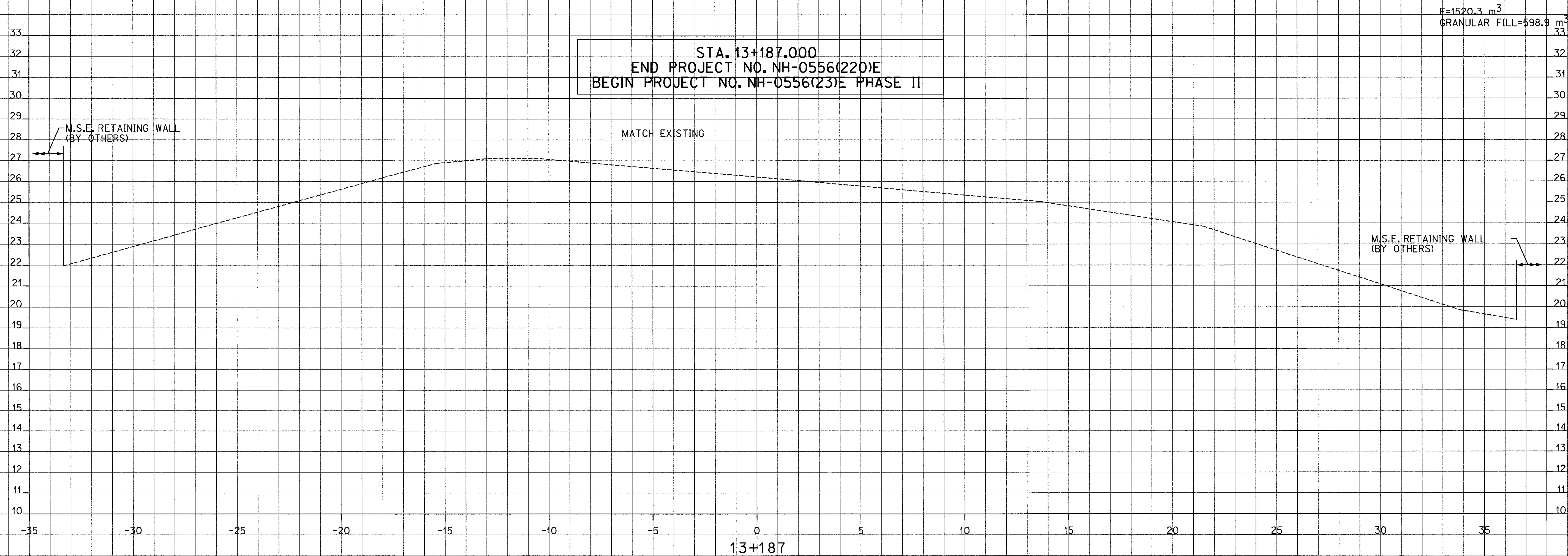
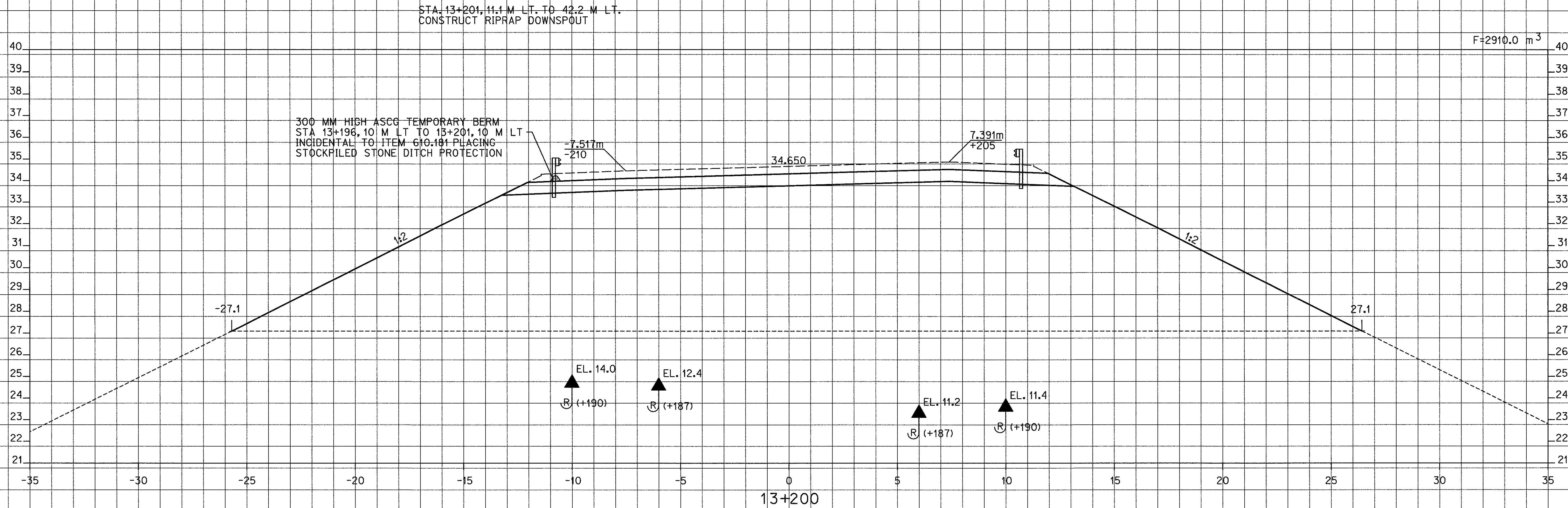
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| PROJECT DESIGN ENGINEER | DOS/JHM | DATE | 09/02 |
| DESIGN DETAILER         | LEF     | DATE | 09/02 |
| CHECKED                 | DSE     |      |       |
| REVISIONS               |         |      |       |
| FIELD CHANGES           |         |      |       |

PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 25           | 105             |



EAST SIDE APPROACH CROSS SECTIONS

CROSS SECTIONS  
STA. 13+200

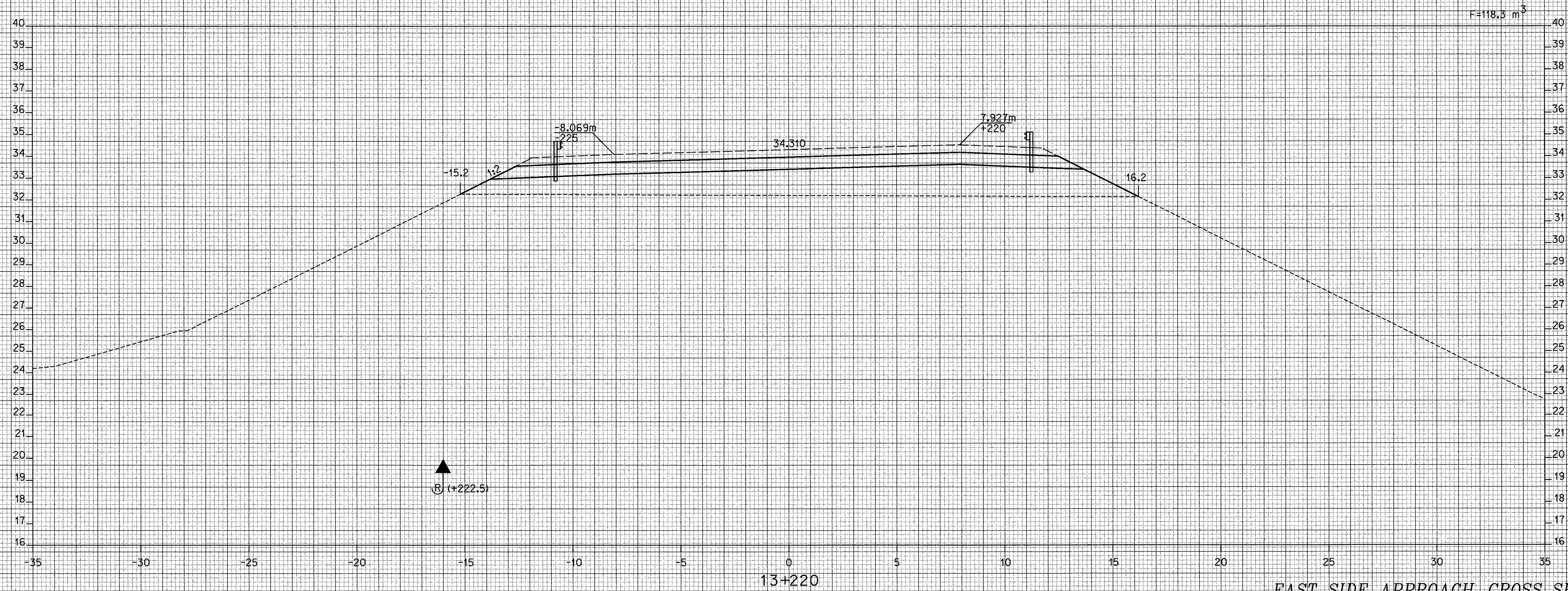
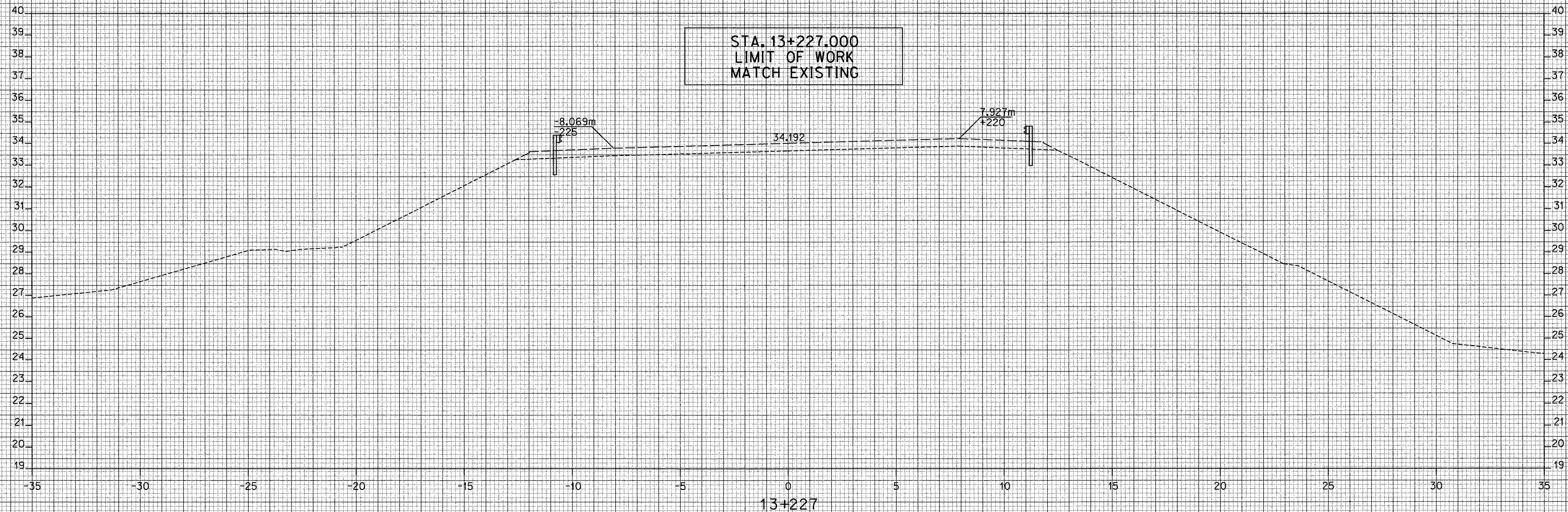
Date: 11/25/02

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|-------------------------|-------|
| PROJECT DESIGN ENGINEER | DATE  |
| DESIGN DETAILER         | 09/02 |
| CHECKED                 | 09/02 |
| REVISIONS               |       |
| FIELD CHANGES           |       |

PLANS

|        |                                                                                                               |                      |       |                |              |                 |
|--------|---------------------------------------------------------------------------------------------------------------|----------------------|-------|----------------|--------------|-----------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. | F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|        |                                                                                                               | 1                    | MAINE | NH-0556(220)E  | 26           | 105             |



EAST SIDE APPROACH CROSS SECTIONS

CROSS SECTIONS STA. 13+200

Date: 26 NOV 2002

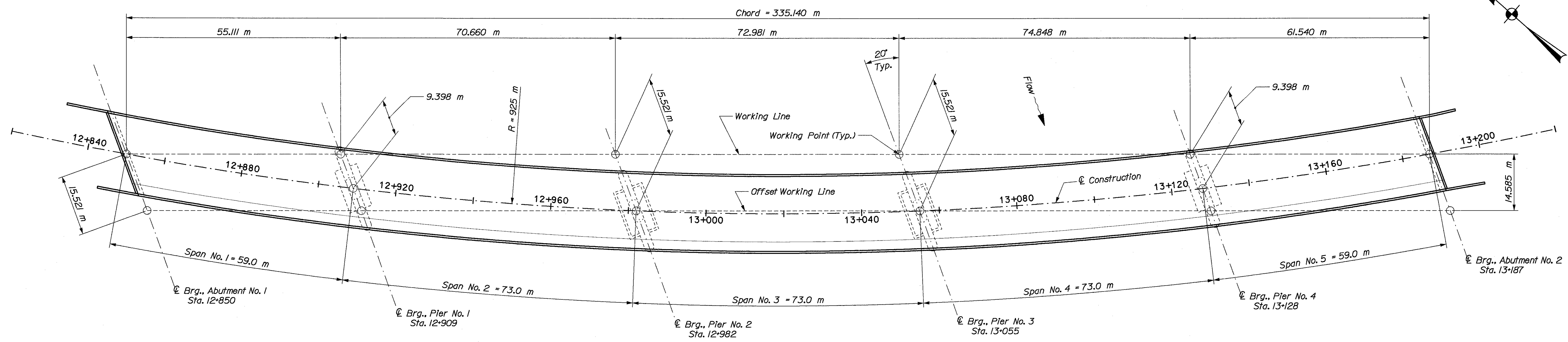
Username: Donal.Damren

Division: BRIDGE

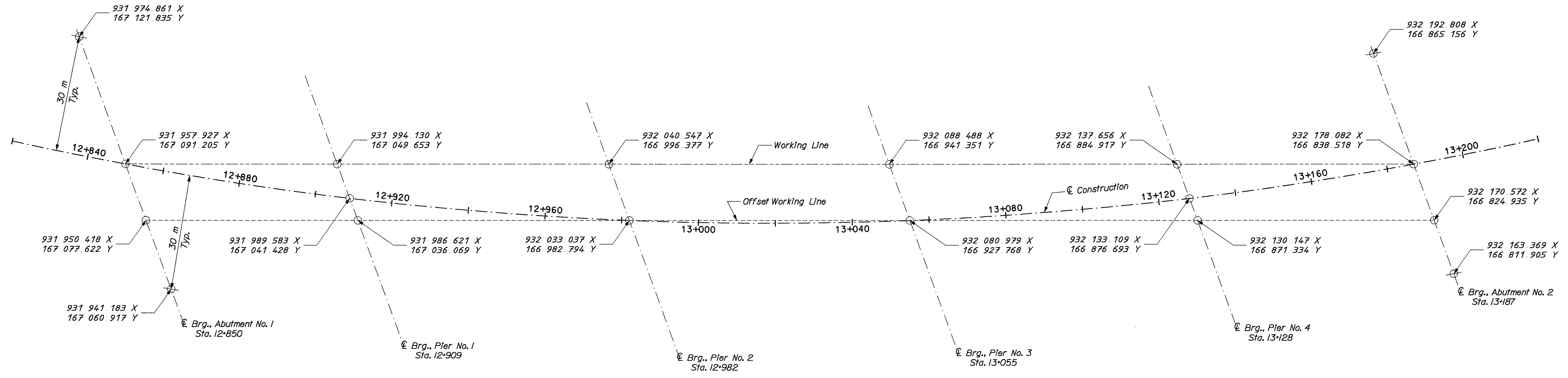
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|                         |           |         |
|-------------------------|-----------|---------|
| PROJECT DESIGN ENGINEER | BY        | DATE    |
| DESIGN-DETAILED         | J. Folsom | D. Damm |
| CHECKED                 |           |         |
| REVISIONS               |           |         |
| FIELD CHANGES           |           |         |

|       |
|-------|
| PLANS |
|-------|



GEOMETRIC LAYOUT PLAN



LAYOUT COORDINATES PLAN

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

GEOMETRIC LAYOUT PLAN

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

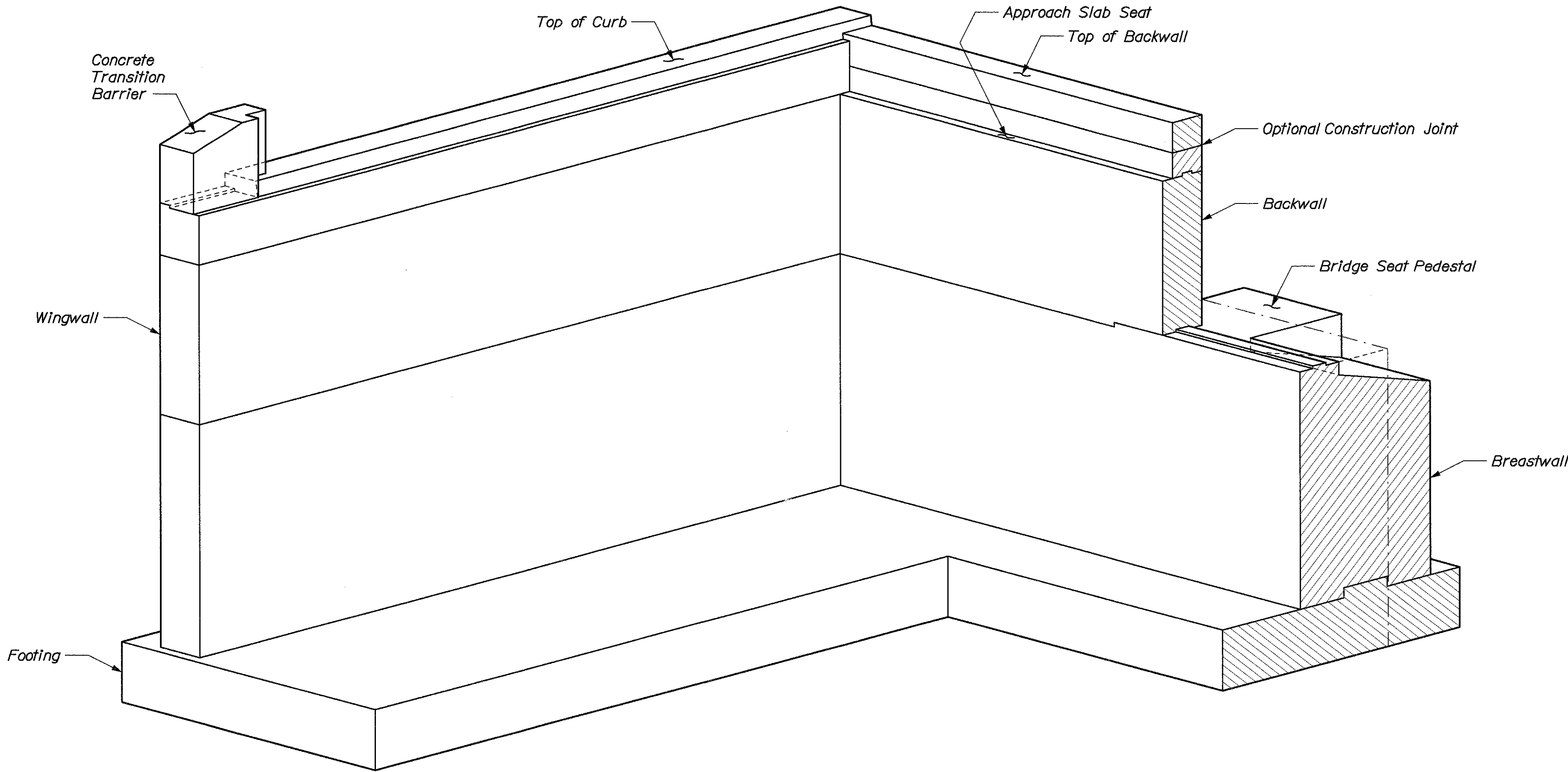
Username: Dana.Damren

Division: BRIDGE

Filename: ...\\028\_Abutments.dgn

|                         |           |           |
|-------------------------|-----------|-----------|
| PROJECT DESIGN ENGINEER | BY        | DATE      |
| DESIGN-DETAILED         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

PLANS



TYPICAL REAR ISOMETRIC VIEW

|        |                                                                                                               |       |                |           |              |
|--------|---------------------------------------------------------------------------------------------------------------|-------|----------------|-----------|--------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. |       |                |           |              |
|        | F.H.W.A. REG. NO.                                                                                             | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|        | 1                                                                                                             | MAINE | NH-0556(220)E  | 28        | 105          |

000556.22

ABUTMENT PILE NOTES

- Piles marked with an arrow shall be battered 100 V : 25 H in the direction of the arrow.
- Maximum calculated pile loads: Abutment No. 1 = 640 kN  
Abutment No. 2 = 710 kN
- Estimate of piles required: Abutment No. 1 = HP 360x108 ~ 53 @ 38 m  
Abutment No. 2 = HP 360x108 ~ 47 @ 16 m
- HP pile material shall be ASTM A 709M, Grade 345.
- All piles shall be equipped with a pile tip in accordance with Standard Specifications Section 501.10.
- The Contractor shall perform and submit a wave equation analysis for review and acceptance by the Engineer. The cost of performing the wave equation analysis will be considered incidental to Item No. 501.92, Pile Driving Equipment Mobilization.
- At each abutment foundation, the Contractor shall perform four (4) dynamic load tests to confirm the ultimate capacity of the piles. The ultimate capacity shall be 2.25 times the maximum calculated design load per LFD Specifications. At the start of production pile driving, two (2) piles from different locations within the pile foundation shall be dynamically tested. Two (2) additional piles shall be tested after approximately one half of the piles have been installed.

ABUTMENT NOTES

- Reinforcing steel shall have 50 mm cover in the walls and 75 mm cover in the footings unless otherwise noted.
- Cover joints without waterstops on the back face in accordance with Standard Detail 502(1).
- Place 100 mm diameter drains in the breastwall and wingwalls at 6 000 mm maximum spacing. The exact location will be determined by the Engineer in the field.
- The bridge seat elevations shown are based on the bearing heights given on the "Pier Bearing Layout" drawing. Adjustments to the elevations shall be made, if necessary, according to the provisions stated there.
- Install HDPE concrete liners on the bridge seats between alternate pairs of bridge seat pedestals. Refer to "Curtain Troughs" drawing for details.
- Tire shreds and low permeability fill shall be placed behind Abutment No. 1 in accordance with details shown on the "Highway Approach Plans" drawings.
- Construct a French drain behind Abutment No. 2 breastwall and wingwalls in accordance with Standard Specification Section 512, French Drains.
- Abutment No. 2 shall be backfilled with granular borrow. Pay limits will be a vertical plane located three (3) meters behind the back faces of the breastwall and wingwalls.
- Protective Coating for Concrete Surfaces shall be applied to all exposed surfaces of the abutments and wingwalls, to 300 mm below the tops of backwalls on the back sides, and to all surfaces of Concrete Transition Barriers.
- Payment for concrete curbs on the wingwalls will be made under Item No. 502.49, Structural Concrete Curbs and Sidewalks.

LEGEND

N.F. = Near Face  
F.F. = Far Face  
E.F. = Each Face  
eq. sp. = equally spaced

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

ABUTMENTS

SHEET OF AUGUSTA, MAINE

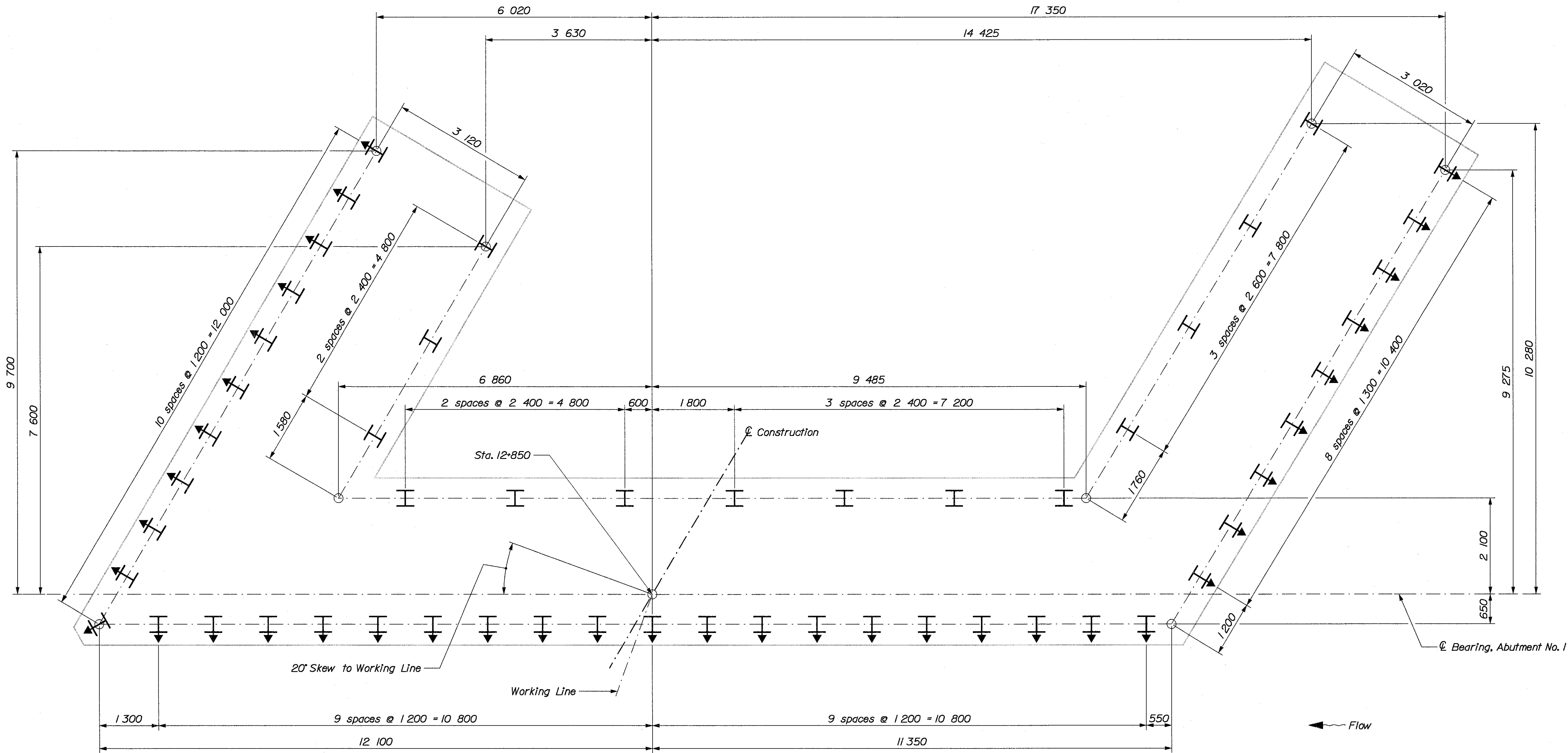
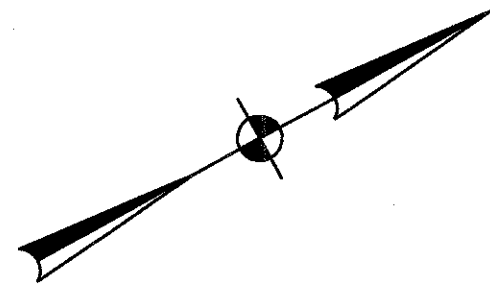
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Username: Dana.Damren

Division: BRIDGE

Date: 26 NOV 2002

| PROJECT DESIGN ENGINEER | BY              |  | DATE |
|-------------------------|-----------------|--|------|
|                         | DESIGN-DETAILED |  |      |
|                         | CHECKED         |  |      |
|                         | REVISIONS       |  |      |
|                         | FIELD CHANGES   |  |      |
| PLANS                   | J. Folsom       |  |      |
|                         | D. Damren       |  |      |
|                         |                 |  |      |
|                         |                 |  |      |
|                         |                 |  |      |



ABUTMENT NO. 1 PILE PLAN  
Battered piles shown at bottom of footing EL. 33.0

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 29           | 105             |

000556.22

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUT. NO. 1 PILE PLAN**

SHEET OF AUGUSTA, MAINE

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

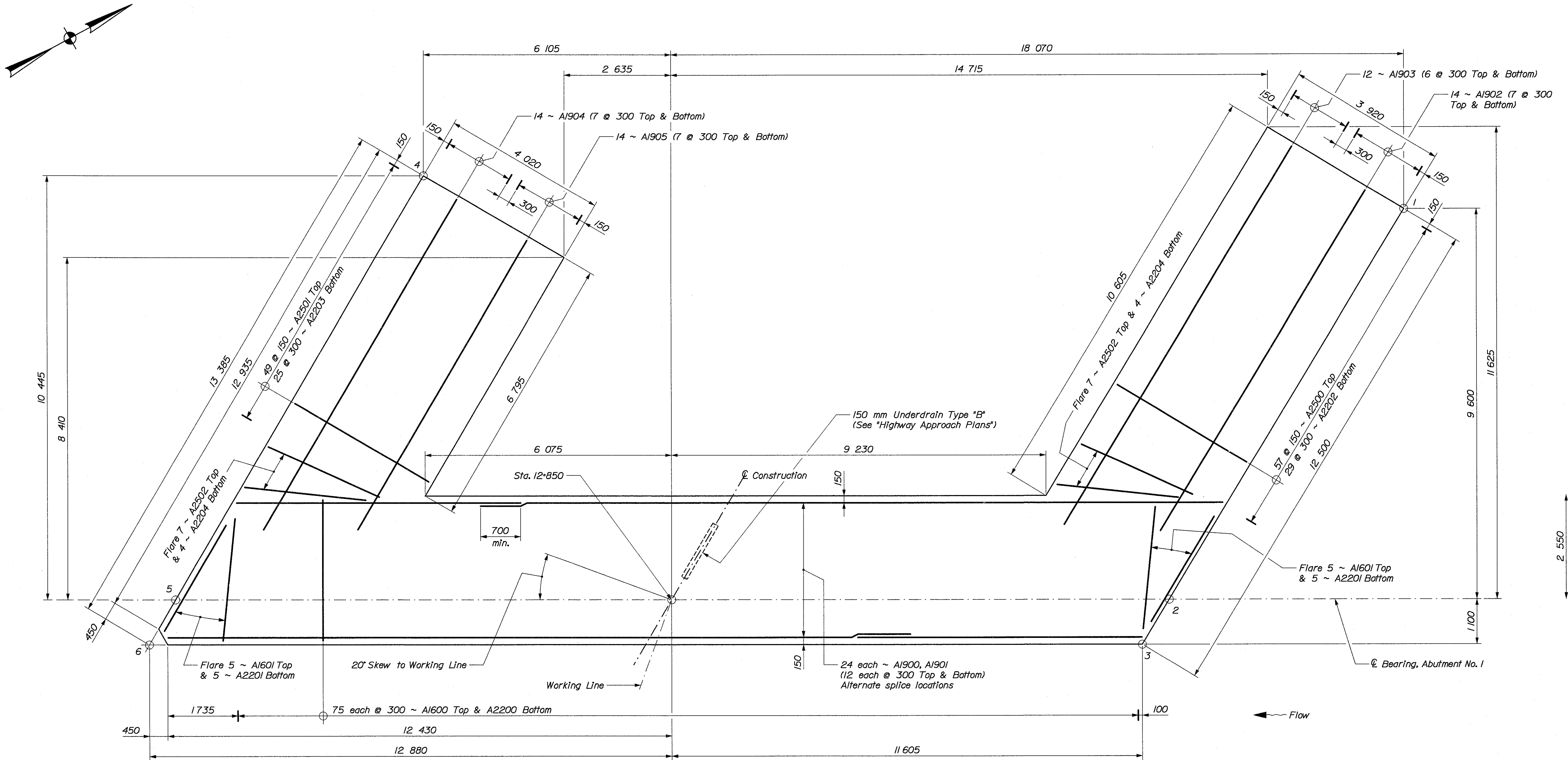
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                      |       |                |              |                 |
|----------------------|-------|----------------|--------------|-----------------|
| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                    | MAINE | NH-0556(220)E  | 30           | 105             |

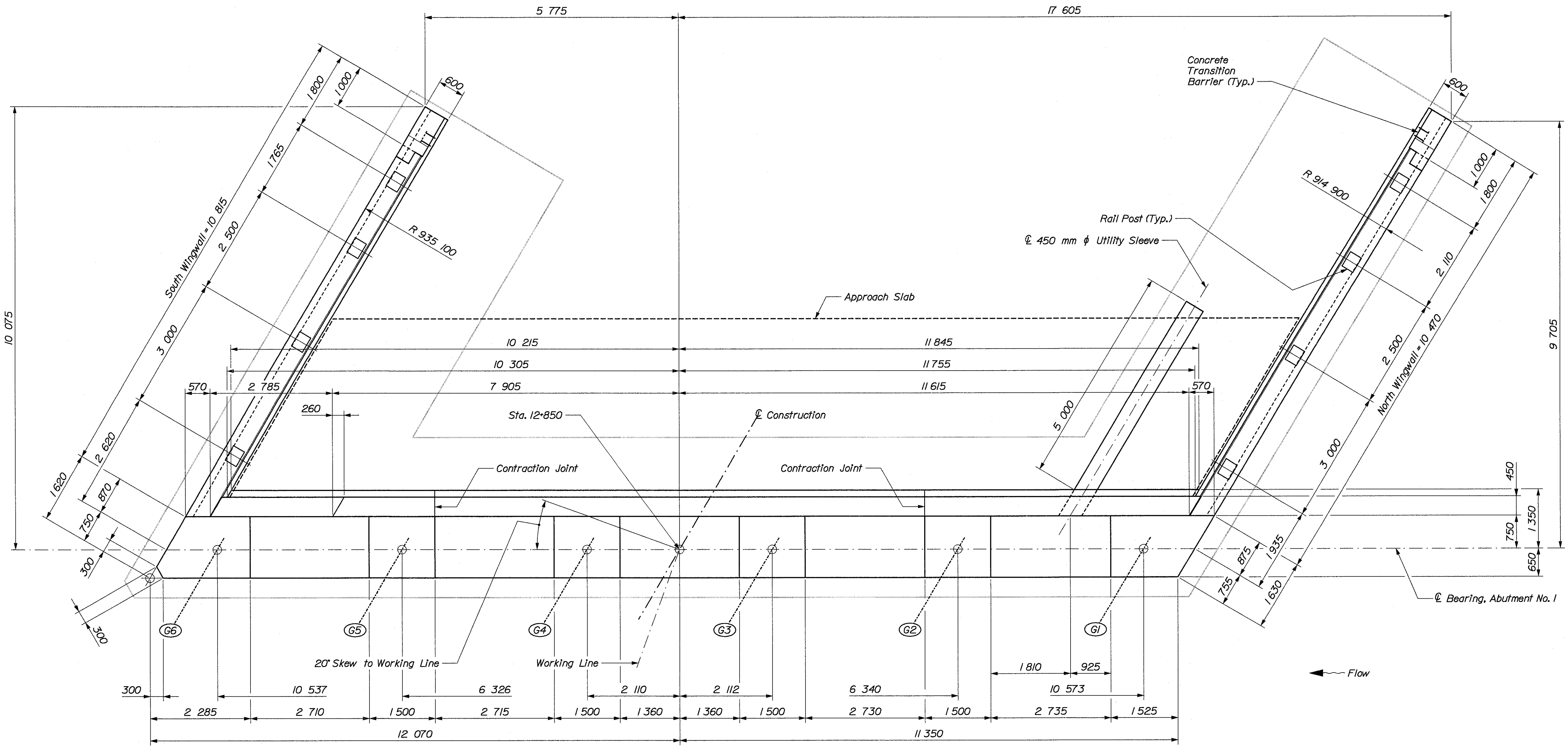
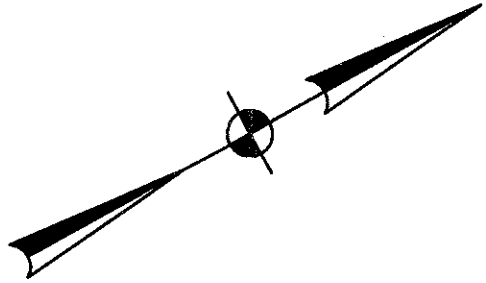
000556.22



ABUTMENT NO. 1 FOOTING PLAN

| FOUNDATION COORDINATES |                                |
|------------------------|--------------------------------|
| Point No.              | Coordinates                    |
| 1                      | 931 958 268 X<br>167 111 662 Y |
| 2                      | 931 963 863 X<br>167 101 941 Y |
| 3                      | 931 964 504 X<br>167 100 827 Y |
| 4                      | 931 952 659 X<br>167 079 402 Y |
| 5                      | 931 952 009 X<br>167 080 499 Y |
| 6                      | 931 945 834 X<br>167 090 917 Y |

BRIDGE NO. 6400  
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
ABUT. NO. 1 FOOTING PLAN



Date:26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ...\\032\_abut\_1 elevation.dgn

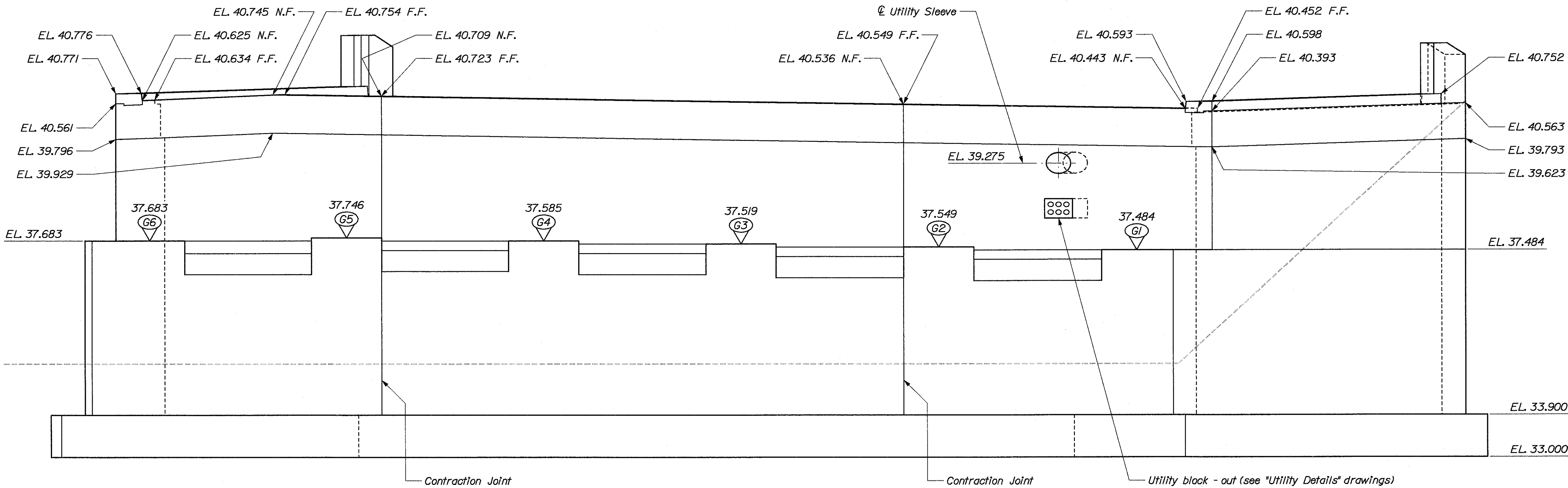
|                         |                 |           |           |
|-------------------------|-----------------|-----------|-----------|
| PROJECT DESIGN ENGINEER | DESIGN-DETAILED | BY        | DATE      |
|                         | CHECKED         | J. Folsom | D. Damren |
|                         | REVISIONS       |           |           |
|                         | FIELD CHANGES   |           |           |
| PLANS                   |                 |           |           |

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                      |       |                |              |                 |
|----------------------|-------|----------------|--------------|-----------------|
| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                    | MAINE | NH-0556(220)E  | 32           | 105             |

000556.22



ABUTMENT NO. 1 ELEVATION

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
ABUT. NO. 1 ELEVATION

SHEET OF AUGUSTA, MAINE

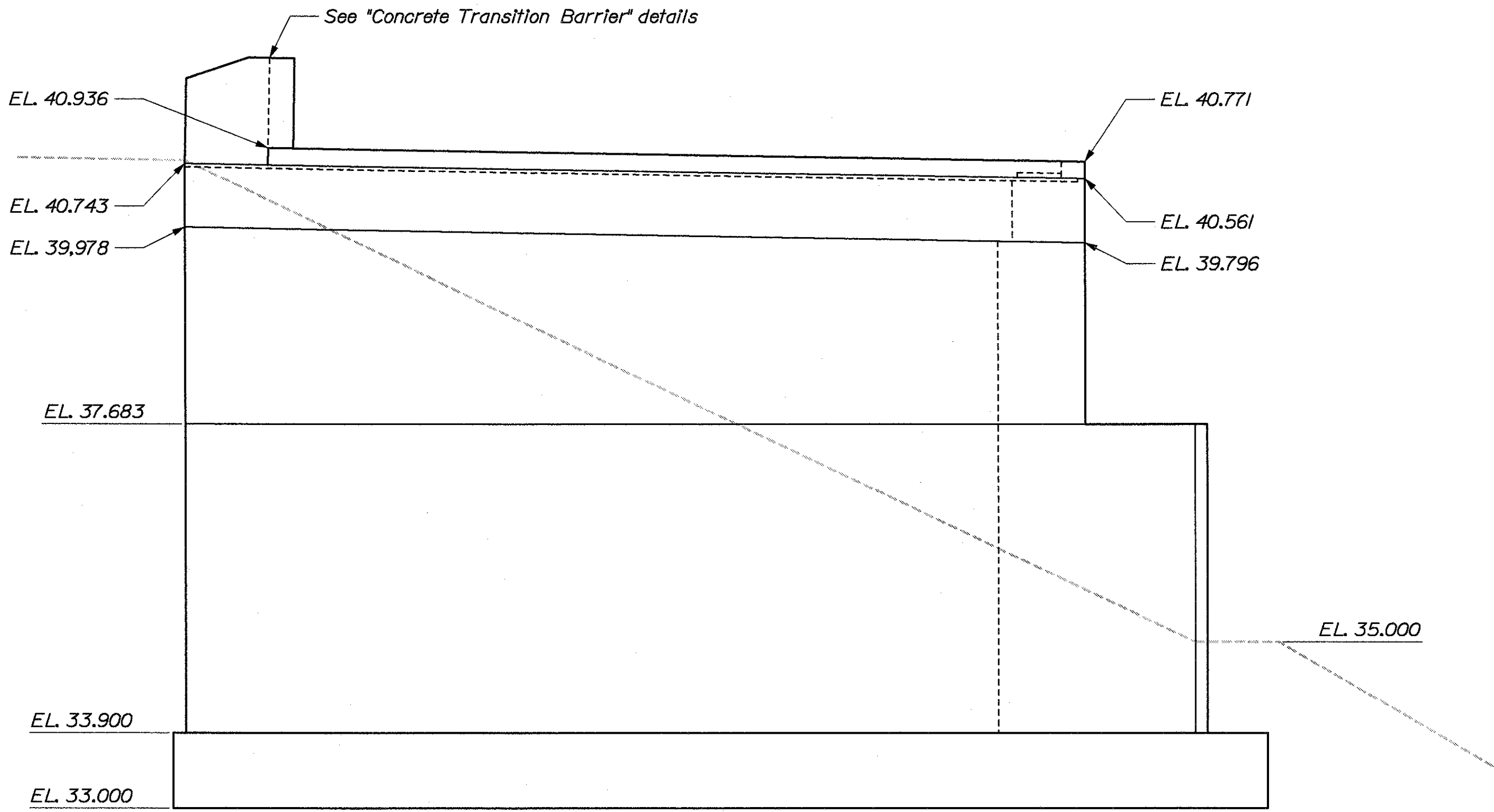
Filename: ... \033\_abut\_1 wingwalls.dgn

Username: Dana.Darren

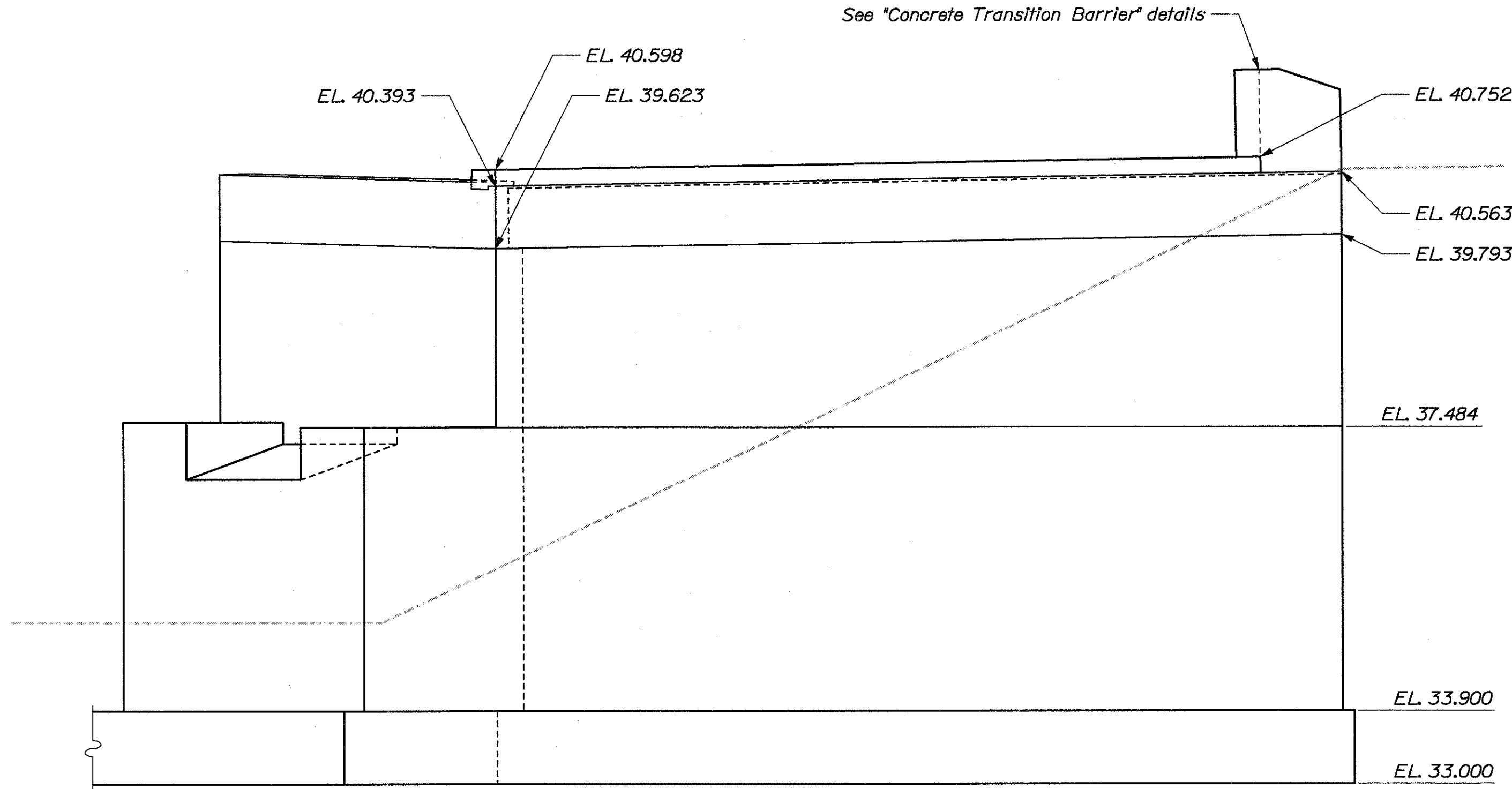
Division: BRIDGE

Date: 26 NOV 2002

| PROJECT DESIGN ENGINEER |                 | BY        | DATE      |
|-------------------------|-----------------|-----------|-----------|
| PLANS                   | DESIGN-DETAILED | J. Folsom | D. Darren |
|                         | CHECKED         |           |           |
|                         | REVISIONS       |           |           |
|                         | FIELD CHANGES   |           |           |



ABUTMENT NO. 1 SOUTH WINGWALL ELEVATION



ABUTMENT NO. 1 NORTH WINGWALL ELEVATION

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 33           | 105             |

000556.22

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUT. NO. 1 WINGWALLS**

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ... \034\_abut\_1 reinforcing.dgn

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

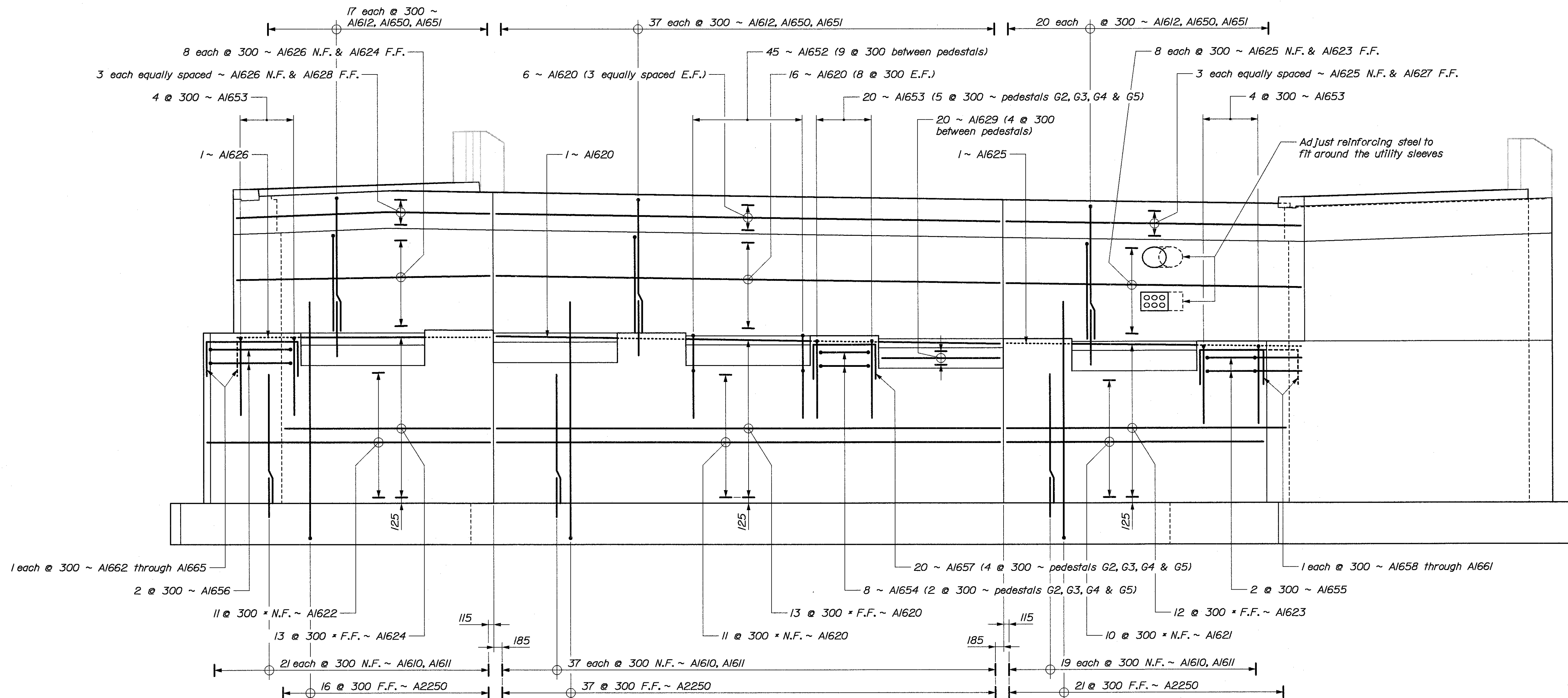
|       |
|-------|
| PLANS |
|-------|

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 34           | 105             |

000556.22



**ABUTMENT NO. 1 REINFORCING**  
\* Adjust spacing to fit cross - slope of breastwall

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUT. NO. 1 REINFORCING**

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

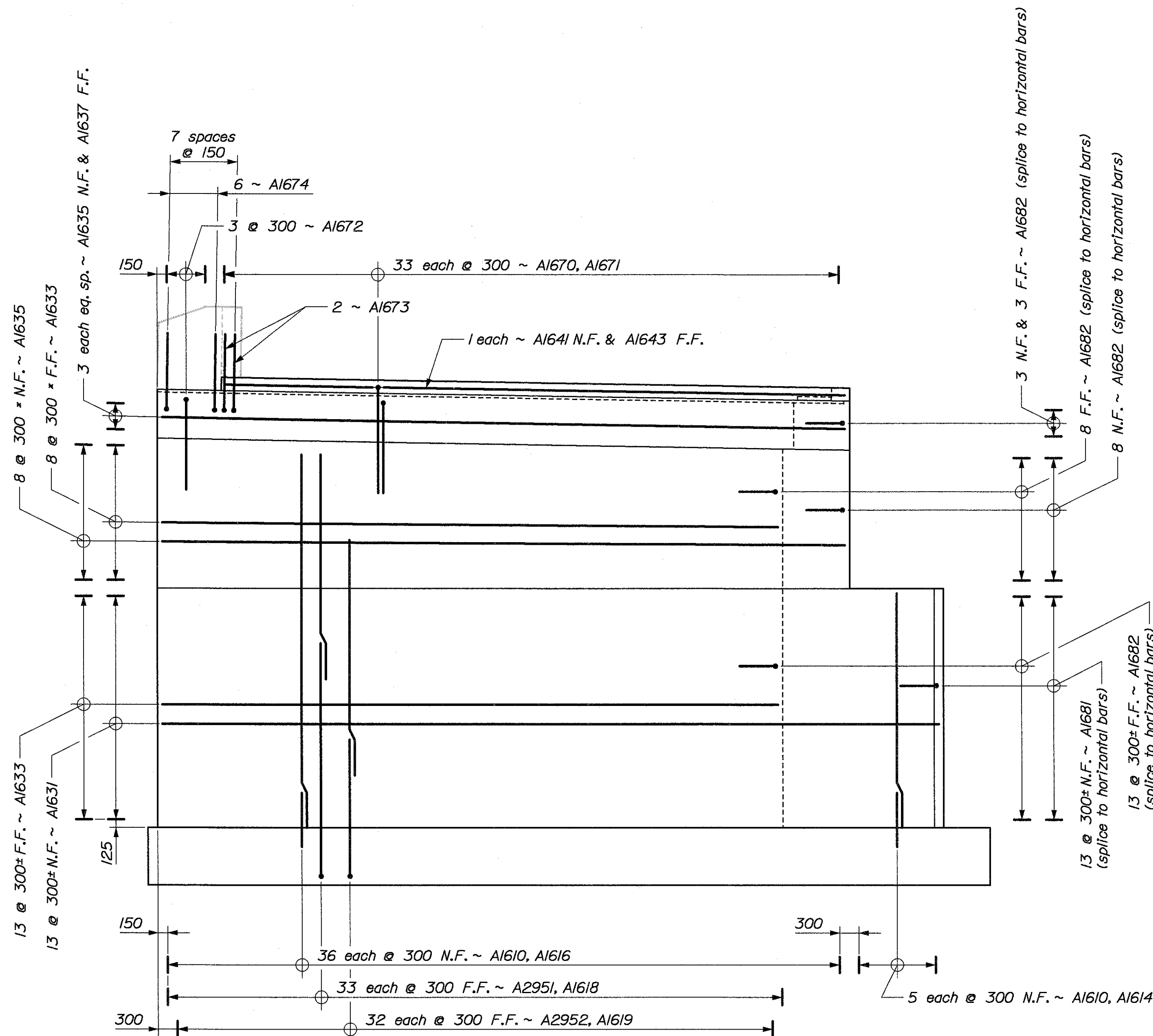
PLANS

**METRIC**

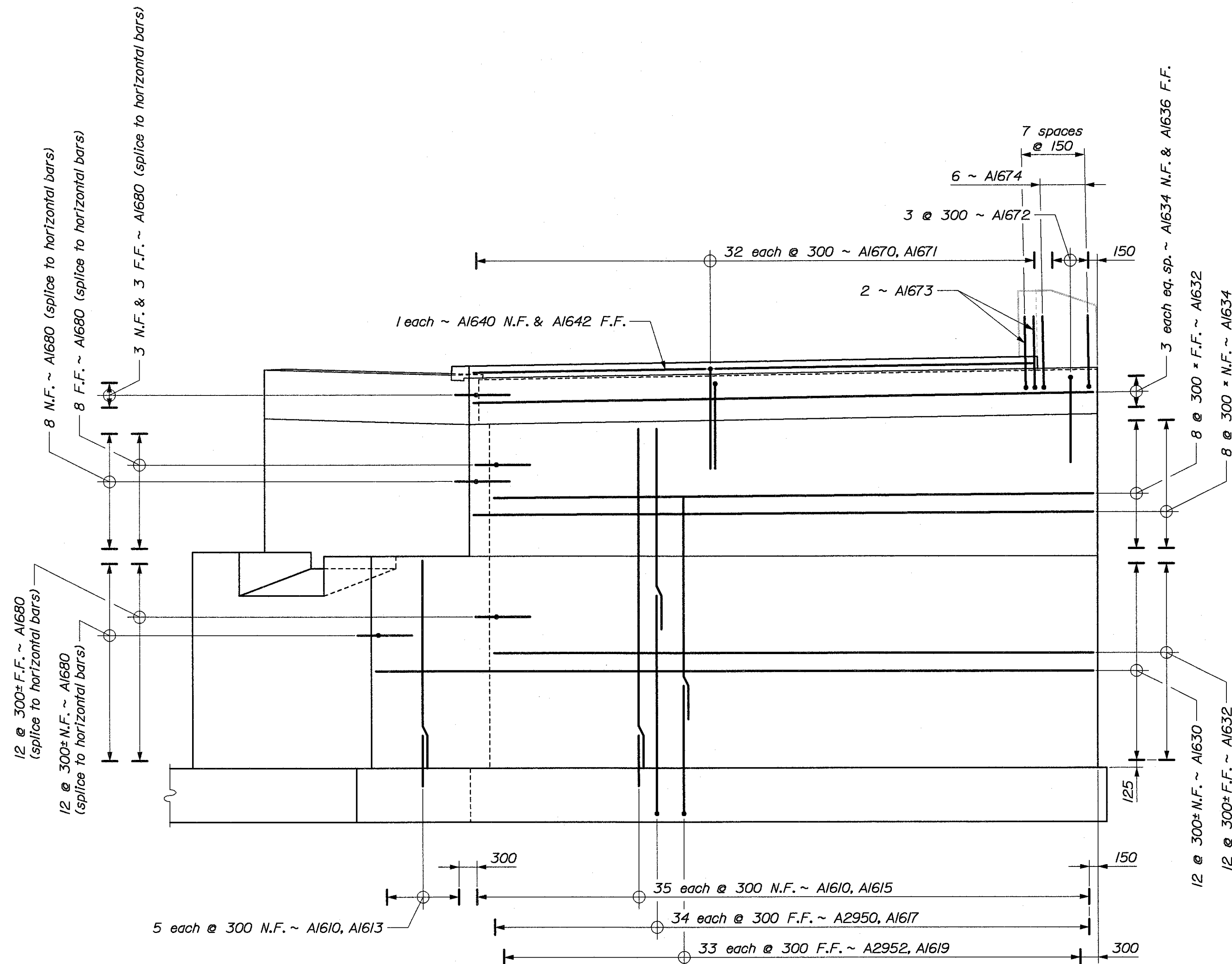
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 35           | 105             |

000556.22



**ABUTMENT NO. 1 SOUTH WINGWALL REINFORCING**  
\* Adjust spacing to fit slope of wingwall



**ABUTMENT NO. 1 NORTH WINGWALL REINFORCING**  
\* Adjust spacing to fit slope of wingwall

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

**ABUT. NO. 1 WINGWALL REINF.**

SHEET OF AUGUSTA, MAINE

Filename: ... \036\_abut\_1 sections.dgn

Username: Dana.Damren

Division: BRIDGE

Date: 26 NOV 2002

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

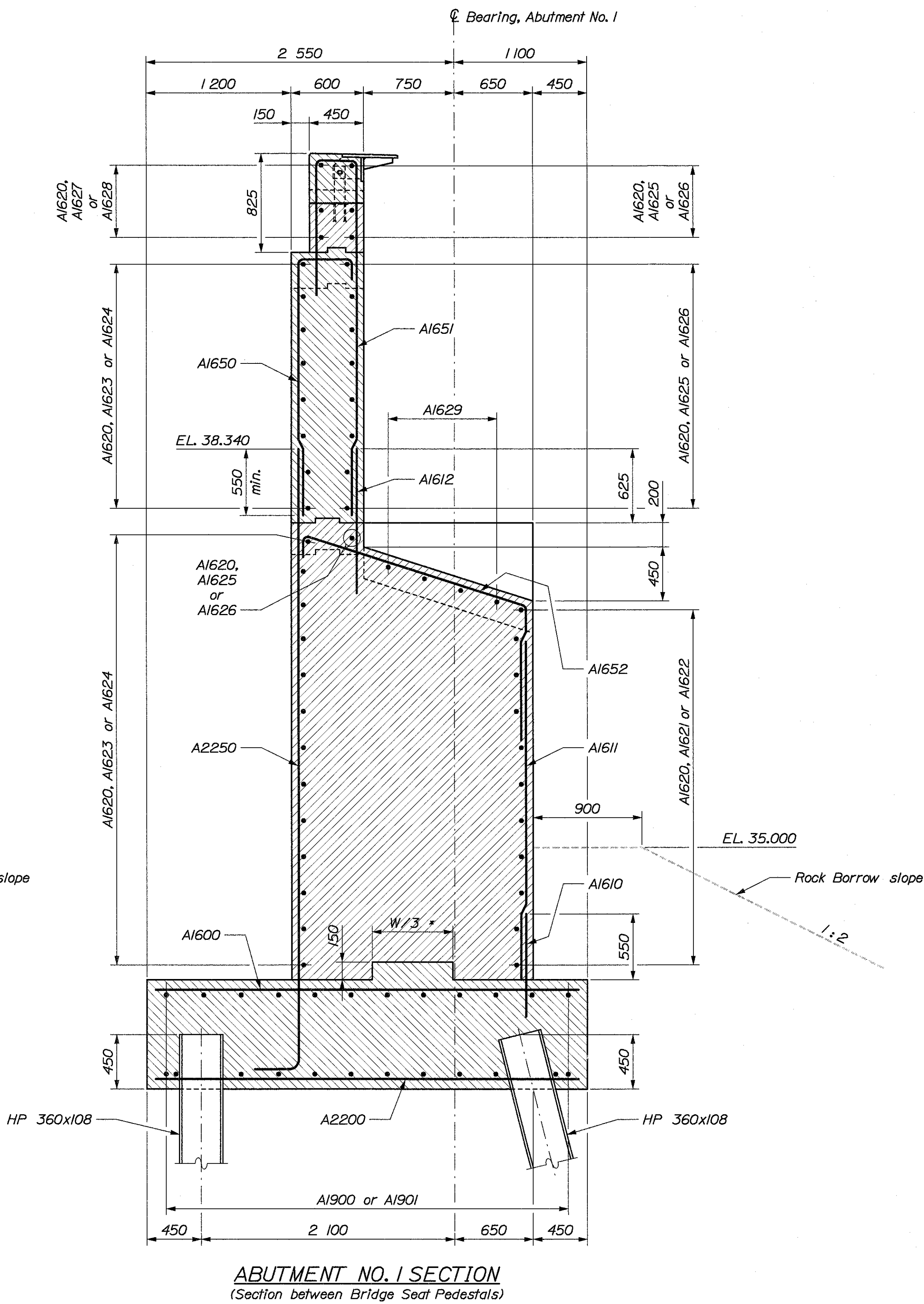
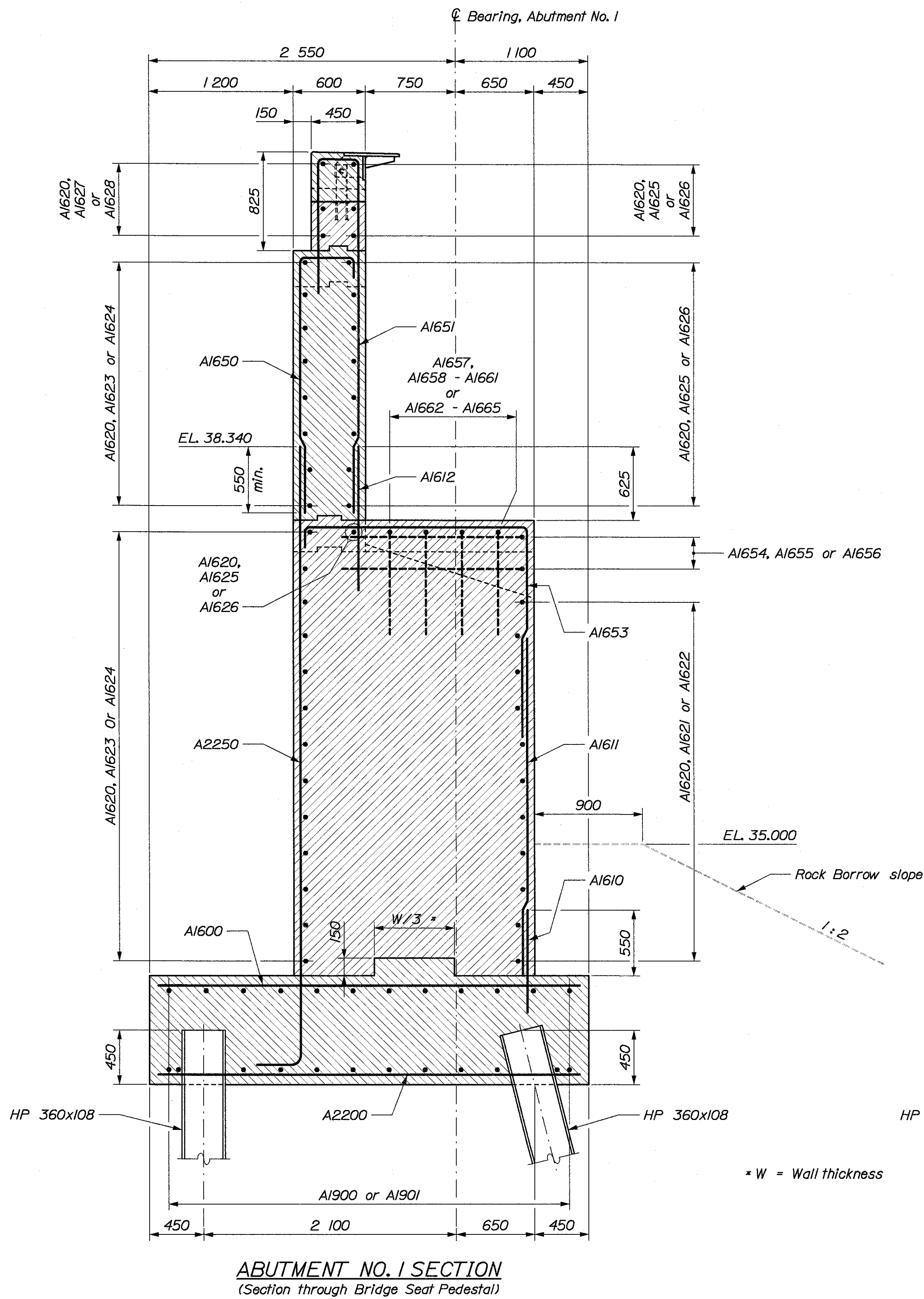
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 36           | 105             |

000556.22



BRIDGE NO. 6400  
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUT. NO. 1 SECTIONS**

SHEET OF AUGUSTA, MAINE

Filename: ... \037\_abut\_1 sections.dgn

Project: 037 - abutment sections

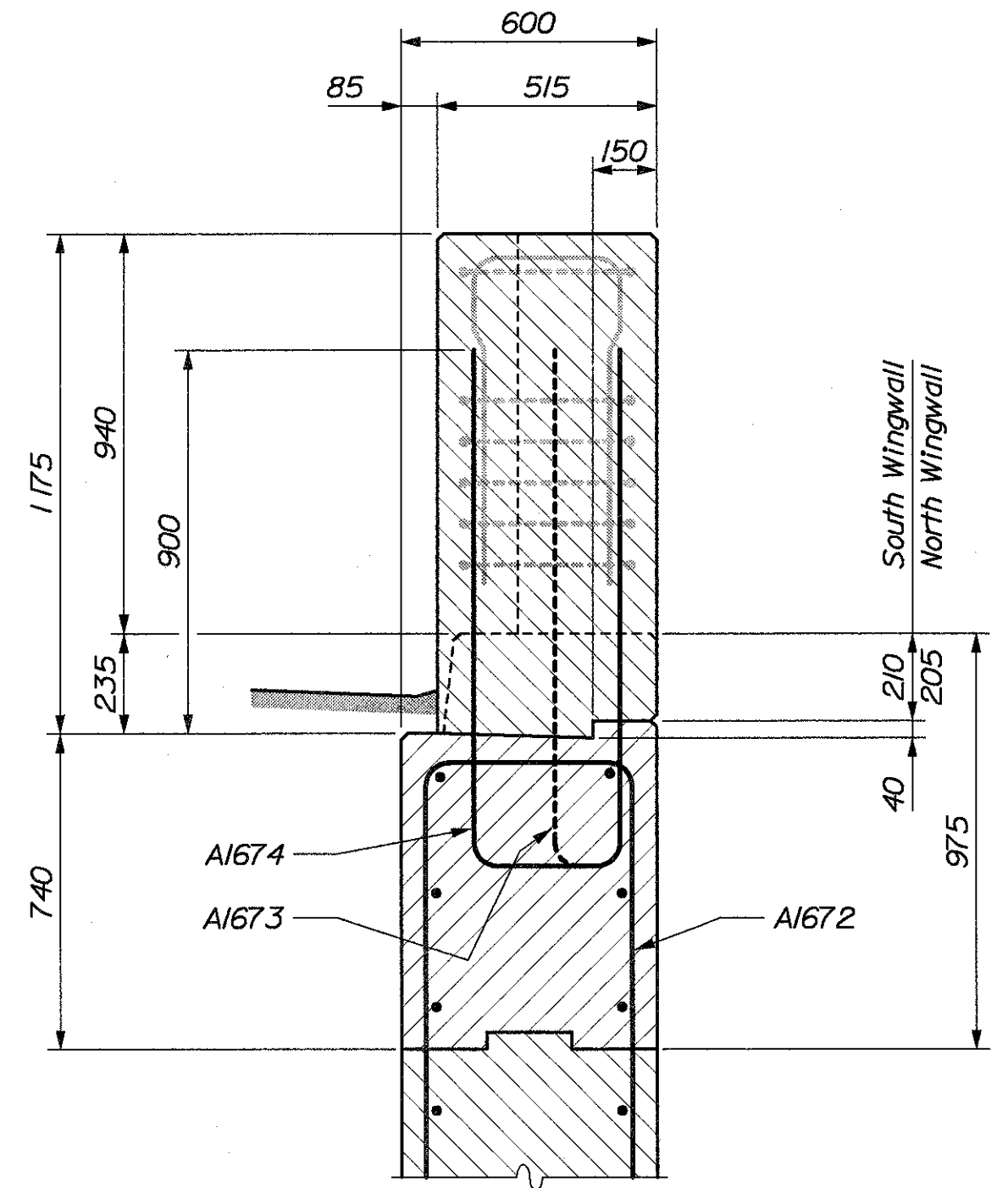
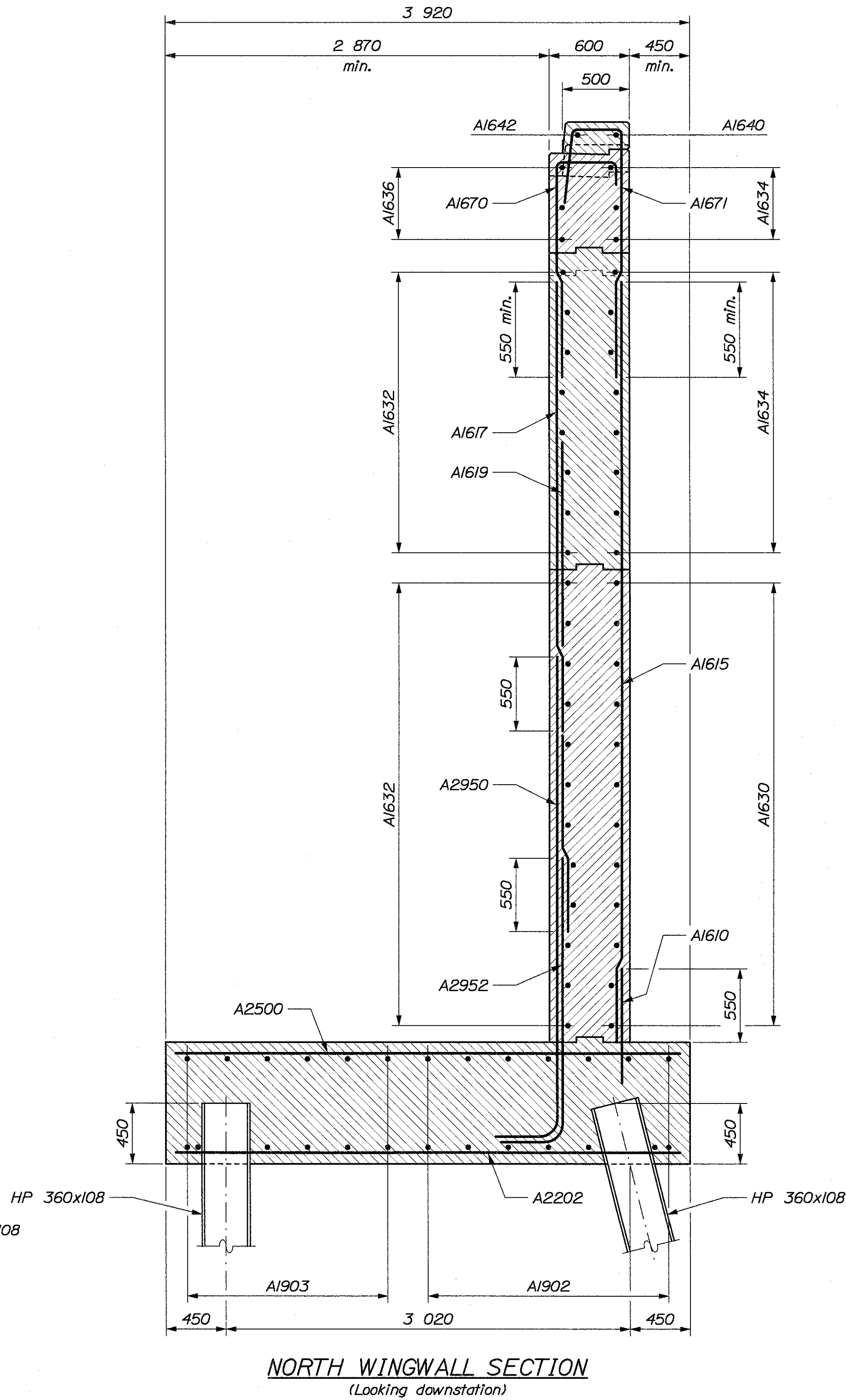
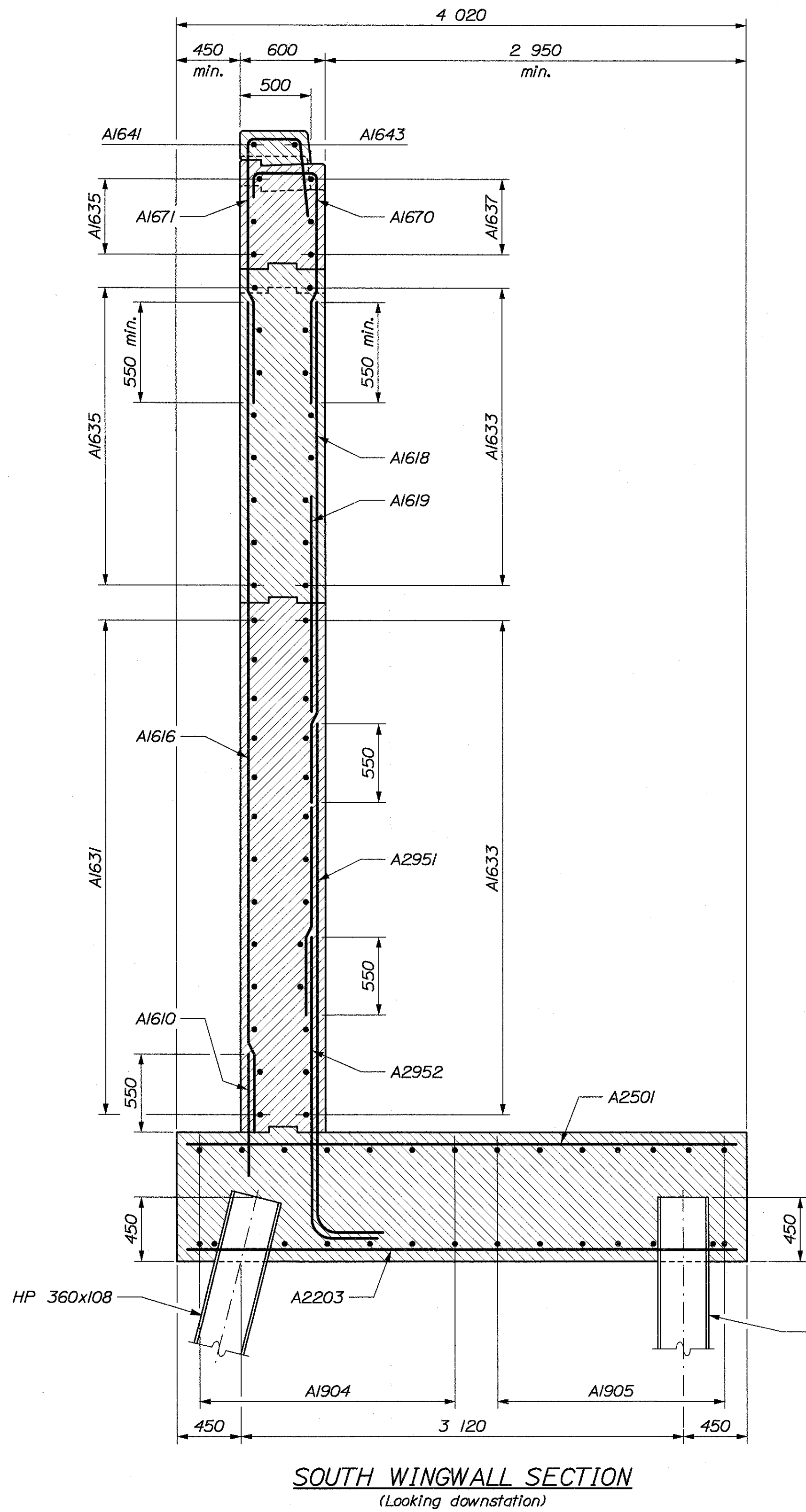
Division: BRIDGE

Date: 26 NOV 2002

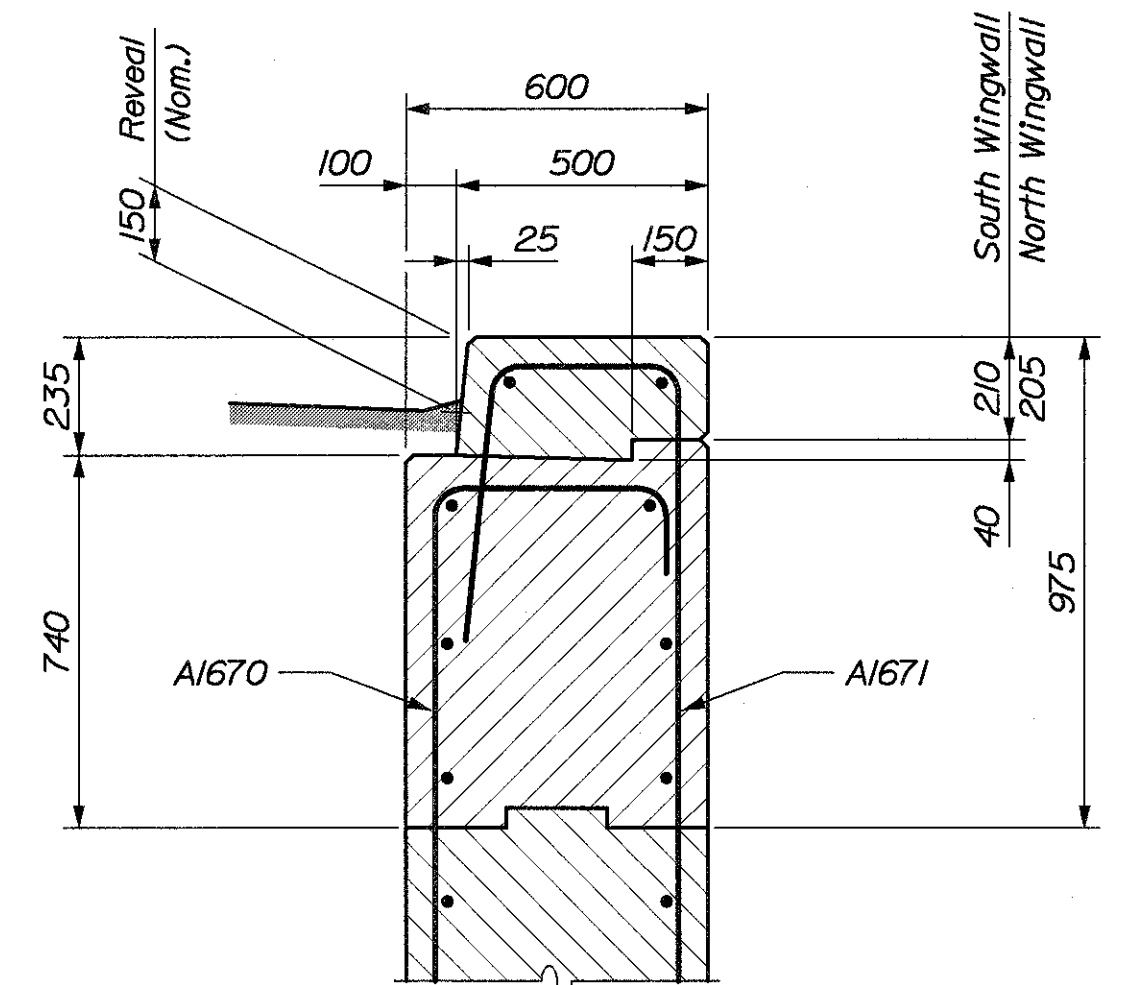
Username: Dana.Darren

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Darren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS



SECTION THROUGH TRANSITION BARRIER



TYPICAL TOP OF WINGWALL SECTION

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

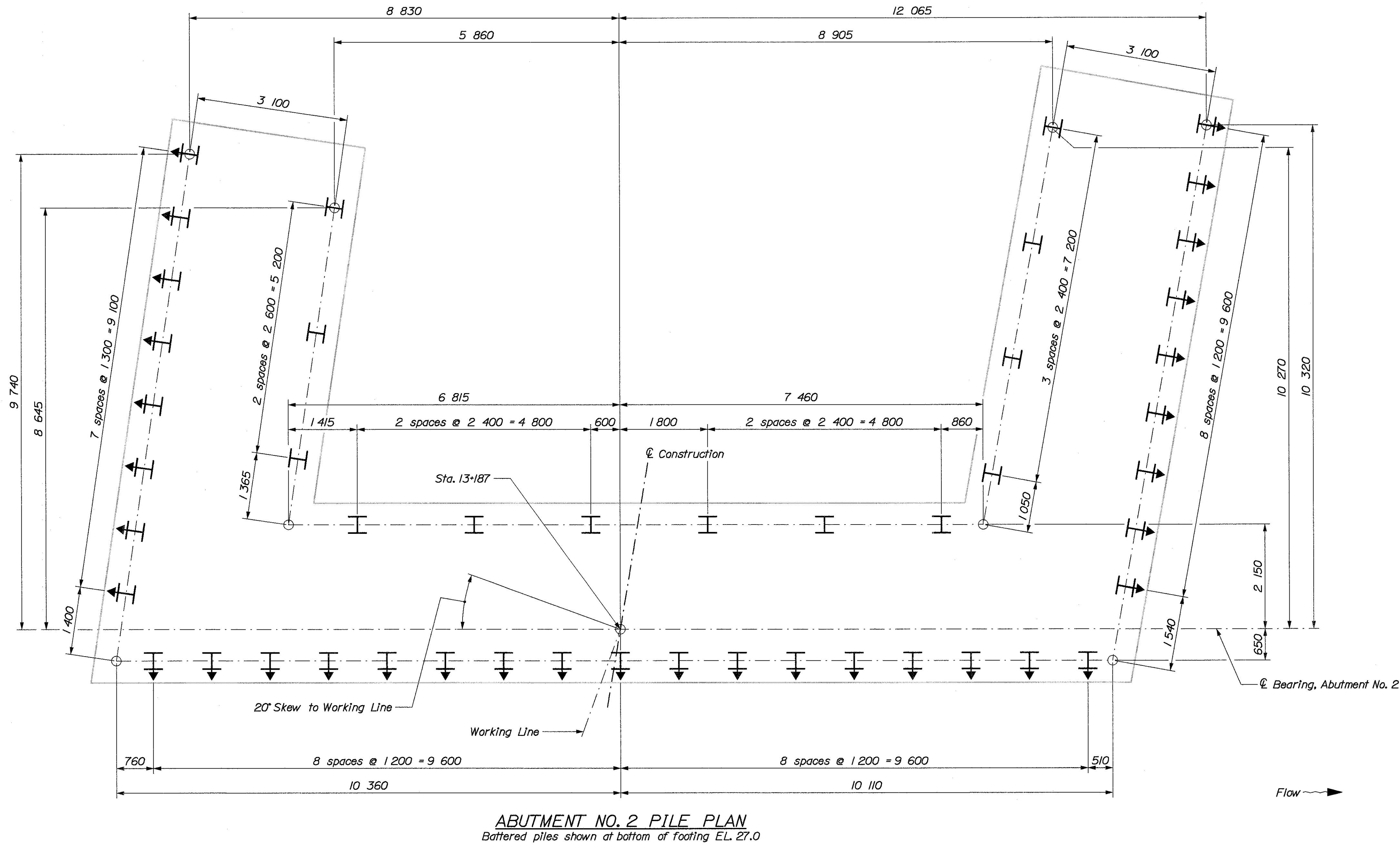
STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

**ABUT. NO. 1 SECTIONS**

SHEET OF AUGUSTA, MAINE

|        |                                                                                                               |  |                   |       |                |              |                 |
|--------|---------------------------------------------------------------------------------------------------------------|--|-------------------|-------|----------------|--------------|-----------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. |  | FILWA<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|        |                                                                                                               |  | 1                 | MAINE | NH-0556(220)E  | 38           | 105             |

000556.22



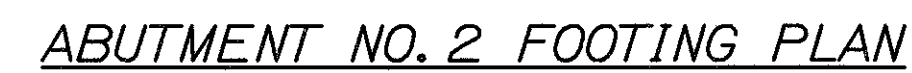
|                                  |           |               |
|----------------------------------|-----------|---------------|
| PROJECT DESIGN ENGINEER<br>PLANS | BY        | DATE          |
|                                  | J. Folsom |               |
|                                  | D. Damren |               |
|                                  |           |               |
| CHECKED                          | REVISIONS | FIELD CHANGES |
|                                  |           |               |
|                                  |           |               |
|                                  |           |               |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUT. NO. 2 PILE PLAN**

SHEET OF AUGUSTA, MAINE



BRIDGE NO. 6400

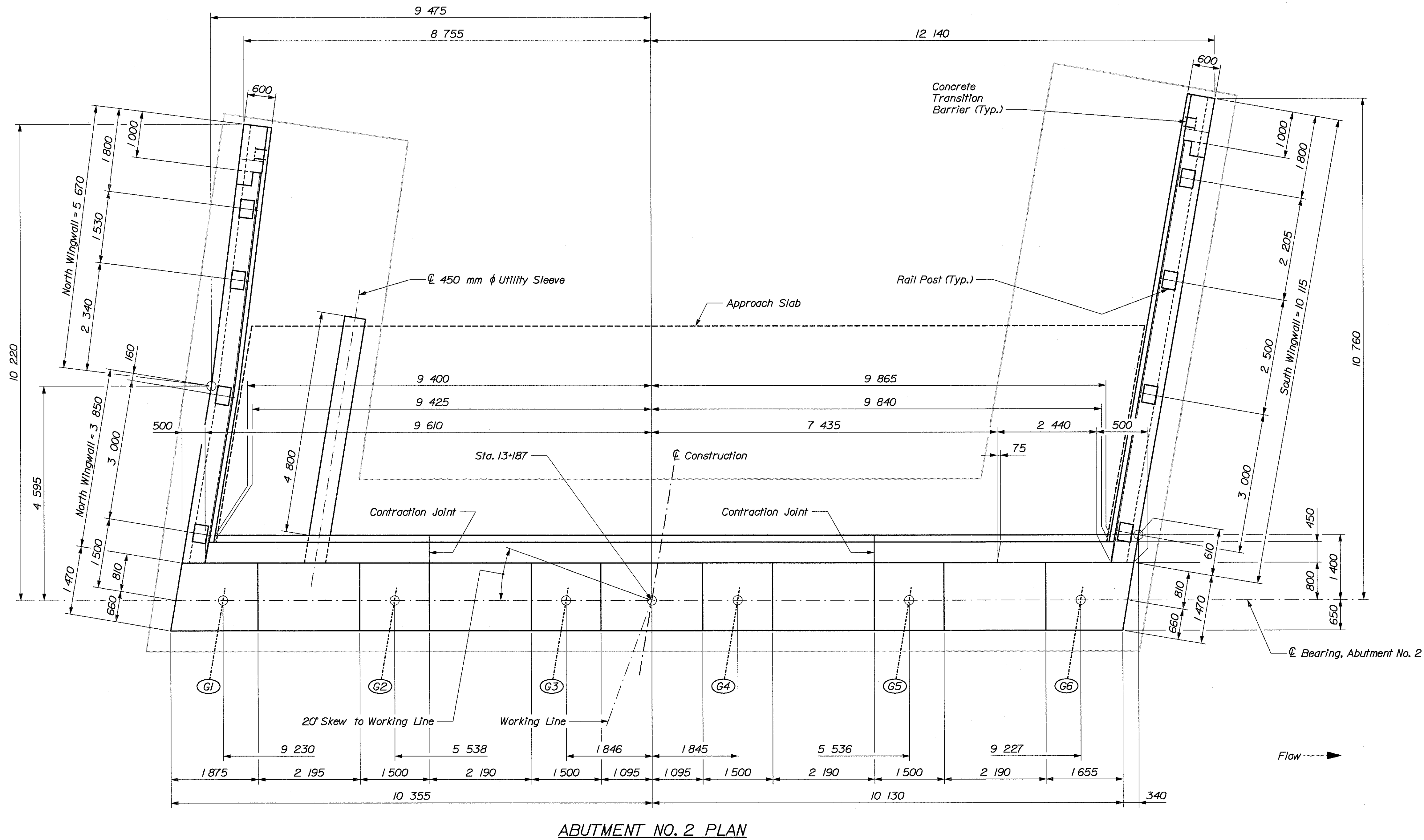
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

*ABUT. NO. 2 FOOTING PLAN*

|                         |           |           |
|-------------------------|-----------|-----------|
| PROJECT DESIGN ENGINEER | BY        | DATE      |
| DESIGN-DETAILED         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

# PLANS



ABUTMENT NO. 2 PLAN

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUTMENT NO. 2 PLAN**

Date: 26 NOV 2002

Username: Dana.Damren

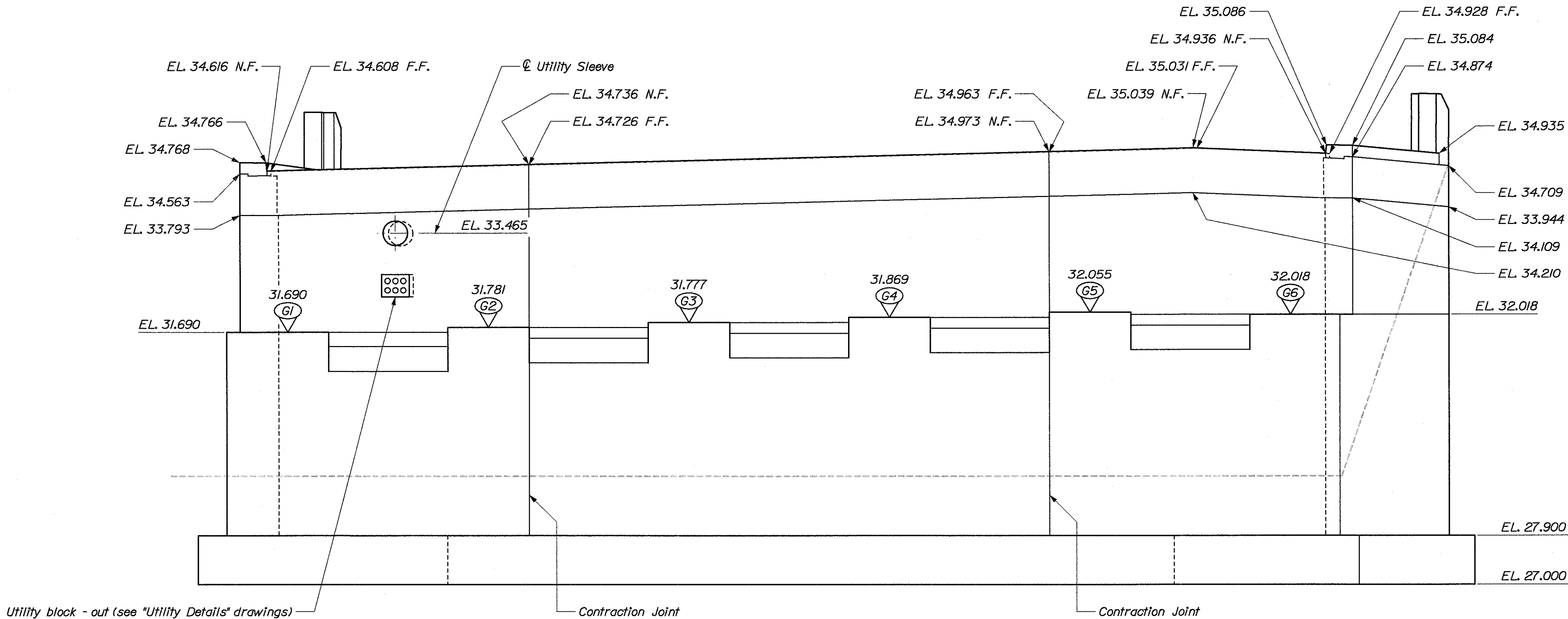
Division: BRIDGE

Filename: ...\\041\_abut\_2\_elevation.dgn

|                         |                 |           |           |
|-------------------------|-----------------|-----------|-----------|
| PROJECT DESIGN ENGINEER | DESIGN-DETAILED | BY        | DATE      |
|                         | CHECKED         | J. Folsom | D. Damren |
|                         | REVISIONS       |           |           |
|                         | FIELD CHANGES   |           |           |
| PLANS                   |                 |           |           |

|        |                                                              |  |                   |       |                |           |              |
|--------|--------------------------------------------------------------|--|-------------------|-------|----------------|-----------|--------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted. |  | F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|        | 2. All elevations and stations are in meters.                |  | 1                 | MAINE | NH-0556(220)E  | 41        | 105          |

000556.22



ABUTMENT NO. 2 ELEVATION

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
ABUT. NO. 2 ELEVATION

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

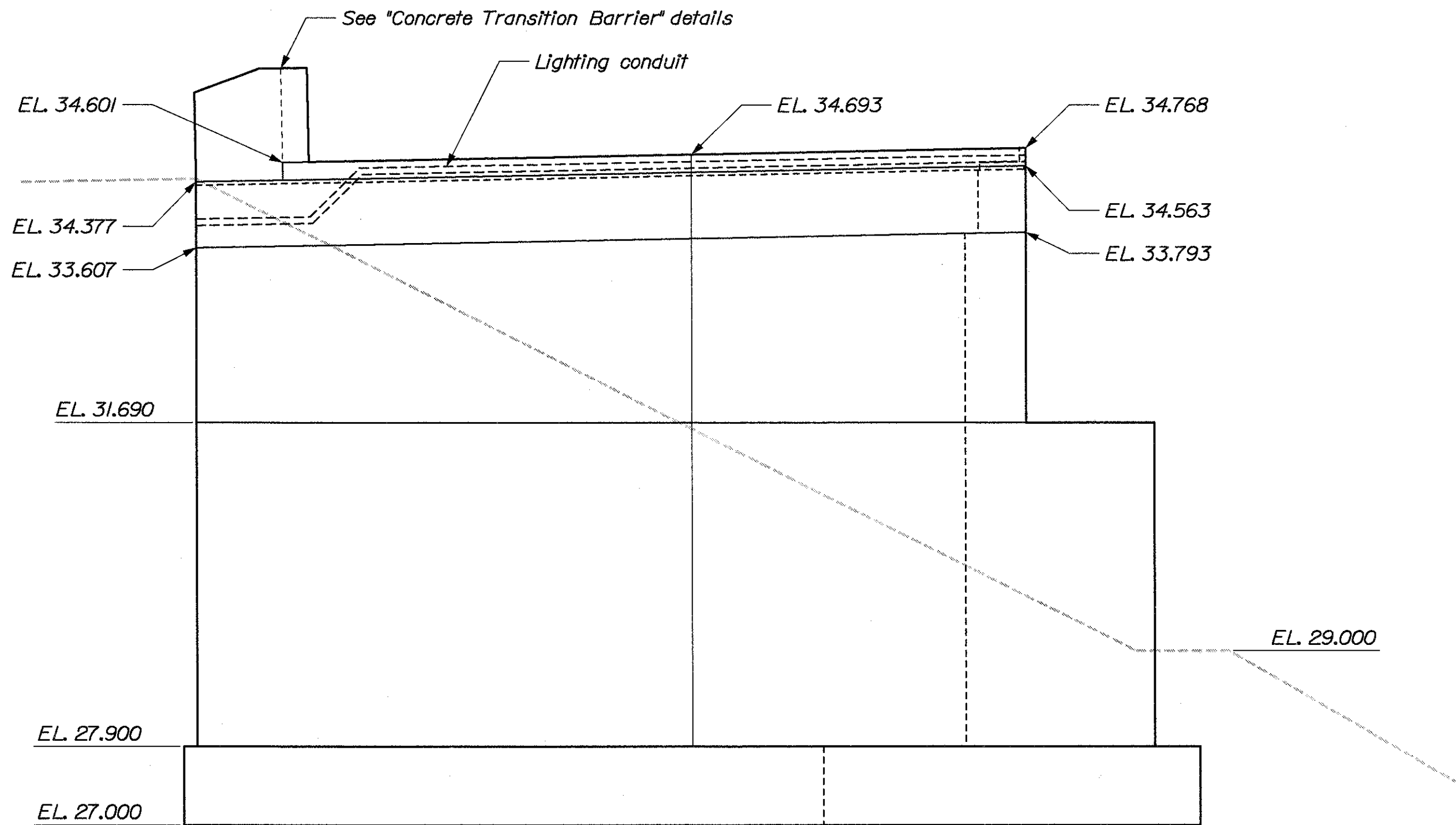
Filename: ...\\042\_abut\_2\_wingwalls.dgn

|                         |           |           |
|-------------------------|-----------|-----------|
| PROJECT DESIGN ENGINEER | BY        | DATE      |
| DESIGN-DETAILED         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

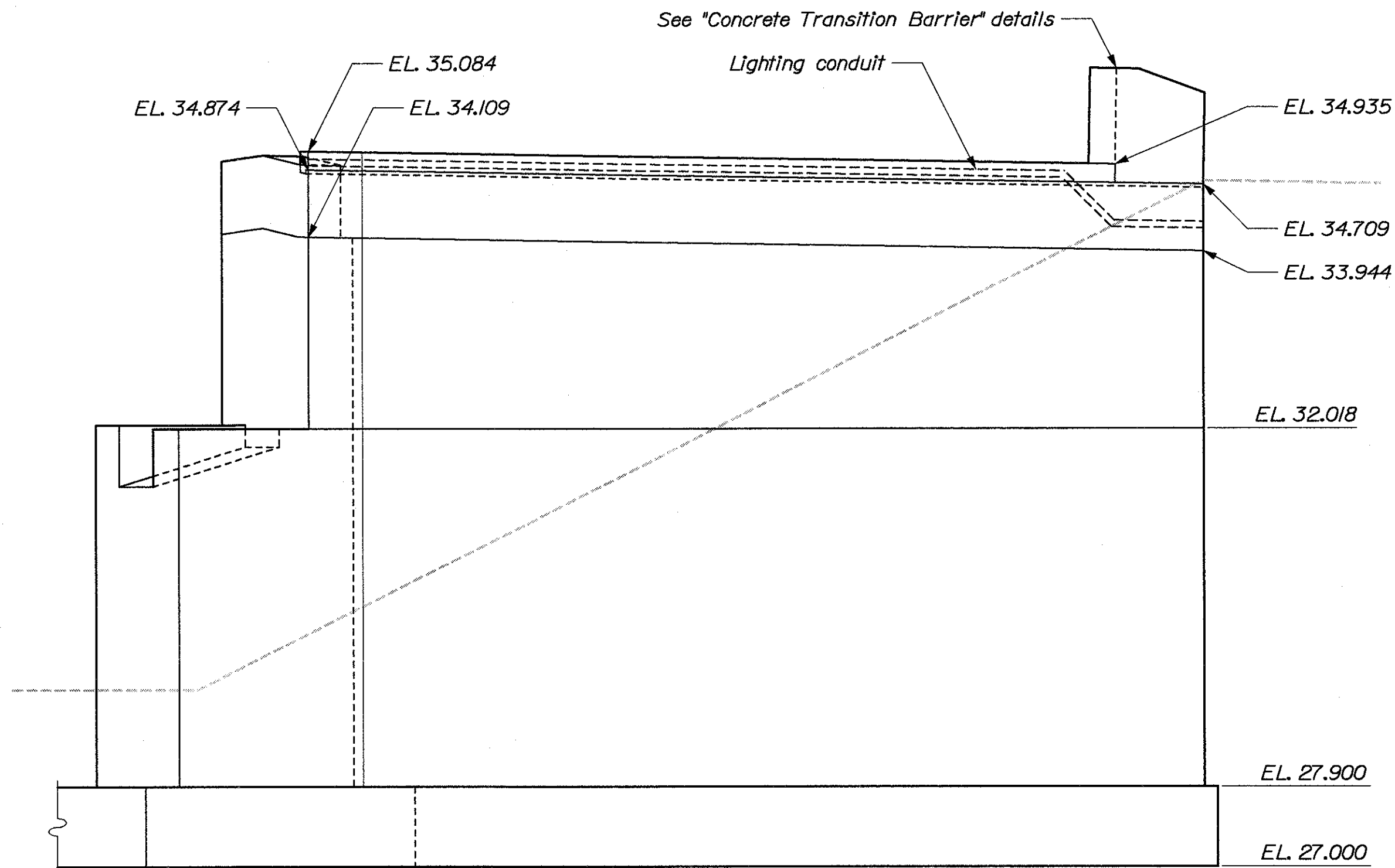
PLANS

|        |                                                                                                               |       |                |           |              |
|--------|---------------------------------------------------------------------------------------------------------------|-------|----------------|-----------|--------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. |       |                |           |              |
|        | F.H.W.A. REG. NO.                                                                                             | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|        | 1                                                                                                             | MAINE | NH-0556(220)E  | 42        | 105          |

000556,22



ABUTMENT NO. 2 NORTH WINGWALL ELEVATION



ABUTMENT NO. 2 SOUTH WINGWALL ELEVATION

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUT. NO. 2 WINGWALLS**

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ...\\043\_abut\_2\_reinforcing.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

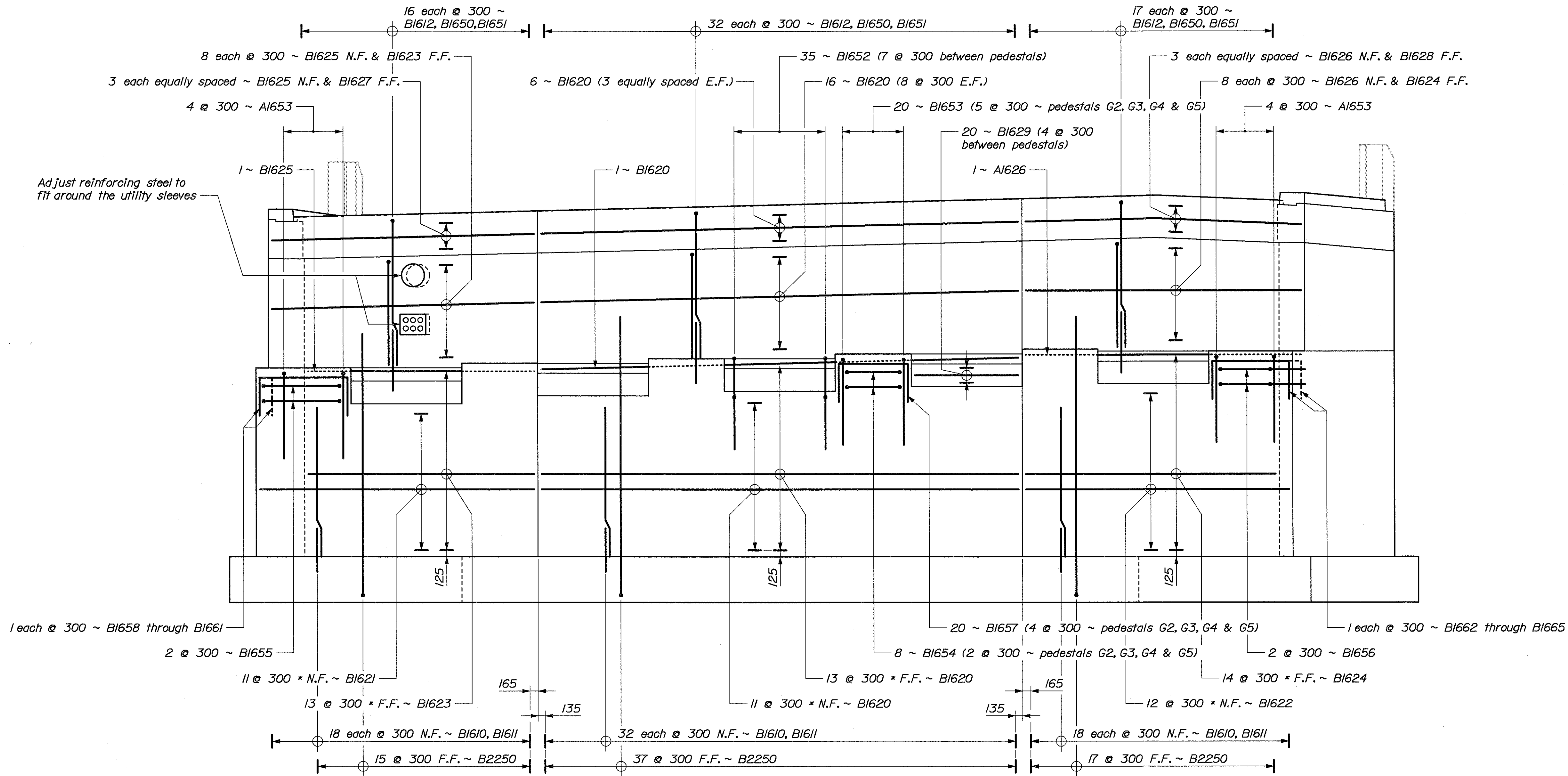
|       |
|-------|
| PLANS |
|-------|

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|-----------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 43        | 105             |

000556.22



ABUTMENT NO. 2 REINFORCING  
\* Adjust spacing to fit cross-slope of breastwall

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUT. NO. 2 REINFORCING**

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

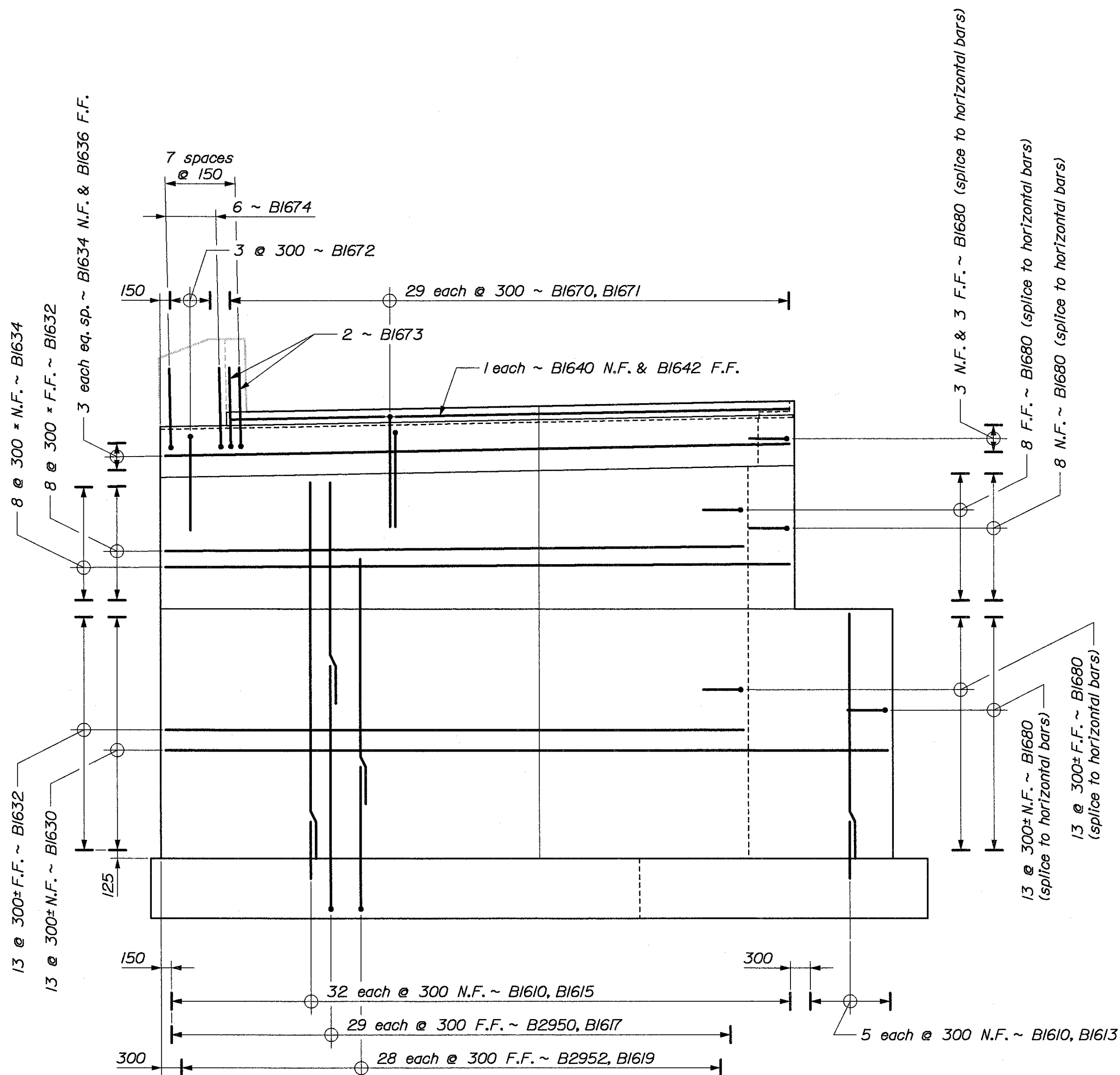
PLANS

METRIC

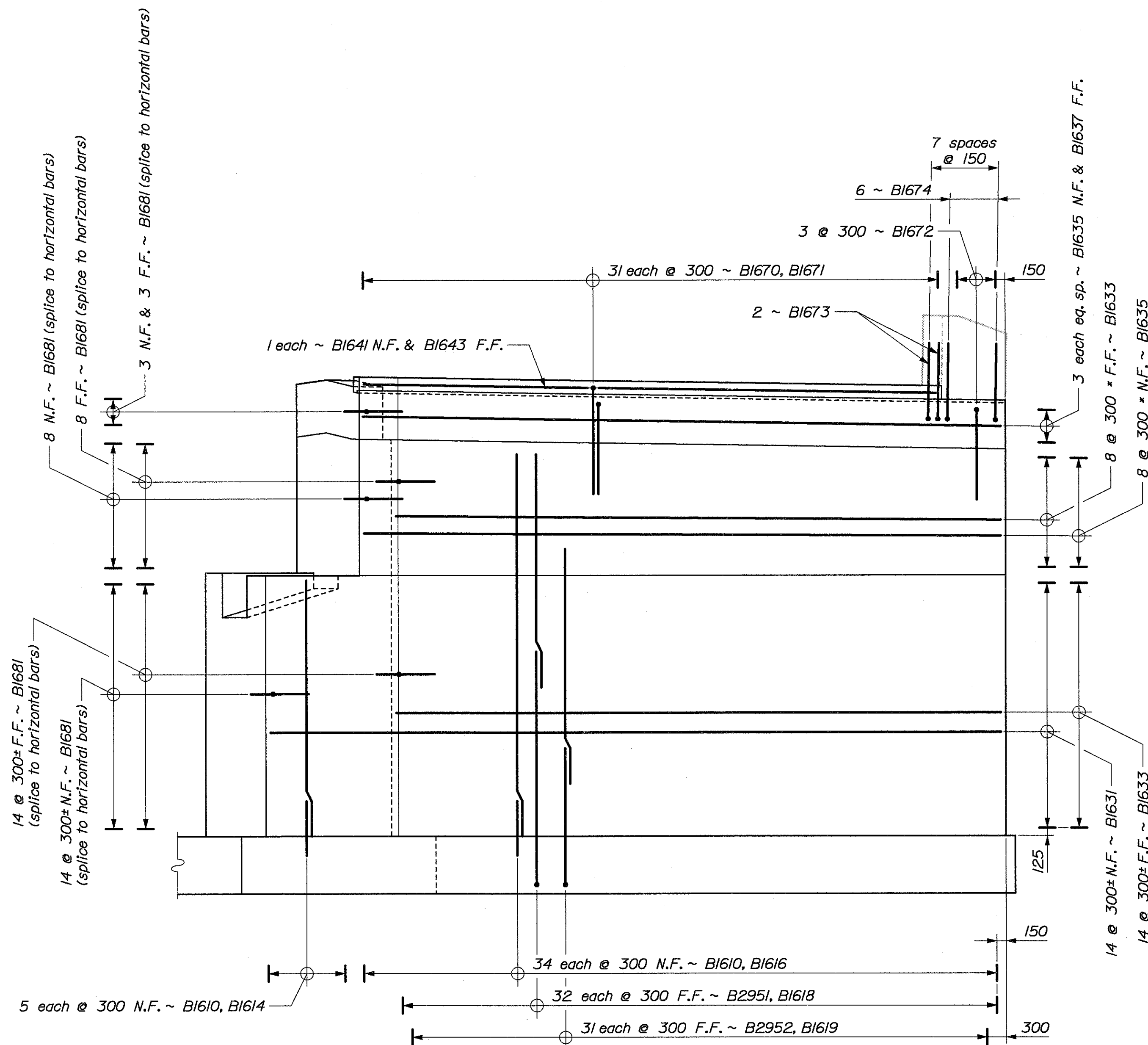
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| FHWA<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|------------------|-------|----------------|--------------|-----------------|
| 1                | MAINE | NH-0556(220)E  | 44           | 105             |

000556.22



**ABUTMENT NO. 2 NORTH WINGWALL REINFORCING**  
\* Adjust spacing to fit slope of wingwall



**ABUTMENT NO. 2 SOUTH WINGWALL REINFORCING**  
\* Adjust spacing to fit slope of wingwall

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

**ABUT. NO. 2 WINGWALL REINF.**

SHEET OF AUGUSTA, MAINE

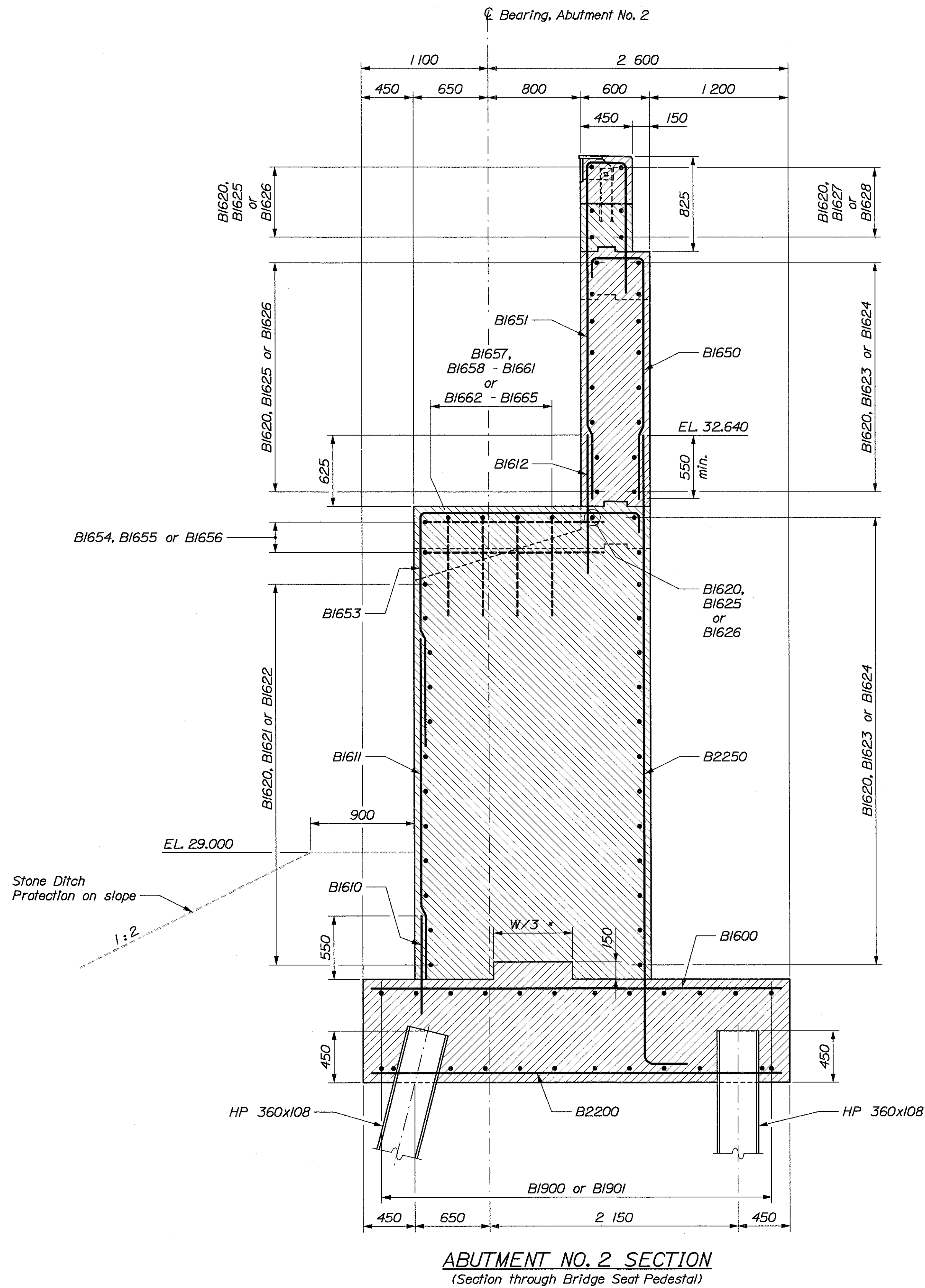
| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 |           |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS

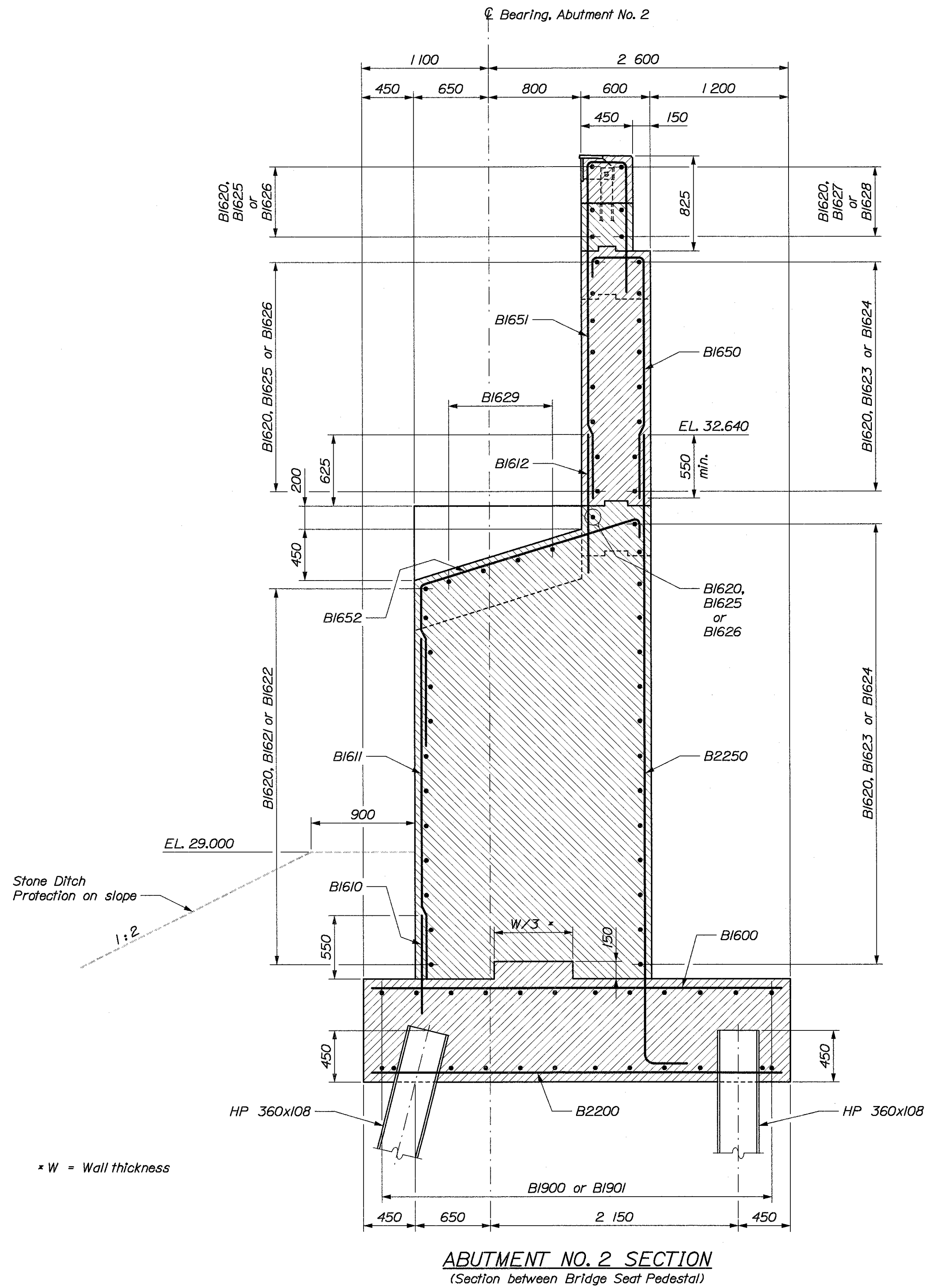
METRIC 1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 45           | 105             |

000556.22



\* W = Wall thickness



BRIDGE NO. 6400

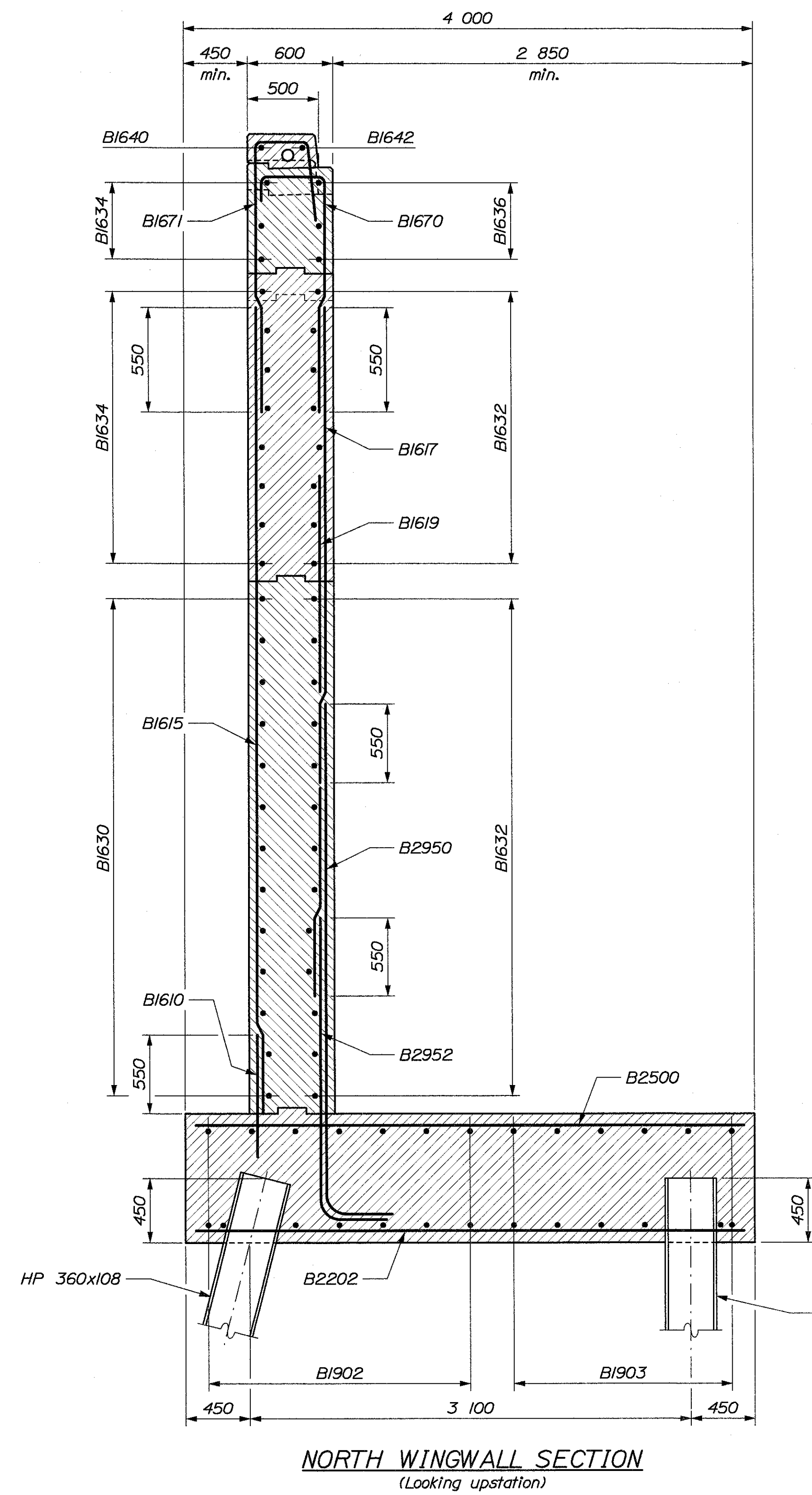
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

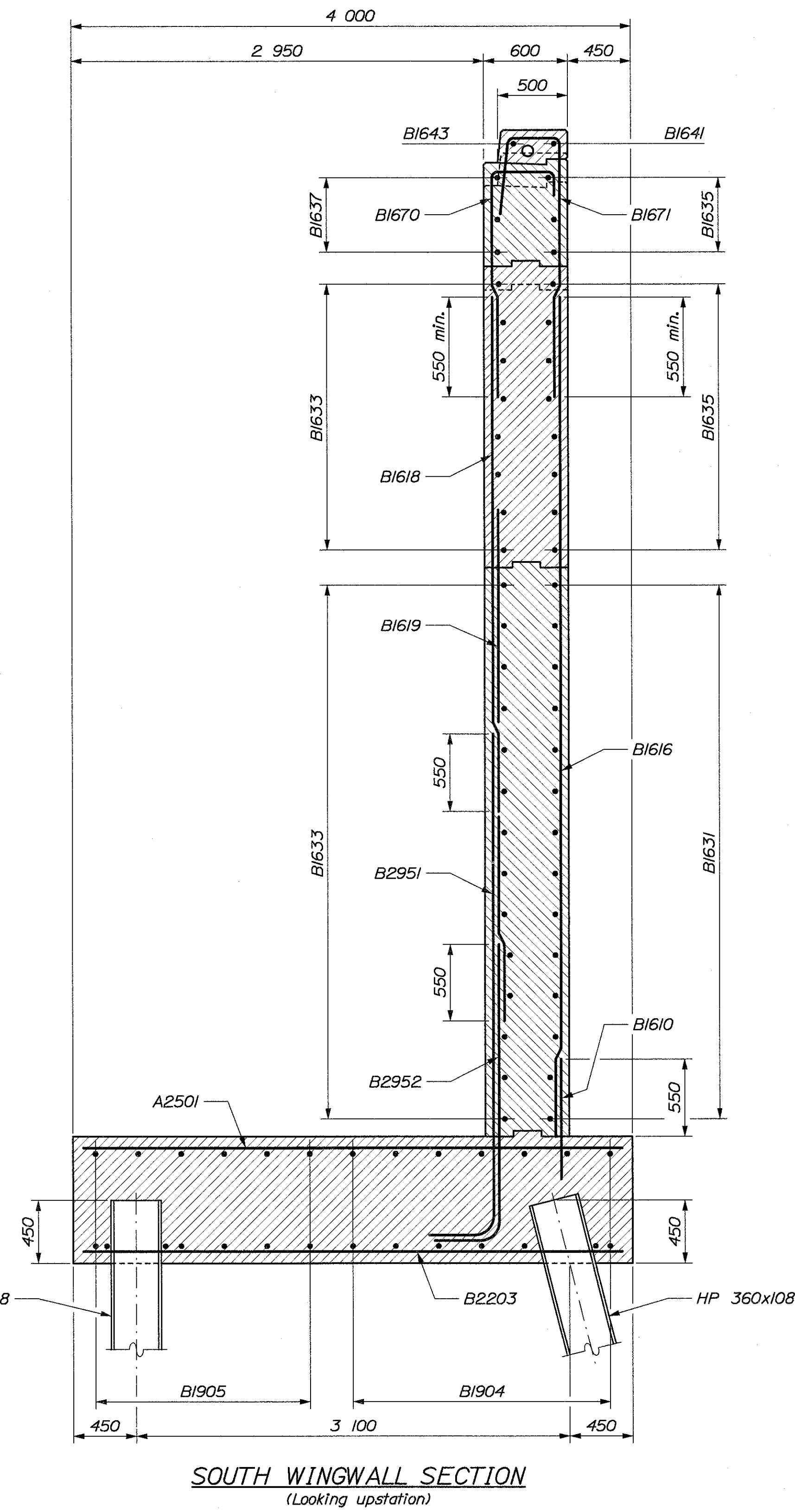
ABUT. NO. 2 SECTIONS

SHEET OF AUGUSTA, MAINE

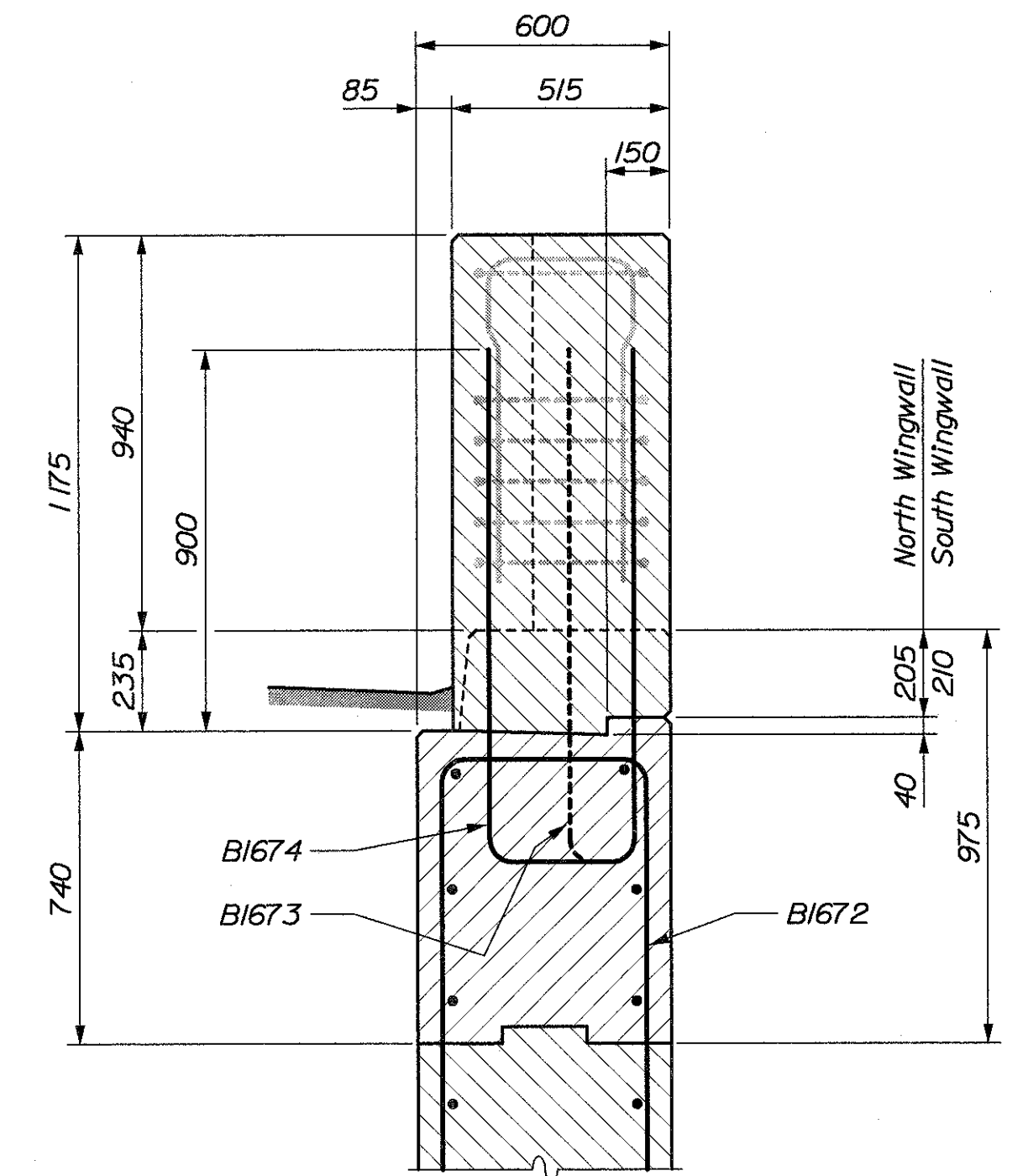
| PROJECT DESIGN ENGINEER |  | BY        | DATE      |
|-------------------------|--|-----------|-----------|
| DESIGN-DETAILED         |  | J. Folsom | D. Dammen |
| CHECKED                 |  |           |           |
| REVISIONS               |  |           |           |
| FIELD CHANGES           |  |           |           |



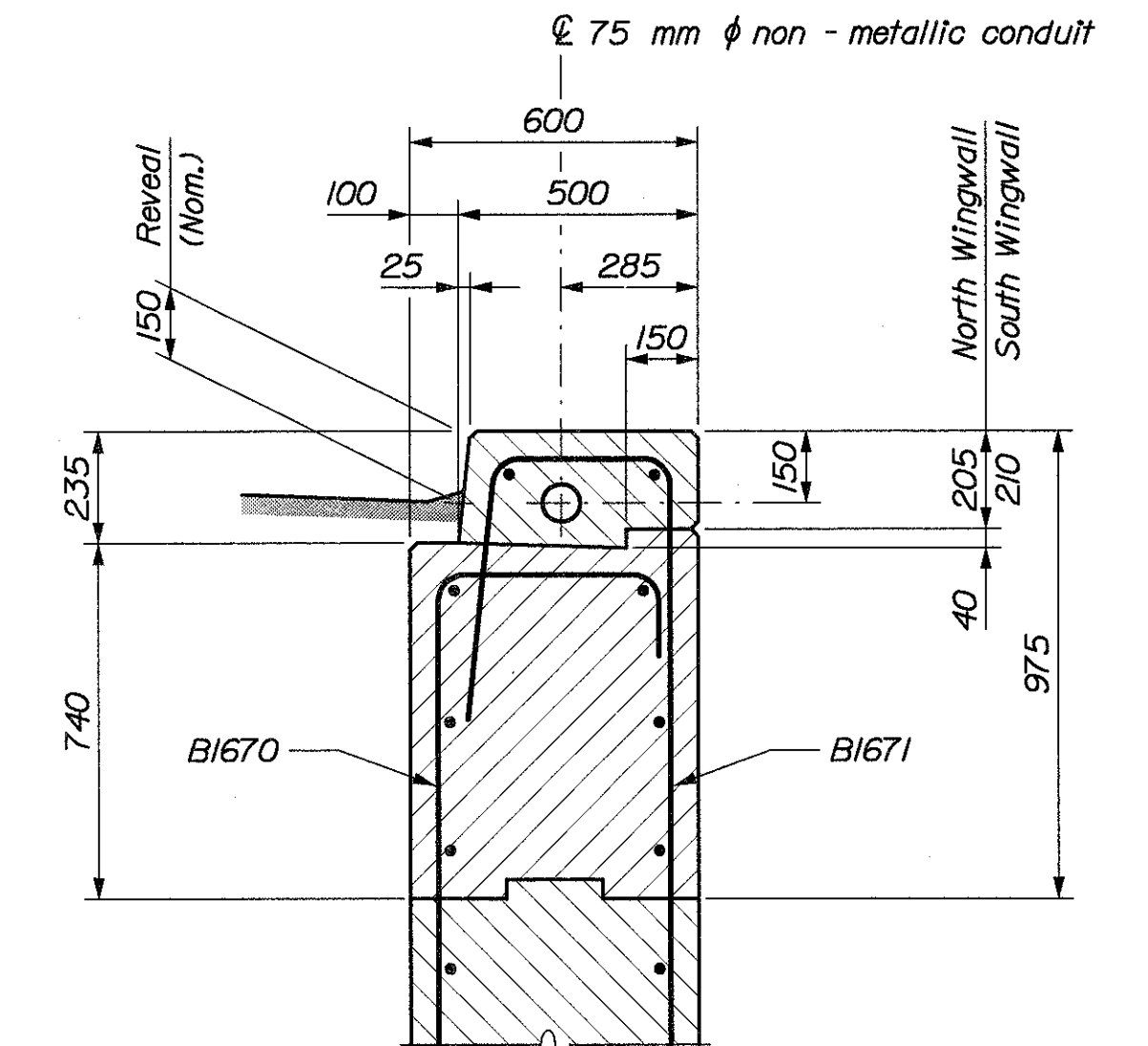
NORTH WINGWALL SECTION  
(Looking upstation)



SOUTH WINGWALL SECTION  
(Looking upstation)



SECTION THROUGH TRANSITION BARRIER



TYPICAL TOP OF WINGWALL SECTION

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
ABUT. NO. 2 SECTIONS

SHEET OF AUGUSTA, MAINE

Username: Dana.Damren

Date: 26 NOV 2002

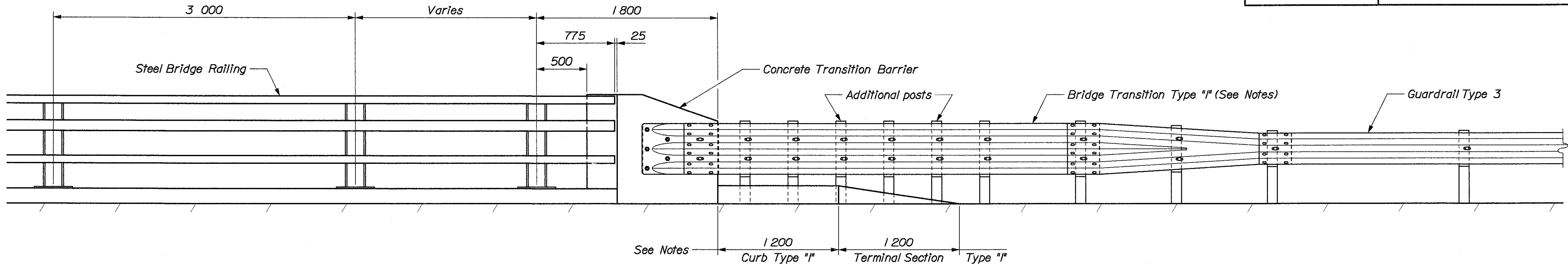
Division: BRIDGE

Filename: ...\\047\_rail\_transition barriers.dgn

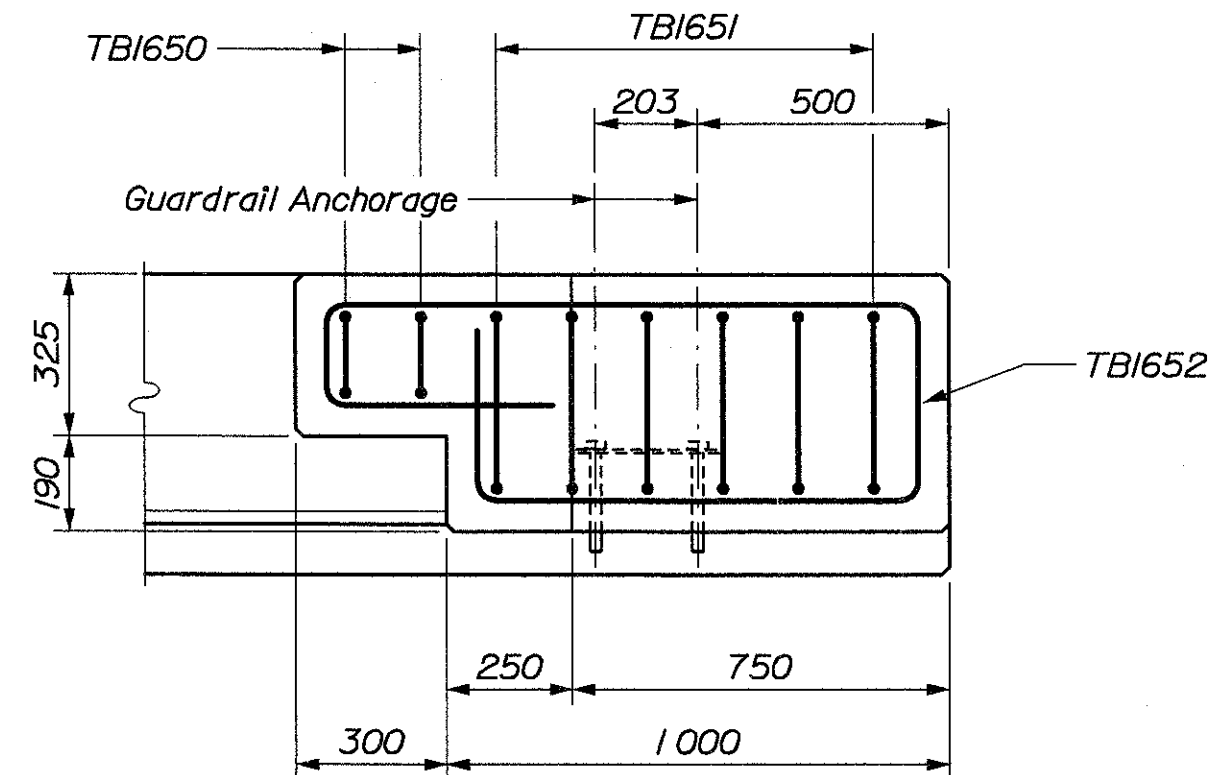
|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 |           |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |
| PLANS                   |           |      |

|        |                                                                                                               |          |       |                |           |              |
|--------|---------------------------------------------------------------------------------------------------------------|----------|-------|----------------|-----------|--------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. | FHWA     | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|        |                                                                                                               | REG. NO. | MAINE | NH-0556(220)E  | 47        | 105          |

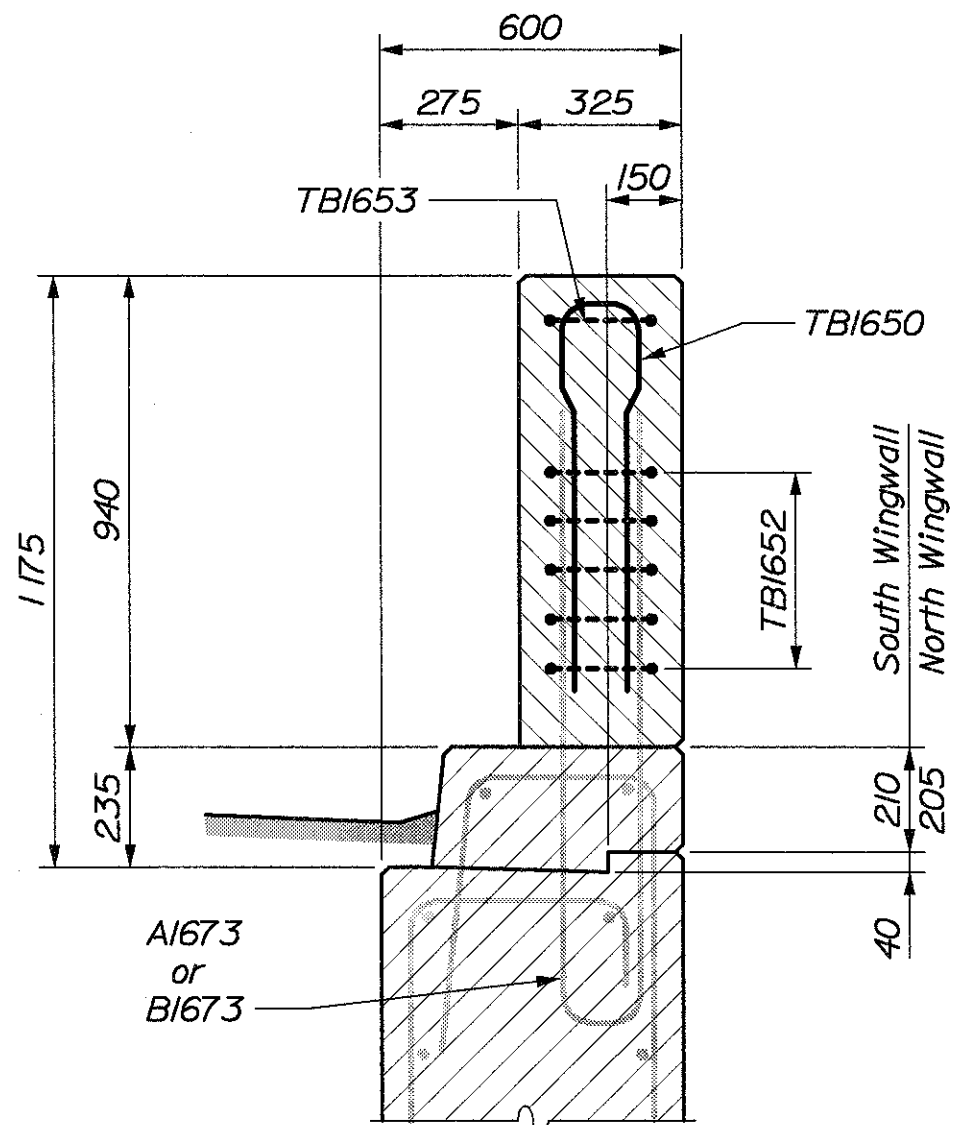
000556.22



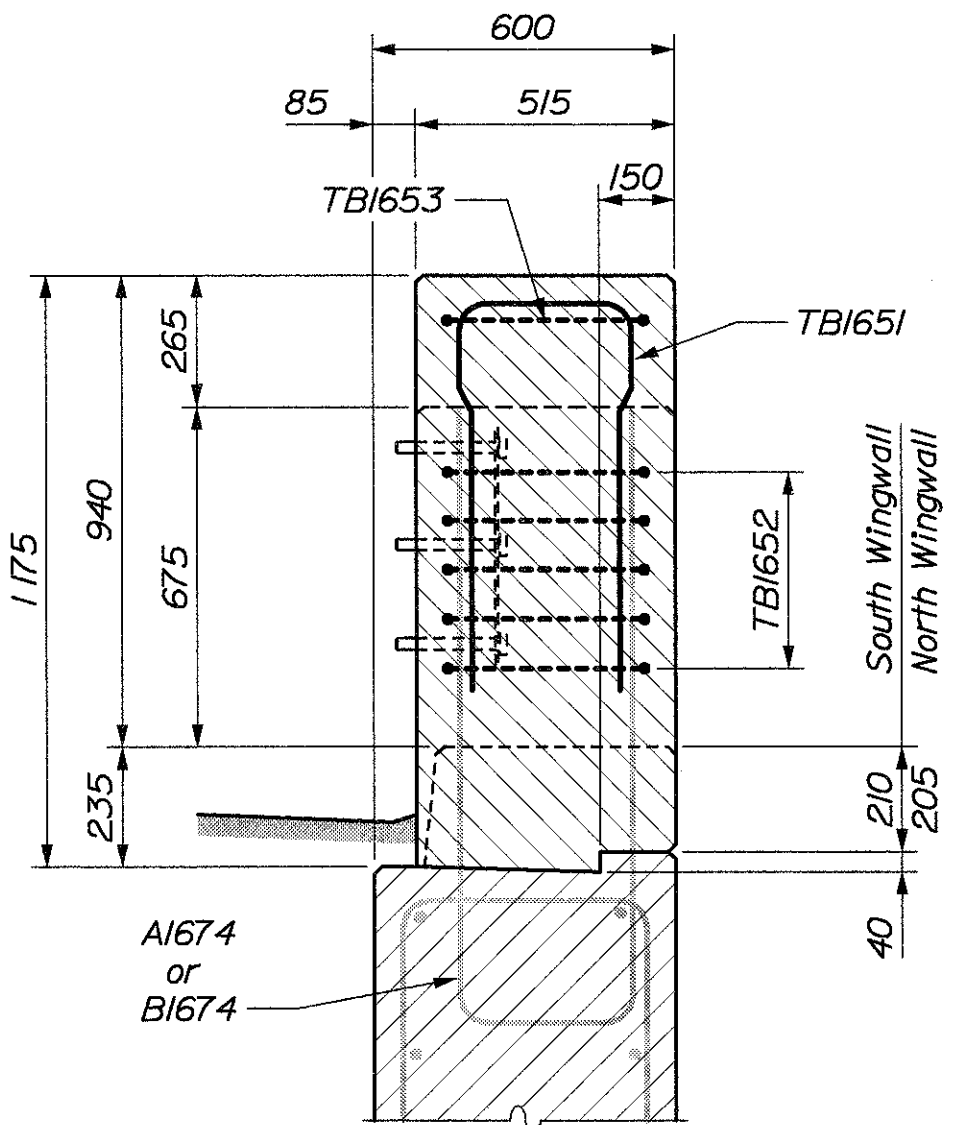
BRIDGE RAILING TRANSITION



TRANSITION BARRIER PLAN



SECTION THROUGH RECESS



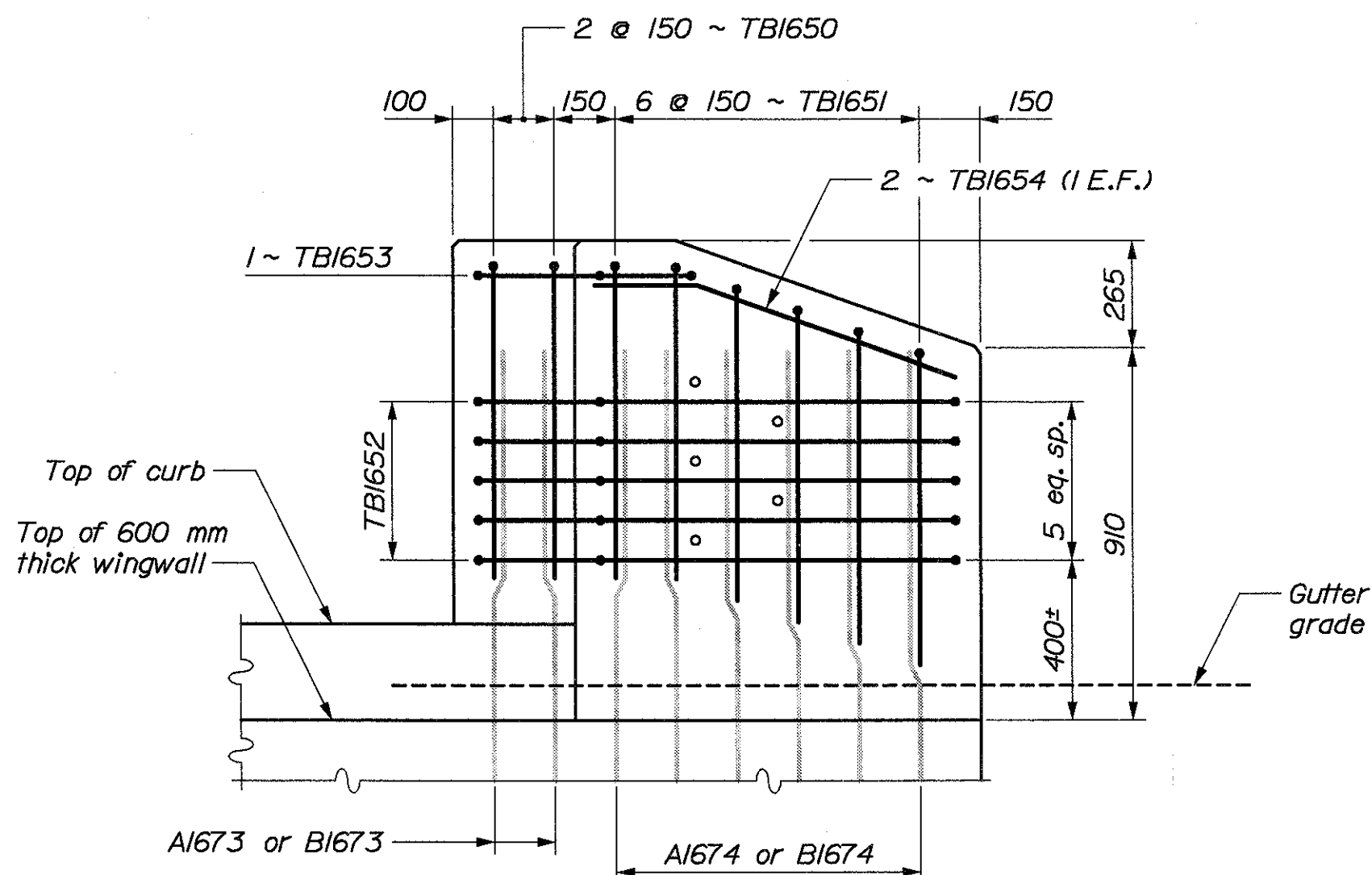
TRANSITION BARRIER SECTION

TRANSITION BARRIER NOTES

1. All work and materials shall conform to the requirements of Special Provision, Section 526, Concrete Barrier (Permanent Concrete Barrier).
  2. Reinforcing bars designated "AI673 or BI673" and "AI674 or BI674" are detailed with the reinforcing plans for the abutment wingwalls and are included for payment in the reinforcing steel pay items.
  3. Reinforcing steel shall have 50 mm minimum concrete cover.
  4. The quantities of reinforcing bars shown are for one transition barrier only.
  5. Payment for the guardrail anchorage will be considered incidental to the transition barrier pay item.
- The following work will be done under a future contract and is not a part of this project:
6. Refer to Standard Details Section 606 for the Bridge Transition Type "I". All posts are to be connected to the guardrail beam. Payment for the additional posts (with offset brackets) will be considered incidental to the Bridge Transition pay item.
  7. Payment for Curb Type "I" and Terminal Section Type "I" will be made under their respective pay items.
  8. After installation of the guardrail is complete, upset the threads on the anchor bolts in three (3) places around each bolt, at the junction of the nut and the exposed thread, with a center punch or similar tool. Payment for nuts will be considered incidental to the guardrail pay times.

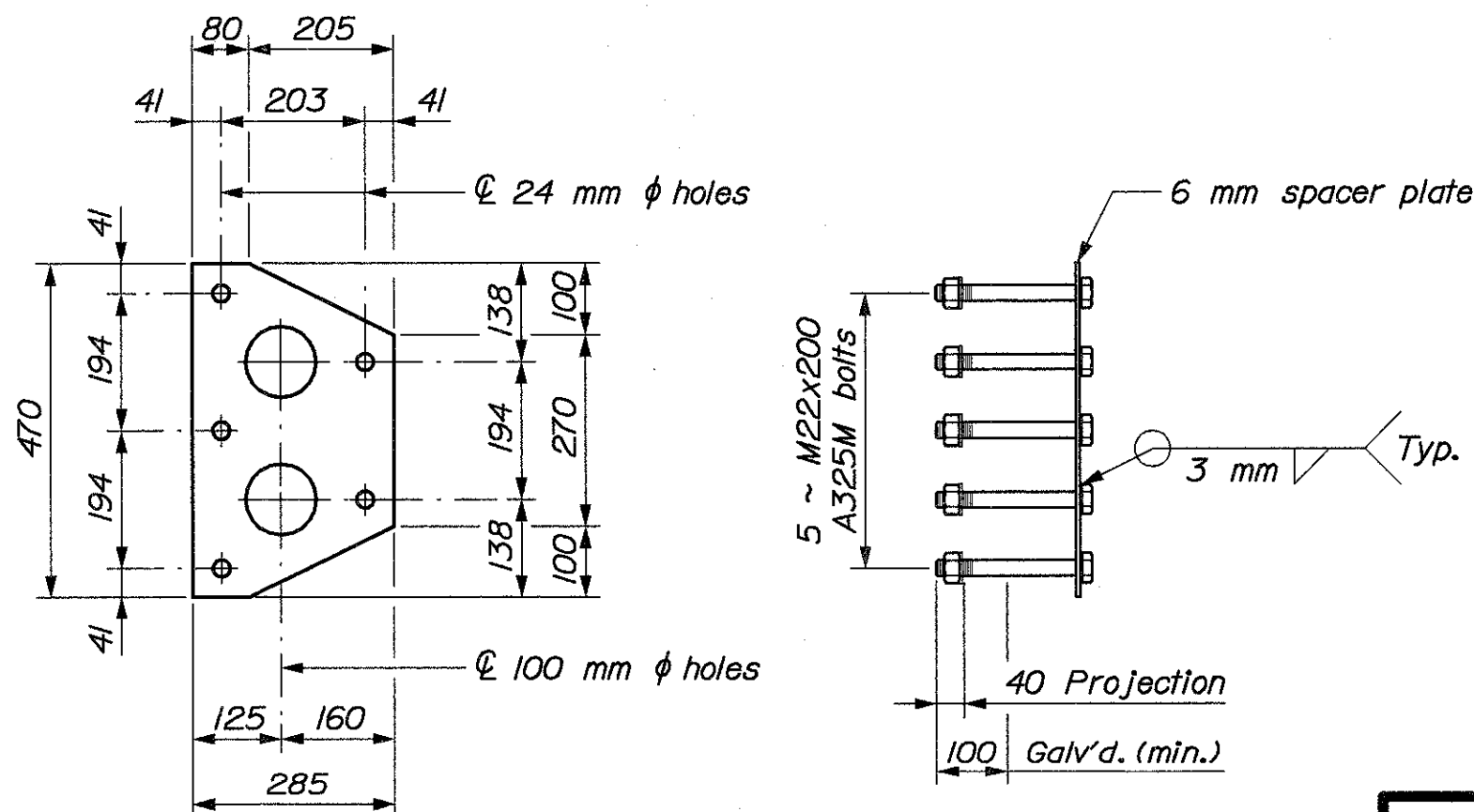
MATERIALS

Concrete..... Class "LP"  
Reinforcing steel..... ASTM A 615/A 615M, Grade 420  
Spacer plate..... ASTM A 709/A 709 M, Grade 250  
Anchor bolts..... ASTM A 325M, Type 1



TRANSITION BARRIER ELEVATION

| REINFORCING STEEL SCHEDULE |      |        |
|----------------------------|------|--------|
| Mark                       | Qty. | Length |
| TBI650                     | 2    | 1725   |
| TBI651                     | 6    | 1925   |
| TBI652                     | 5    | 3500   |
| TBI653                     | 1    | 2250   |
| TBI654                     | 2    | 925    |



GUARDRAIL ANCHORAGE

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
TRANSITION BARRIERS

SHEET OF AUGUSTA, MAINE

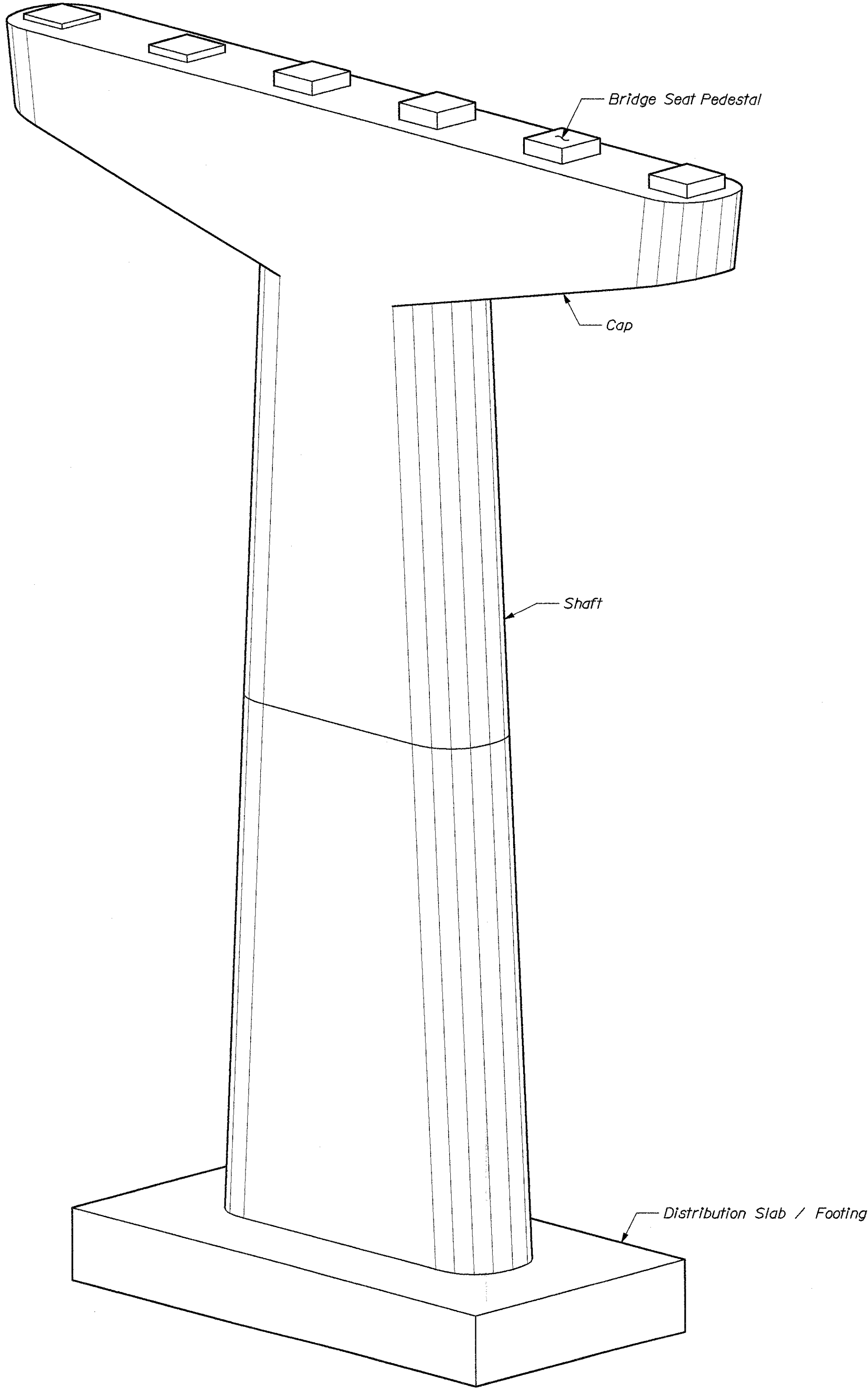
Date:02 DEC 2002

Username: jeff.Folsom

Division: BRIDGE

Filename: ...048\_Piers.dgn

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Darnen |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |
| PLANS                   |           |      |



PIER ISOMETRIC VIEW  
Pier No. 2 shown

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 48           | 105             |
| 000556.22            |       |                |              |                 |

PIER PILE NOTES

1. Piles marked with an arrow shall be battered 100 V : 12.5 H in the direction of the arrow.
2. Maximum calculated pile loads: Pier No. 1 = 1 813 kN Group VI
3. Estimate of piles required: Pier No. 1 = HP 360x132 ~ 28 @ 20 m
4. HP pile material shall be ASTM A 709M, Grade 345.
5. All piles shall be equipped with a pile tip in accordance with Standard Specifications Section 501.10.
6. The Contractor shall perform and submit a wave equation analysis for review and acceptance by the Engineer. The cost of performing the wave equation analysis will be considered incidental to Item No. 501.92, Pile Driving Equipment Mobilization.
7. At the Pier No. 1 foundation, the Contractor shall perform three (3) dynamic load tests to confirm the ultimate capacity of the piles and verify driving criteria. The ultimate capacity shall be 2.25 times the maximum calculated design load per LFD Specifications. At the start of production pile driving, two (2) piles from different locations within the pile foundation shall be dynamically tested. One (1) additional pile will be tested after approximately one half of the piles have been installed.

SEAL COFFERDAM NOTES

1. The seal concrete dimensions shown represent the minimum seal size necessary for design capacity and are not based on any particular sheet pile section.
2. Appropriate rolled corners shall be used for seal cofferdam sheet piling and the inside face of the sheet piling shall be at or outside of the seal concrete dimensions shown.
3. The Pier # 3 seal cofferdam shall be vented at elevation 2.5 meters.
4. Pier seal concrete shall be placed on sound bedrock cleaned of all weathered or fractured rock or loose soil. Smooth ledge shall be roughened prior to placing concrete. The slope of the bedrock surface shall be less than 1 V : 4 H, or it shall be benched as directed by the Engineer.
5. The method of placing dowels in the seal concrete shall be approved by the Engineer.
6. Each seal shall be cored full depth in at least three (3) locations to ensure that the seal was satisfactorily placed. These locations will be determined by the Engineer. In the event that voids or any other defects are found, the Contractor shall correct any and all defects in a manner approved by the Engineer. For each core that reveals a void or defect, two additional cores shall be taken after repairs are made. One additional core shall be taken in approximately the same location as the original core. The other core will be located by the Engineer. All cores holes shall be refilled using a non - shrink grout. The cost of all coring and repairs will be considered incidental to related contract items.
7. The horizontal pay limit for seal concrete shall be to the dimensions shown on the plans. No additional payment will be made for concrete placed outside of these limits.

PIER NOTES

1. Reinforcing steel shall have 50 mm cover in the caps and 75 mm cover in the shafts and distribution slabs / footings unless otherwise noted.
2. Maximum calculated bearing pressures: Pier No. 2 = 1 609 kPa Group IX  
Pier No. 3 = 1 489 kPa Group VI  
Pier No. 4 = 1 201 kPa Group VI
4. The bridge seat elevations shown are based on the bearing heights given on the "Pot Bearing Layout" drawing. Adjustments to the elevations shall be made, if necessary, according to the provisions stated there.
5. Payment for Pier No. 4 footing will be made under Item No. 502.23, Structural Concrete Piers (m³).

DESIGN CRITERIA

Buoyancy - Water level assumed at elevation 10.31 meters.

Stream flow - Velocity of 2.75 m/s (no skew)

Wind - 160 km/h

Ice - Thickness 600 mm at elevation 11.14 meters

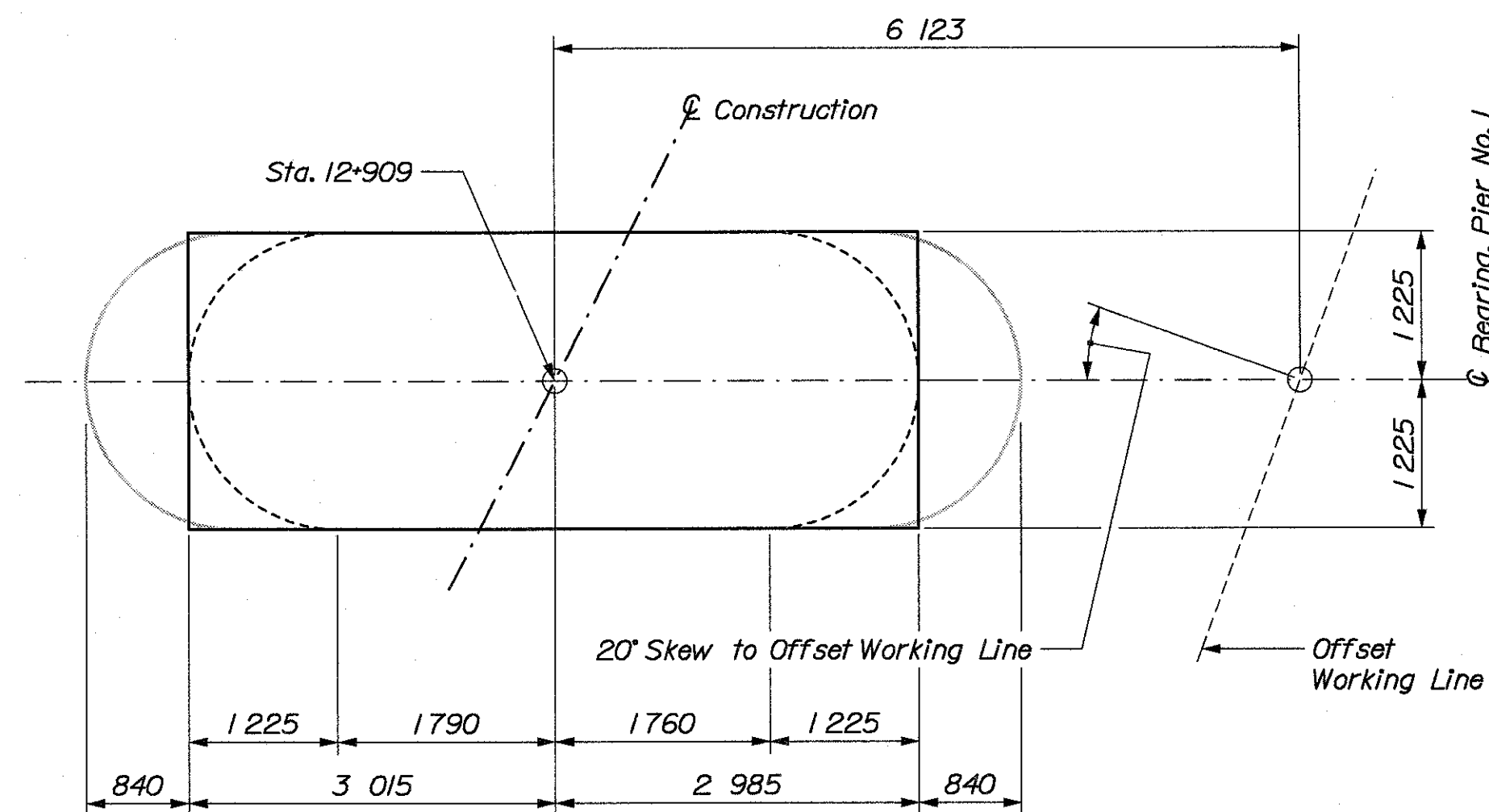
BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

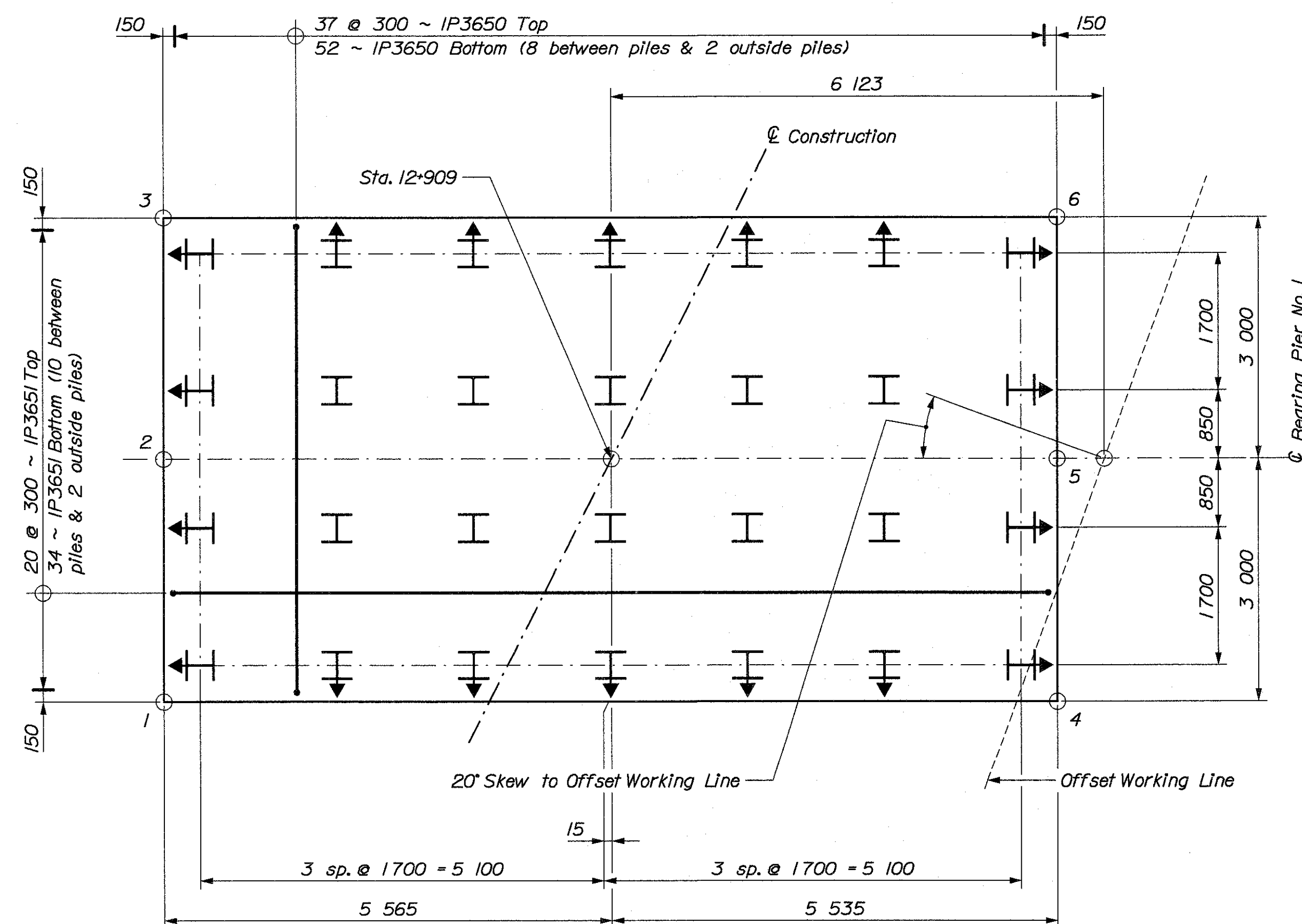
PIERS

SHEET 48 OF 105  
AUGUSTA, MAINE



PIER NO. 1 SHAFT TOP PLAN  
EL. 31.157

PIER NO. 1 CAP PLAN



PIER NO.1 SHAFT BASE PLAN

PIER NO.1 PILE & FOOTING PLAN

| FOUNDATION<br>COORDINATES |                                |
|---------------------------|--------------------------------|
| Point No.                 | Coordinates                    |
| 1                         | 931 989 649 X<br>167 047 748 Y |
| 2                         | 931 992 274 X<br>167 046 296 Y |
| 3                         | 931 994 900 X<br>167 044 845 Y |
| 4                         | 931 984 279 X<br>167 038 033 Y |
| 5                         | 931 986 904 X<br>167 036 582 Y |
| 6                         | 931 989 529 X<br>167 035 130 Y |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
*PIER NO. 1 PLAN*

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER |  | BY        | DATE      |
|-------------------------|--|-----------|-----------|
| DESIGN-DETAILED         |  | J. Folsom | D. Damren |
| CHECKED                 |  |           |           |
| REVISIONS               |  |           |           |
| FIELD CHANGES           |  |           |           |

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

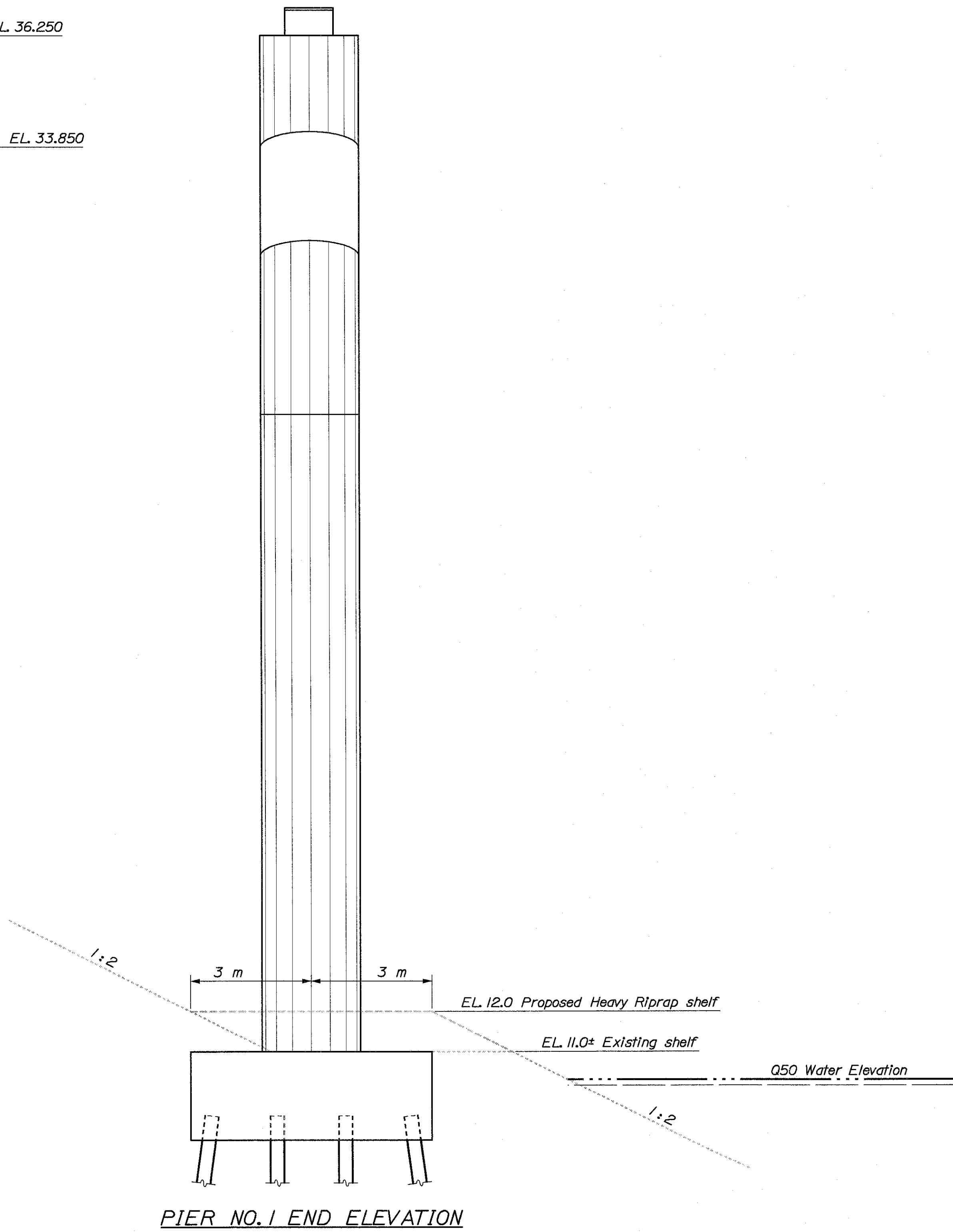
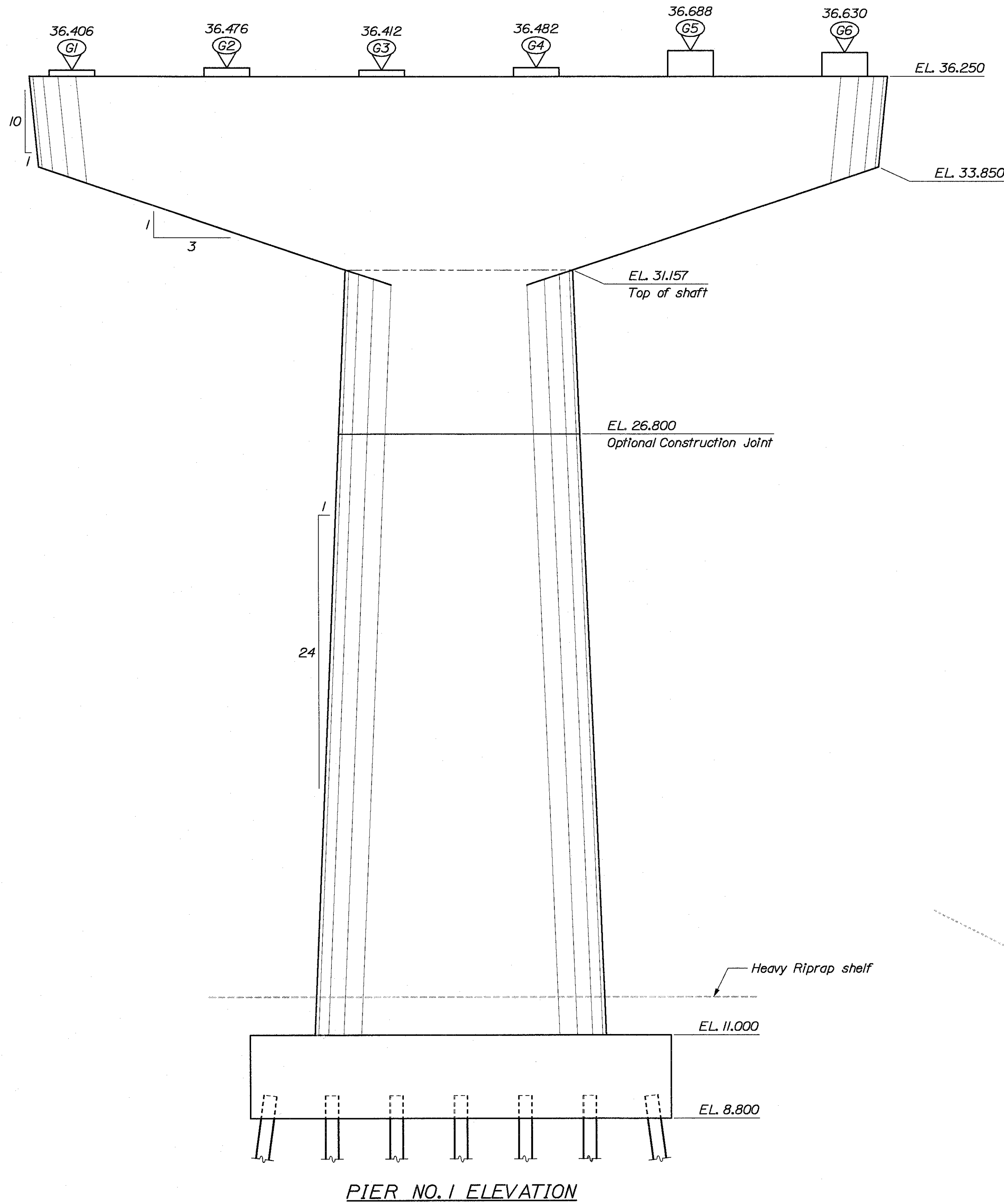
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 50           | 105             |

000556.22



BRIDGE NO. 6400

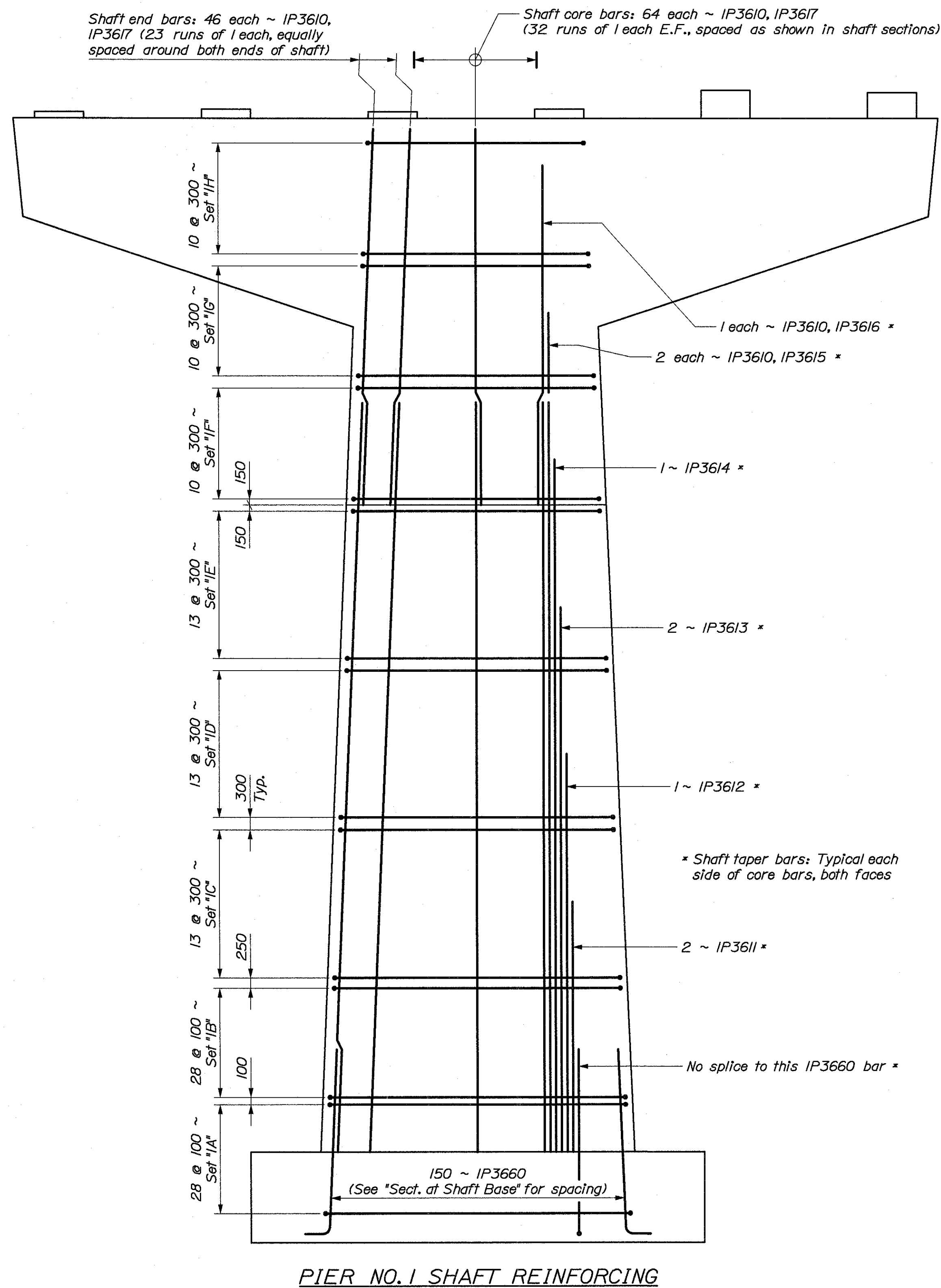
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

**PIER NO. 1 ELEVATION**

| PROJECT DESIGN ENGINEER | BY        | DATE      |
|-------------------------|-----------|-----------|
| DESIGN DETAILER         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

PLANS



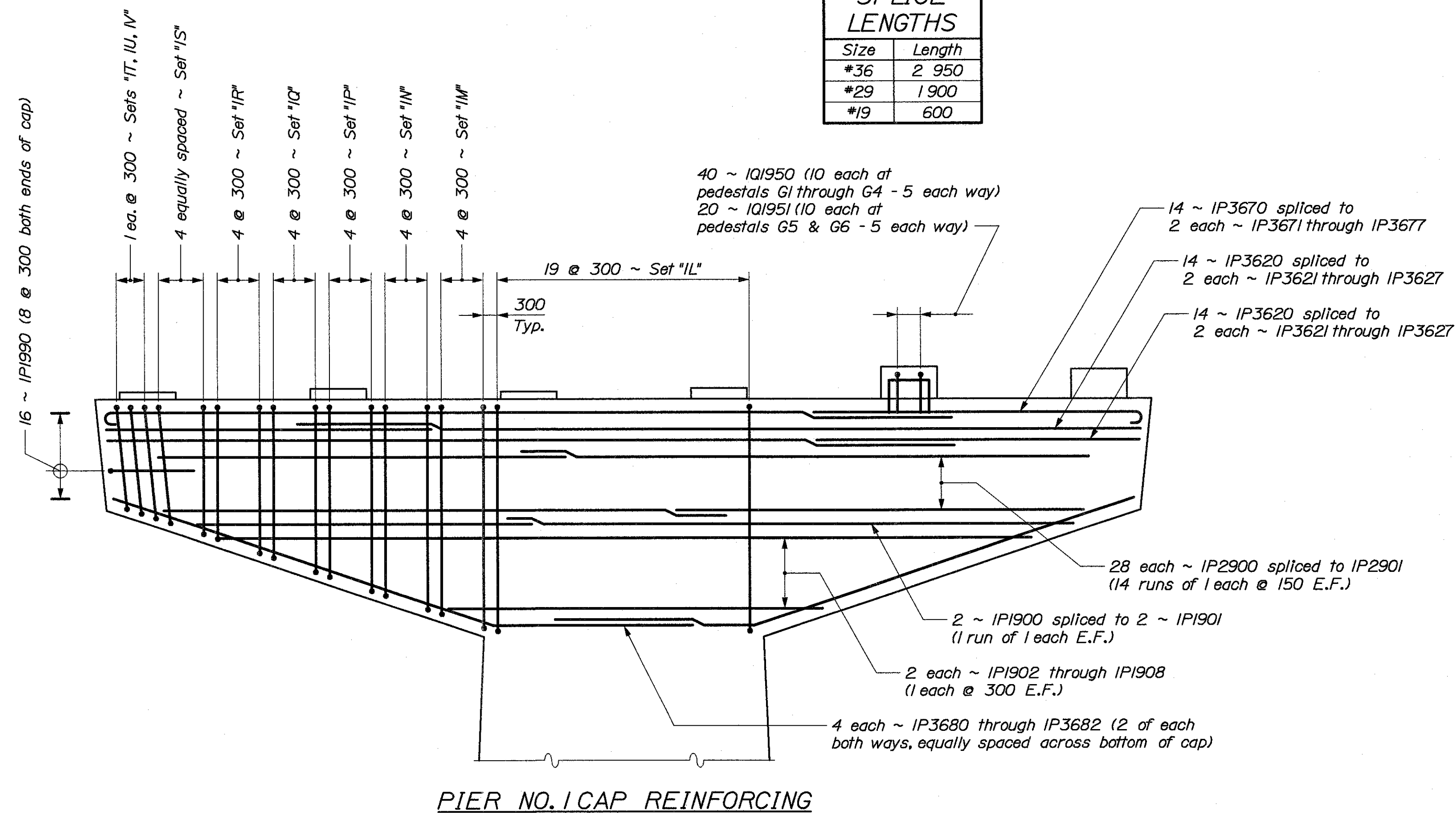
METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 51        | 105          |

000556.22

| CAP BAR SPLICE LENGTHS |        |
|------------------------|--------|
| Size                   | Length |
| #36                    | 2 950  |
| #29                    | 1 900  |
| #19                    | 600    |



PIER NO. 1 REINFORCING STEEL SET SCHEDULE

| Set | Qty. | Location | Bar Designations                                                                                |
|-----|------|----------|-------------------------------------------------------------------------------------------------|
| IA  | 28   | Shaft    | 2 ~ IP1950, 2 ~ IP1960, 6 ~ IP1970, 2 ~ IP1971, 2 ~ IP1972, 2 ~ IP1973, 15 ~ IP1974, 4 ~ IP1975 |
| IB  | 28   | Shaft    | 2 ~ IP1951, 2 ~ IP1961, 6 ~ IP1970, 2 ~ IP1971, 2 ~ IP1972, 2 ~ IP1973, 13 ~ IP1974, 4 ~ IP1975 |
| IC  | 13   | Shaft    | 2 ~ IP1350, 2 ~ IP1360, 6 ~ IP1370, 2 ~ IP1371, 2 ~ IP1372, 2 ~ IP1373, 13 ~ IP1374, 4 ~ IP1375 |
| ID  | 13   | Shaft    | 2 ~ IP1351, 2 ~ IP1361, 6 ~ IP1370, 2 ~ IP1371, 2 ~ IP1372, 2 ~ IP1373, 11 ~ IP1374, 4 ~ IP1375 |
| IE  | 13   | Shaft    | 2 ~ IP1352, 2 ~ IP1362, 6 ~ IP1370, 2 ~ IP1371, 2 ~ IP1372, 2 ~ IP1373, 11 ~ IP1374, 4 ~ IP1375 |
| IF  | 10   | Shaft    | 2 ~ IP1353, 2 ~ IP1363, 6 ~ IP1370, 2 ~ IP1371, 2 ~ IP1372, 2 ~ IP1373, 9 ~ IP1374, 4 ~ IP1375  |
| IG  | 10   | Shaft    | 2 ~ IP1354, 2 ~ IP1364, 6 ~ IP1370, 2 ~ IP1371, 2 ~ IP1372, 2 ~ IP1373, 9 ~ IP1374, 4 ~ IP1375  |
| IH  | 10   | Shaft    | 2 ~ IP1355, 2 ~ IP1365, 6 ~ IP1370, 2 ~ IP1371, 2 ~ IP1372, 2 ~ IP1373, 7 ~ IP1374, 4 ~ IP1375  |
| IL  | 19   | Cap      | 2 ~ IP1980                                                                                      |
| IM  | 8    | Cap      | 2 ~ IP1981, 1 ~ IP1991                                                                          |
| IN  | 8    | Cap      | 2 ~ IP1982, 1 ~ IP1992                                                                          |
| IP  | 8    | Cap      | 2 ~ IP1983, 1 ~ IP1993                                                                          |
| IQ  | 8    | Cap      | 2 ~ IP1984, 1 ~ IP1994                                                                          |
| IR  | 8    | Cap      | 2 ~ IP1985, 1 ~ IP1995                                                                          |
| IS  | 8    | Cap      | 2 ~ IP1986, 1 ~ IP1996                                                                          |
| IT  | 2    | Cap      | 2 ~ IP1987, 1 ~ IP1997                                                                          |
| IU  | 2    | Cap      | 2 ~ IP1988, 1 ~ IP1998                                                                          |
| IV  | 2    | Cap      | 2 ~ IP1989, 1 ~ IP1999                                                                          |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**PIER NO. 1 REINFORCING**

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

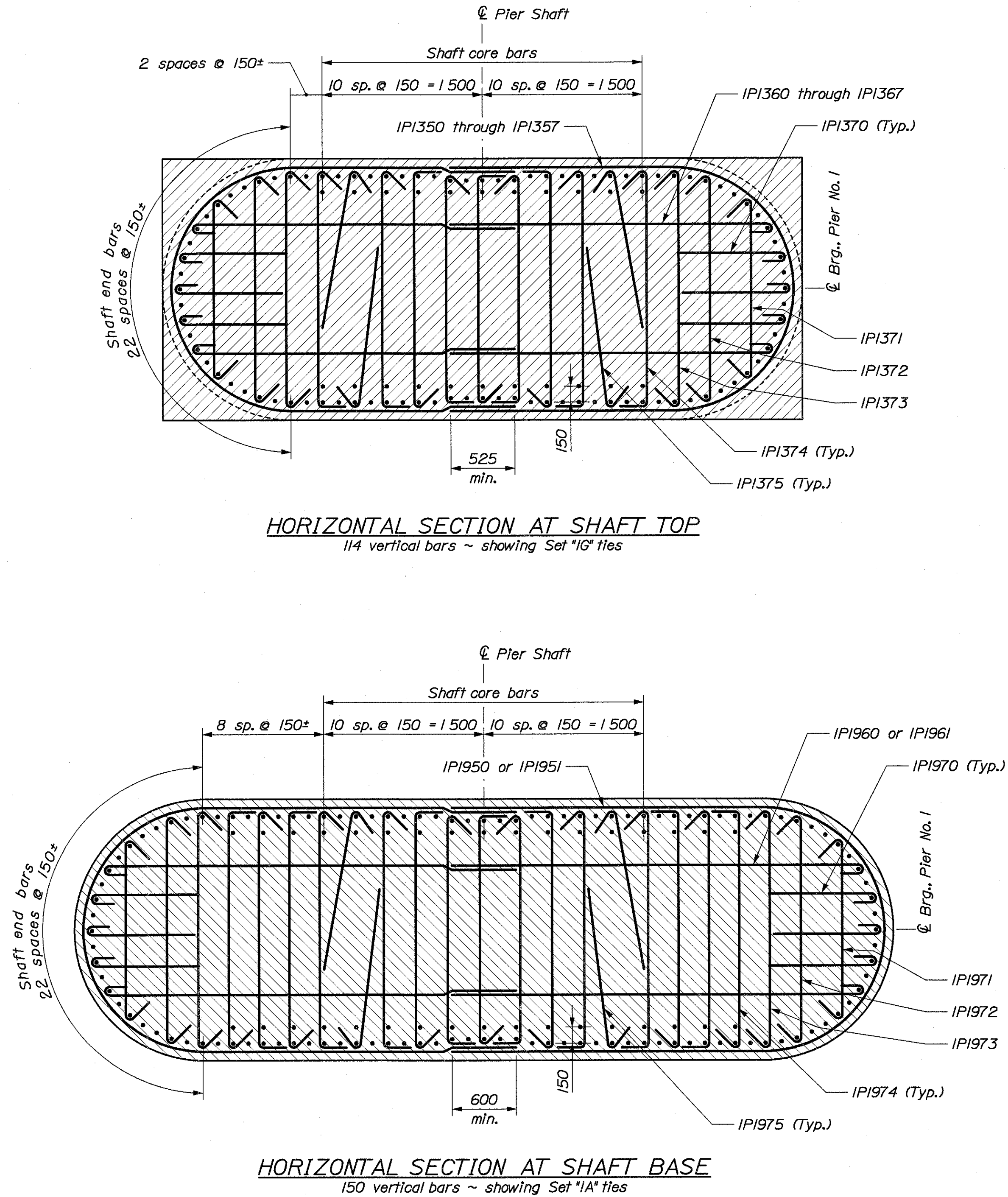
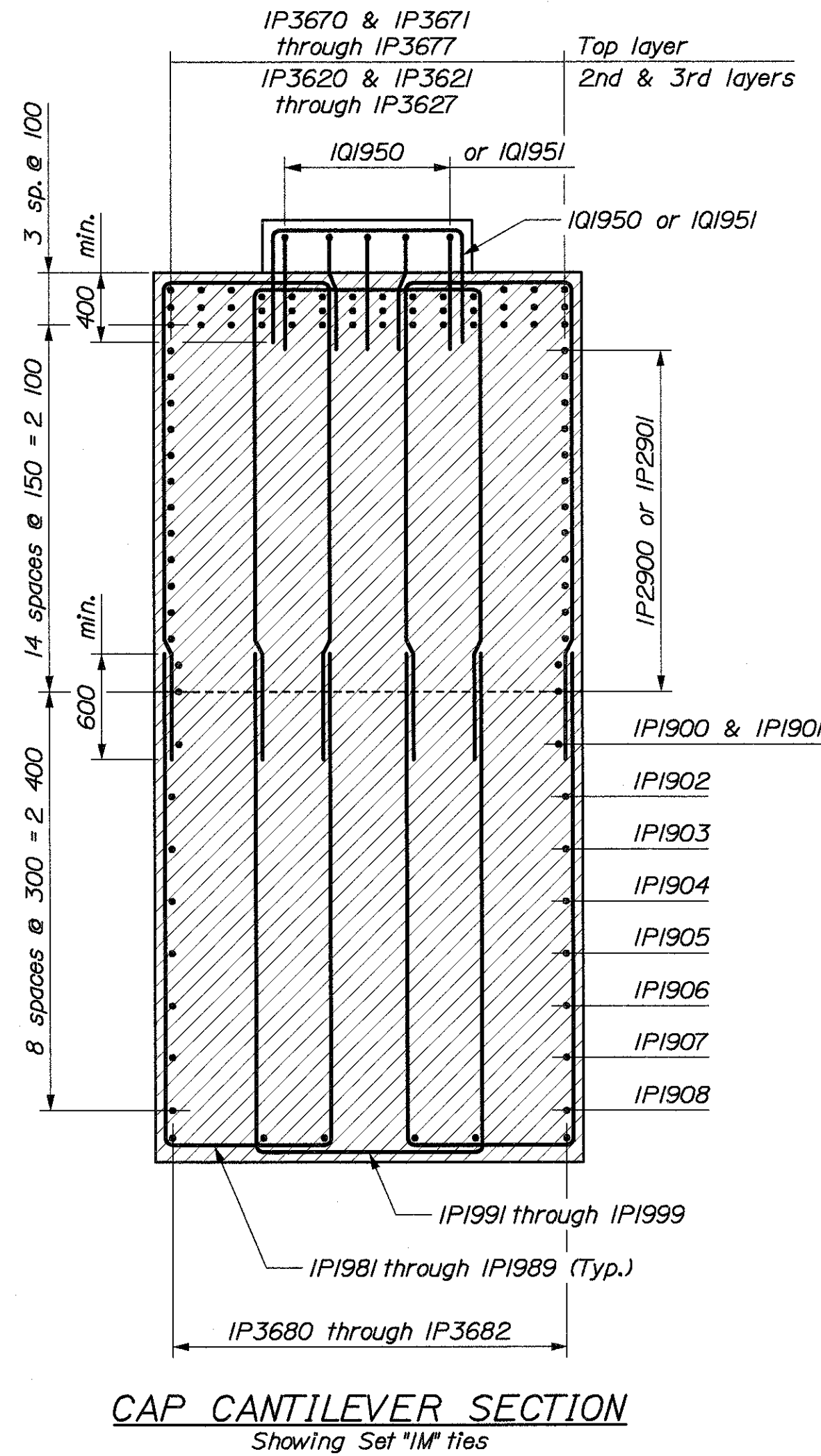
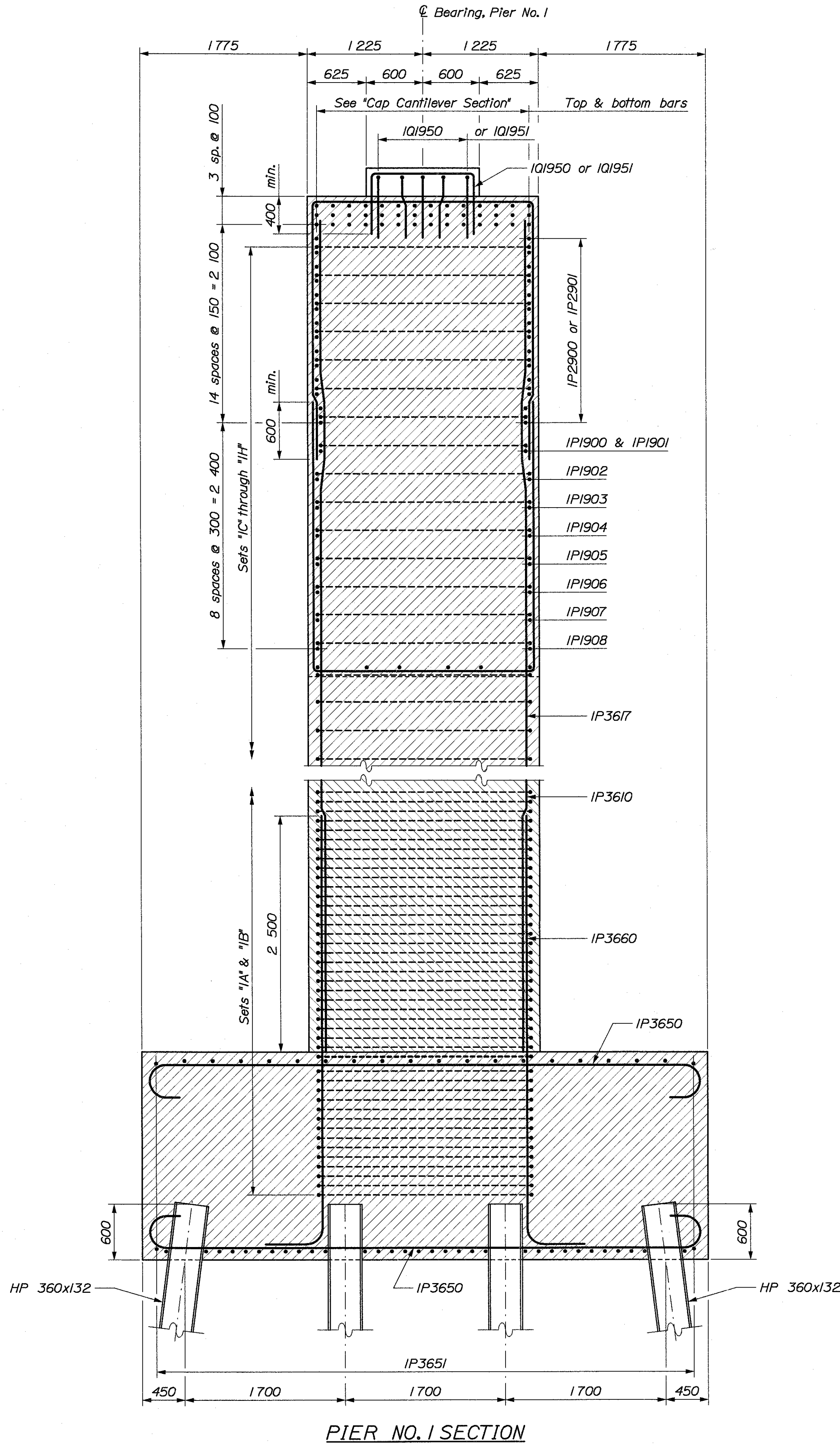
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| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

|       |
|-------|
| PLANS |
|-------|

|        |                                                                                                               |                      |       |                |              |                 |
|--------|---------------------------------------------------------------------------------------------------------------|----------------------|-------|----------------|--------------|-----------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. | F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|        |                                                                                                               | 1                    | MAINE | NH-0556(220)E  | 52           | 105             |

000556.22



BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

PIER NO. 1 SECTIONS

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ...\\053\_pier\_2 plan.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

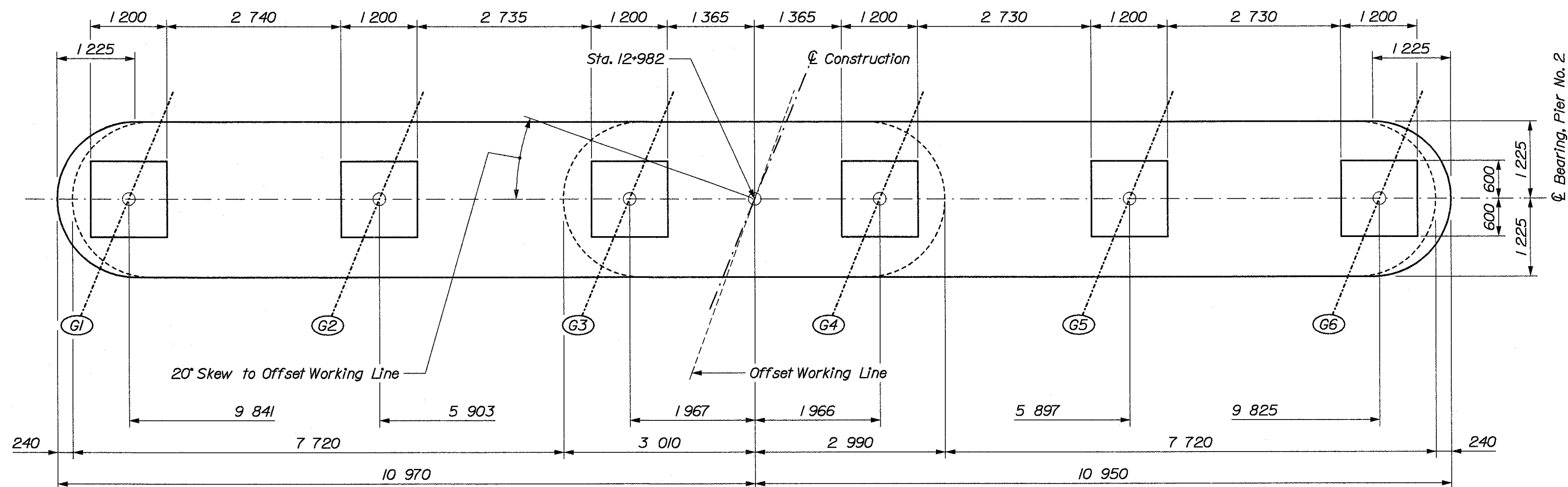
PLANS

METRIC

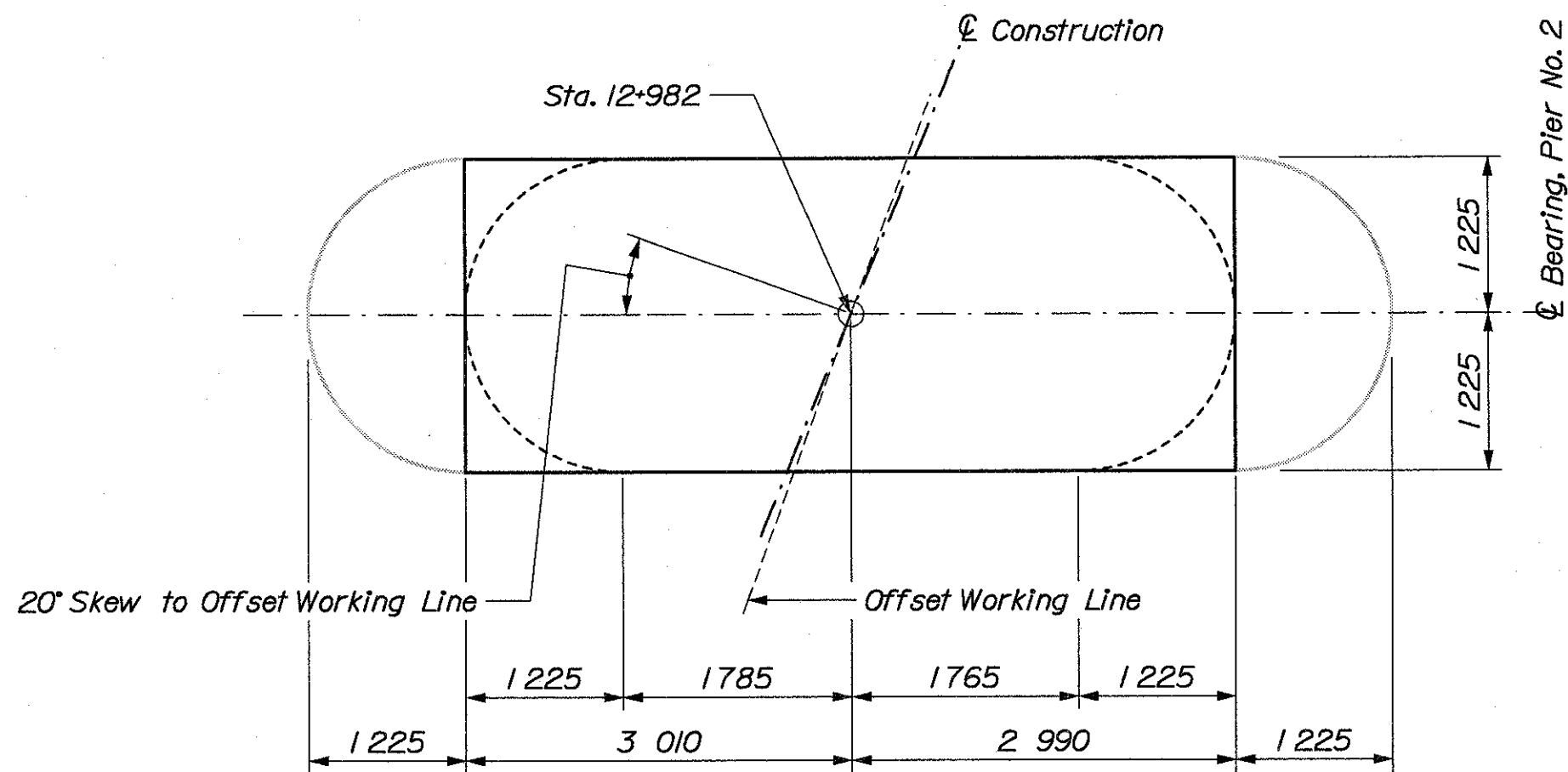
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 53           | 105             |

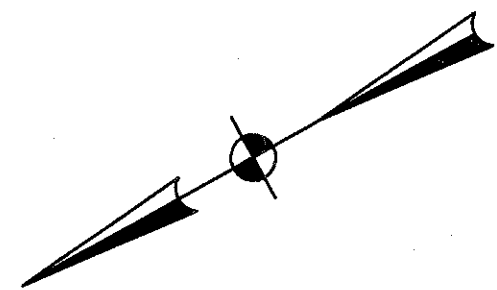
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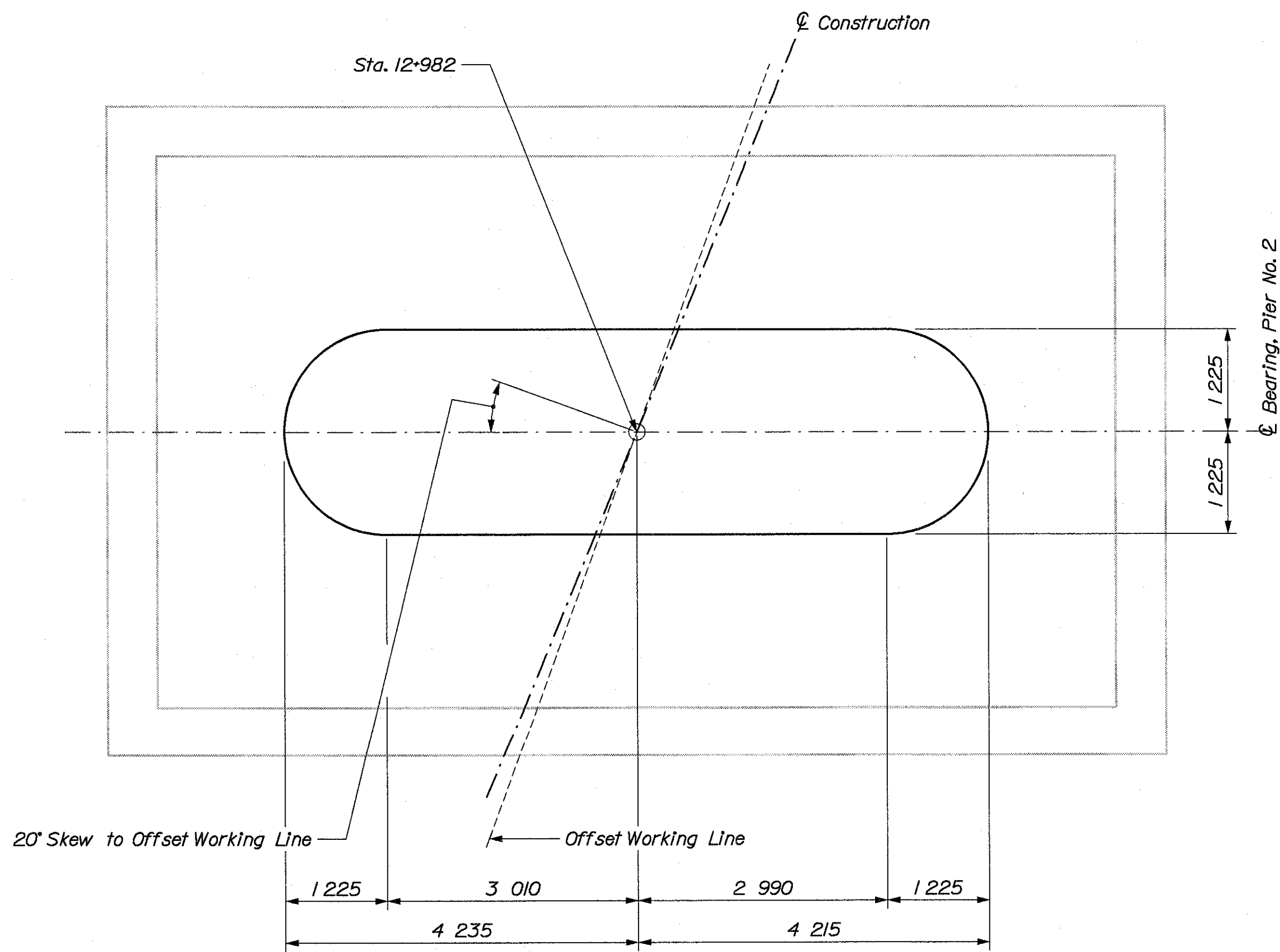
PIER NO. 2 CAP PLAN



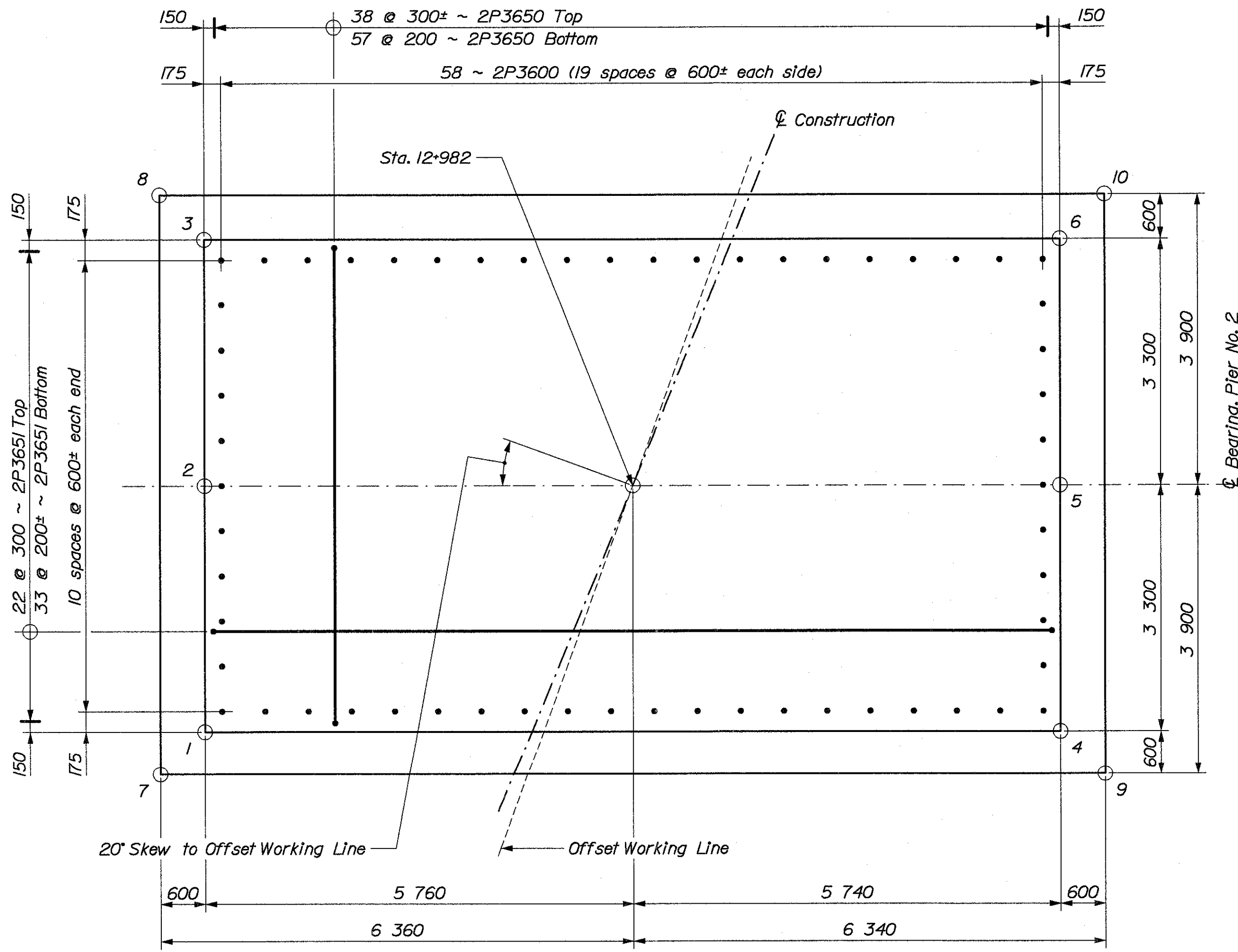
PIER NO. 2 SHAFT TOP PLAN  
EL. 29.885



Flow



PIER NO. 2 SHAFT BASE PLAN



PIER NO. 2 SEAL & DISTRIBUTION SLAB PLAN

| FOUNDATION COORDINATES |                                |
|------------------------|--------------------------------|
| Point No.              | Coordinates                    |
| 1                      | 932 032.935 X<br>166 989.430 Y |
| 2                      | 932 035.823 X<br>166 987.833 Y |
| 3                      | 932 038.711 X<br>166 986.236 Y |
| 4                      | 932 037.371 X<br>166 979.365 Y |
| 5                      | 932 030.259 X<br>166 977.769 Y |
| 6                      | 932 033.147 X<br>166 976.172 Y |
| 7                      | 932 032.700 X<br>166 990.245 Y |
| 8                      | 932 039.527 X<br>166 986.471 Y |
| 9                      | 932 026.556 X<br>166 979.130 Y |
| 10                     | 932 033.382 X<br>166 975.357 Y |

BRIDGE NO. 6400  
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 2 PLAN

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

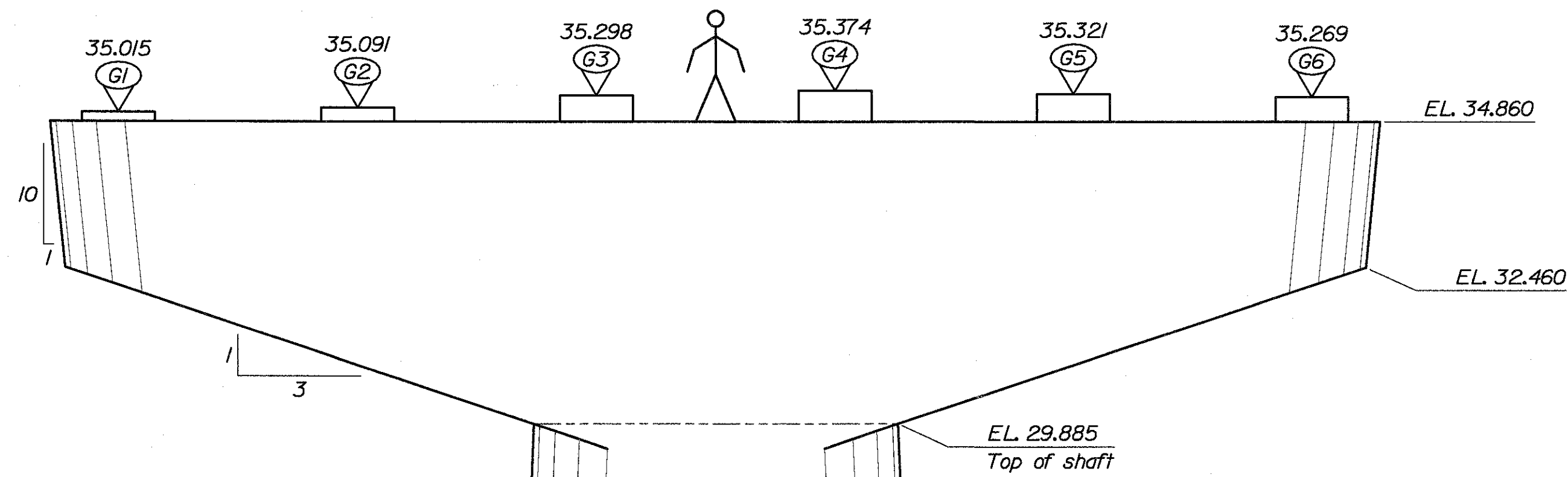
Username: Dana.Damren

Division: BRIDGE

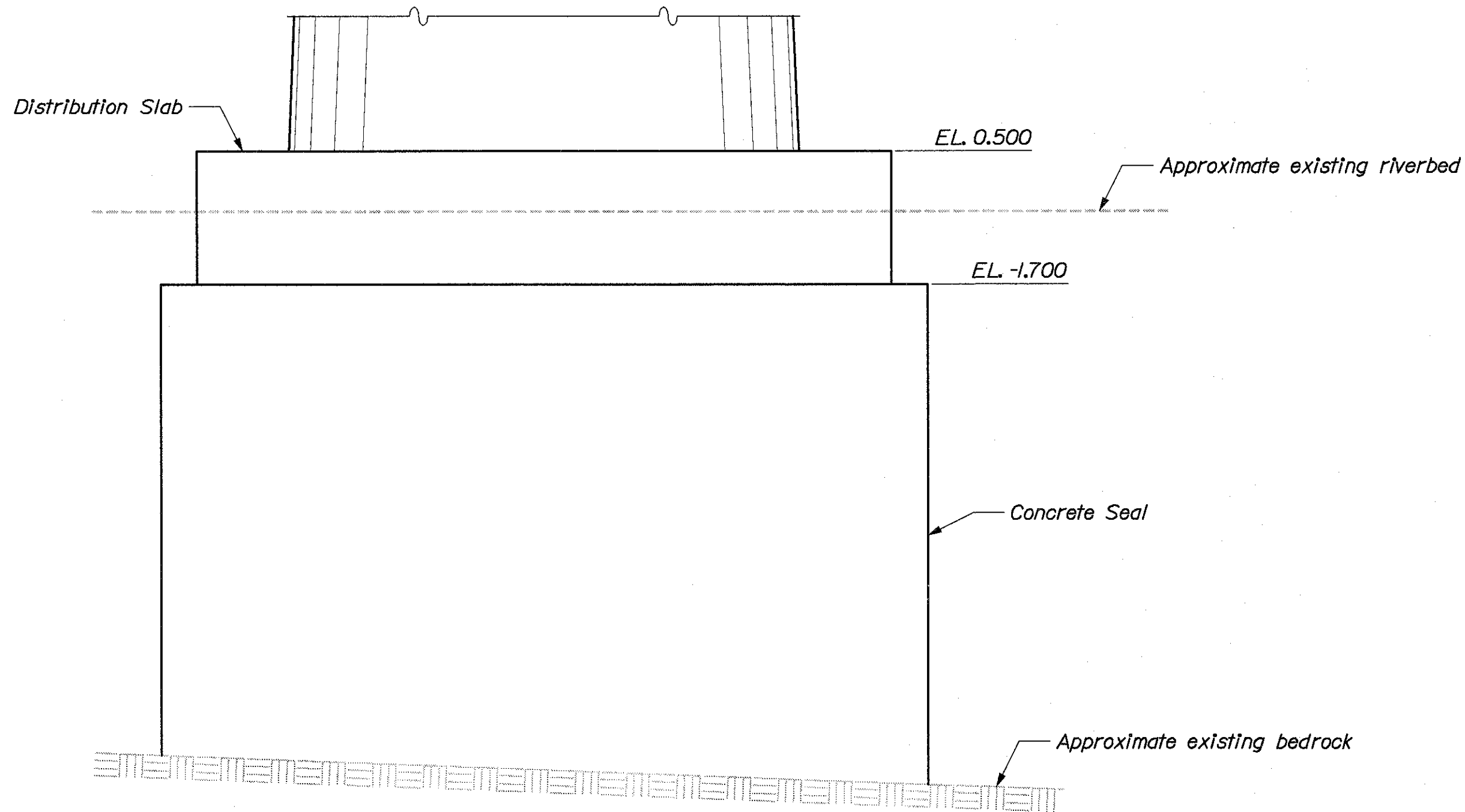
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| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

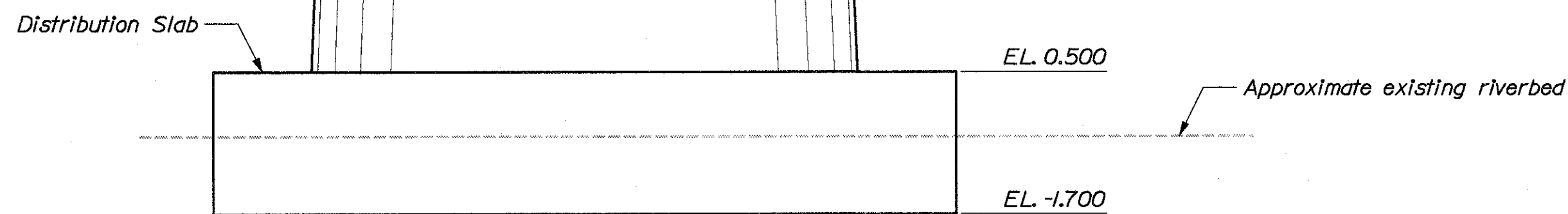
PLANS



PIER NO. 2 PART END ELEVATION



PIER NO. 2 FOUNDATION ELEVATION



PIER NO. 2 ELEVATION

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 54           | 105             |

000556.22

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 2 ELEVATION

SHEET OF AUGUSTA, MAINE

Date:26 NOV 2002

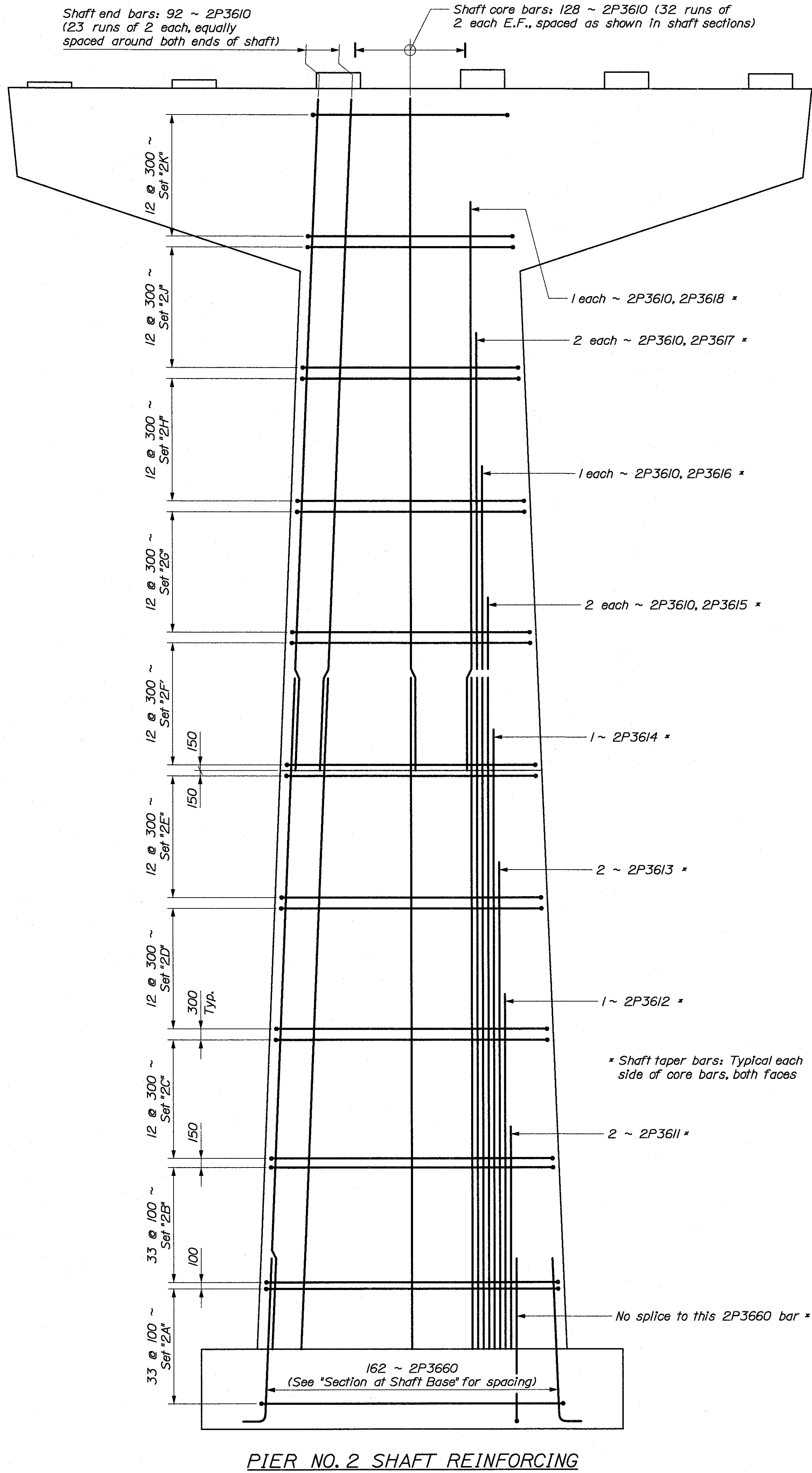
Username: Dana.Damren

Division: BRIDGE

Filename: ... \055\_pier\_2 reinforcing.dgn

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

|       |
|-------|
| PLANS |
|-------|



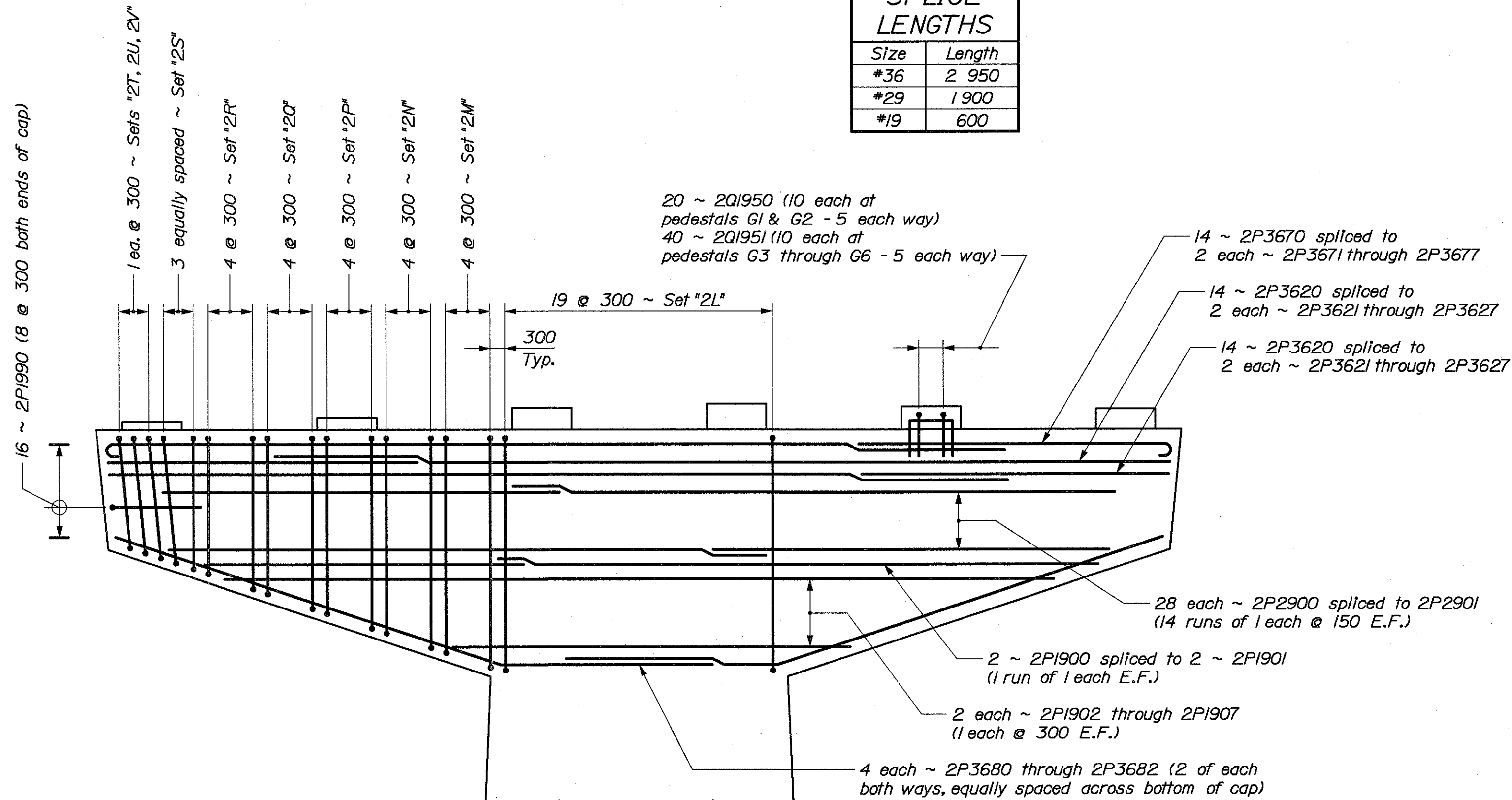
METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                        |       |                |           |              |
|------------------------|-------|----------------|-----------|--------------|
| F.H.W.A.<br>(REG. NO.) | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
| 1                      | MAINE | NH-0556(220)E  | 55        | 105          |

000556.22

| Size | Length |
|------|--------|
| #36  | 2 950  |
| #29  | 1 900  |
| #19  | 600    |



PIER NO. 2 REINFORCING STEEL SET SCHEDULE

| Set | Qty. | Location | Bar Designations                                                                                |
|-----|------|----------|-------------------------------------------------------------------------------------------------|
| 2A  | 33   | Shaft    | 2 ~ 2P1950, 2 ~ 2P1960, 6 ~ 2P1970, 2 ~ 2P1971, 2 ~ 2P1972, 2 ~ 2P1973, 17 ~ 2P1974, 4 ~ 2P1975 |
| 2B  | 33   | Shaft    | 2 ~ 2P1951, 2 ~ 2P1961, 6 ~ 2P1970, 2 ~ 2P1971, 2 ~ 2P1972, 2 ~ 2P1973, 15 ~ 2P1974, 4 ~ 2P1975 |
| 2C  | 12   | Shaft    | 2 ~ 2P1350, 2 ~ 2P1360, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 15 ~ 2P1374, 4 ~ 2P1375 |
| 2D  | 12   | Shaft    | 2 ~ 2P1351, 2 ~ 2P1361, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 13 ~ 2P1374, 4 ~ 2P1375 |
| 2E  | 12   | Shaft    | 2 ~ 2P1352, 2 ~ 2P1362, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 13 ~ 2P1374, 4 ~ 2P1375 |
| 2F  | 12   | Shaft    | 2 ~ 2P1353, 2 ~ 2P1363, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 11 ~ 2P1374, 4 ~ 2P1375 |
| 2G  | 12   | Shaft    | 2 ~ 2P1354, 2 ~ 2P1364, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 11 ~ 2P1374, 4 ~ 2P1375 |
| 2H  | 12   | Shaft    | 2 ~ 2P1355, 2 ~ 2P1365, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 9 ~ 2P1374, 4 ~ 2P1375  |
| 2J  | 12   | Shaft    | 2 ~ 2P1356, 2 ~ 2P1366, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 9 ~ 2P1374, 4 ~ 2P1375  |
| 2K  | 12   | Shaft    | 2 ~ 2P1357, 2 ~ 2P1367, 6 ~ 2P1370, 2 ~ 2P1371, 2 ~ 2P1372, 2 ~ 2P1373, 7 ~ 2P1374, 4 ~ 2P1375  |
| 2L  | 19   | Cap      | 2 ~ 2P1980                                                                                      |
| 2M  | 8    | Cap      | 2 ~ 2P1981, 1 ~ 2P1991                                                                          |
| 2N  | 8    | Cap      | 2 ~ 2P1982, 1 ~ 2P1992                                                                          |
| 2P  | 8    | Cap      | 2 ~ 2P1983, 1 ~ 2P1993                                                                          |
| 2Q  | 8    | Cap      | 2 ~ 2P1984, 1 ~ 2P1994                                                                          |
| 2R  | 8    | Cap      | 2 ~ 2P1985, 1 ~ 2P1995                                                                          |
| 2S  | 6    | Cap      | 2 ~ 2P1986, 1 ~ 2P1996                                                                          |
| 2T  | 2    | Cap      | 2 ~ 2P1987, 1 ~ 2P1997                                                                          |
| 2U  | 2    | Cap      | 2 ~ 2P1988, 1 ~ 2P1998                                                                          |
| 2V  | 2    | Cap      | 2 ~ 2P1989, 1 ~ 2P1999                                                                          |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 2 REINFORCING

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

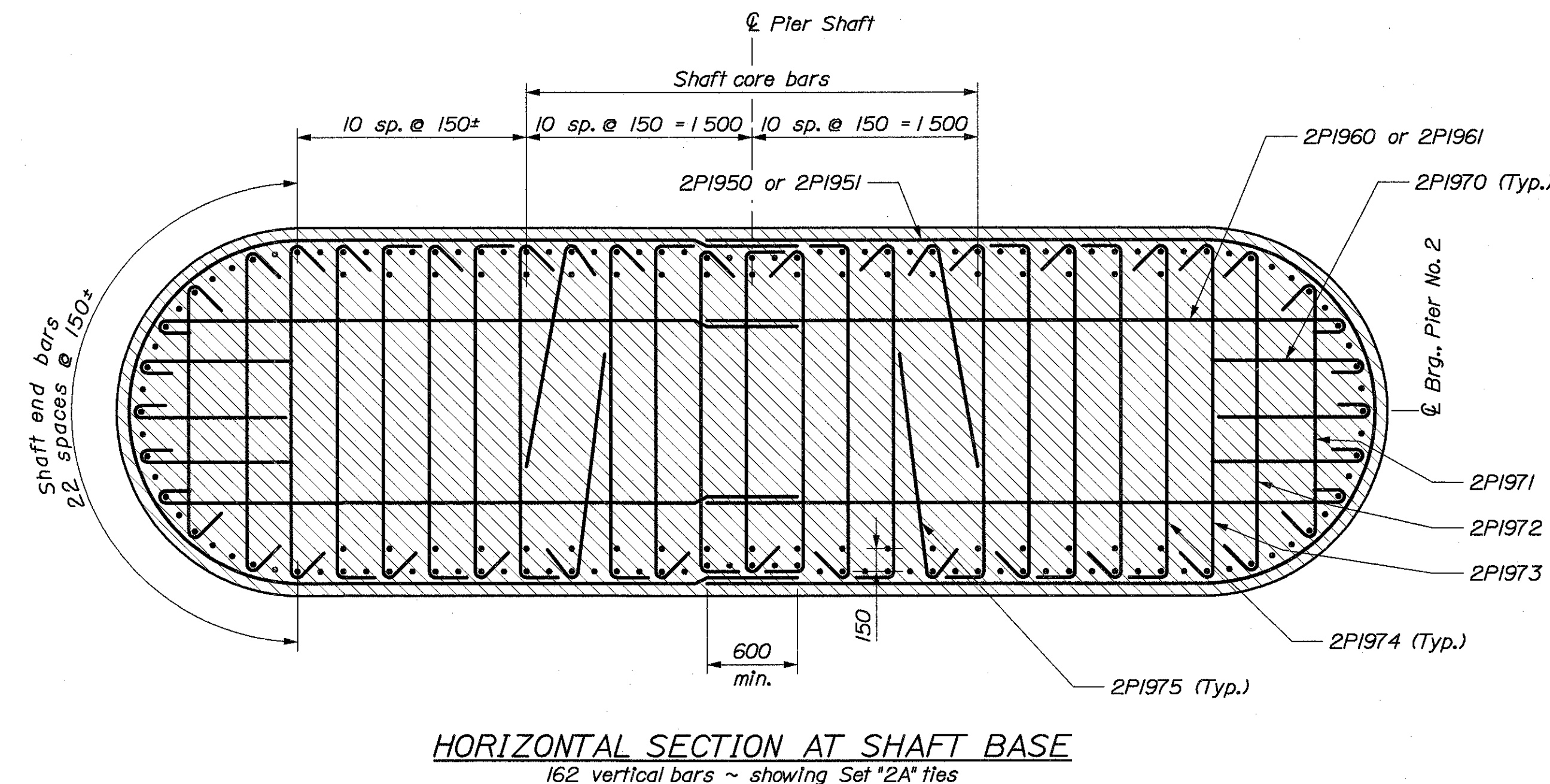
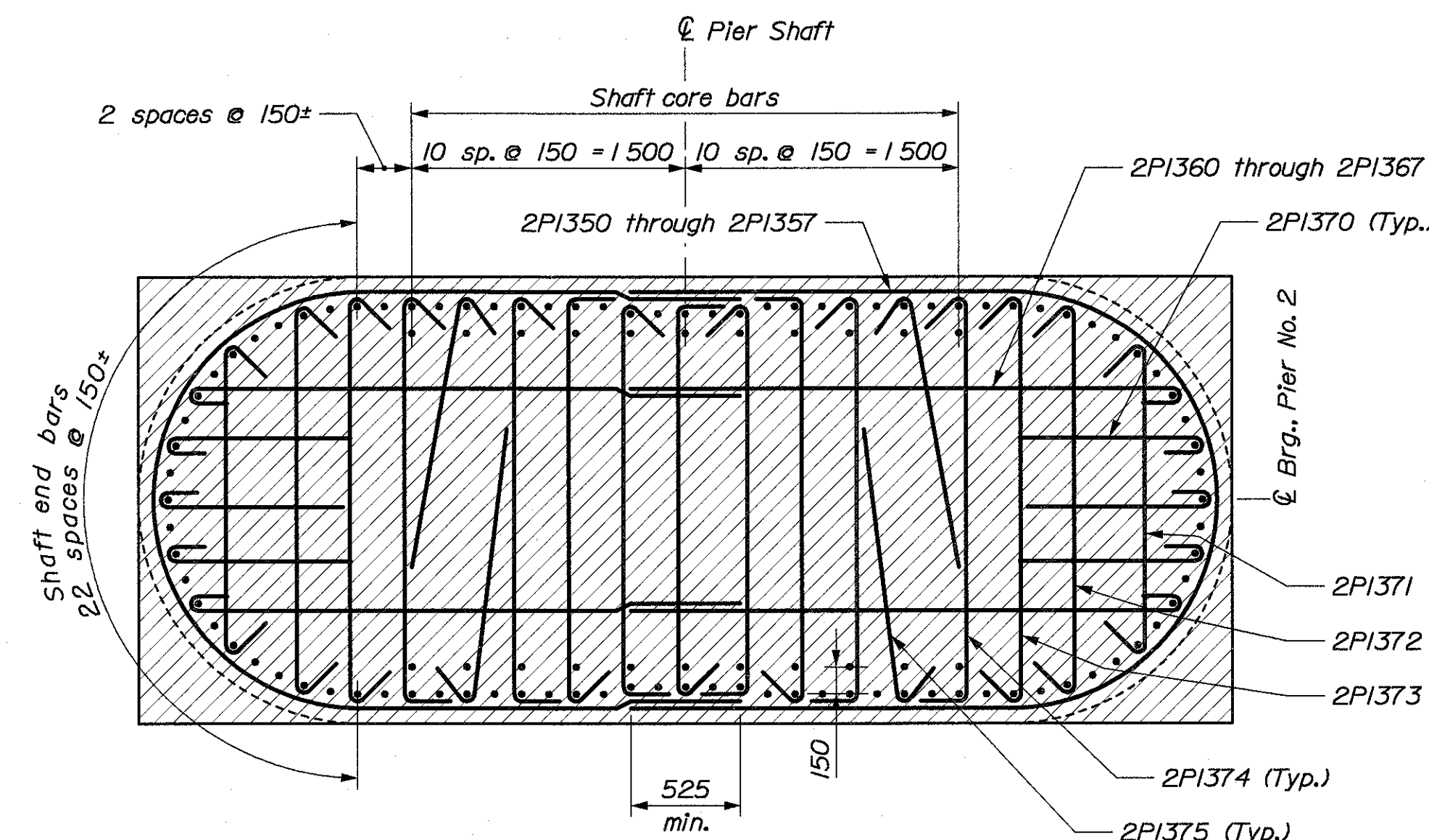
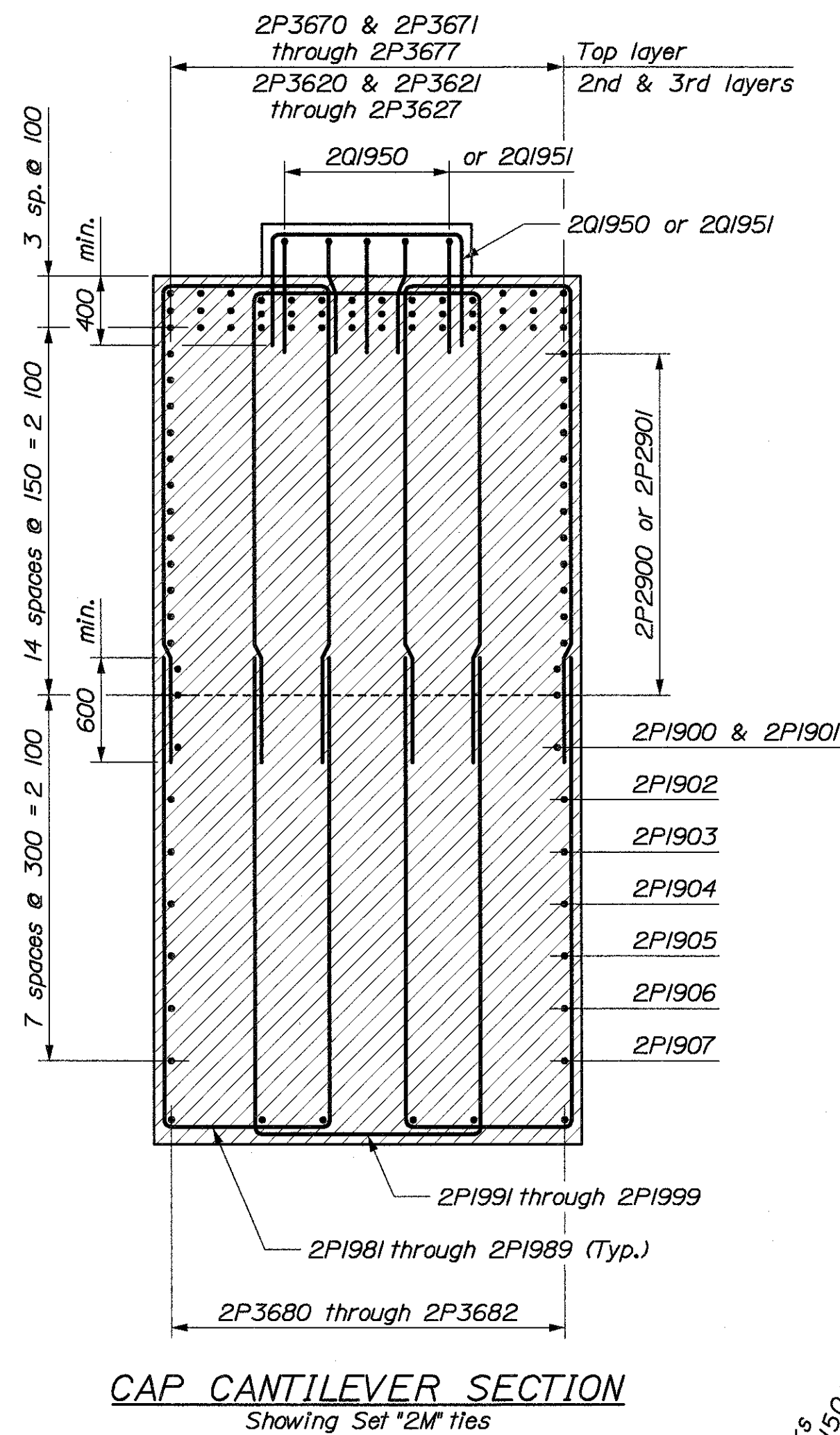
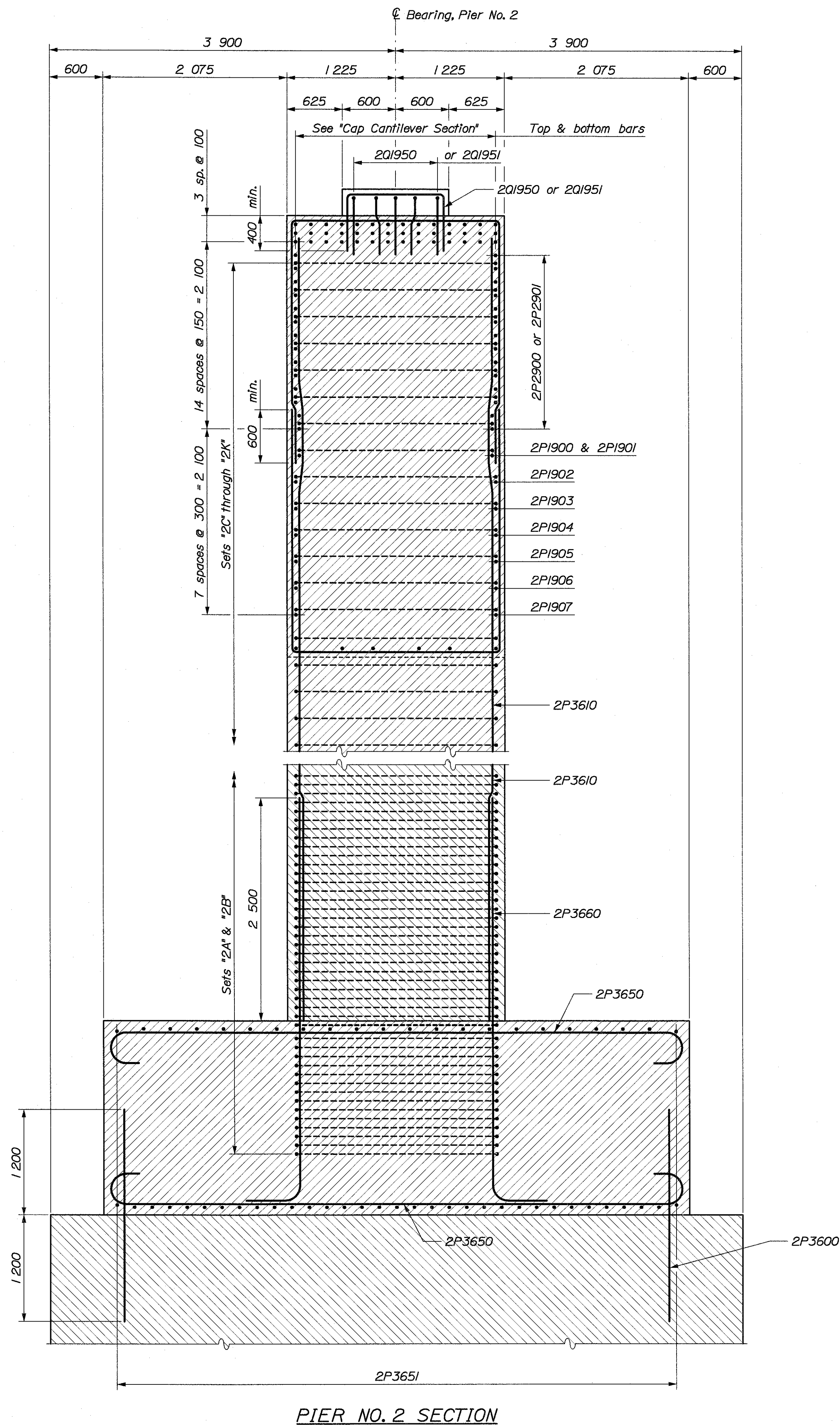
PLANS

**METRIC**

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 56        | 105          |

000556.22



BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 2 SECTIONS

SHEET OF AUGUSTA, MAINE

|                         |           |           |
|-------------------------|-----------|-----------|
| PROJECT DESIGN ENGINEER | BY        | DATE      |
| DESIGN-DETAILED         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

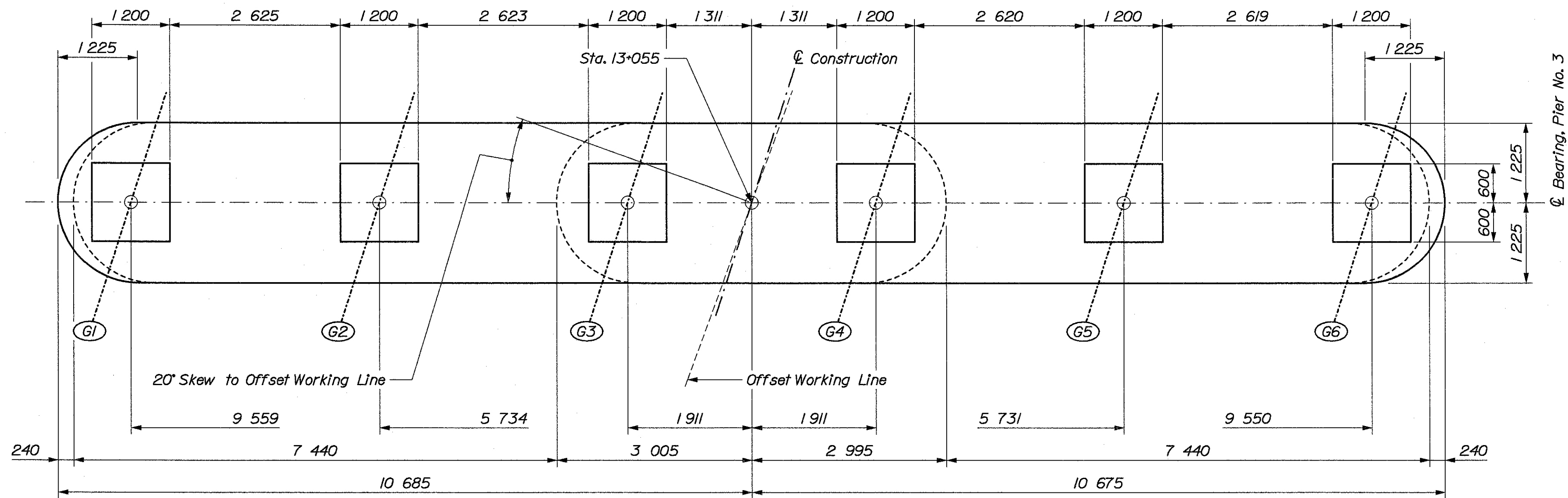
PLANS

METRIC

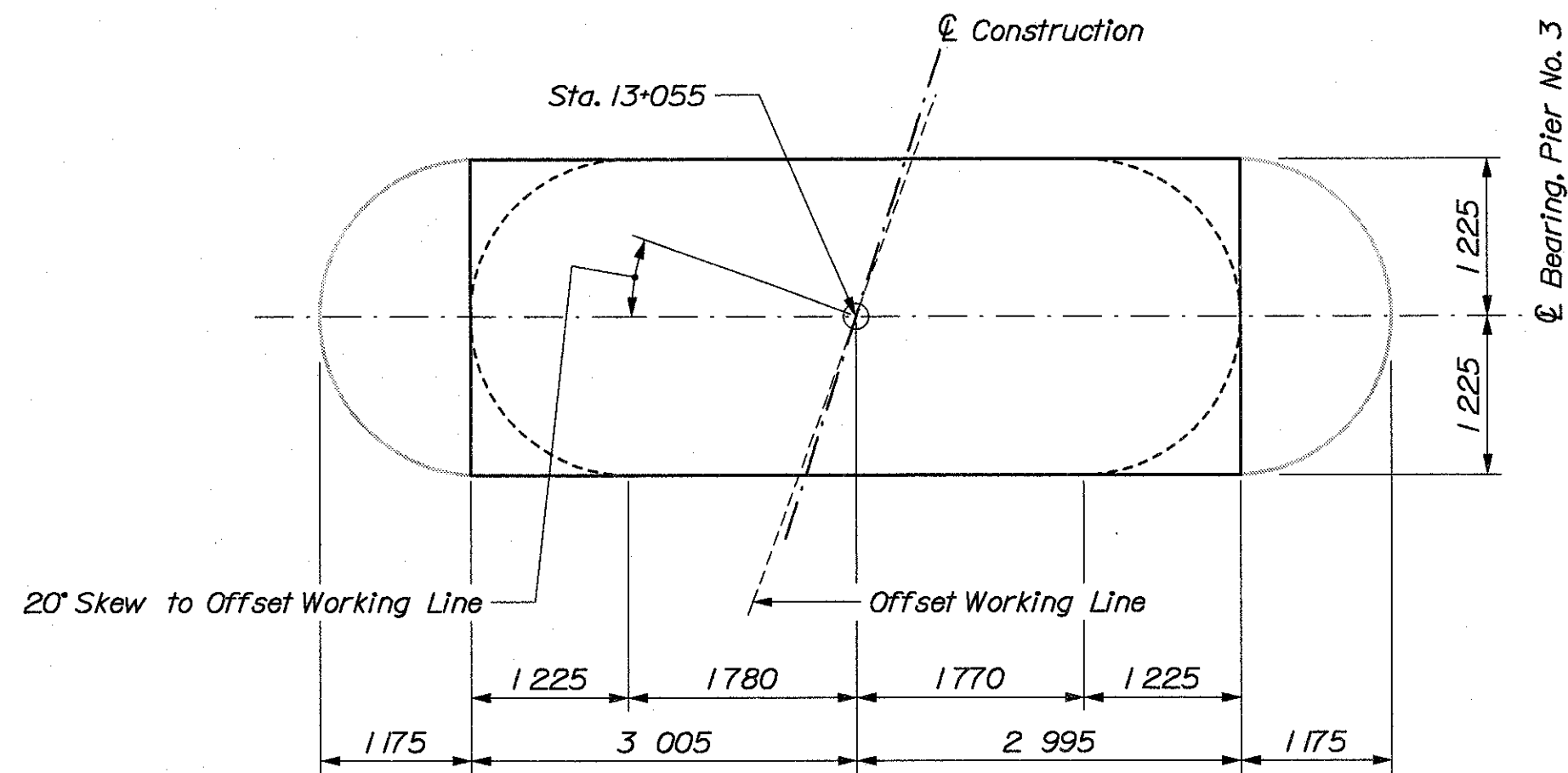
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2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 57           | 105             |

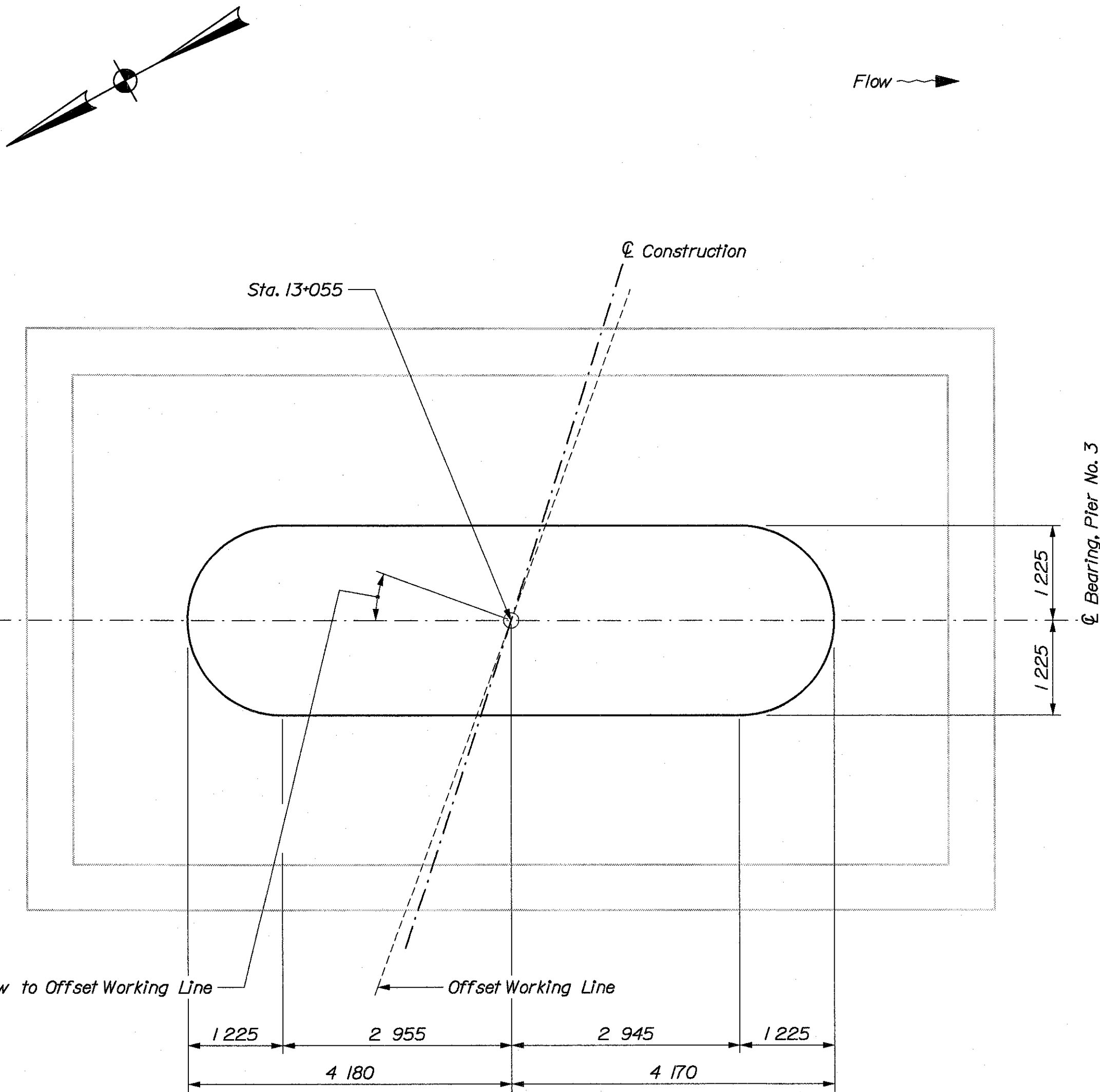
000556.22



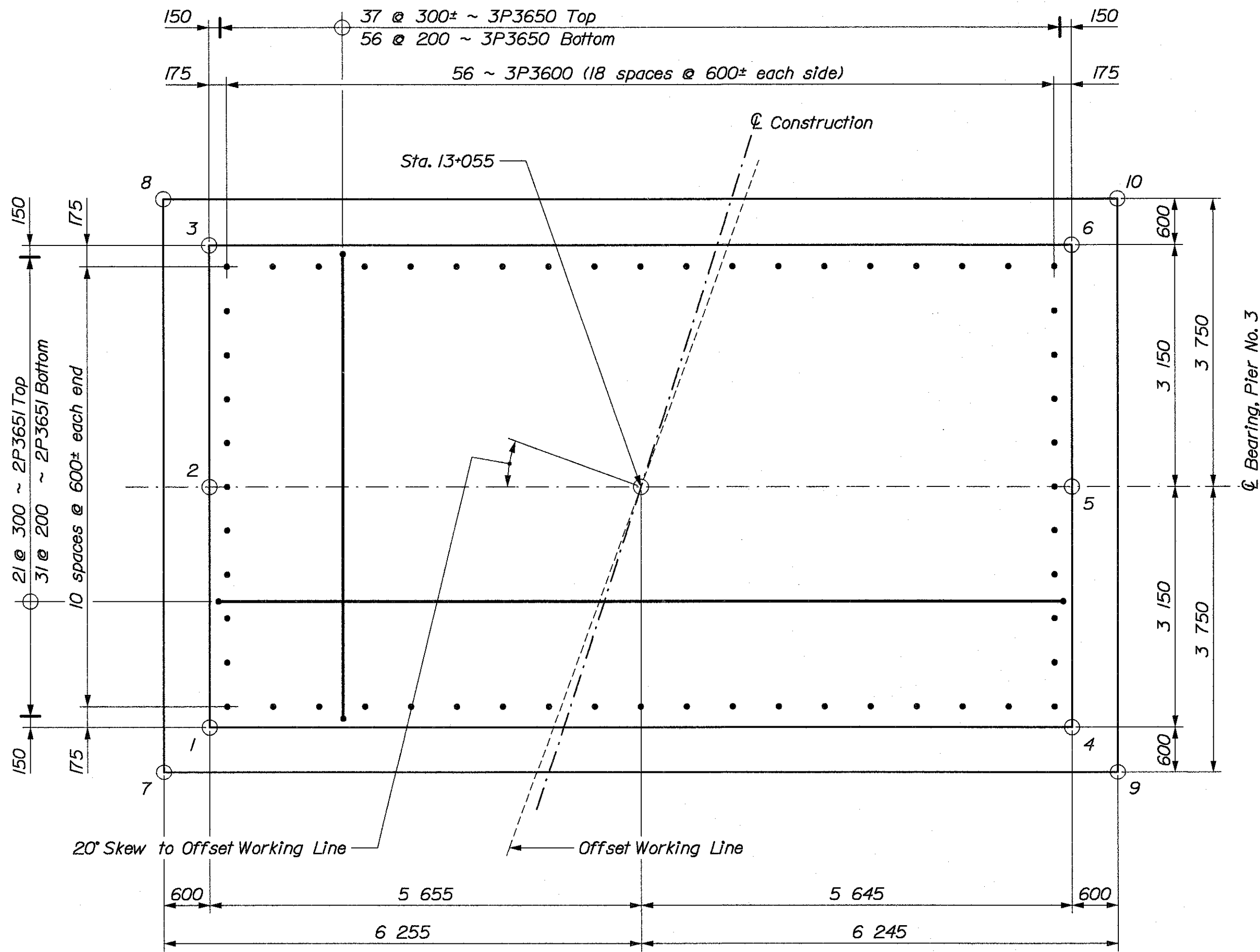
PIER NO. 3 CAP PLAN



PIER NO. 3 SHAFT TOP PLAN  
EL. 28.730



PIER NO. 3 SHAFT BASE PLAN



PIER NO. 3 SEAL & DISTRIBUTION SLAB PLAN

| FOUNDATION COORDINATES |                                |
|------------------------|--------------------------------|
| Point No.              | Coordinates                    |
| 1                      | 932 080.958 X<br>166 934.241 Y |
| 2                      | 932 083.715 X<br>166 932.717 Y |
| 3                      | 932 086.472 X<br>166 931.193 Y |
| 4                      | 932 075.491 X<br>166 924.351 Y |
| 5                      | 932 078.248 X<br>166 922.827 Y |
| 6                      | 932 081.004 X<br>166 921.303 Y |
| 7                      | 932 080.723 X<br>166 935.056 Y |
| 8                      | 932 087.287 X<br>166 931.427 Y |
| 9                      | 932 074.675 X<br>166 924.117 Y |
| 10                     | 932 081.239 X<br>166 920.488 Y |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 3 PLAN

Date: 26 NOV 2002

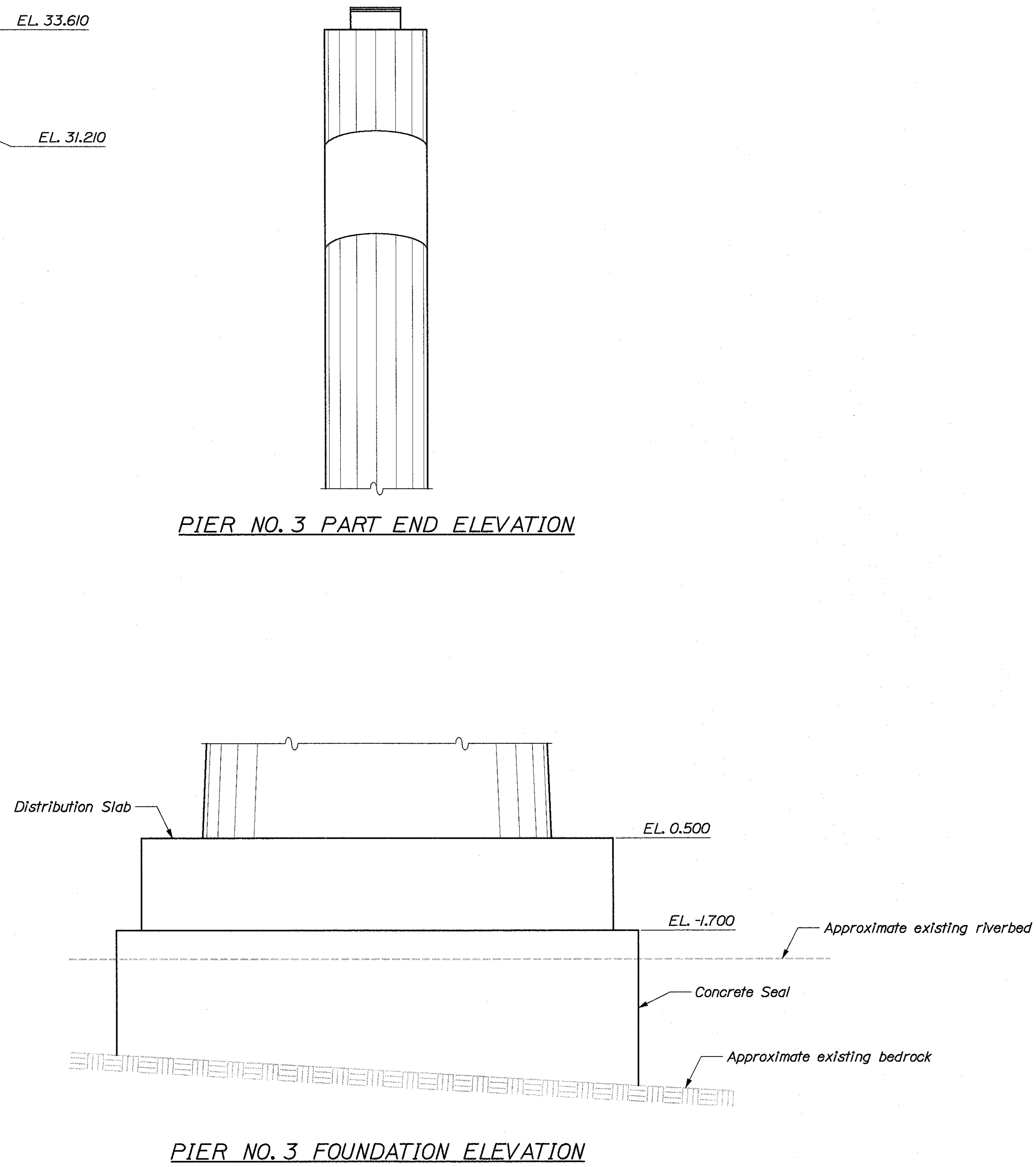
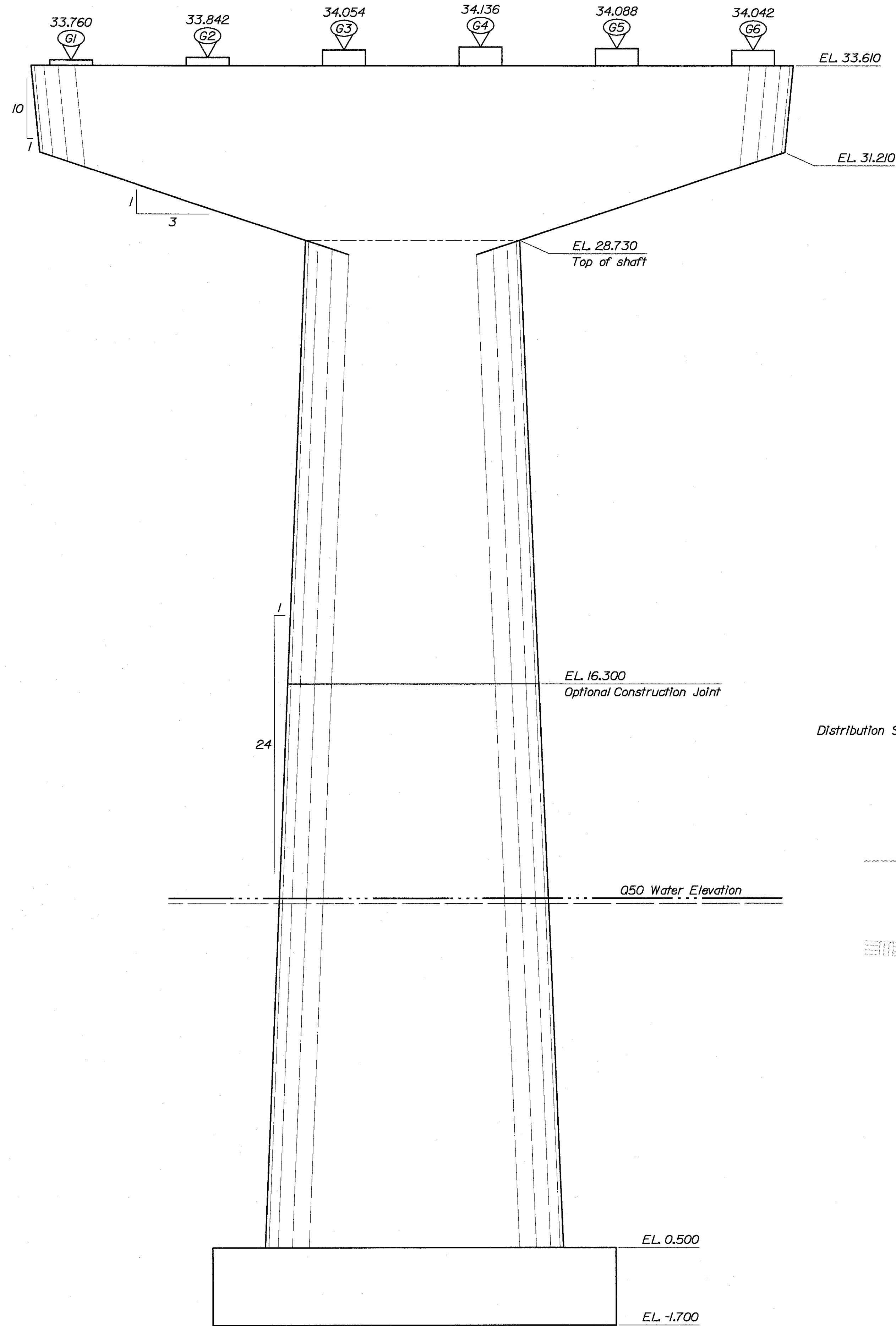
Username: Dana.Damren

Division: BRIDGE

Filename: ...058\_pier\_3\_elevation.dgn

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS



METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                      |       |                |              |                 |
|----------------------|-------|----------------|--------------|-----------------|
| F.H.W.A.<br>REQ. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                    | MAINE | NH-0556(220)E  | 58           | 105             |

000556.22

BRIDGE NO. 6400

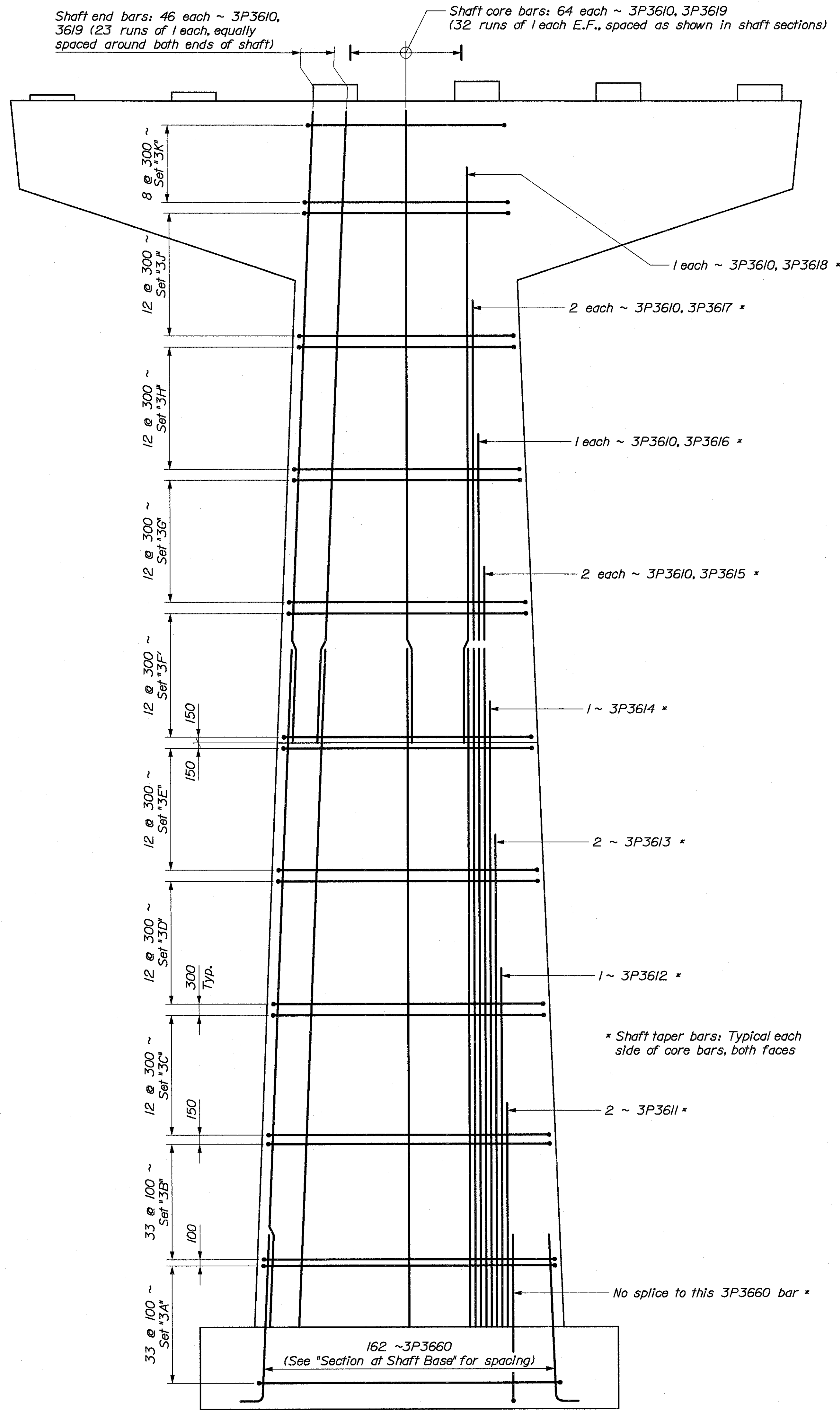
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 3 ELEVATION

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS



PIER NO. 3 SHAFT REINFORCING

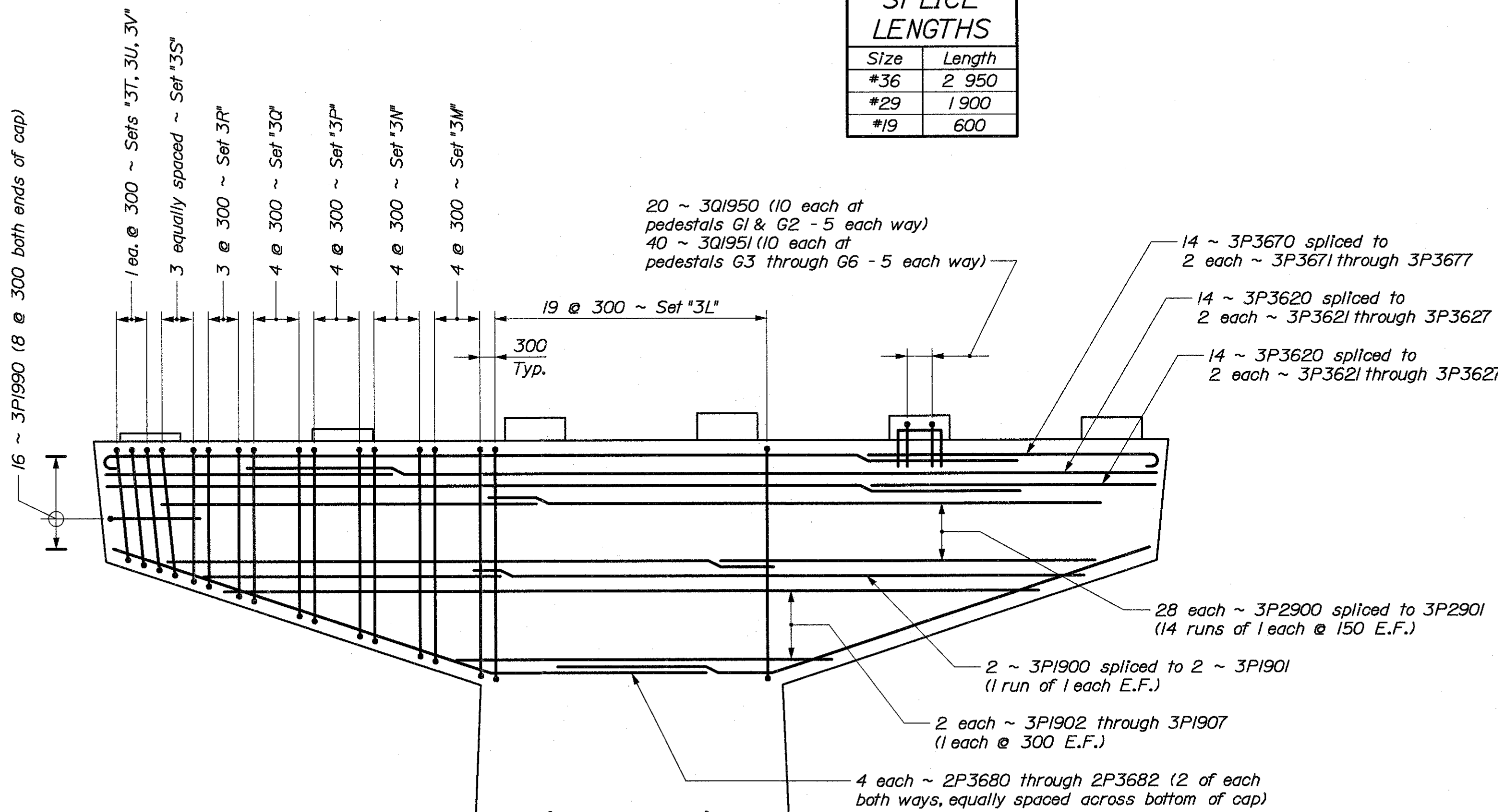
METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 59        | 105          |

000556.22

| CAP BAR SPLICE LENGTHS |        |
|------------------------|--------|
| Size                   | Length |
| *36                    | 2 950  |
| *29                    | 1 900  |
| *19                    | 600    |



PIER NO. 3 CAP REINFORCING

PIER NO. 3 REINFORCING STEEL SET SCHEDULE

| Set | Qty. | Location | Bar Designations                                                                                |
|-----|------|----------|-------------------------------------------------------------------------------------------------|
| 3A  | 33   | Shaft    | 2 ~ 3P1950, 2 ~ 3P1960, 6 ~ 3P1970, 2 ~ 3P1971, 2 ~ 3P1972, 2 ~ 3P1973, 17 ~ 3P1974, 4 ~ 3P1975 |
| 3B  | 33   | Shaft    | 2 ~ 3P1951, 2 ~ 3P1961, 6 ~ 3P1970, 2 ~ 3P1971, 2 ~ 3P1972, 2 ~ 3P1973, 15 ~ 3P1974, 4 ~ 3P1975 |
| 3C  | 12   | Shaft    | 2 ~ 3P1350, 2 ~ 3P1360, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 15 ~ 3P1374, 4 ~ 3P1375 |
| 3D  | 12   | Shaft    | 2 ~ 3P1351, 2 ~ 3P1361, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 13 ~ 3P1374, 4 ~ 3P1375 |
| 3E  | 12   | Shaft    | 2 ~ 3P1352, 2 ~ 3P1362, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 13 ~ 3P1374, 4 ~ 3P1375 |
| 3F  | 12   | Shaft    | 2 ~ 3P1353, 2 ~ 3P1363, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 11 ~ 3P1374, 4 ~ 3P1375 |
| 3G  | 12   | Shaft    | 2 ~ 3P1354, 2 ~ 3P1364, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 11 ~ 3P1374, 4 ~ 3P1375 |
| 3H  | 12   | Shaft    | 2 ~ 3P1355, 2 ~ 3P1365, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 9 ~ 3P1374, 4 ~ 3P1375  |
| 3J  | 12   | Shaft    | 2 ~ 3P1356, 2 ~ 3P1366, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 9 ~ 3P1374, 4 ~ 3P1375  |
| 3K  | 8    | Shaft    | 2 ~ 3P1357, 2 ~ 3P1367, 6 ~ 3P1370, 2 ~ 3P1371, 2 ~ 3P1372, 2 ~ 3P1373, 7 ~ 3P1374, 4 ~ 3P1375  |
| 3L  | 19   | Cap      | 2 ~ 3P1980                                                                                      |
| 3M  | 8    | Cap      | 2 ~ 3P1981, 1 ~ 3P1991                                                                          |
| 3N  | 8    | Cap      | 2 ~ 3P1982, 1 ~ 3P1992                                                                          |
| 3P  | 8    | Cap      | 2 ~ 3P1983, 1 ~ 3P1993                                                                          |
| 3Q  | 8    | Cap      | 2 ~ 3P1984, 1 ~ 3P1994                                                                          |
| 3R  | 6    | Cap      | 2 ~ 3P1985, 1 ~ 3P1995                                                                          |
| 3S  | 6    | Cap      | 2 ~ 3P1986, 1 ~ 3P1996                                                                          |
| 3T  | 2    | Cap      | 2 ~ 3P1987, 1 ~ 3P1997                                                                          |
| 3U  | 2    | Cap      | 2 ~ 3P1988, 1 ~ 3P1998                                                                          |
| 3V  | 2    | Cap      | 2 ~ 3P1989, 1 ~ 3P1999                                                                          |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 3 REINFORCING

SHEET OF AUGUSTA, MAINE



SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

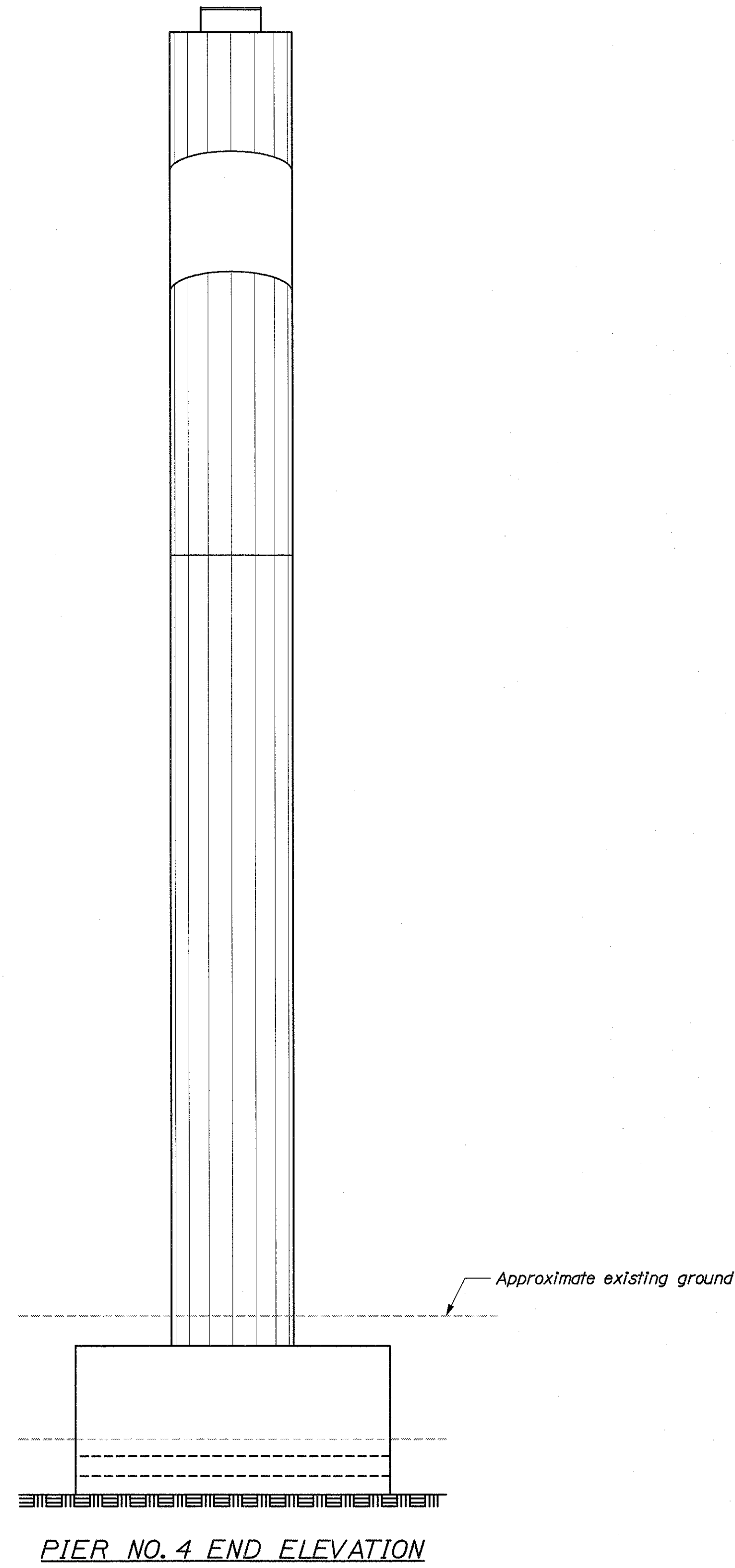
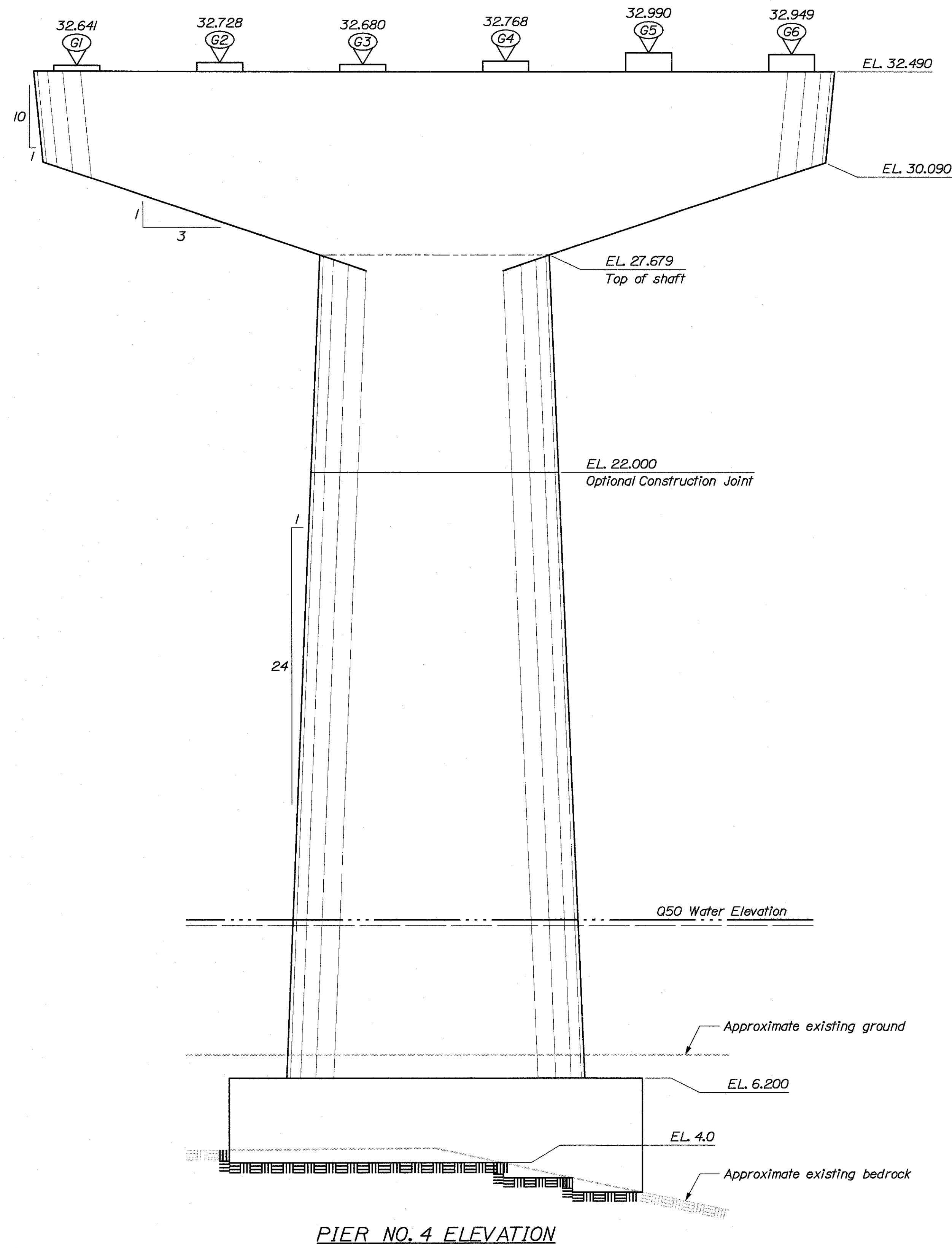
Username: Dana.Damren

Division: BRIDGE

Filename: ...\\062\_pier\_4 elevation.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

| PLANS |
|-------|
|-------|



METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 62           | 105             |

000556.22

BRIDGE NO. 6400

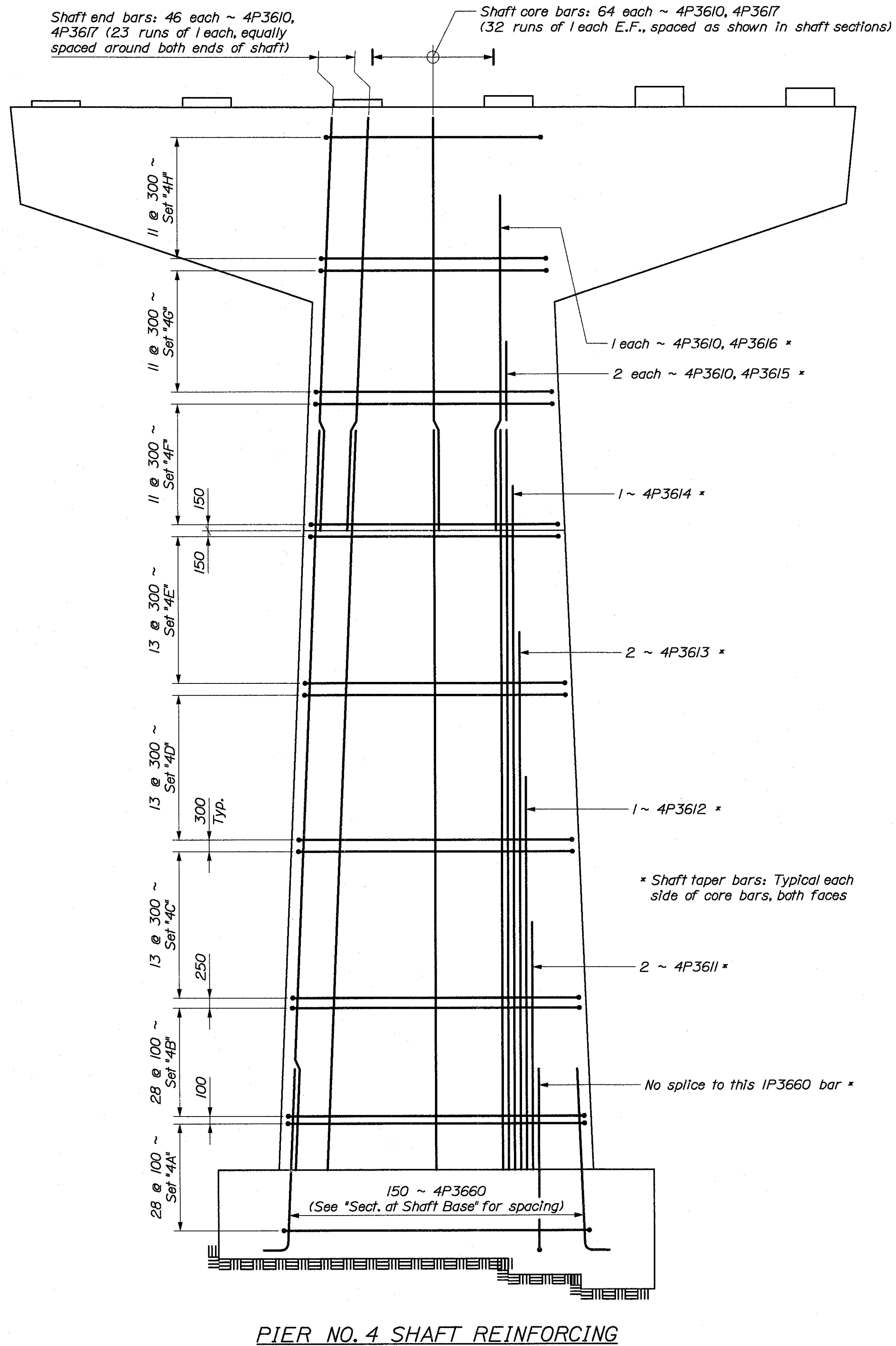
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**PIER NO. 4 ELEVATION**

SHEET OF AUGUSTA, MAINE

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

|       |
|-------|
| PLANS |
|-------|

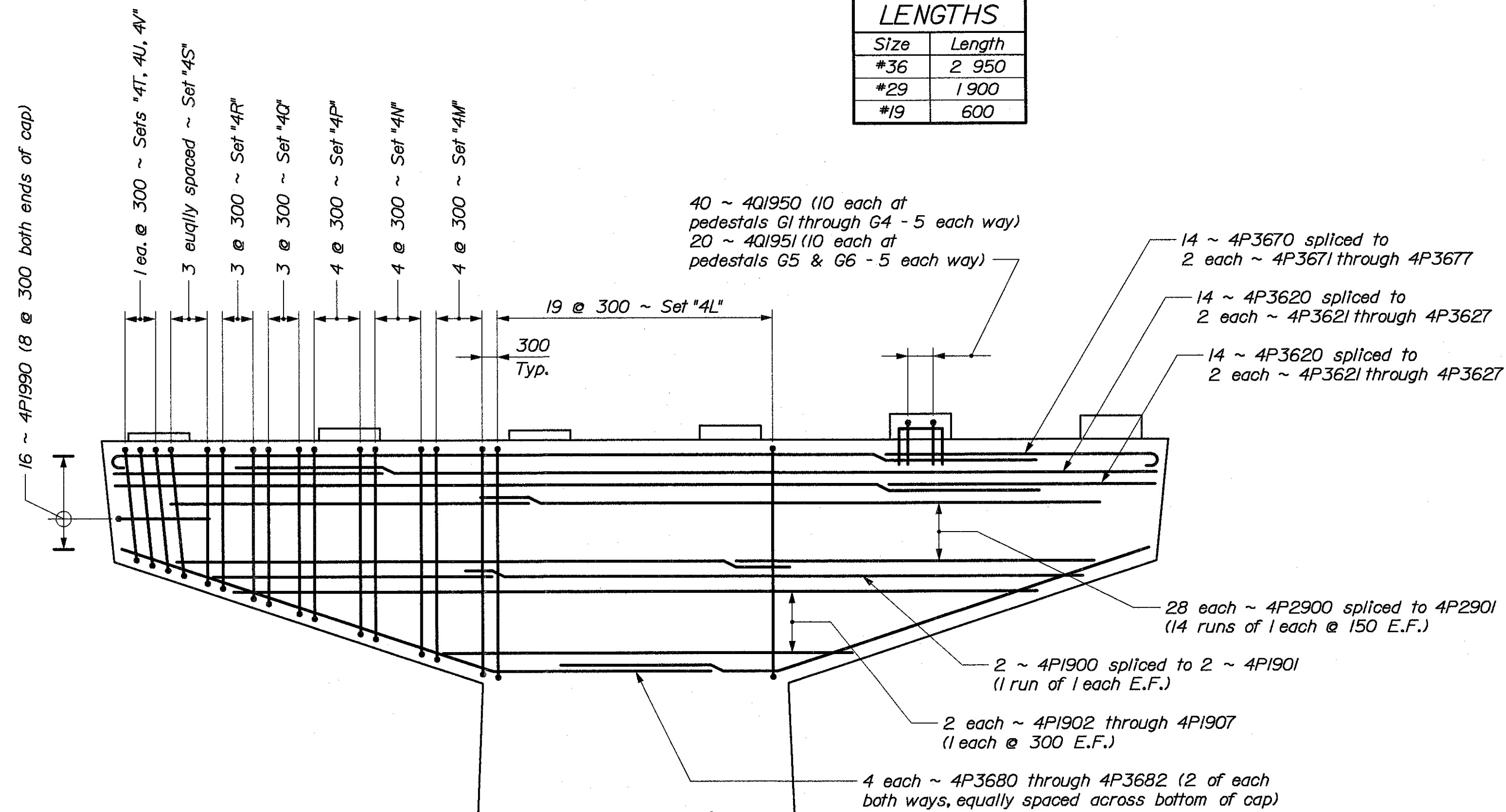
**METRIC**

- All dimensions are in millimeters unless otherwise noted.
- All elevations and stations are in meters.

|               |       |                |           |              |
|---------------|-------|----------------|-----------|--------------|
| FHWA RES. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
| 1             | MAINE | NH-0556(22)01E | 63        | 105          |

000556.22

| CAP BAR SPLICE LENGTHS |        |
|------------------------|--------|
| Size                   | Length |
| #36                    | 2 950  |
| #29                    | 1 900  |
| #19                    | 600    |



PIER NO. 4 REINFORCING STEEL SET SCHEDULE

| Set | Qty. | Location | Bar Designations                                                                                |
|-----|------|----------|-------------------------------------------------------------------------------------------------|
| 4A  | 28   | Shaft    | 2 ~ 4P1950, 2 ~ 4P1960, 6 ~ 4P1970, 2 ~ 4P1971, 2 ~ 4P1972, 2 ~ 4P1973, 15 ~ 4P1974, 4 ~ 4P1975 |
| 4B  | 28   | Shaft    | 2 ~ 4P1951, 2 ~ 4P1961, 6 ~ 4P1970, 2 ~ 4P1971, 2 ~ 4P1972, 2 ~ 4P1973, 13 ~ 4P1974, 4 ~ 4P1975 |
| 4C  | 13   | Shaft    | 2 ~ 4P1350, 2 ~ 4P1360, 6 ~ 4P1370, 2 ~ 4P1371, 2 ~ 4P1372, 2 ~ 4P1373, 13 ~ 4P1374, 4 ~ 4P1375 |
| 4D  | 13   | Shaft    | 2 ~ 4P1351, 2 ~ 4P1361, 6 ~ 4P1370, 2 ~ 4P1371, 2 ~ 4P1372, 2 ~ 4P1373, 11 ~ 4P1374, 4 ~ 4P1375 |
| 4E  | 13   | Shaft    | 2 ~ 4P1352, 2 ~ 4P1362, 6 ~ 4P1370, 2 ~ 4P1371, 2 ~ 4P1372, 2 ~ 4P1373, 11 ~ 4P1374, 4 ~ 4P1375 |
| 4F  | 11   | Shaft    | 2 ~ 4P1353, 2 ~ 4P1363, 6 ~ 4P1370, 2 ~ 4P1371, 2 ~ 4P1372, 2 ~ 4P1373, 9 ~ 4P1374, 4 ~ 4P1375  |
| 4G  | 11   | Shaft    | 2 ~ 4P1354, 2 ~ 4P1364, 6 ~ 4P1370, 2 ~ 4P1371, 2 ~ 4P1372, 2 ~ 4P1373, 9 ~ 4P1374, 4 ~ 4P1375  |
| 4H  | 11   | Shaft    | 2 ~ 4P1355, 2 ~ 4P1365, 6 ~ 4P1370, 2 ~ 4P1371, 2 ~ 4P1372, 2 ~ 4P1373, 7 ~ 4P1374, 4 ~ 4P1375  |
| 4L  | 19   | Cap      | 2 ~ 4P1980                                                                                      |
| 4M  | 8    | Cap      | 2 ~ 4P1981, 1 ~ 4P1991                                                                          |
| 4N  | 8    | Cap      | 2 ~ 4P1982, 1 ~ 4P1992                                                                          |
| 4P  | 8    | Cap      | 2 ~ 4P1983, 1 ~ 4P1993                                                                          |
| 4Q  | 6    | Cap      | 2 ~ 4P1984, 1 ~ 4P1994                                                                          |
| 4R  | 6    | Cap      | 2 ~ 4P1985, 1 ~ 4P1995                                                                          |
| 4S  | 6    | Cap      | 2 ~ 4P1986, 1 ~ 4P1996                                                                          |
| 4T  | 2    | Cap      | 2 ~ 4P1987, 1 ~ 4P1997                                                                          |
| 4U  | 2    | Cap      | 2 ~ 4P1988, 1 ~ 4P1998                                                                          |
| 4V  | 2    | Cap      | 2 ~ 4P1989, 1 ~ 4P1999                                                                          |

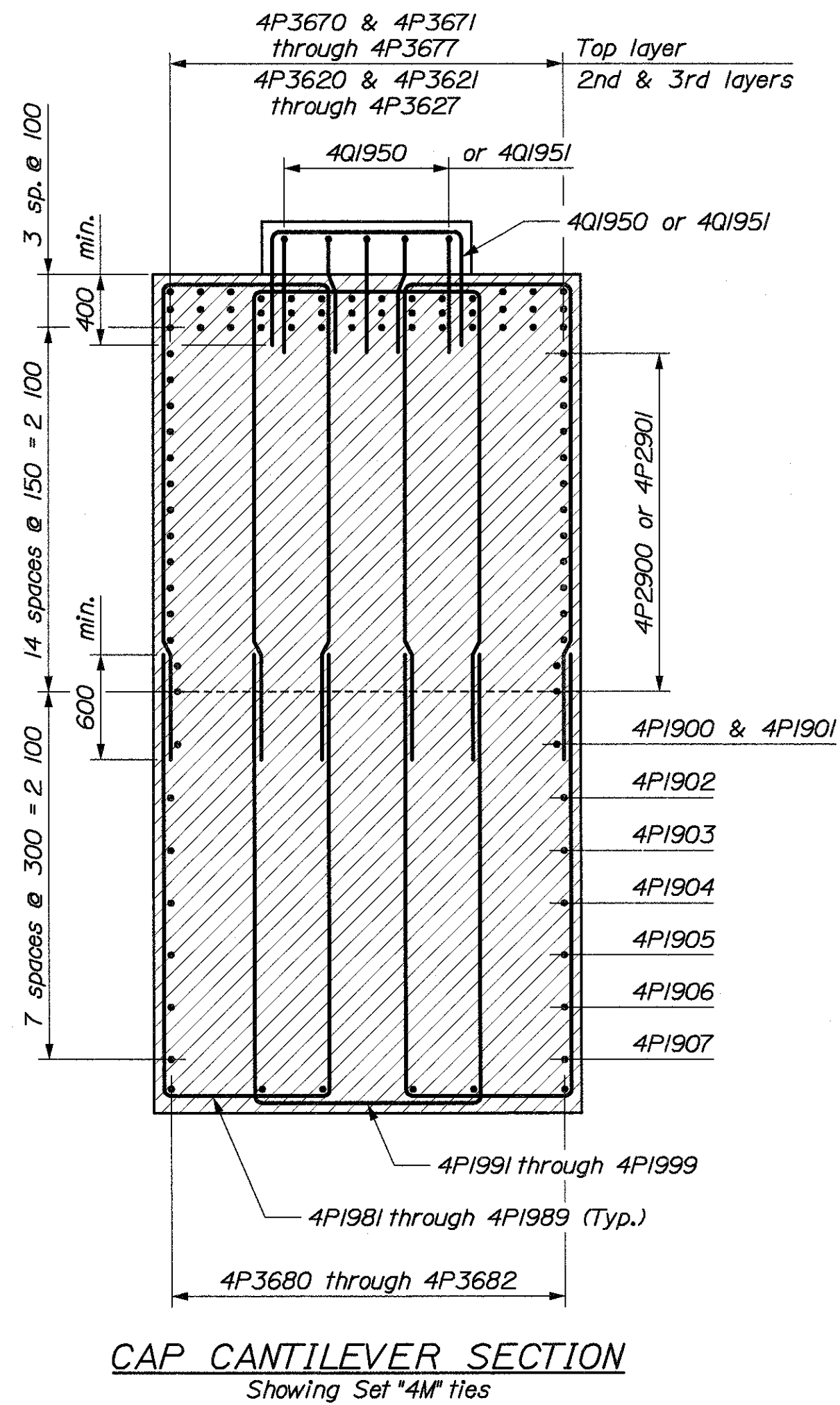
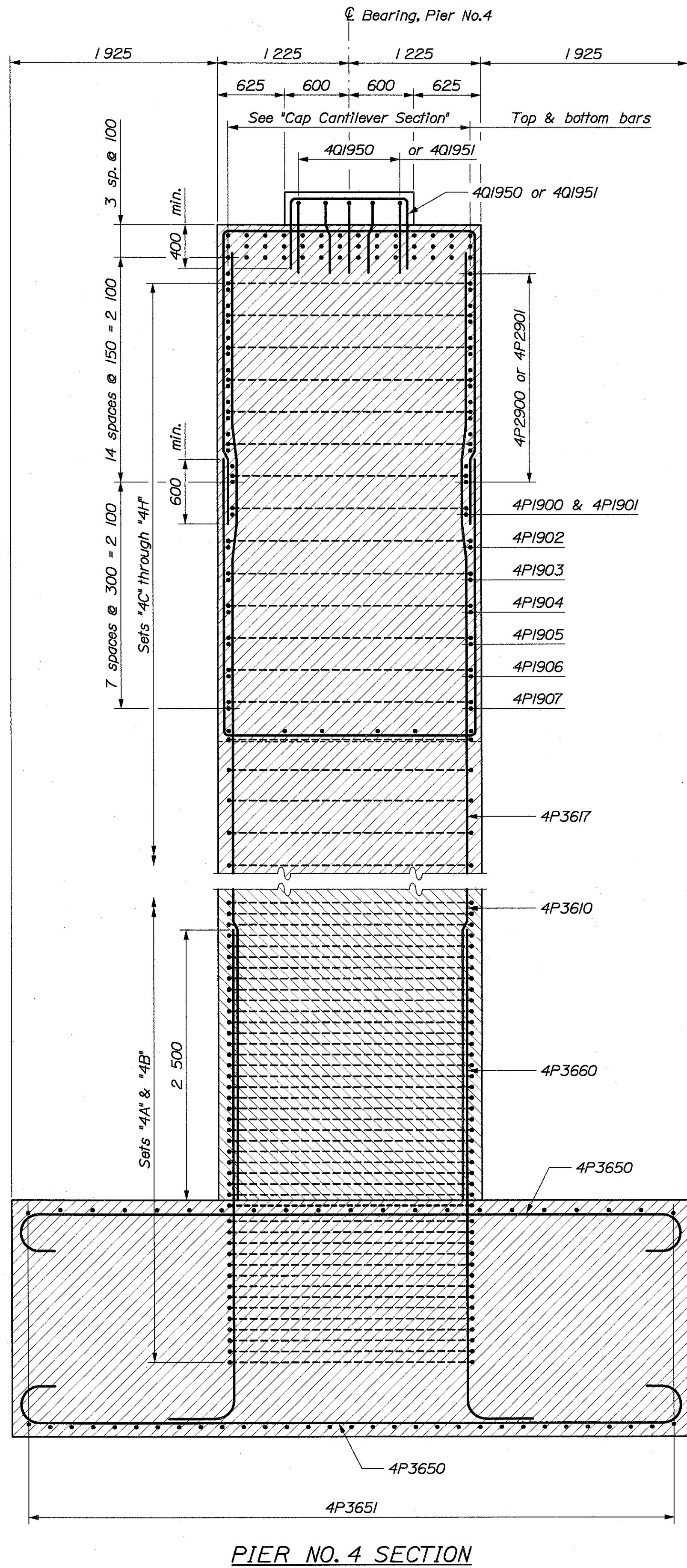
BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATIONSTATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
PIER NO. 4 REINFORCING

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

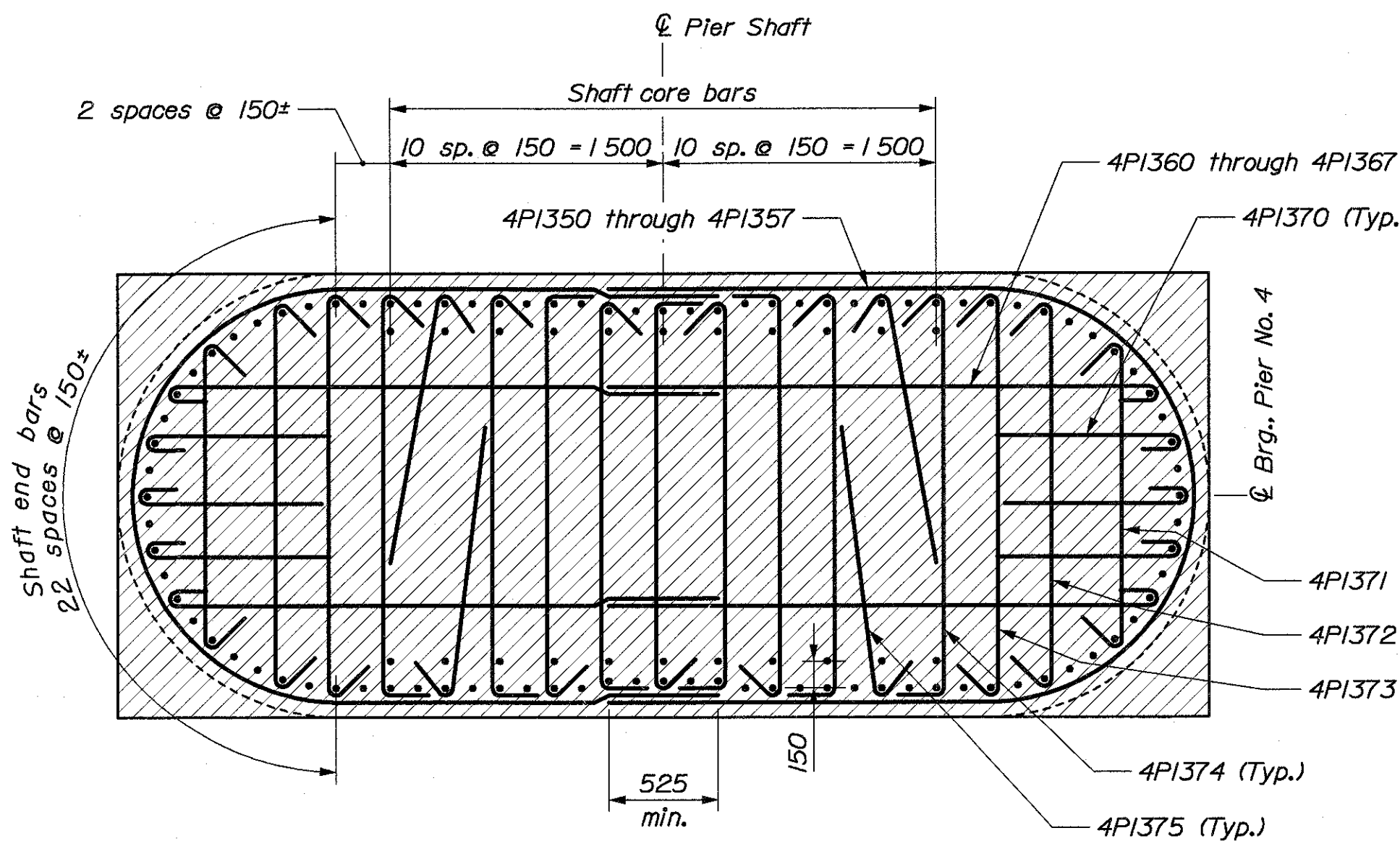
PLANS



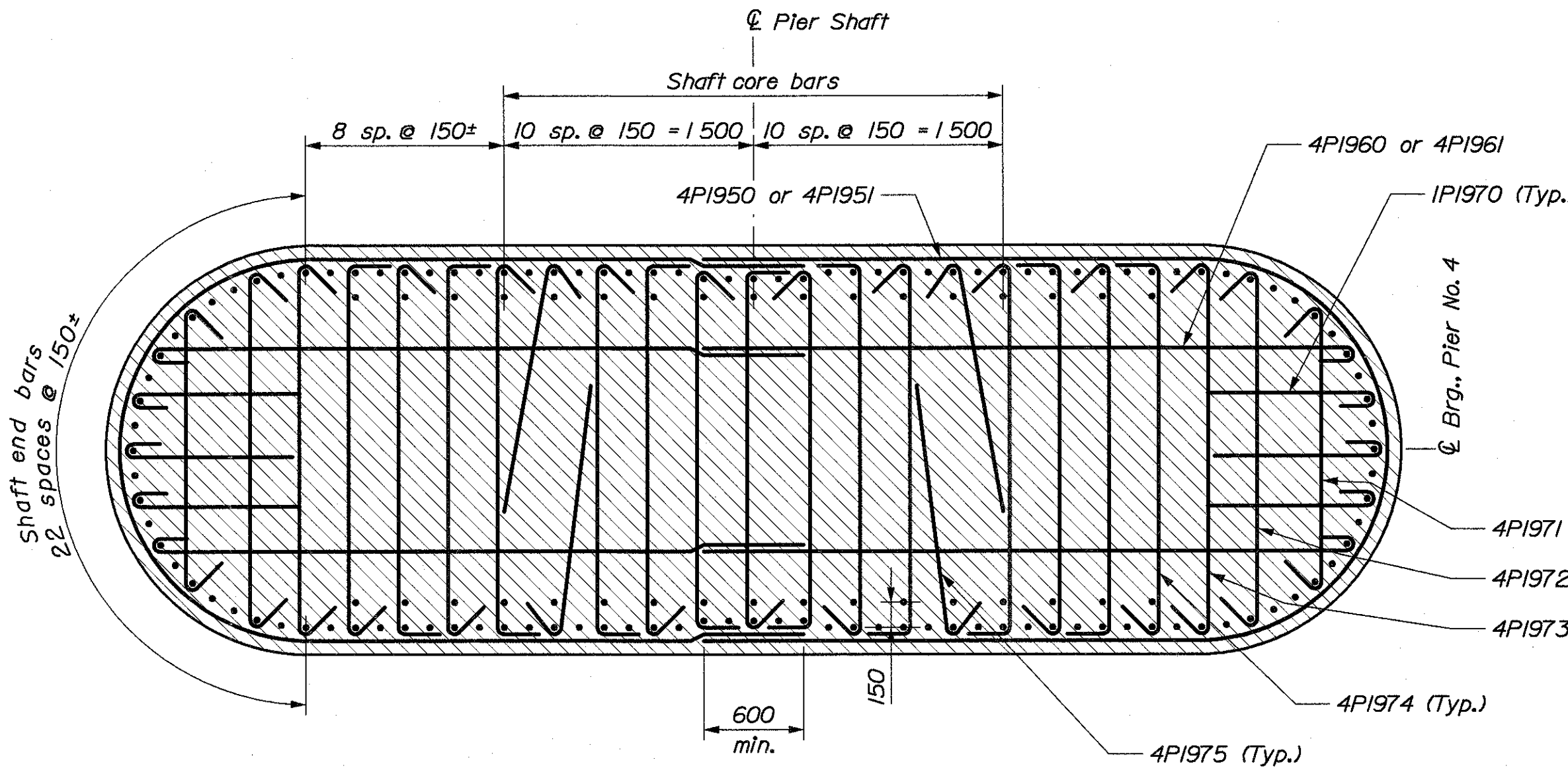
**METRIC** 1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 64        | 105          |

000556.22



**HORIZONTAL SECTION AT SHAFT TOP**  
114 vertical bars ~ showing Set "4G" ties



**HORIZONTAL SECTION AT SHAFT BASE**  
150 vertical bars ~ showing Set "4A" ties

BRIDGE NO. 6400  
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
**STATE ROUTE 3**  
OVER  
**KENNEBEC RIVER**  
IN THE CITY OF  
**AUGUSTA**  
KENNEBEC COUNTY  
**PIER NO. 4 SECTIONS**

Filename: ... \065\_pier\_drip\_pans.dgn

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

| PROJECT DESIGN ENGINEER | BY           | DATE |
|-------------------------|--------------|------|
| DESIGN-DETAILED         | J. J. Folsom |      |
| CHECKED                 | D. Damren    |      |
| REVISIONS               |              |      |
| FIELD CHANGES           |              |      |

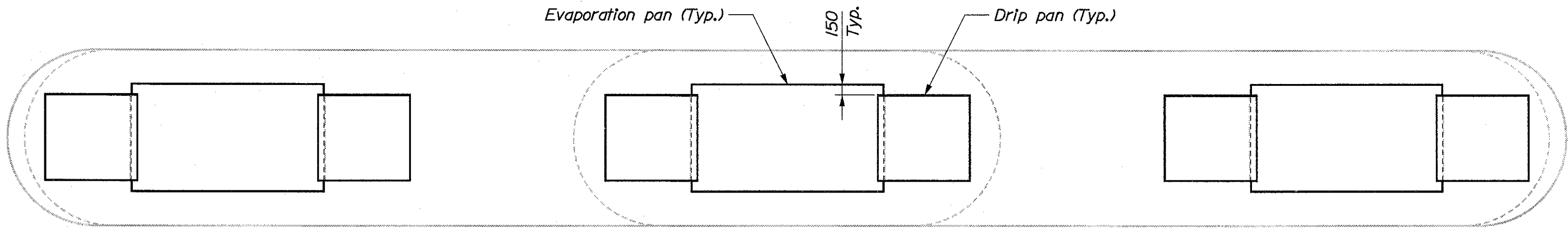
PLANS

METRIC

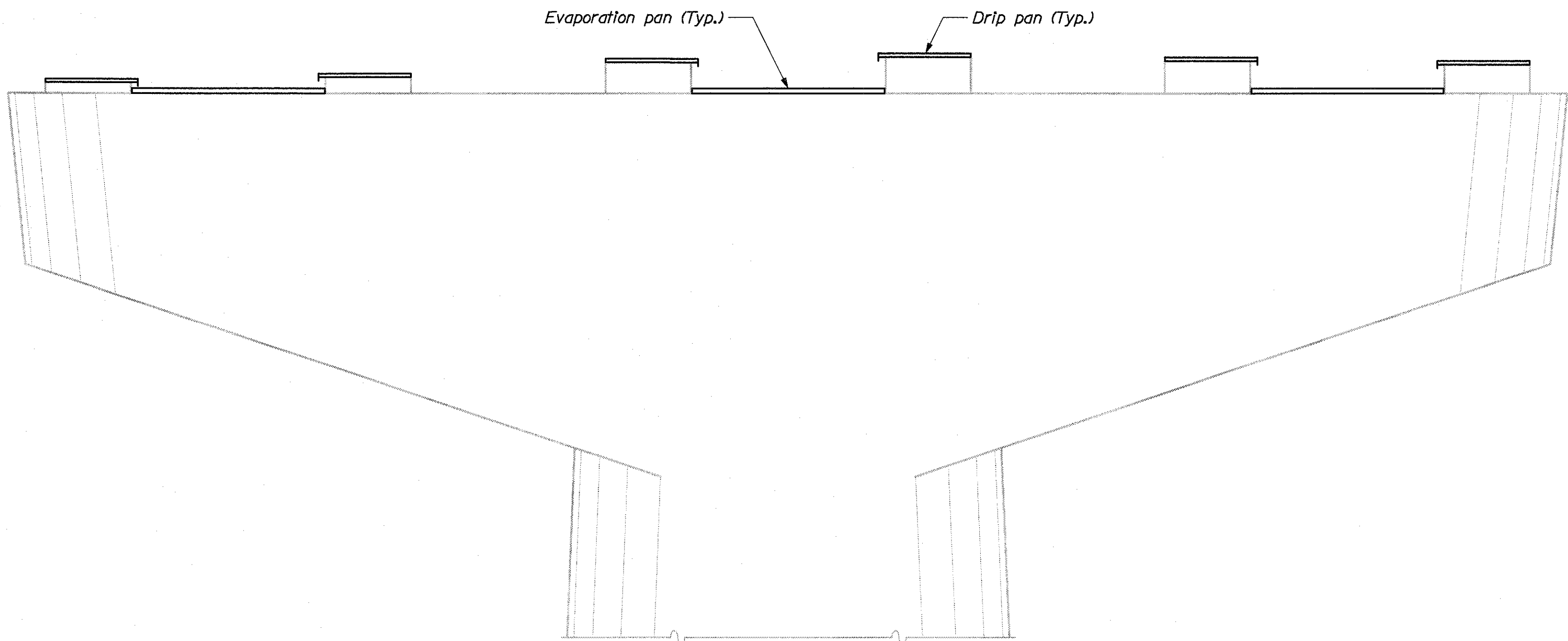
1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 65           | 105             |

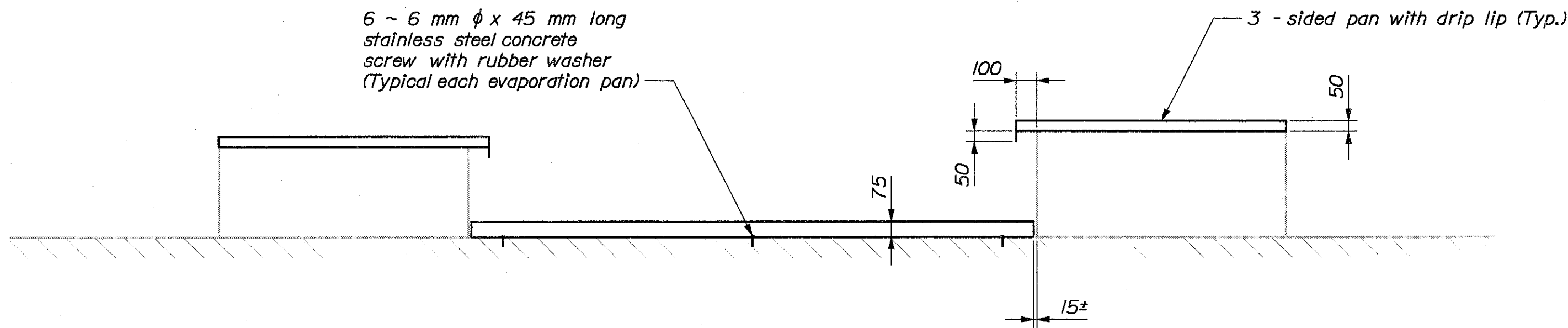
000556.22



TYPICAL PIER CAP PLAN



TYPICAL PIER CAP ELEVATION



PAN DETAILS

DRIP PAN NOTES

1. Pans shall be cold formed from 16 gauge stainless steel sheets.
2. Payment for drip and evaporation pans will be considered incidental to the structural steel pay items.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**PIER DRIP PANS**

SHEET OF AUGUSTA, MAINE

Date:26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ... \066\_pot bearing layout.dgn

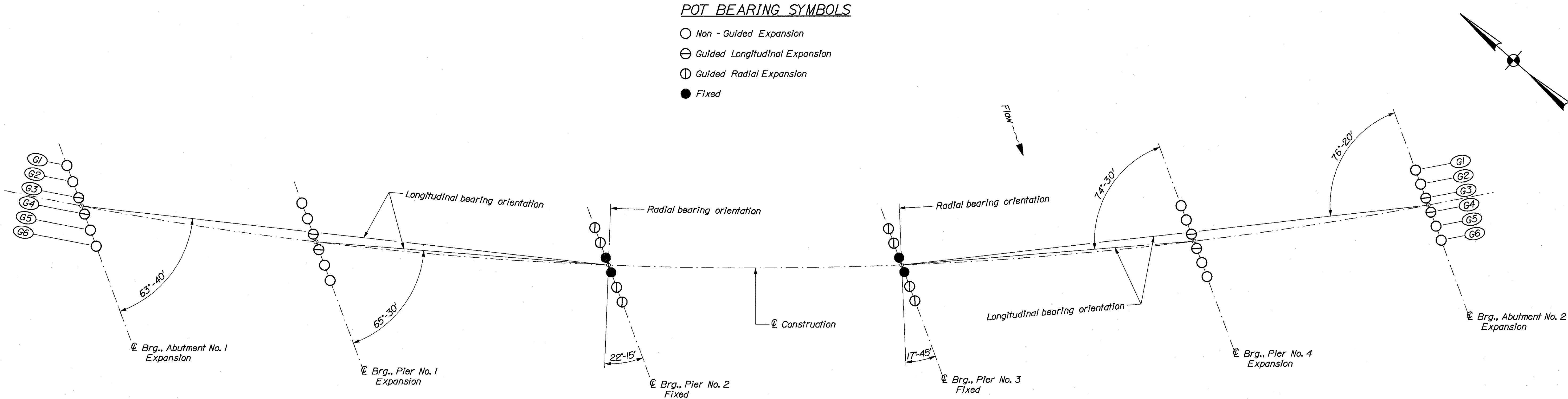
| PROJECT DESIGN ENGINEER | DESIGN-DETAILED | CHECKED | REVISIONS | FIELD CHANGES | PLANS | BY        | DATE |
|-------------------------|-----------------|---------|-----------|---------------|-------|-----------|------|
|                         |                 |         |           |               |       |           |      |
|                         |                 |         |           |               |       |           |      |
|                         |                 |         |           |               |       |           |      |
|                         |                 |         |           |               |       | J. Folsom |      |
|                         |                 |         |           |               |       | D. Damren |      |

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A. RES. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 66        | 105          |

000556.22



POT BEARING LAYOUT PLAN

POT BEARING NOTES

1. Refer to the Special Provisions for design, materials, fabrication and general construction requirements.
2. The actual dimension "H" shall be the responsibility of the Contractor. Dimensions and sizes of plates not shown are dependent on design loads, bearing type, capacity and the manufacturer of the bearings. The shop drawings, prepared by the manufacturer, shall provide all pertinent bearing information. The final bridge seat elevations shall be determined by the Contractor and submitted with the shop drawings for approval prior to construction of the substructure units.
3. Masonry plates shall be placed on 3 mm thick preformed pads in accordance with the specifications.
4. All steel, including anchor bolts, shall be AASHTO M 270, Grade 345W.
5. Anchor bolt spacing shall be coordinated with the bearing manufacturer.
6. Bearing installation shall be in strict conformance with the Special Provisions and the manufacturer's recommendations.
7. The abbreviation "PTFE" indicates polytetrafluoroethylene.
8. The design temperature range shall be 85 °C (-35 °C to 50 °C).
9. At abutment bearings only, all steel located below the PTFE sliding surface shall be coated in accordance with Special Provision, Section 506, Protective Coating - Steel (Thermal Spray Coating). All remaining steel at abutment bearings shall be coated in accordance with Special Provision, Section 506, Protective Coating - Steel (Zinc Rich System).
10. In the Bearing Setting Corrections portion of the Pot Bearing Table, a negative dimension (-39) indicates a direction away from the nearer abutment backwall. A positive dimension (+13) indicates a direction toward the nearer abutment backwall.

POT BEARING TABLE

| POT BEARING TABLE |                      |                     |        |              |                |                     |        |              |                             |                             |       |      |       |       |       |               |                     |        |              |             |
|-------------------|----------------------|---------------------|--------|--------------|----------------|---------------------|--------|--------------|-----------------------------|-----------------------------|-------|------|-------|-------|-------|---------------|---------------------|--------|--------------|-------------|
|                   | Non - Guided Bearing |                     |        |              | Guided Bearing |                     |        |              | Total Longitudinal Movement | Bearing Setting Corrections |       |      |       |       |       | Fixed Bearing |                     |        |              |             |
| Location          | Girder               | Design Loads * (kN) |        | H (mm)<br>xx | Girder         | Design Loads * (kN) |        | H (mm)<br>xx |                             | Dimension "X" (mm)          |       |      |       |       |       | Girder        | Design Loads * (kN) |        | H (mm)<br>xx | Location    |
|                   |                      | Vertical            | Horiz. |              |                | Vertical            | Horiz. |              |                             | -13 °C                      | -3 °C | 7 °C | 17 °C | 27 °C | 37 °C |               | Vertical            | Horiz. |              |             |
| Abut. No. 1       | 1, 2, 5, 6           | 1 321.7             | —      | 190          | 3 & 4          | 1 321.7             | 390    | 285          | 167.6 mm                    | -39                         | -19   | 0    | +19   | +39   | +59   | —             | —                   | —      | Abut. No. 1  |             |
| Pier No. 1        | 1, 2, 5, 6           | 3 961.1             | —      | 230          | 3 & 4          | 3 961.1             | 570    | 365          | 108.9 mm                    | -26                         | -13   | 0    | +13   | +26   | +38   | —             | —                   | —      | Pier No. 1   |             |
| Pier No. 2        | —                    | —                   | —      | —            | 1, 2, 5, 6     | 4 024.9             | 580    | 365          | —                           | —                           | —     | —    | —     | —     | —     | 3 & 4         | 4 024.9             | 725    | 235          | Pier No. 2  |
| Pier No. 3        | —                    | —                   | —      | —            | 1, 2, 5, 6     | 3 990.2             | 580    | 365          | —                           | —                           | —     | —    | —     | —     | —     | 3 & 4         | 3 990.2             | 725    | 235          | Pier No. 3  |
| Pier No. 4        | 1, 2, 5, 6           | 4 010.9             | —      | 230          | 3 & 4          | 4 010.9             | 570    | 365          | 108.9 mm                    | -26                         | -13   | 0    | +13   | +26   | +38   | —             | —                   | —      | —            | Pier No. 4  |
| Abut. No. 2       | 1, 2, 5, 6           | 1 332.8             | —      | 190          | 3 & 4          | 1 332.8             | 390    | 285          | 167.6 mm                    | -39                         | -19   | 0    | +19   | +39   | +59   | —             | —                   | —      | —            | Abut. No. 2 |

\* Service Loads

\*\* See Note No. 2

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
POT BEARING LAYOUT

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

Username: Dana.Darren

Division: BRIDGE

Filename: ...067\_pot bearing details.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Darren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

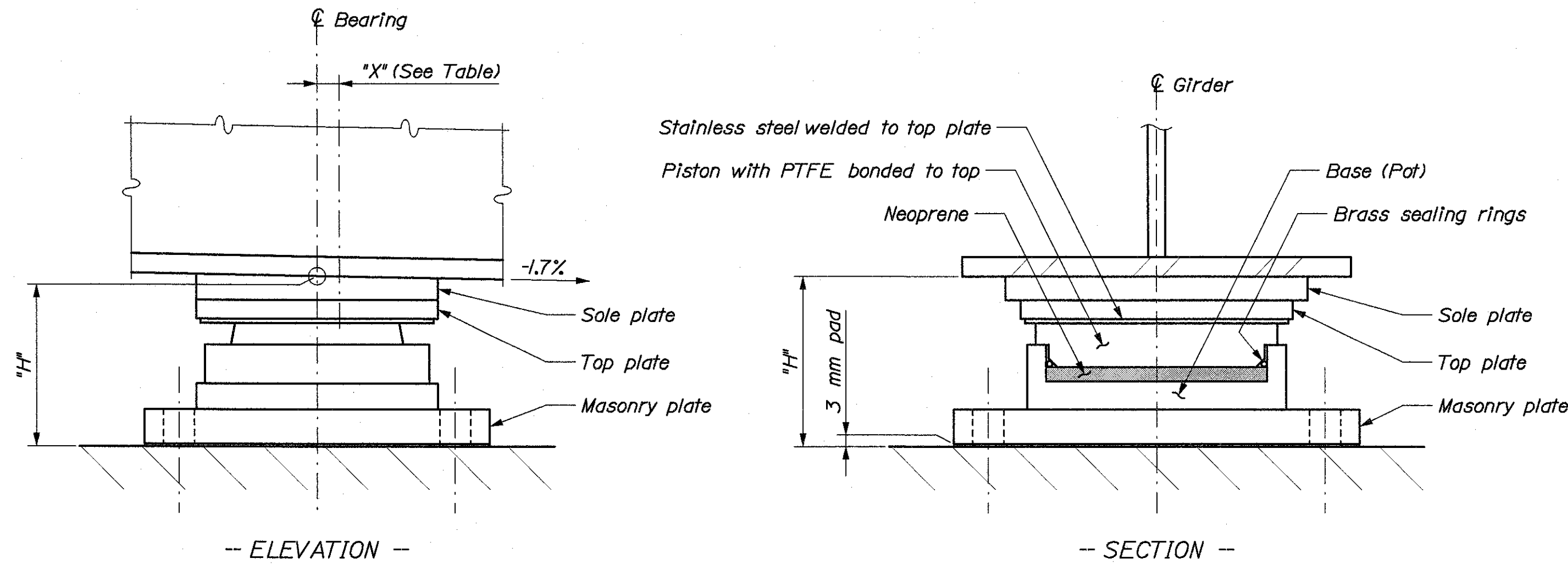
PLANS

METRIC

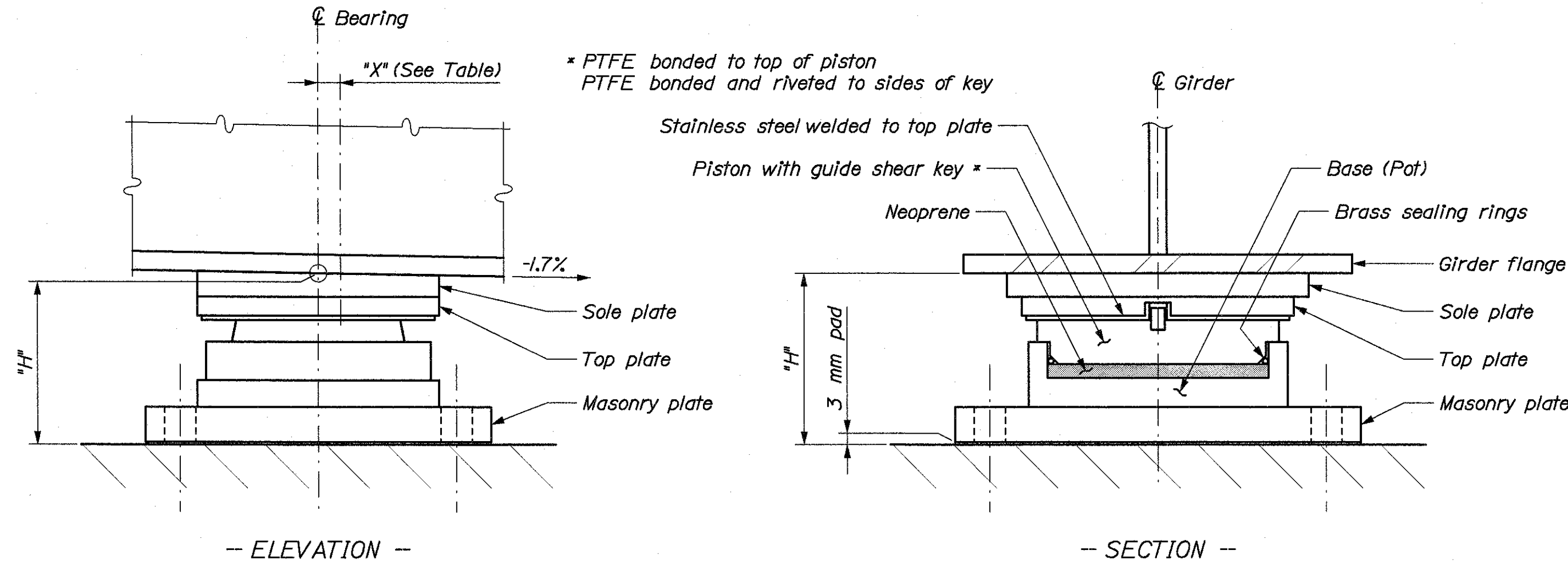
1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 67           | 105             |

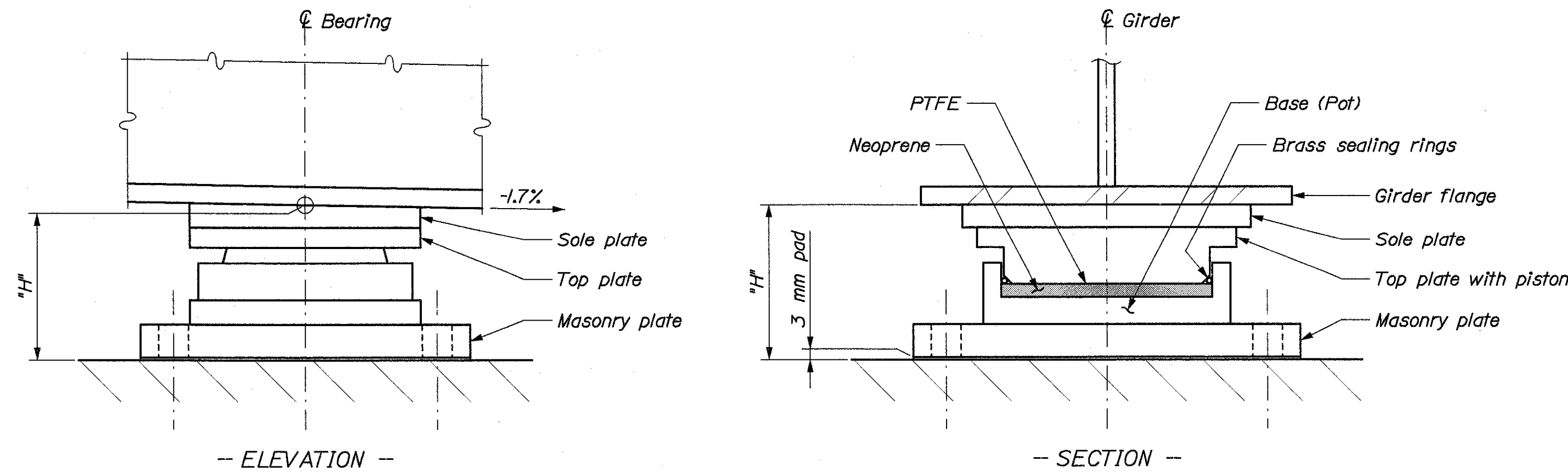
000556.22



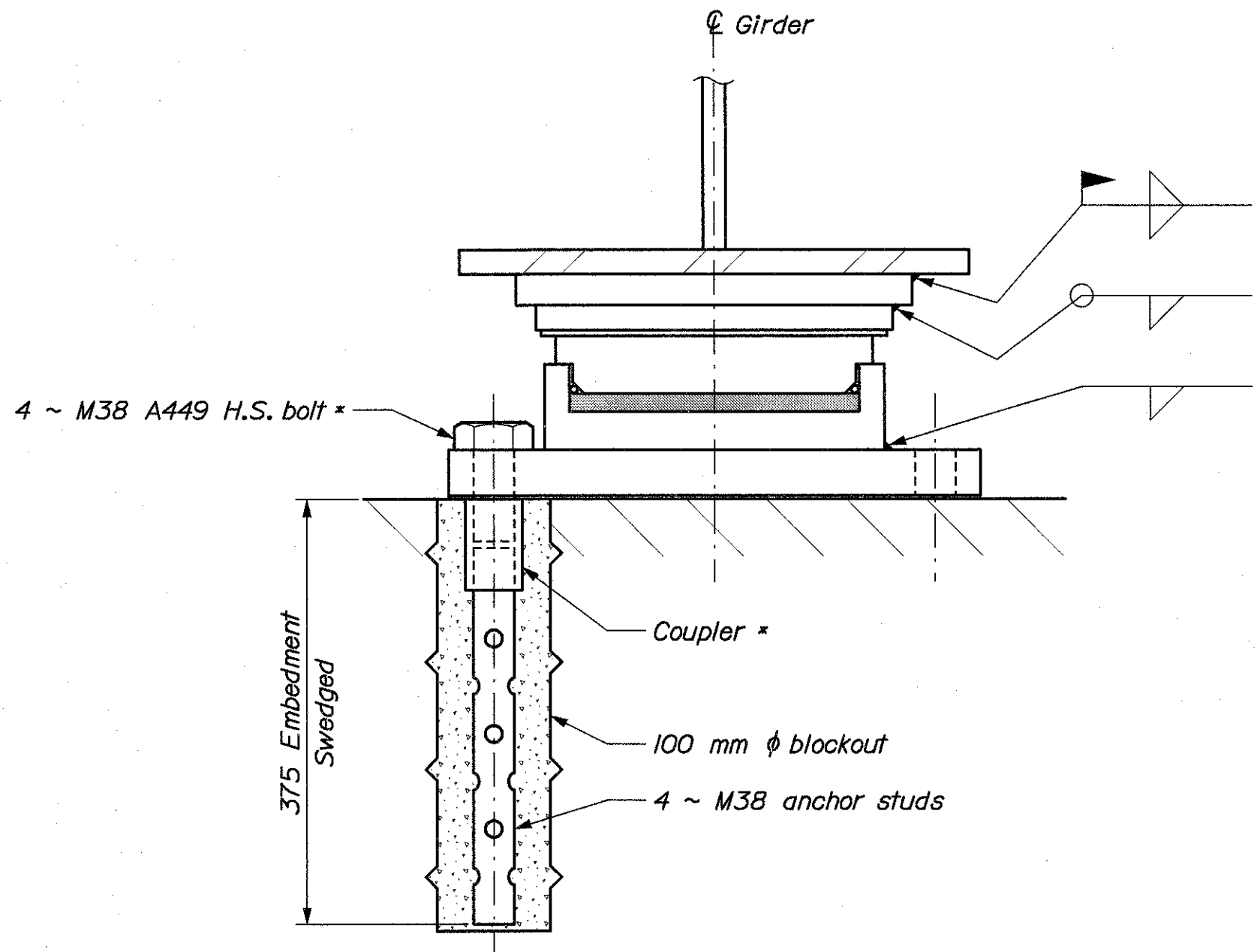
NON - GUIDED POT BEARING



GUIDED POT BEARING



FIXED POT BEARING



TYPICAL DETAILS  
\* Drilled and anchored studs similar to the details shown on Standard Detail 504(12) may be used as an alternate.

|                                                                                                                |
|----------------------------------------------------------------------------------------------------------------|
| BRIDGE NO. 6400                                                                                                |
| STATE OF MAINE<br>DEPARTMENT OF TRANSPORTATION                                                                 |
| STATE ROUTE 3<br>OVER<br>KENNEBEC RIVER<br>IN THE CITY OF<br>AUGUSTA<br>KENNEBEC COUNTY<br>POT BEARING DETAILS |
| SHEET OF<br>AUGUSTA, MAINE                                                                                     |

Date:26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ... \068\_framing layout plan.dgn

| PROJECT DESIGN ENGINEER | BY           | DATE |
|-------------------------|--------------|------|
| DESIGN/DETAILED         | J. F. Damren |      |
| CHECKED                 |              |      |
| REVISIONS               |              |      |
| FIELD CHANGES           |              |      |

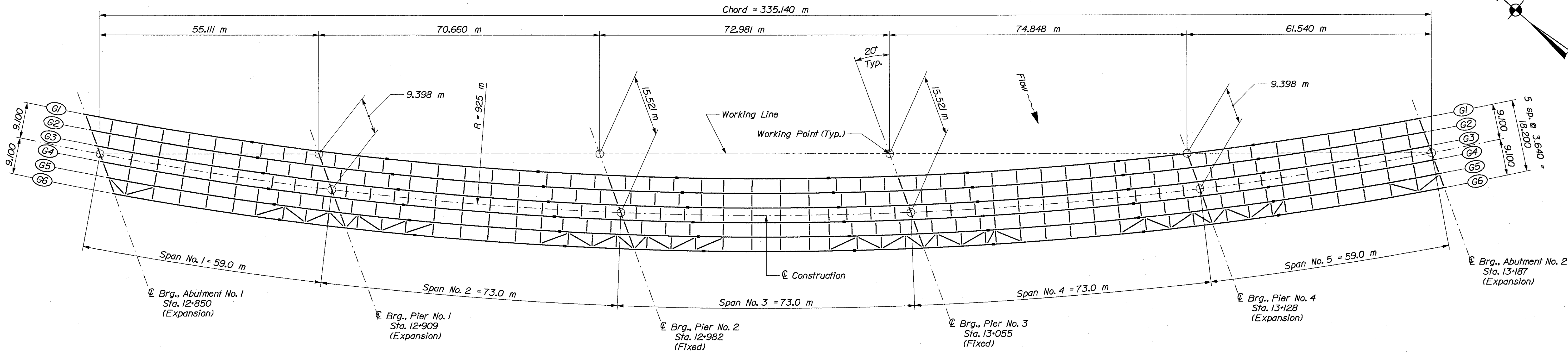
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 68           | 105             |

000556.22



FRAMING LAYOUT PLAN

### STRUCTURAL STEEL NOTES

- Girders may be either heat curved or cut curved in accordance with AASHTO specifications at the option of the Contractor.
- All web plates and all flange plates located in the negative moment sections shall be steel conforming to the requirements of ASTM A 709M, Grade HPS 485W. All other steel shall be ASTM A 709M, Grade 345W unless otherwise noted.
- Camber ordinates, as shown, are computed to compensate for all dead load deflections and for the curvature of the finished grade profile.
- Girder webs shall be vertical under full dead load.
- No transverse butt-weld splices will be allowed in the flange plates or web plates within 3.0 m or 10 percent of the span length (whichever is greater) from the points of maximum negative moment or maximum positive moment. Butt-weld splices in flanges shall be not less than 1.0 m from transverse butt-welds in the web plates and no transverse web or flange butt-welds shall be located within 1.0 m of other transverse welds (e.g. connection plates to web welds) on either flange or web. No transverse butt-weld splices will be allowed in areas of stress reversal.
- Sections of flange plates or web plates between transverse shop splices or between a transverse shop splice and a field splice shall be not less than 6.0 m in length unless otherwise shown on the plans.
- Use of undermatching fillet welds shall be in accordance with Special Provision, Section 504.
- Bolted field splice connections shall be made using M22 H.S. bolts. Hole size shall be 24 mm diameter unless otherwise shown. Bolt threads shall be excluded from the shear plane of field splice connections.
- Filler plates may be steel conforming to the requirements of ASTM A 709/A 709M, Grade 250.
- Steel for crossframes, connection plates and stiffeners shall be ASTM A 709/A 709M, Grade 345W.
- Bearing stiffeners shall be plumb after erection and dead loading of the structure. Intermediate web stiffeners may be either plumb or normal to the top flange.
- Crossframe connection plates may be either plumb or normal to the top flange.
- All crossframes except those at bearing locations are radial.
- Crossframes shall be detailed and fabricated to fit under full dead load.
- Bolted crossframe and lateral bracing connections shall be made using M22 H.S. bolts. Hole size shall be 24 mm diameter. The minimum edge distance shall be 38 mm unless otherwise shown. Oversized holes may be used with the permission of the Engineer.
- Connection plates shall be 14 mm minimum thickness and 260 mm minimum width. The plate thicknesses and widths for stiffeners shall be as shown on the "Girder Elevation" details.
- Connection plates shall be full web depth and shall be connected to flanges using the "Flange Connection Plate" detail.
- Bearing stiffeners shall be mill-to-bear on the bottom flange and tight fit to the top flange. Bearing stiffeners used as connection plates shall be connected to flanges in tension and stress reversal using the "Flange Connection Plate" detail.

- Intermediate stiffeners not intended to carry concentrated loads shall be tight fit to both flanges. Intermediate stiffeners used as connection plates shall be detailed as connection plates.
- Stiffeners not used as connection plates shall be welded to the web only. Connection plates and stiffeners used as connection plates shall be welded to the web and flange connection plate on both sides of the plates. The welds shall terminate 15 mm  $\pm$  3 mm from the ends of the plates.
- Prior to structural steel erection, the Contractor shall submit an erection sequence for approval.
- Bottom flange lateral bracing is designed to control flange bending stresses due to wind load during erection of the structural steel. The lateral bracing is intended to be placed between the first pair of girders to be erected. If the Contractor chooses to erect a pair of girders other than G5 and G6 first, the location of the bottom flange lateral bracing shall be modified and so shown on the shop drawings. No additional payment will be made for this change.
- The Contractor may eliminate the bottom flange lateral bracing system by providing other methods of ensuring stability and controlling wind loads during erection of the structural steel. If the Contractor chooses to eliminate the lateral bracing system, the Contractor shall submit for approval a plan which includes any relevant calculations and working drawings, signed and sealed by a Professional Engineer.
- The "Drip Plate" detail is for use on all girders.
- Handhold bars shall be installed in accordance with the Standard Details 502(23-24).
- Outside and bottom surfaces of fascia girders shall be blast-cleaned to SSPC SP-6 after fabrication and prior to delivery in order to present a uniform appearance free of dirt, oil or other deleterious material.
- Girder ends and end crossframes shall be coated to a distance of three (3) meters from the centerline of bearing at the abutments in accordance with Special Provision, Section 506, Protective Coating - Steel (Zinc Rich System).
- Prior to erection of the structural steel, the Contractor shall wrap abutments and piers with polyethylene sheeting. The polyethylene sheeting shall remain in place and kept free from any tears or separations until the deck slab has been placed. After removing the polyethylene sheeting, the Contractor shall thoroughly clean the abutments and piers of all stains by a method approved by the Engineer. Payment will be considered incidental to related contract items.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
FRAMING LAYOUT PLAN

SHEET OF AUGUSTA, MAINE

Filename: ...\\069\_framing plan span no. 1.dgn

Username: Dana.Damren

Division: BRIDGE

Date: 26 NOV 2002

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
|                         |           |      |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

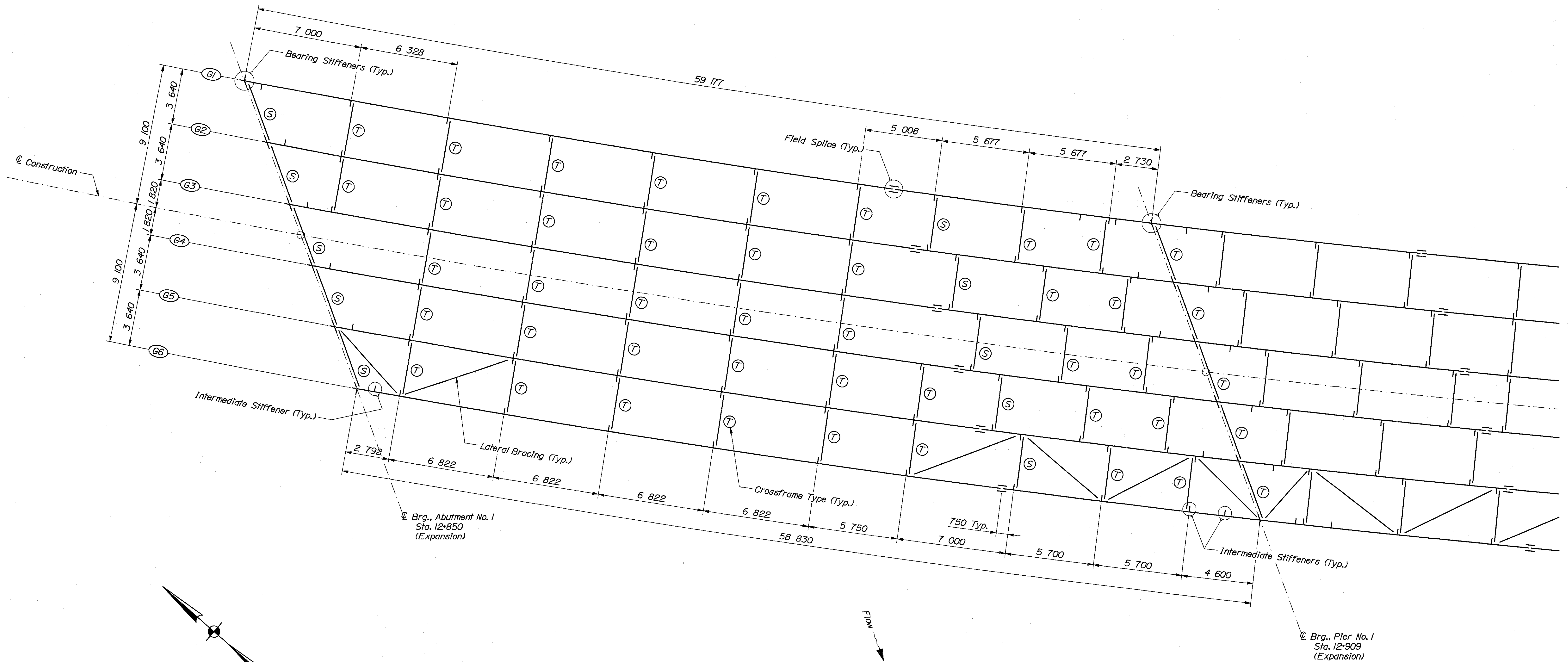
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 69           | 105             |

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FRAMING PLAN ~ SPAN NO. 1

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
FRAMING PLAN ~ SP. NO. 1

SHEET OF AUGUSTA, MAINE

Filename: ... \070\_framing plan span no. 2.dgn

Username: Dana.Damren

Division: BRIDGE

Date: 26 NOV 2002

| PROJECT DESIGN ENGINEER | BY        | DATE      |
|-------------------------|-----------|-----------|
| DESIGN-DETAILED         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

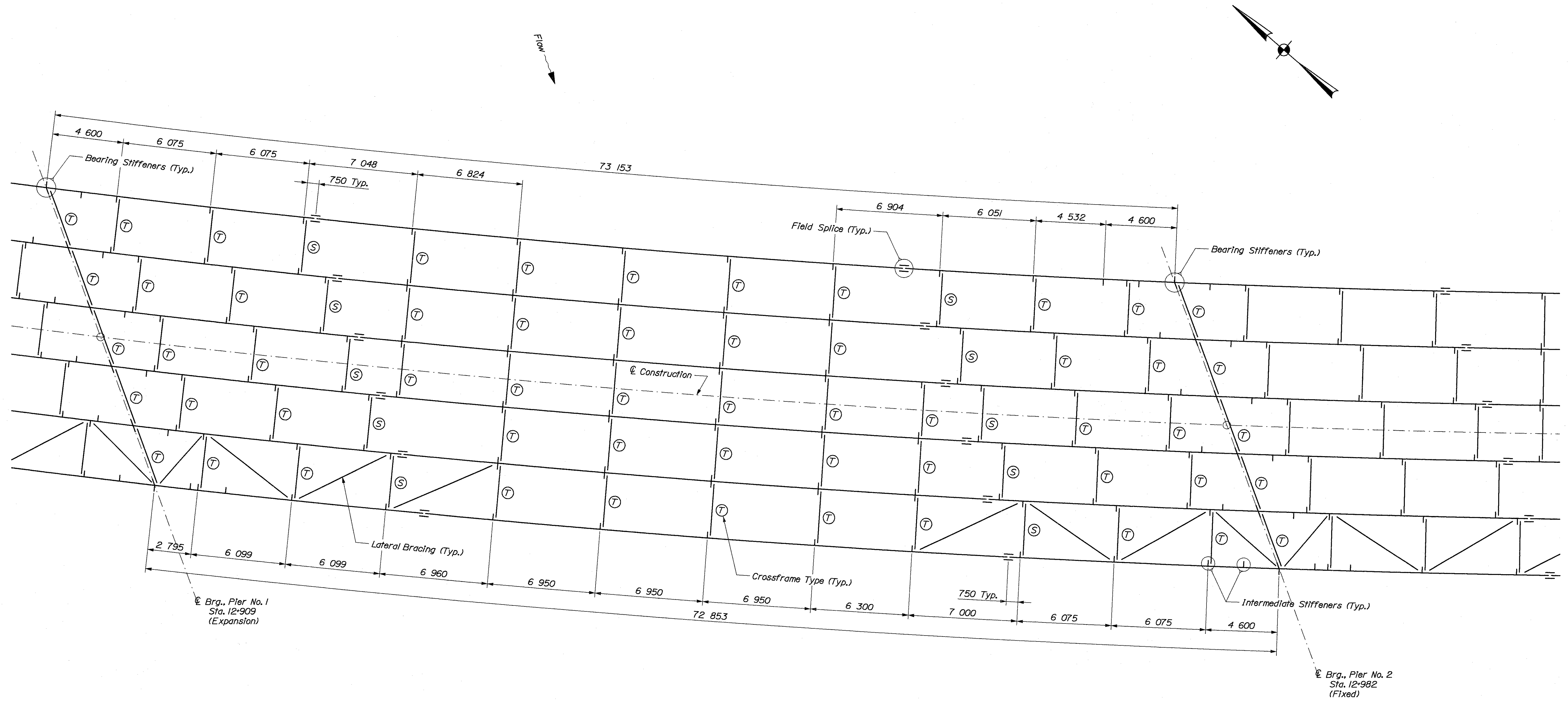
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 70           | 105             |

000556.22



FRAMING PLAN ~ SPAN NO. 2

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

FRAMING PLAN ~ SP. NO. 2

SHEET OF AUGUSTA, MAINE

File: ...\\071\_framing plan span no. 3.dgn

Date: 26 NOV 2002

User: Dora.Damren

Division: BRIDGE

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 |           |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

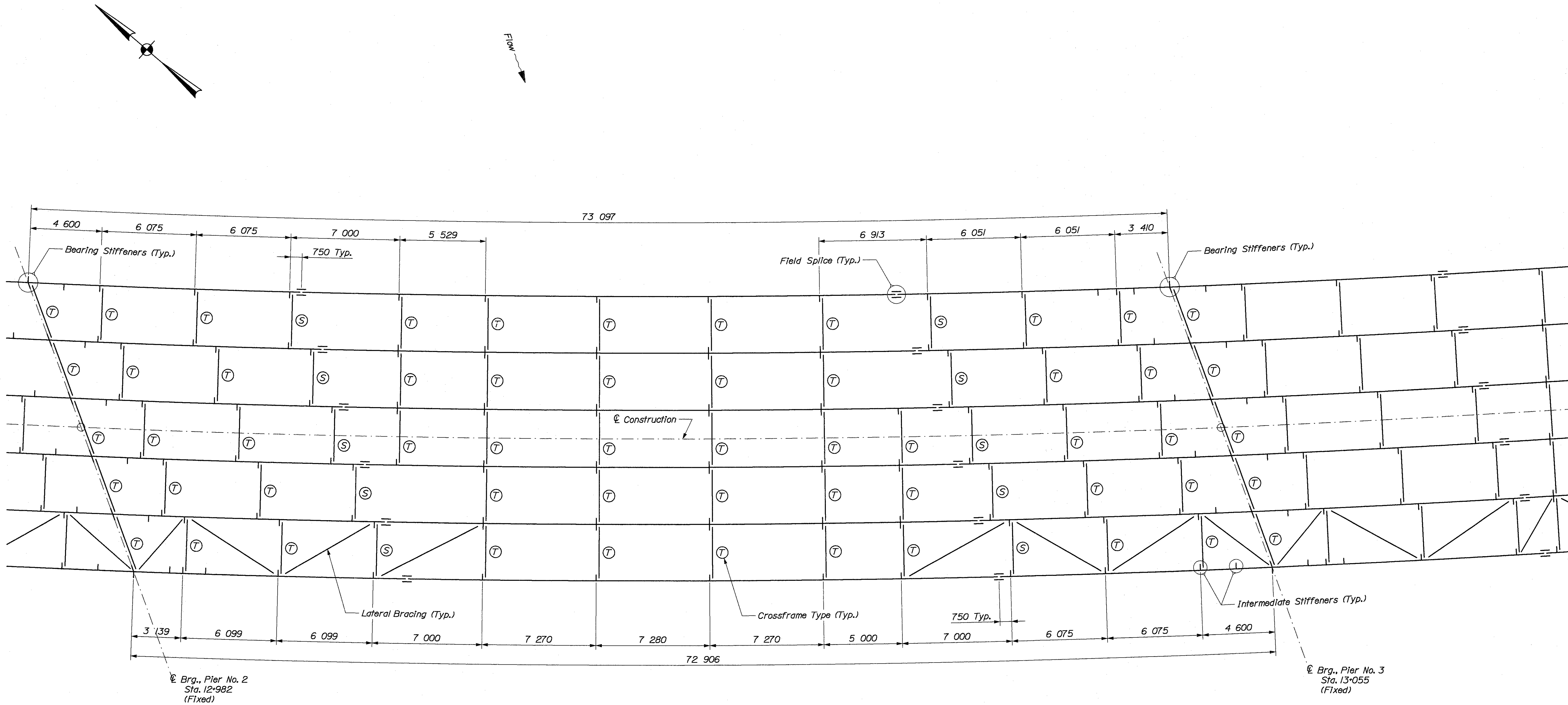
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

|                   |       |                |              |                 |
|-------------------|-------|----------------|--------------|-----------------|
| FILWA<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                 | MAINE | NH-0556(220)E  | 71           | 105             |

000556.22



FRAMING PLAN ~ SPAN NO. 3

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

FRAMING PLAN ~ SP. NO. 3

SHEET OF AUGUSTA, MAINE

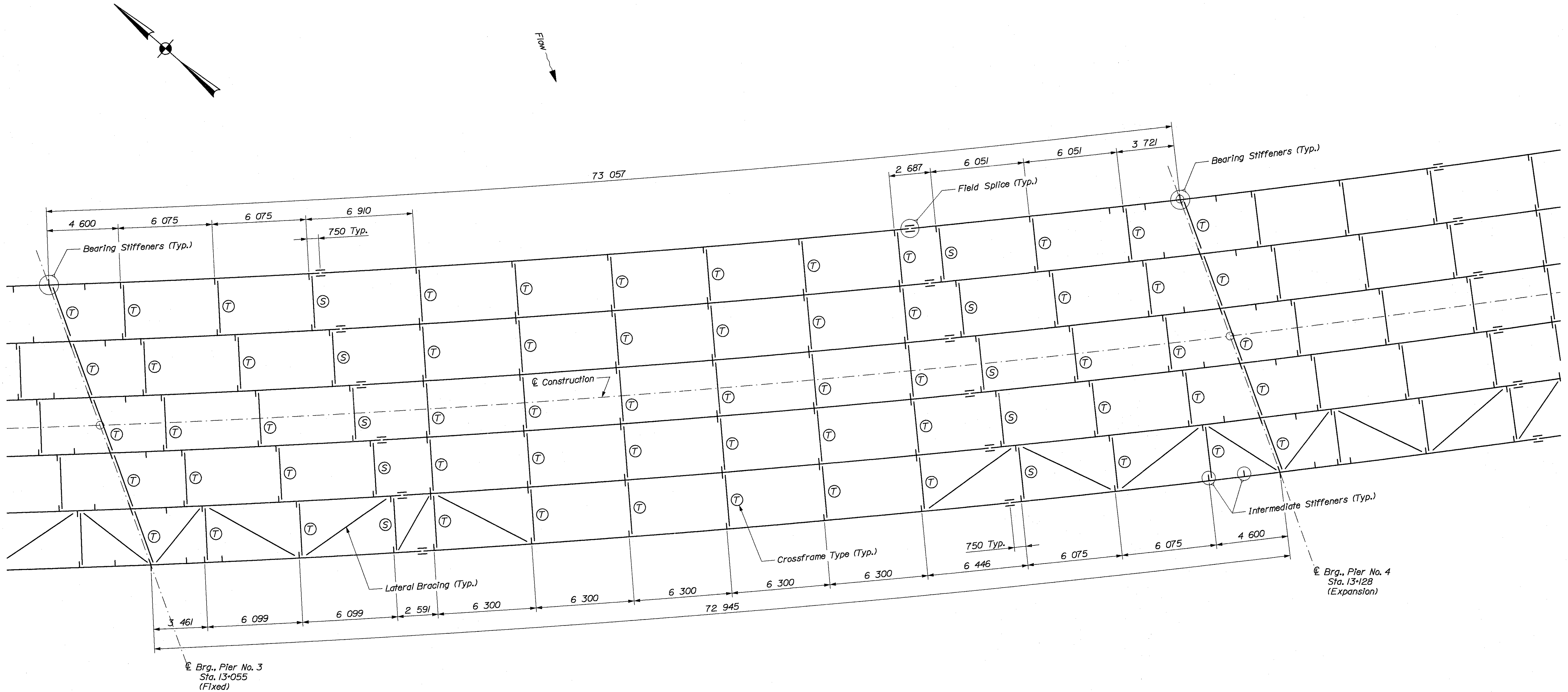
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| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN/DETAILED         | J. Folsom |      |
| CHECKED                 | D. Darren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS

|        |                                                              |  |                   |       |                |           |              |
|--------|--------------------------------------------------------------|--|-------------------|-------|----------------|-----------|--------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted. |  | F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|        | 2. All elevations and stations are in meters.                |  | 1                 | MAINE | NH-0556(220)E  | 72        | 105          |

000556.22



FRAMING PLAN ~ SPAN NO. 4

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

FRAMING PLAN ~ SP. NO. 4

SHEET OF  
AUGUSTA, MAINE

Filename: ...\\073\_framing plan span no. 5.dgn

Username: Dana.Damren

Division: BRIDGE

Date: 26 NOV 2002

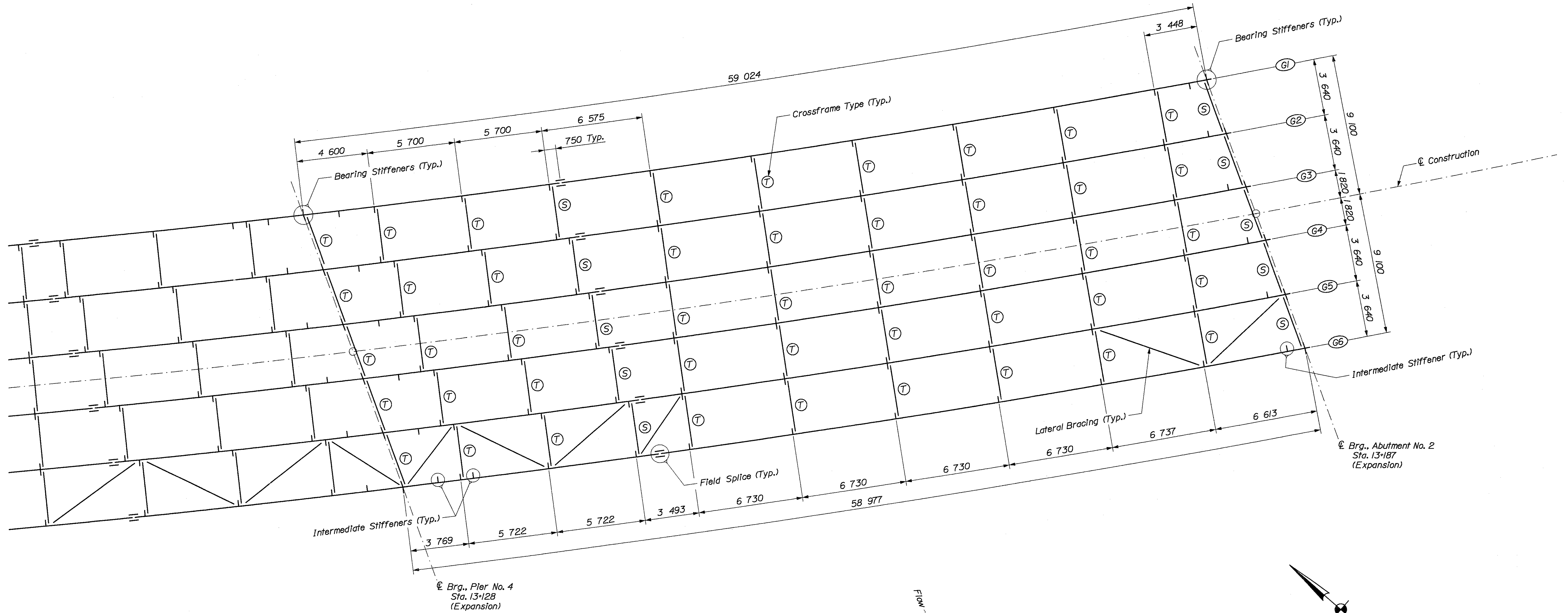
| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
|                         |           |      |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 |           |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS

METRIC 1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 73           | 105             |

000556.22



FRAMING PLAN ~ SPAN NO. 5

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
FRAMING PLAN ~ SP. NO. 5

SHEET OF AUGUSTA, MAINE

Date:26 NOV 2002

Username: Dono.Damren

Division: BRIDGE

Filename: ...\\074\_girder elevation.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

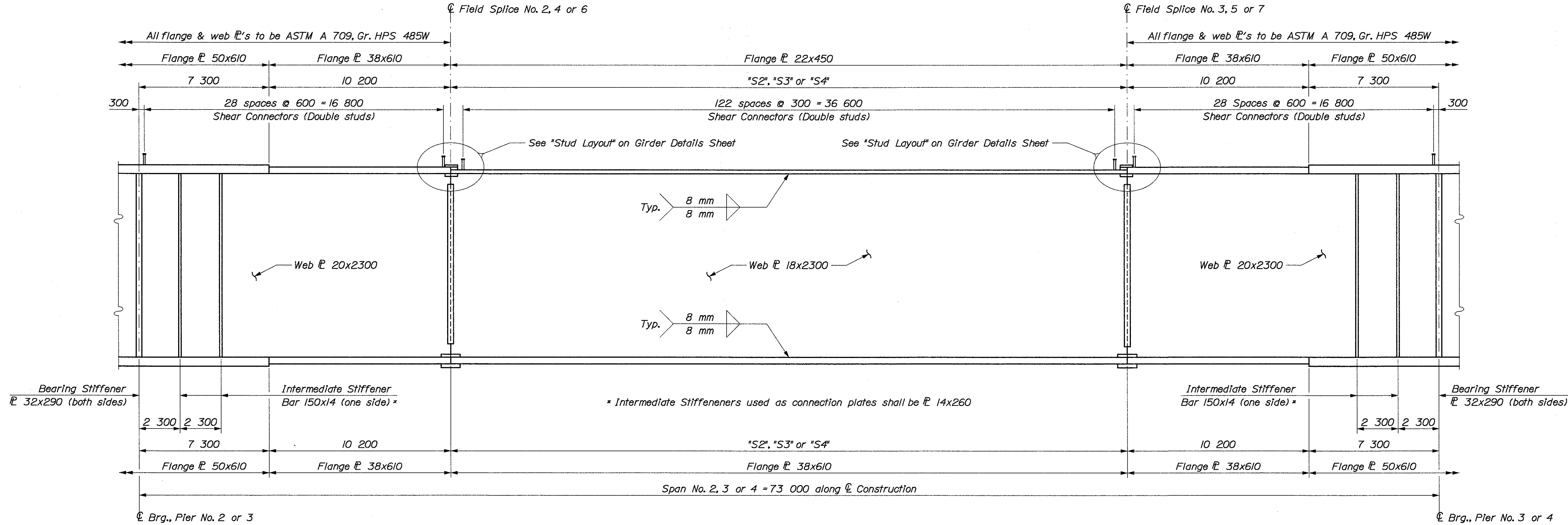
PLANS

METRIC

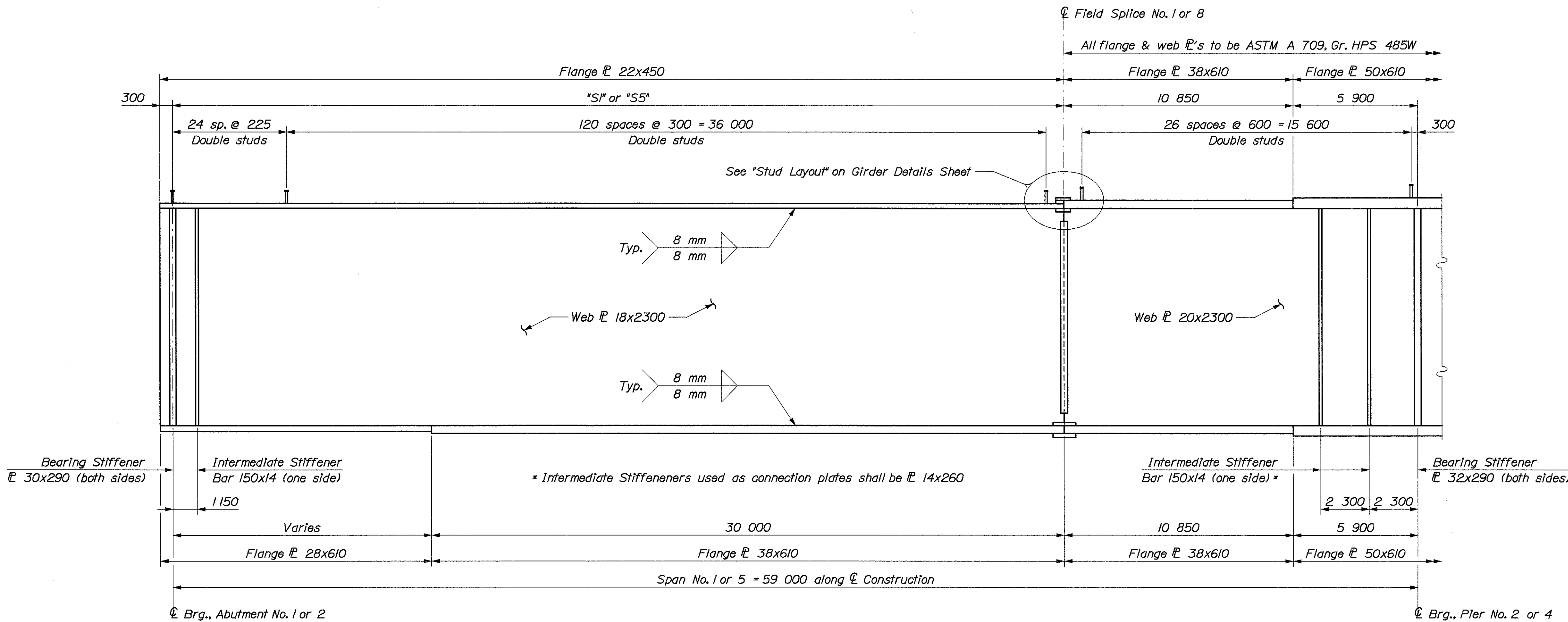
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 74           | 105             |

000556.22



GIRDER ELEVATION ~ SPAN NO. 2, 3 OR 4



GIRDER ELEVATION ~ SPAN NO. 1 OR 5

| TABLE OF GIRDER DIMENSIONS |            |        |        |        |        |        |
|----------------------------|------------|--------|--------|--------|--------|--------|
| Girder                     | Radius (m) | "S1"   | "S2"   | "S3"   | "S4"   | "S5"   |
| 1                          | 915.900    | 42 427 | 38 153 | 38 097 | 38 057 | 42 274 |
| 2                          | 919.540    | 42 355 | 38 091 | 38 058 | 38 034 | 42 264 |
| 3                          | 923.180    | 42 285 | 38 030 | 38 019 | 38 011 | 42 255 |
| 4                          | 926.820    | 42 215 | 37 970 | 37 981 | 37 989 | 42 245 |
| 5                          | 930.460    | 42 147 | 37 911 | 37 943 | 37 967 | 42 236 |
| 6                          | 934.100    | 42 080 | 37 853 | 37 906 | 37 945 | 42 227 |

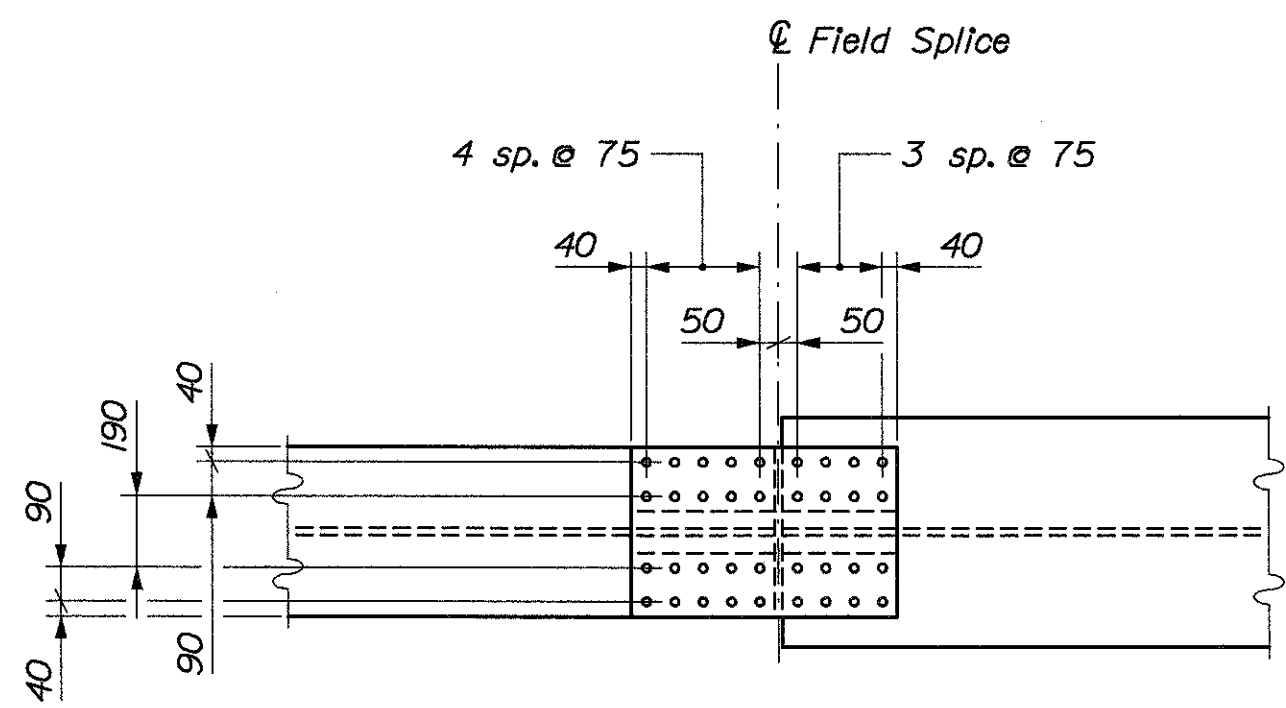
"S1" = Span No. 1, "S2" = Span No. 2, etc.

BRIDGE NO. 6400

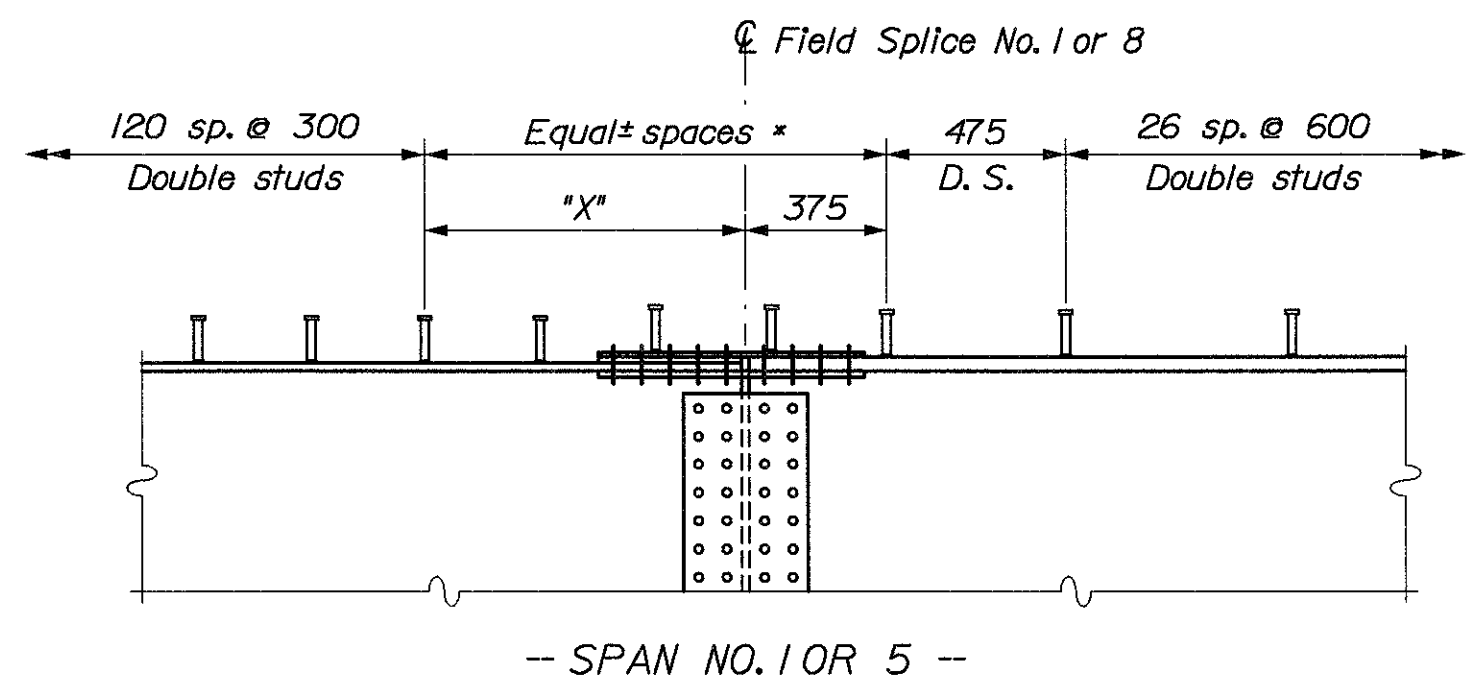
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
GIRDER ELEVATION

SHEET OF AUGUSTA, MAINE

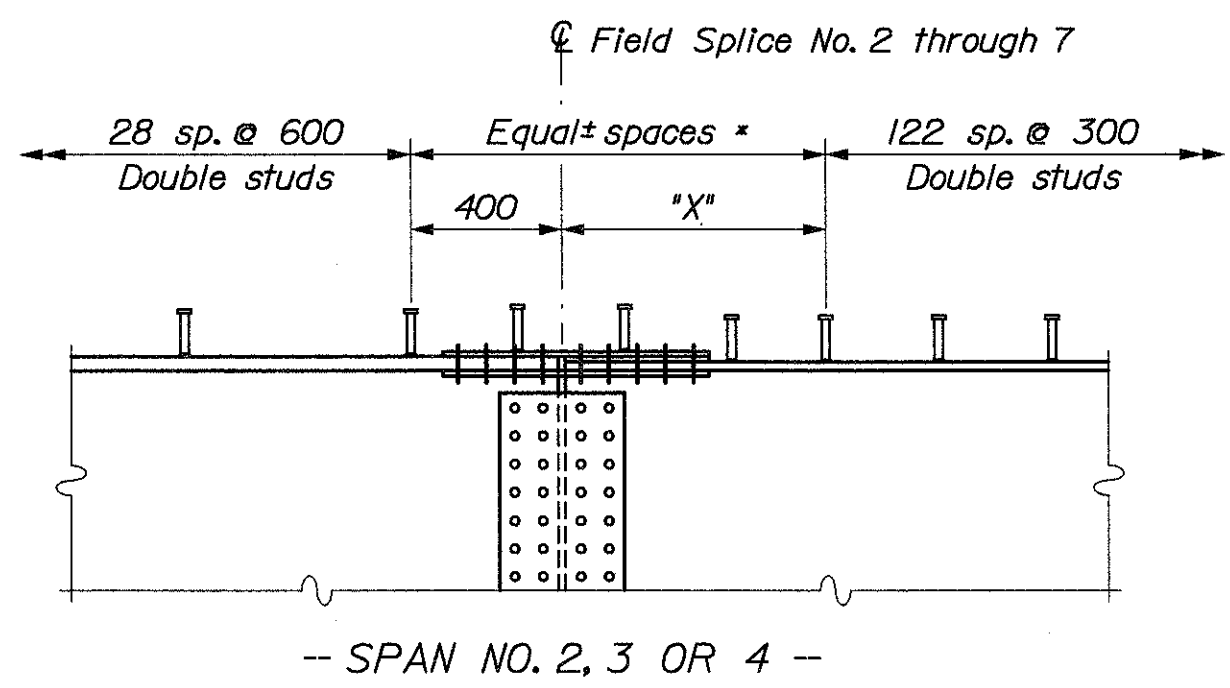


FIELD SPICE TOP VIEW

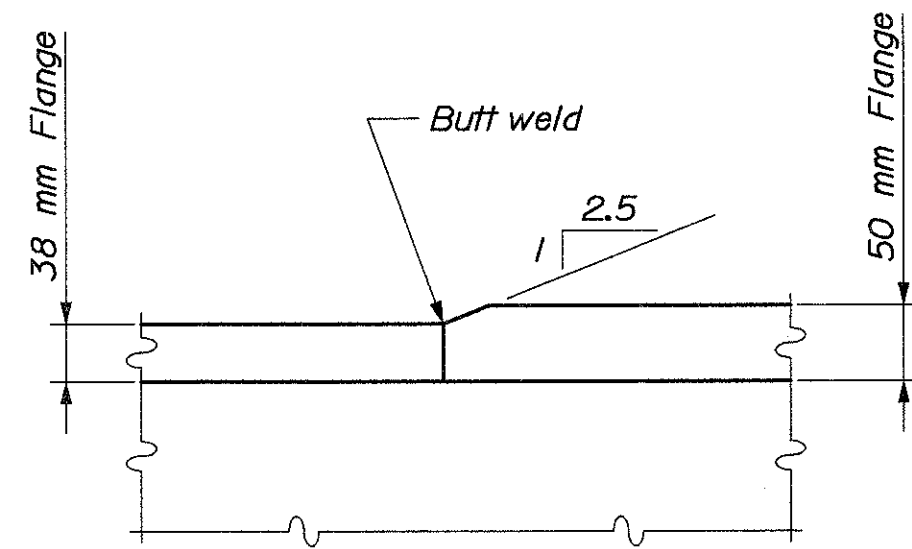


| STUD LAYOUT DIM. "X" |      |      |      |      |      |
|----------------------|------|------|------|------|------|
| Girder               | "S1" | "S2" | "S3" | "S4" | "S5" |
| 1                    | 1025 | 775  | 750  | 725  | 875  |
| 2                    | 955  | 745  | 730  | 715  | 865  |
| 3                    | 885  | 715  | 710  | 705  | 855  |
| 4                    | 815  | 685  | 690  | 695  | 845  |
| 5                    | 745  | 655  | 670  | 685  | 835  |
| 6                    | 680  | 625  | 650  | 675  | 825  |

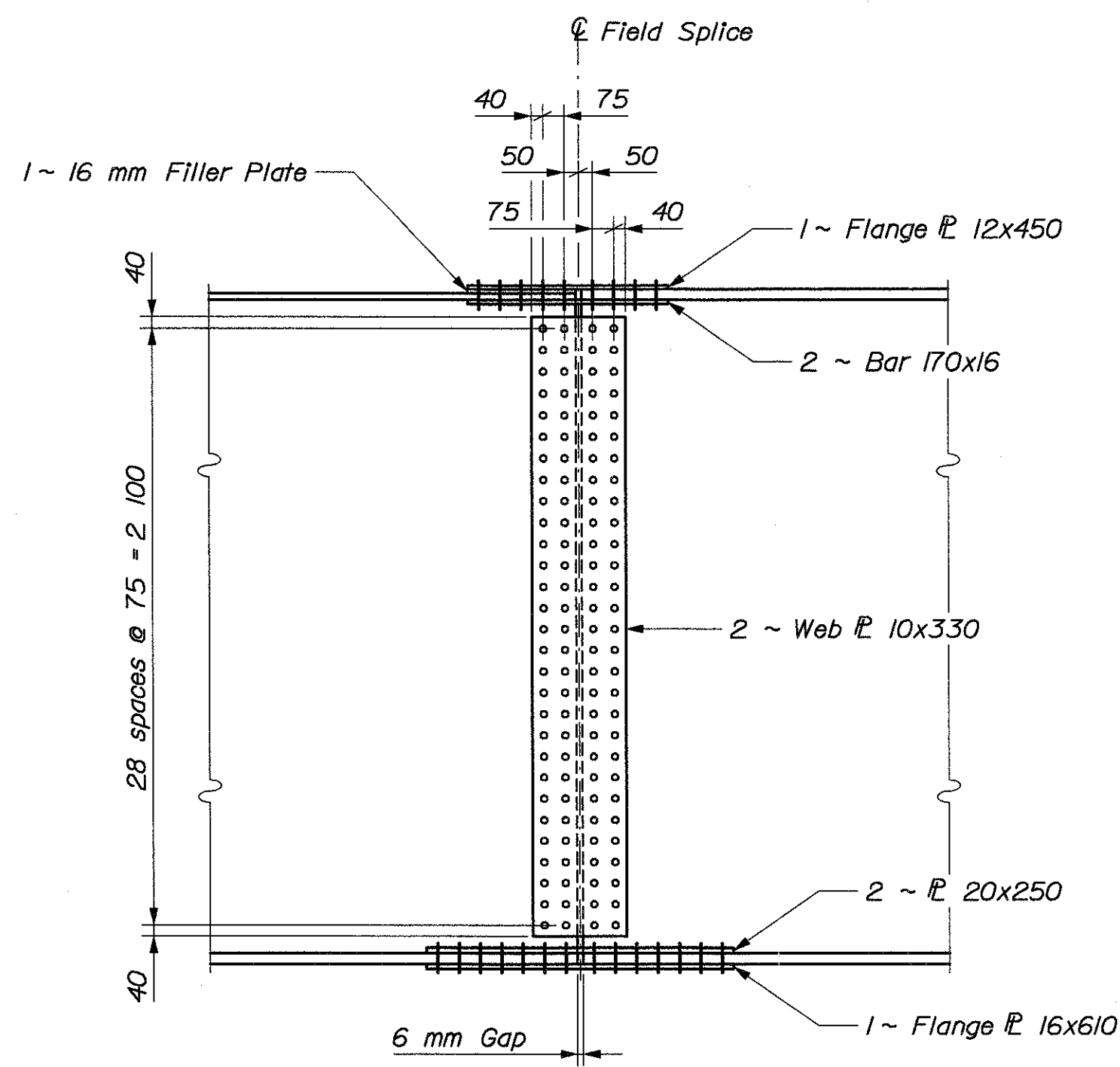
\*S1" = Span No. 1, \*S2" = Span No. 2 (both ends), etc.



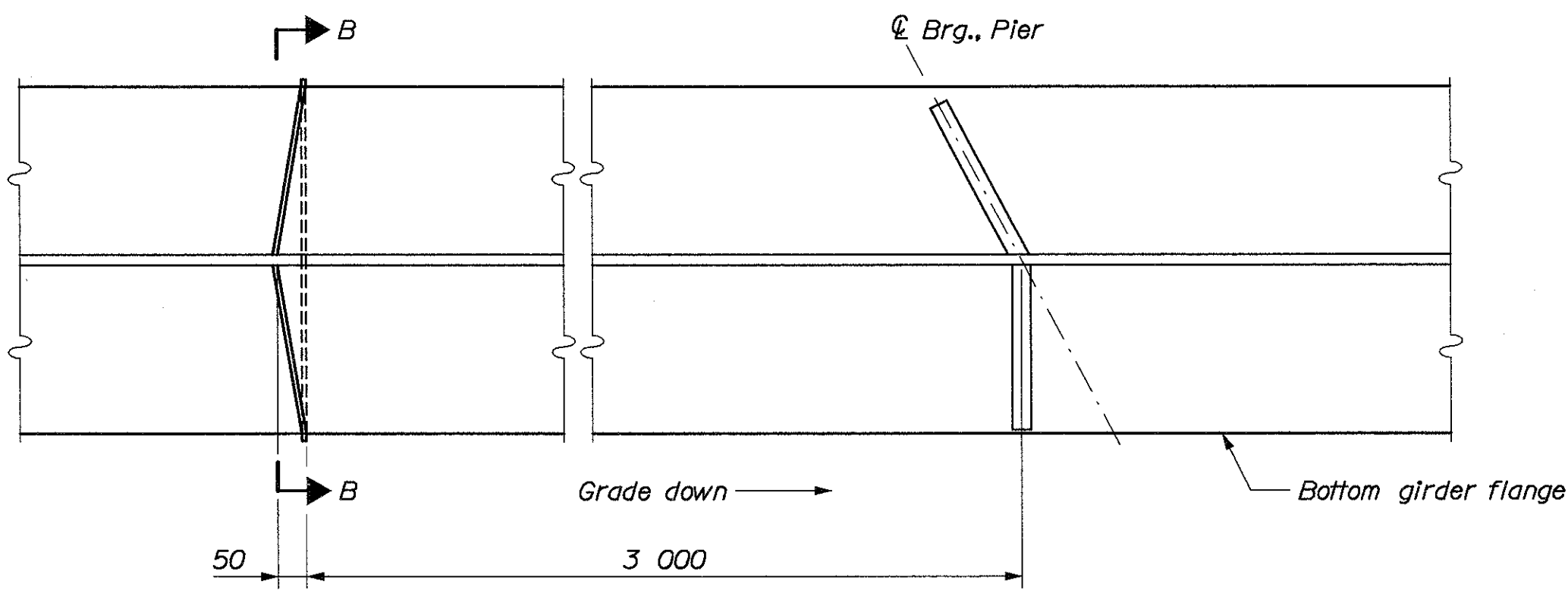
STUD LAYOUT  
\* Use single studs on splice plates



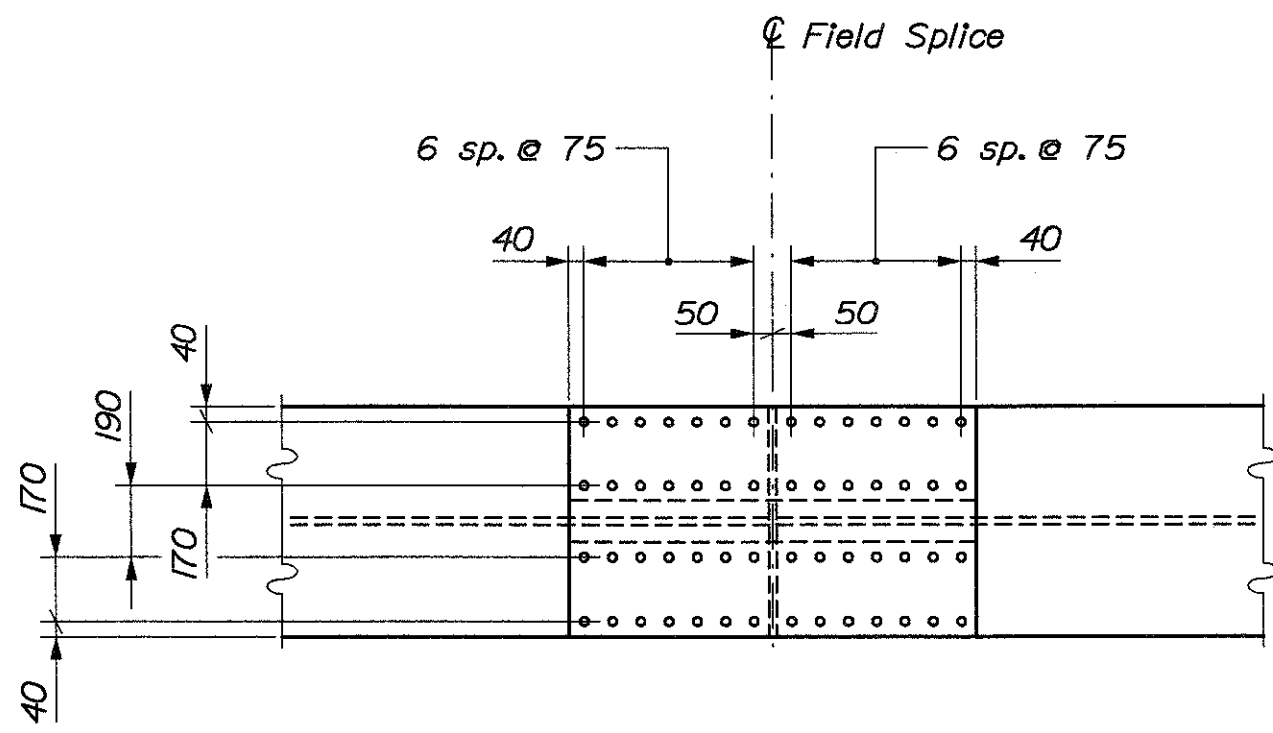
FLANGE THICKNESS TRANSITION  
Typical top & bottom flanges



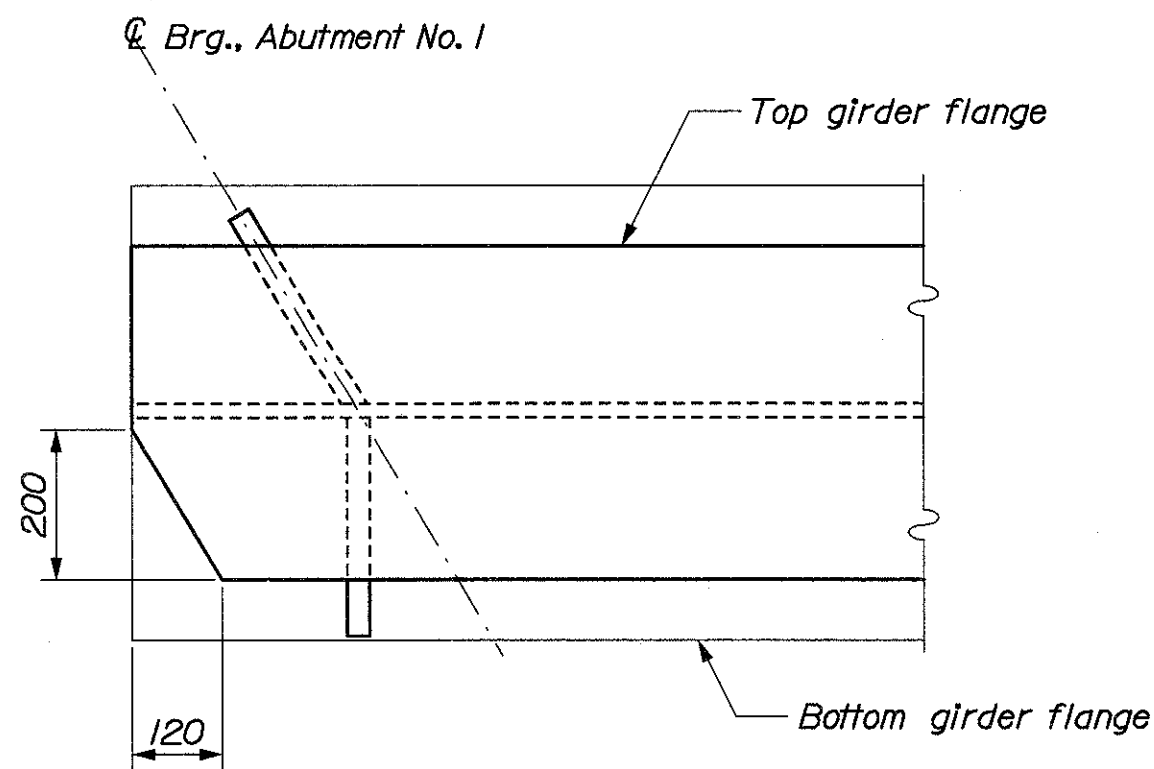
FIELD SPICE ELEVATION



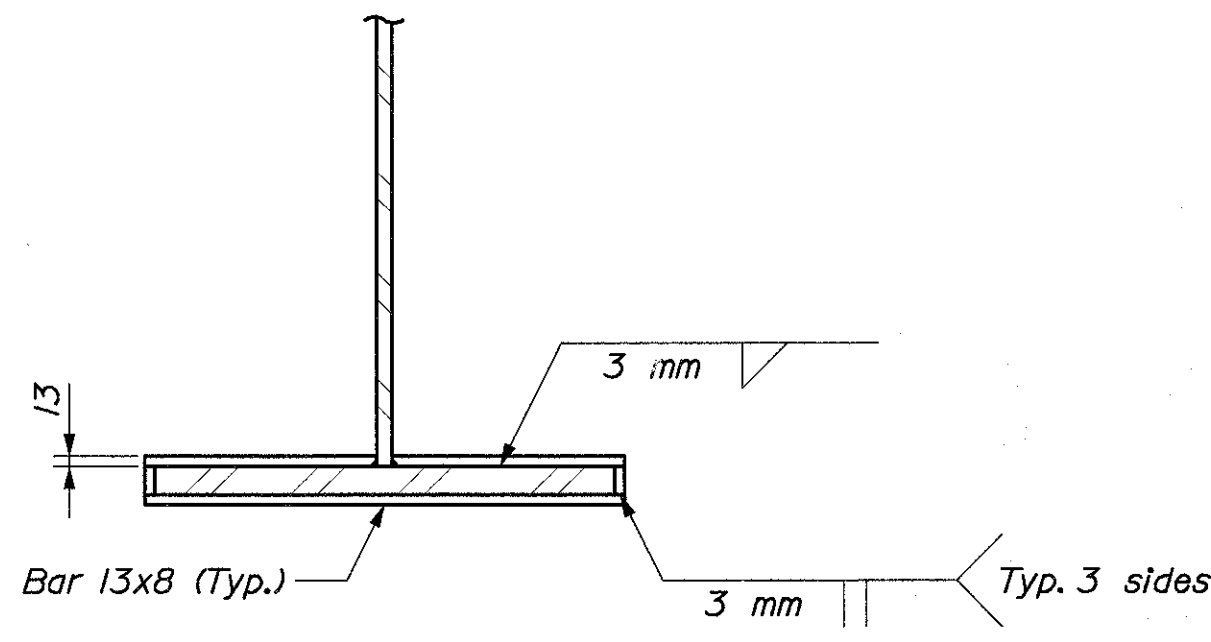
FLANGE DRIP PLATE PLAN



FIELD SPICE BOTTOM VIEW



TOP FLANGE CLIP  
Typical all girders at Abutment No. 1 end only



SECTION B-B

BRIDGE NO. 6400  
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
GIRDER DETAILS

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS

Date:26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ...\\076\_camber diagram.dgn

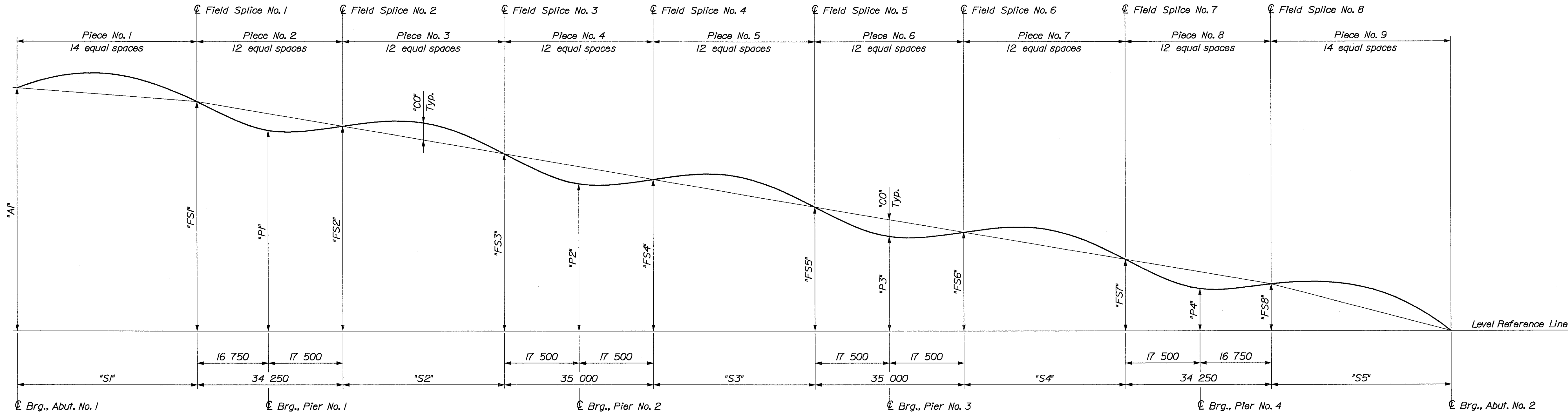
|                                                                                     |           |      |
|-------------------------------------------------------------------------------------|-----------|------|
| PROJECT DESIGN ENGINEER<br>DESIGN-DETAILED<br>CHECKED<br>REVISIONS<br>FIELD CHANGES | BY        | DATE |
|                                                                                     | J. Folsom |      |
|                                                                                     | D. Damren |      |
|                                                                                     |           |      |
|                                                                                     |           |      |
| PLANS                                                                               |           |      |

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                      |       |                |              |                 |
|----------------------|-------|----------------|--------------|-----------------|
| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                    | MAINE | NH-0556(220)E  | 76           | 105             |

000556.22



CAMBER DIAGRAM

| TABLE OF CAMBER DIMENSIONS |        |        |        |        |        |       |       |       |       |       |       |       |       |       |       |       |       |       |        |
|----------------------------|--------|--------|--------|--------|--------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|--------|
| Girder                     | *S1*   | *S2*   | *S3*   | *S4*   | *S5*   | *A*   | *FS1* | *P1*  | *FS2* | *FS3* | *P2*  | *FS4* | *FS5* | *P3*  | *FS6* | *FS7* | *P4*  | *FS8* | Girder |
| 1                          | 42 427 | 38 153 | 38 097 | 38 057 | 42 274 | 5 795 | 5 143 | 4 779 | 4 555 | 3 900 | 3 523 | 3 300 | 2 647 | 2 268 | 2 043 | 1 390 | 1 014 | 799   | 1      |
| 2                          | 42 355 | 38 091 | 38 058 | 38 034 | 42 264 | 5 768 | 5 123 | 4 757 | 4 536 | 3 886 | 3 507 | 3 288 | 2 638 | 2 258 | 2 037 | 1 386 | 1 009 | 801   | 2      |
| 3                          | 42 285 | 38 030 | 38 019 | 38 011 | 42 255 | 5 742 | 5 102 | 4 737 | 4 518 | 3 871 | 3 493 | 3 277 | 2 630 | 2 249 | 2 030 | 1 381 | 1 005 | 801   | 3      |
| 4                          | 42 215 | 37 970 | 37 981 | 37 989 | 42 245 | 5 716 | 5 079 | 4 715 | 4 497 | 3 852 | 3 477 | 3 262 | 2 617 | 2 239 | 2 020 | 1 374 | 1 001 | 798   | 4      |
| 5                          | 42 147 | 37 911 | 37 943 | 37 967 | 42 236 | 5 691 | 5 057 | 4 695 | 4 475 | 3 834 | 3 463 | 3 243 | 2 606 | 2 230 | 2 011 | 1 367 | 997   | 794   | 5      |
| 6                          | 42 080 | 37 853 | 37 906 | 37 945 | 42 227 | 5 665 | 5 036 | 4 675 | 4 449 | 3 816 | 3 448 | 3 230 | 2 593 | 2 221 | 1 998 | 1 357 | 993   | 790   | 6      |

| TABLE OF CAMBER ORDINATES "CO" |             |    |    |    |     |     |     |     |     |    |    |    |             |    |    |    |    |    |    |    |    |    |             |    |    |    |    |    |    |    |    |    |             |    |    |    |    |     |     |     |     |     |             |    |    |    |    |    |    |    |    |    |        |    |    |    |    |   |
|--------------------------------|-------------|----|----|----|-----|-----|-----|-----|-----|----|----|----|-------------|----|----|----|----|----|----|----|----|----|-------------|----|----|----|----|----|----|----|----|----|-------------|----|----|----|----|-----|-----|-----|-----|-----|-------------|----|----|----|----|----|----|----|----|----|--------|----|----|----|----|---|
| Girder                         | Piece No. 1 |    |    |    |     |     |     |     |     |    |    |    | Piece No. 2 |    |    |    |    |    |    |    |    |    | Piece No. 3 |    |    |    |    |    |    |    |    |    | Piece No. 4 |    |    |    |    |     |     |     |     |     | Piece No. 5 |    |    |    |    |    |    |    |    |    | Girder |    |    |    |    |   |
| 1                              | 26          | 48 | 69 | 85 | 99  | 106 | 109 | 106 | 98  | 87 | 68 | 50 | 23          | 20 | 37 | 52 | 65 | 74 | 76 | 73 | 65 | 53 | 35          | 18 | 20 | 38 | 54 | 66 | 74 | 77 | 74 | 66 | 55          | 39 | 21 | 18 | 36 | 53  | 64  | 74  | 77  | 74  | 65          | 54 | 39 | 19 | 21 | 39 | 55 | 66 | 74 | 76 | 74     | 66 | 55 | 38 | 20 | 1 |
| 2                              | 24          | 50 | 71 | 87 | 102 | 110 | 112 | 110 | 101 | 90 | 70 | 50 | 24          | 19 | 38 | 55 | 67 | 77 | 79 | 74 | 67 | 54 | 36          | 18 | 22 | 40 | 57 | 68 | 77 | 78 | 76 | 68 | 57          | 41 | 23 | 18 | 36 | 54  | 67  | 76  | 80  | 77  | 68          | 54 | 36 | 18 | 23 | 41 | 58 | 69 | 77 | 80 | 78     | 69 | 58 | 42 | 23 | 2 |
| 3                              | 25          | 51 | 72 | 88 | 104 | 113 | 114 | 112 | 104 | 90 | 72 | 51 | 26          | 20 | 39 | 56 | 68 | 78 | 80 | 76 | 68 | 55 | 36          | 18 | 22 | 40 | 57 | 68 | 78 | 79 | 77 | 69 | 58          | 41 | 22 | 18 | 37 | 56  | 69  | 78  | 81  | 78  | 68          | 57 | 37 | 19 | 23 | 42 | 59 | 70 | 79 | 81 | 78     | 70 | 59 | 42 | 23 | 3 |
| 4                              | 27          | 52 | 72 | 90 | 105 | 113 | 115 | 113 | 104 | 92 | 73 | 51 | 27          | 19 | 38 | 55 | 67 | 77 | 79 | 75 | 66 | 54 | 37          | 19 | 21 | 39 | 56 | 68 | 75 | 80 | 77 | 67 | 57          | 40 | 21 | 19 | 37 | 54  | 67  | 77  | 80  | 76  | 67          | 54 | 35 | 19 | 22 | 41 | 58 | 69 | 79 | 82 | 80     | 71 | 59 | 42 | 22 | 4 |
| 5                              | 26          | 51 | 73 | 91 | 104 | 113 | 115 | 111 | 103 | 90 | 73 | 51 | 27          | 19 | 37 | 52 | 65 | 73 | 77 | 72 | 65 | 52 | 36          | 18 | 21 | 37 | 54 | 66 | 73 | 77 | 73 | 66 | 55          | 39 | 21 | 18 | 35 | 51  | 64  | 72  | 76  | 72  | 63          | 49 | 31 | 15 | 26 | 43 | 60 | 72 | 79 | 81 | 79     | 69 | 58 | 42 | 21 | 5 |
| 6                              | 25          | 51 | 72 | 89 | 105 | 112 | 114 | 110 | 102 | 90 | 71 | 50 | 26          | 18 | 34 | 51 | 64 | 72 | 74 | 71 | 63 | 52 | 34          | 18 | 21 | 37 | 54 | 65 | 72 | 75 | 73 | 64 | 54          | 38 | 20 | 18 | 35 | 51  | 62  | 71  | 75  | 72  | 63          | 51 | 34 | 18 | 22 | 40 | 57 | 66 | 75 | 78 | 75     | 67 | 56 | 39 | 21 | 6 |
| Girder                         | Piece No. 6 |    |    |    |     |     |     |     |     |    |    |    | Piece No. 7 |    |    |    |    |    |    |    |    |    | Piece No. 8 |    |    |    |    |    |    |    |    |    | Piece No. 9 |    |    |    |    |     |     |     |     |     | Girder      |    |    |    |    |    |    |    |    |    |        |    |    |    |    |   |
| 1                              | 19          | 35 | 52 | 65 | 73  | 77  | 74  | 65  | 53  | 36 | 18 | 21 | 39          | 55 | 66 | 74 | 76 | 73 | 64 | 54 | 38 | 21 | 17          | 35 | 52 | 62 | 72 | 75 | 72 | 64 | 52 | 35 | 19          | 23 | 47 | 69 | 86 | 97  | 105 | 110 | 106 | 99  | 85          | 69 | 49 | 26 |    |    |    |    |    |    | 1      |    |    |    |    |   |
| 2                              | 18          | 36 | 53 | 67 | 76  | 80  | 76  | 68  | 55  | 37 | 18 | 21 | 40          | 56 | 67 | 76 | 77 | 76 | 67 | 56 | 40 | 22 | 18          | 36 | 54 | 67 | 75 | 78 | 76 | 67 | 55 | 38 | 18          | 24 | 49 | 71 | 89 | 102 | 110 | 114 | 112 | 104 | 91          | 72 | 51 | 26 |    |    |    |    |    |    | 2      |    |    |    |    |   |
| 3                              | 20          | 36 | 55 | 69 | 77  | 81  | 79  | 71  | 55  | 38 | 18 | 22 | 42          | 58 | 69 | 77 | 79 | 77 | 69 | 58 | 41 | 22 | 18          | 38 | 55 | 68 | 76 | 80 | 78 | 68 | 56 | 39 | 19          | 26 | 51 | 72 | 91 | 104 | 113 | 118 | 114 | 107 | 92          | 75 | 52 | 28 |    |    |    |    |    |    | 3      |    |    |    |    |   |
| 4                              | 19          | 36 | 53 | 66 | 76  | 80  | 75  | 67  | 54  | 37 | 18 | 21 | 40          | 57 | 68 | 77 | 79 | 76 | 68 | 58 | 41 | 22 | 19          | 37 | 54 | 67 | 75 | 79 | 75 | 67 | 54 | 36 | 19          | 28 | 52 | 73 | 92 | 106 | 114 | 118 | 115 | 105 | 92          | 75 | 52 | 27 |    |    |    |    |    |    | 4      |    |    |    |    |   |
| 5                              | 19          | 35 | 52 | 65 | 74  | 78  | 74  | 66  | 53  | 36 | 18 | 20 | 38          | 55 | 66 | 74 | 76 | 74 | 66 | 56 | 39 | 21 | 19          | 36 | 53 | 65 | 74 | 77 | 76 | 66 | 54 | 36 | 19          | 27 | 51 | 72 | 91 | 104 | 113 | 116 | 115 | 106 | 91          | 74 | 52 | 25 |    |    |    |    |    |    | 5      |    |    |    |    |   |
| 6                              | 17          | 34 | 51 | 62 | 72  | 75  | 73  | 64  | 52  | 35 | 19 | 20 | 38          | 54 | 64 | 73 | 75 | 73 | 64 | 54 | 38 | 22 | 18          | 34 | 50 | 63 | 72 | 75 | 73 | 64 | 53 | 37 | 19          | 26 | 49 | 71 | 89 | 103 | 111 | 114 | 112 | 104 | 90          | 73 | 52 | 27 |    |    |    |    |    |    | 6      |    |    |    |    |   |

| TABLE OF DEFLECTIONS |                        |                            |                           |                             |                           |                             |
|----------------------|------------------------|----------------------------|---------------------------|-----------------------------|---------------------------|-----------------------------|
| Girder               | Load                   | Span No. 1 ~ 10th Points   | Span No. 2 ~ 10th Points  | Span No. 3 ~ 10th Points    | Span No. 4 ~ 10th Points  | Span No. 5 ~ 10th Points    |
| 1                    | Steel Dead Load        | 11 20 26 28 27 22 15 8 2   | 3 12 21 28 30 28 21 12 4  | 3 11 20 27 30 27 20 11 3    | 3 11 20 27 30 28 21 12 4  | 2 8 15 22 26 28 26 20 11    |
|                      | Fluid Dead Load        | 36 66 85 92 88 73 50 27 8  | 8 31 58 79 88 80 60 33 10 | 10 34 61 83 91 83 62 35 11  | 9 32 58 79 86 79 58 32 9  | 8 26 48 70 86 91 84 65 36   |
|                      | Superimposed Dead Load | 11 20 26 28 27 23 16 9 2   | 5 15 25 32 35 32 25 14 5  | 4 13 23 31 33 31 24 14 4    | 4 14 24 31 34 32 25 15 5  | 2 8 16 23 27 28 26 20 11    |
| 2                    | Steel Dead Load        | 11 20 26 29 27 22 15 8 2   | 3 12 21 28 30 28 21 12 3  | 3 11 21 27 30 27 21 11 3    | 3 12 21 28 30 28 21 12 3  | 2 8 15 22 27 29 27 21 11    |
|                      | Fluid Dead Load        | 38 69 90 98 93 77 54 29 9  | 9 33 62 84 93 85 64 35 10 | 11 37 66 88 97 89 67 38 12  | 9 34 62 84 91 83 62 33 9  | 9 28 53 76 93 98 91 71 39   |
|                      | Superimposed Dead Load | 11 20 26 28 27 23 16 9 2   | 5 15 25 32 34 32 24 14 4  | 4 14 23 30 33 31 23 14 4    | 4 14 24 31 34 32 25 15 5  | 2 9 16 23 27 28 26 20 11    |
| 3                    | Steel Dead Load        | 11 20 26 29 27 22 15 8 2   | 3 12 21 28 30 28 21 12 3  | 3 11 20 27 30 28 21 11 3    | 3 12 21 28 30 28 21 12 3  | 2 8 16 22 27 29 27 21 12    |
|                      | Fluid Dead Load        | 39 72 93 101 95 79 55 30 9 | 9 34 62 86 95 87 65 36 10 | 11 38 68 92 100 92 69 39 12 | 10 35 64 86 93 85 63 34 9 | 9 30 56 80 97 102 95 73 41  |
|                      | Superimposed Dead Load | 11 19 25 28 27 23 16 9 2   | 5 15 25 32 34 31 24 14 4  | 4 14 23 30 33 30 23 13 4    | 4 14 24 31 34 31 24 14 5  | 2 9 16 23 27 28 26 20 11    |
| 4                    | Steel Dead Load        | 11 21 27 29 27 22 15 8 2   | 3 11 20 27 30 27 20 11 4  | 4 11 20 27 30 27 20 11 4    | 4 12 21 28 30 28 21 11 4  | 2 8 16 23 28 29 27 21 12    |
|                      | Fluid Dead Load        | 40 72 93 101 95 78 54 29 9 | 9 34 62 84 93 85 63 34 10 | 12 39 69 91 100 91 68 38 12 | 10 34 63 84 92 83 61 33 9 | 10 30 56 80 97 103 95 74 40 |
|                      | Superimposed Dead Load | 11 19 25 28 27 23 16 9 2   | 5 14 24 31 34 31 24 14 4  | 4 13 23 30 33 30 23 13 4    | 4 14 24 31 34 31 24 14 5  | 2 9 16 23 27 28 26 20 11    |
| 5                    | Steel Dead Load        | 11 21 27 29 28 23 16 8 2   | 3 11 20 27 30 27 20 11 3  | 4 11 20 27 30 28 21 11 4    | 4 11 20 27 30 27 20 11 3  | 3 8 16 23 28 30 28 21 12    |
|                      | Fluid Dead Load        | 39 71 91 98 93 76 53 28 9  | 9 32 58 80 88 80 59 32 9  | 12 37 66 88 96 88 65 37 12  | 9 33 59 80 87 79 57 31 8  | 10 30 55 78 94 100 92 72 39 |
|                      | Superimposed Dead Load | 11 20 26 29 28 23 17 9 3   | 5 14 24 31 34 31 24 14 4  | 4 13 23 30 33 30 23 13 4    | 4 14 24 31 34 31 24 14 5  | 3 9 17 23 28 29 26 20 11    |
| 6                    | Steel Dead Load        | 12 22 28 30 29 23 16 9 3   | 3 11 20 27 30 27 21 11 4  | 3 11 21 28 30 28 21 12 4    | 3 11 20 27 30 27 20 11 3  | 2 8 16 23 28 30 28 22 12    |
|                      | Fluid Dead Load        | 37 68 88 95 90 74 52 28 9  | 7 27 53 73 81 74 55 30 9  | 10 34 61 83 91 83 62 35 11  | 8 29 54 74 81 73 53 28 7  | 9 28 52 74 90 96 89 69 38   |
|                      | Superimposed Dead Load | 12 21 28 30 29 24 17 9 3   | 4 14 24 32 35 32 24 14 5  | 4 13 24 31 34 31 24 14 4    | 4 14 24 32 35 32 25 14 5  | 3 9 17 24 29 30 27 21 12    |

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
CAMBER DIAGRAM

SHEET OF AUGUSTA, MAINE

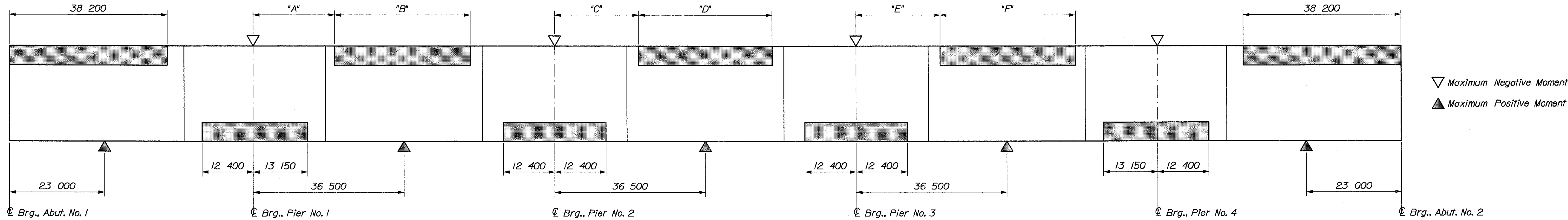
|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | D. Darren |      |
| CHECKED                 | J. Foscm  |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |
| PLANS                   |           |      |

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                      |       |                |              |                 |
|----------------------|-------|----------------|--------------|-----------------|
| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                    | MAINE | NH-0556(220)E  | 77           | 105             |

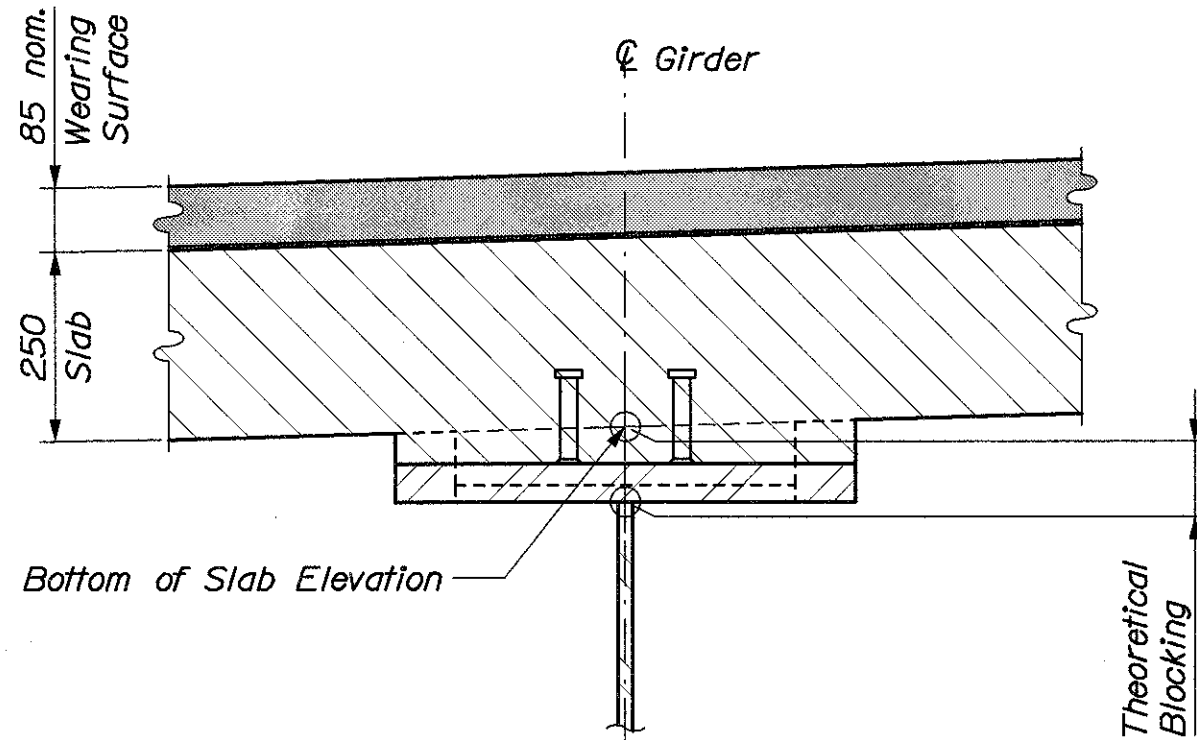
000556.22



GIRDER STRESS DIAGRAM  
Shaded areas are always in compression.  
Other areas are in tension or have stress reversal.

| Girder | "A"    | "B"    | "C"    | "D"    | "E"    | "F"    |
|--------|--------|--------|--------|--------|--------|--------|
| 1      | 19 800 | 32 900 | 20 500 | 32 100 | 20 500 | 32 800 |
| 2      | 19 000 | 34 300 | 19 800 | 33 600 | 19 800 | 34 300 |
| 3      | 19 000 | 34 300 | 19 800 | 33 500 | 19 800 | 34 300 |
| 4      | 19 700 | 32 800 | 19 800 | 33 500 | 19 800 | 33 500 |
| 5      | 19 700 | 32 800 | 20 500 | 32 100 | 20 500 | 32 100 |
| 6      | 21 200 | 29 800 | 21 900 | 29 100 | 21 900 | 29 800 |

| BOTTOM OF SLAB ELEVATIONS |                        |                                                           |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        |        |        |   |
|---------------------------|------------------------|-----------------------------------------------------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|------------------------|--------|--------|---|
| Span No. 1                |                        |                                                           |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        |        |        |   |
| Girder                    | ℄ Brg.,<br>Abut. No. 1 | Blocking points at 3 m intervals from ℄ Brg., Abut. No. 1 |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        | Girder                 |        |        |   |
| 1                         | 40.102                 | 40.076                                                    | 40.046 | 40.017 | 39.983 | 39.944 | 39.904 | 39.859 | 39.811 | 39.758 | 39.701 | 39.641 | 39.578 | 39.512 | 39.445 | 39.378 | 39.311 | 39.247 | 39.184 | 39.126 |        |        |                        | 1      |        |   |
| 2                         | 40.167                 | 40.142                                                    | 40.113 | 40.085 | 40.052 | 40.016 | 39.975 | 39.931 | 39.883 | 39.829 | 39.773 | 39.712 | 39.648 | 39.582 | 39.516 | 39.448 | 39.381 | 39.314 | 39.253 | 39.194 |        |        |                        | 2      |        |   |
| 3                         | 40.232                 | 40.206                                                    | 40.181 | 40.152 | 40.119 | 40.085 | 40.044 | 40.001 | 39.952 | 39.899 | 39.841 | 39.781 | 39.718 | 39.652 | 39.585 | 39.516 | 39.449 | 39.383 | 39.321 | 39.263 |        |        |                        | 3      |        |   |
| 4                         | 40.298                 | 40.274                                                    | 40.248 | 40.218 | 40.186 | 40.151 | 40.112 | 40.068 | 40.020 | 39.967 | 39.910 | 39.849 | 39.785 | 39.719 | 39.651 | 39.583 | 39.517 | 39.452 | 39.391 | 39.333 |        |        |                        | 4      |        |   |
| 5                         | 40.364                 | 40.339                                                    | 40.314 | 40.285 | 40.253 | 40.216 | 40.178 | 40.134 | 40.085 | 40.031 | 39.976 | 39.915 | 39.852 | 39.786 | 39.719 | 39.651 | 39.586 | 39.521 | 39.460 | 39.402 |        |        |                        | 5      |        |   |
| 6                         | 40.301                 | 40.275                                                    | 40.250 | 40.221 | 40.189 | 40.154 | 40.115 | 40.071 | 40.022 | 39.969 | 39.914 | 39.852 | 39.791 | 39.724 | 39.659 | 39.591 | 39.526 | 39.461 | 39.401 | 39.343 |        |        |                        | 6      |        |   |
| Span No. 2                |                        |                                                           |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        |        |        |   |
| Girder                    | ℄ Brg.,<br>Pier No. 1  | Blocking points at 3 m intervals from ℄ Brg., Pier No. 1  |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        | Girder                 |        |        |   |
| 1                         | 39.086                 | 39.036                                                    | 38.992 | 38.951 | 38.914 | 38.878 | 38.841 | 38.805 | 38.766 | 38.727 | 38.685 | 38.640 | 38.591 | 38.539 | 38.482 | 38.423 | 38.360 | 38.297 | 38.230 | 38.162 | 38.096 | 38.031 | 37.968                 | 37.908 | 37.851 | 1 |
| 2                         | 39.156                 | 39.106                                                    | 39.063 | 39.022 | 38.985 | 38.951 | 39.915 | 39.879 | 38.842 | 38.804 | 38.760 | 38.716 | 38.667 | 38.615 | 38.560 | 38.501 | 38.438 | 38.373 | 38.308 | 38.239 | 38.172 | 38.105 | 38.042                 | 37.982 | 37.925 | 2 |
| 3                         | 39.227                 | 39.177                                                    | 39.133 | 39.093 | 39.058 | 39.023 | 38.988 | 38.953 | 38.915 | 38.877 | 38.835 | 38.790 | 38.741 | 38.690 | 38.633 | 38.574 | 38.512 | 38.447 | 38.381 | 38.313 | 38.248 | 38.180 | 38.116                 | 38.057 | 38.000 | 3 |
| 4                         | 39.297                 | 39.247                                                    | 39.205 | 39.165 | 39.128 | 39.093 | 39.059 | 39.023 | 38.985 | 38.947 | 38.906 | 38.861 | 38.814 | 38.761 | 38.705 | 38.646 | 38.583 | 38.519 | 38.453 | 38.385 | 38.319 | 38.253 | 38.191                 | 38.131 | 38.076 | 4 |
| 5                         | 39.368                 | 39.319                                                    | 39.276 | 39.235 | 39.200 | 39.163 | 39.128 | 39.092 | 39.054 | 39.016 | 38.974 | 38.930 | 38.881 | 38.829 | 38.774 | 38.715 | 38.652 | 38.589 | 38.525 | 38.457 | 38.391 | 38.328 | 38.265                 | 38.206 | 38.153 | 5 |
| 6                         | 39.310                 | 39.260                                                    | 39.216 | 39.176 | 39.137 | 39.102 | 39.066 | 39.030 | 38.992 | 38.954 | 38.912 | 38.868 | 38.819 | 38.768 | 38.713 | 38.655 | 38.593 | 38.531 | 38.468 | 38.401 | 38.336 | 38.273 | 38.211                 | 38.153 | 38.099 | 6 |
| Span No. 3                |                        |                                                           |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        |        |        |   |
| Girder                    | ℄ Brg.,<br>Pier No. 2  | Blocking points at 3 m intervals from ℄ Brg., Pier No. 2  |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        | Girder |        |   |
| 1                         | 37.830                 | 37.780                                                    | 37.737 | 37.696 | 37.659 | 37.623 | 37.586 | 37.550 | 37.512 | 37.474 | 37.430 | 37.386 | 37.336 | 37.284 | 37.228 | 37.169 | 37.106 | 37.042 | 36.975 | 36.908 | 36.842 | 36.775 | 36.711                 | 36.653 | 36.595 | 1 |
| 2                         | 37.906                 | 37.856                                                    | 37.814 | 37.774 | 37.738 | 37.704 | 37.668 | 37.632 | 37.595 | 37.557 | 37.514 | 37.469 | 37.415 | 37.368 | 37.313 | 37.253 | 37.191 | 37.125 | 37.059 | 36.991 | 36.924 | 36.856 | 36.793                 | 36.733 | 36.676 | 2 |
| 3                         | 37.983                 | 37.933                                                    | 37.892 | 37.850 | 37.816 | 37.781 | 37.747 | 37.712 | 37.676 | 37.638 | 37.596 | 37.552 | 37.503 | 37.451 | 37.393 | 37.335 | 37.271 | 37.206 | 37.141 | 37.072 | 37.005 | 36.937 | 36.874                 | 36.814 | 36.757 | 3 |
| 4                         | 38.059                 | 38.009                                                    | 37.970 | 37.930 | 37.895 | 37.859 | 37.825 | 37.790 | 37.753 | 37.715 | 37.673 | 37.629 | 37.582 | 37.529 | 37.472 | 37.414 | 37.350 | 37.286 | 37.220 | 37.151 | 37.084 | 37.017 | 36.955                 | 36.895 | 36.839 | 4 |
| 5                         | 38.136                 | 38.086                                                    | 38.046 | 38.007 | 37.971 | 37.936 | 37.901 | 37.866 | 37.833 | 38.791 | 37.749 | 37.704 | 37.657 | 37.605 | 37.548 | 37.490 | 37.427 | 37.362 | 37.299 | 37.230 | 37.164 | 37.097 | 37.036                 | 36.977 | 36.920 | 5 |
| 6                         | 38.084                 | 38.034                                                    | 37.993 | 37.952 | 37.917 | 37.880 | 37.846 | 37.811 | 37.774 | 37.736 | 37.694 | 37.650 | 37.603 | 37.550 | 37.495 | 37.436 | 37.374 | 37.311 | 37.247 | 37.179 | 37.115 | 37.049 | 36.987                 | 36.928 | 36.873 | 6 |
| Span No. 4                |                        |                                                           |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        |        |        |   |
| Girder                    | ℄ Brg.,<br>Pier No. 3  | Blocking points at 3 m intervals from ℄ Brg., Pier No. 3  |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        | Girder |        |   |
| 1                         | 36.575                 | 36.525                                                    | 36.481 | 36.440 | 36.402 | 36.366 | 36.329 | 36.293 | 36.255 | 36.216 | 36.172 | 36.126 | 36.077 | 36.026 | 35.969 | 35.910 | 35.847 | 35.784 | 35.718 | 35.651 | 35.585 | 35.518 | 35.456                 | 35.397 | 35.339 | 1 |
| 2                         | 36.657                 | 36.607                                                    | 36.563 | 36.523 | 36.487 | 36.450 | 36.415 | 36.380 | 36.343 | 36.303 | 36.261 | 36.215 | 36.166 | 36.114 | 36.058 | 35.999 | 35.936 | 35.872 | 35.806 | 35.738 | 35.672 | 35.604 | 35.541                 | 35.483 | 35.426 | 2 |
| 3                         | 36.739                 | 36.689                                                    | 36.647 | 36.607 | 36.570 | 36.535 | 36.499 | 36.464 | 36.424 | 36.389 | 36.347 | 36.301 | 36.253 | 36.202 | 36.144 | 36.084 | 36.022 | 35.957 | 35.892 | 35.823 | 35.757 | 35.690 | 35.628                 | 35.569 | 35.513 | 3 |
| 4                         | 36.821                 | 36.771                                                    | 36.729 | 36.689 | 36.652 | 36.617 | 36.582 | 36.547 | 36.510 | 36.471 | 36.429 | 36.383 | 36.336 | 36.283 | 36.226 | 36.169 | 36.106 | 36.041 | 35.976 | 35.908 | 35.841 | 35.776 | 35.714                 | 35.655 | 35.599 | 4 |
| 5                         | 36.903                 | 36.854                                                    | 36.811 | 36.771 | 36.735 | 36.699 | 36.662 | 36.627 | 36.590 | 36.551 | 36.509 | 36.464 | 36.416 | 36.364 | 36.308 | 36.249 | 36.187 | 36.122 | 36.059 | 35.992 | 35.926 | 35.861 | 35.801                 | 35.741 | 35.688 | 5 |
| 6                         | 36.857                 | 36.806                                                    | 36.764 | 36.723 | 36.686 | 36.650 | 36.613 | 36.578 | 36.541 | 36.501 | 36.459 | 36.414 | 36.366 | 36.314 | 36.259 | 36.201 | 36.139 | 36.076 | 36.012 | 35.945 | 35.881 | 35.817 | 35.758                 | 35.699 | 35.645 | 6 |
| Span No. 5                |                        |                                                           |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |                        |        |        |   |
| Girder                    | ℄ Brg.,<br>Pier No. 4  | Blocking points at 3 m intervals from ℄ Brg., Pier No. 4  |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        | ℄ Brg.,<br>Abut. No. 2 | Girder |        |   |
| 1                         | 35.321                 | 35.269                                                    | 35.228 | 35.187 | 35.150 | 35.113 | 35.077 | 35.041 | 35.003 | 34.963 | 34.920 | 34.872 | 34.822 | 34.766 | 34.708 | 34.645 | 34.578 | 34.508 | 34.436 | 34.359 |        | 34.308 | 1                      |        |        |   |
| 2                         | 35.408                 | 35.358                                                    | 35.316 | 35.277 | 35.241 | 35.205 | 35.171 | 35.136 | 35.098 | 35.059 | 35.016 | 34.969 | 34.918 | 34.863 | 34.804 | 34.741 | 34.673 | 34.602 | 34.527 | 34.452 |        | 34.399 | 2                      |        |        |   |
| 3                         | 35.495                 | 35.445                                                    | 35.404 | 35.366 | 35.331 | 35.296 | 35.263 | 35.228 | 35.191 | 35.152 | 35.109 | 35.063 | 35.012 | 34.957 | 34.898 | 34.834 | 34.767 | 34.695 | 34.620 | 34.543 |        | 34.490 | 3                      |        |        |   |
| 4                         | 35.583                 | 35.534                                                    | 35.493 | 35.455 | 35.419 | 35.384 | 35.351 | 35.316 | 35.280 | 35.242 | 35.199 | 35.153 | 35.098 | 35.048 | 34.988 | 34.924 | 34.857 | 34.785 | 34.711 | 34.635 |        | 34.582 | 4                      |        |        |   |
| 5                         | 35.670                 | 35.622                                                    | 35.582 | 35.542 | 35.507 | 35.473 | 35.439 | 35.404 | 35.368 | 35.329 | 35.287 | 35.241 | 35.191 | 35.136 | 35.077 | 35.013 | 34.946 | 34.875 | 34.801 | 34.725 |        | 34.673 | 5                      |        |        |   |
| 6                         | 35.629                 | 35.580                                                    | 35.540 | 35.500 | 35.465 | 35.429 | 35.395 | 35.362 | 35.325 | 35.286 | 35.244 | 35.198 | 35.149 | 35.094 | 35.036 | 34.972 | 34.906 | 34.835 | 34.762 | 34.686 |        | 34.636 | 6                      |        |        |   |



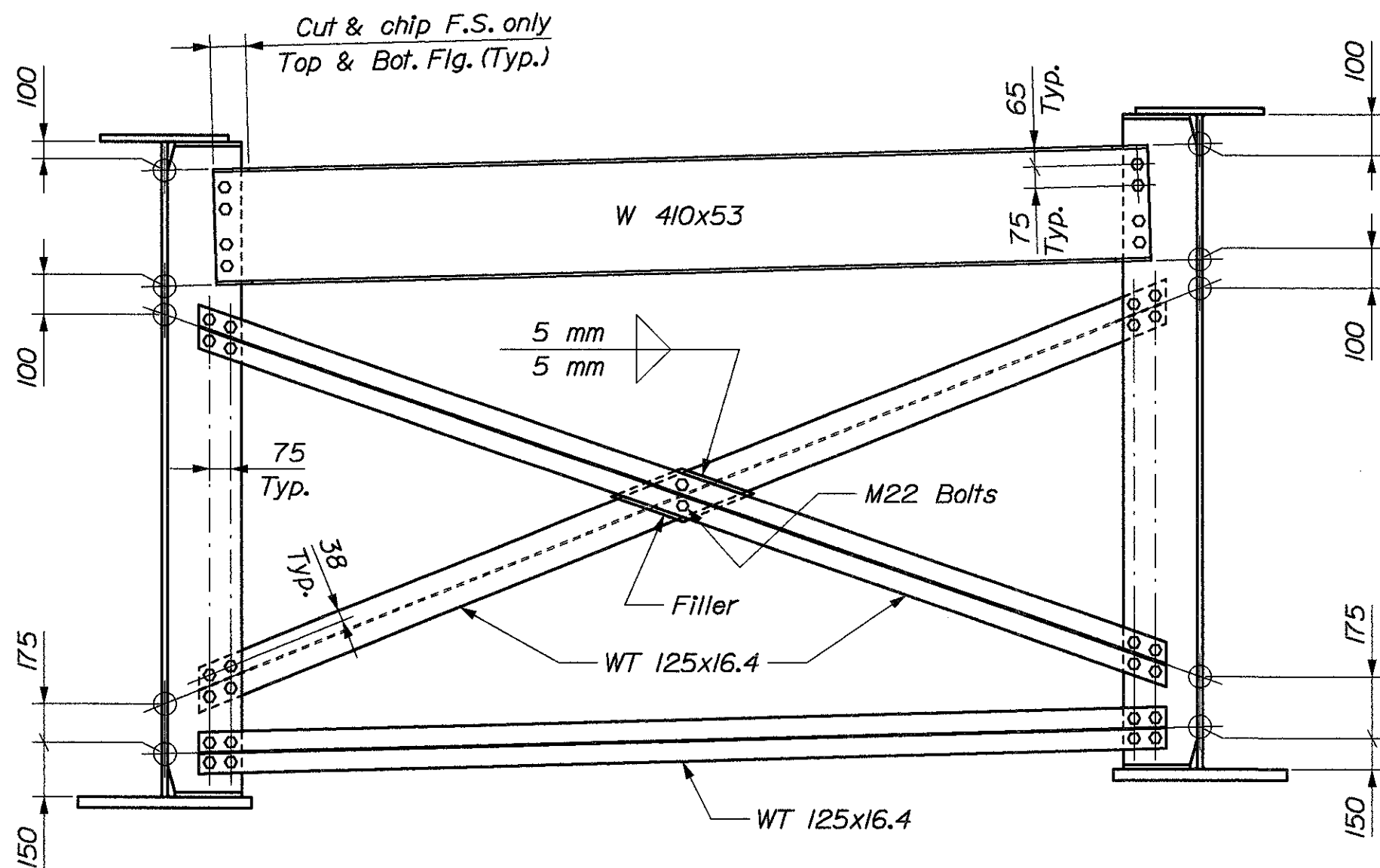
BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

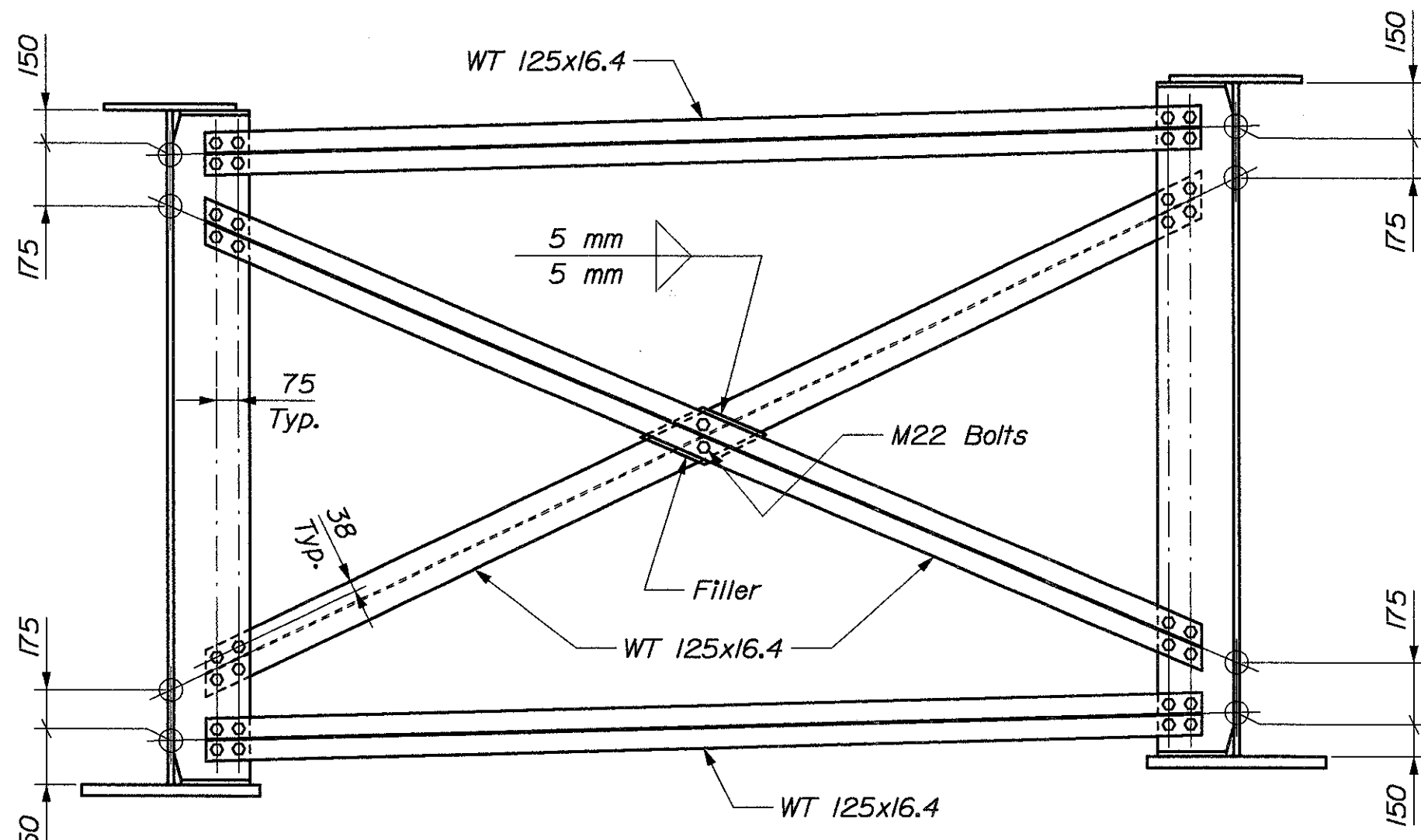
STATE ROUTE 3  
OVER  
KENNEBEC RIVER

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 |           |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

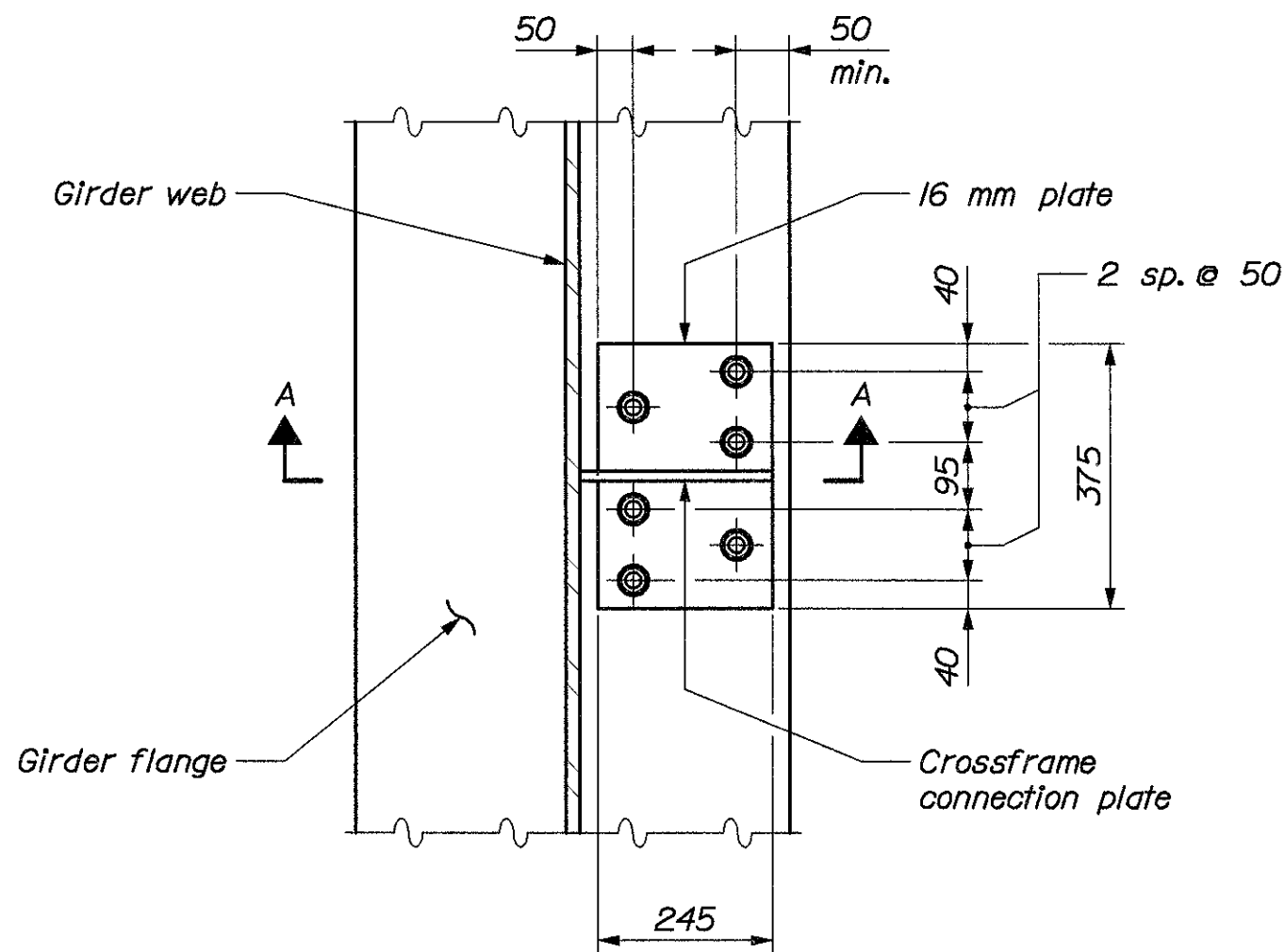
PLANS



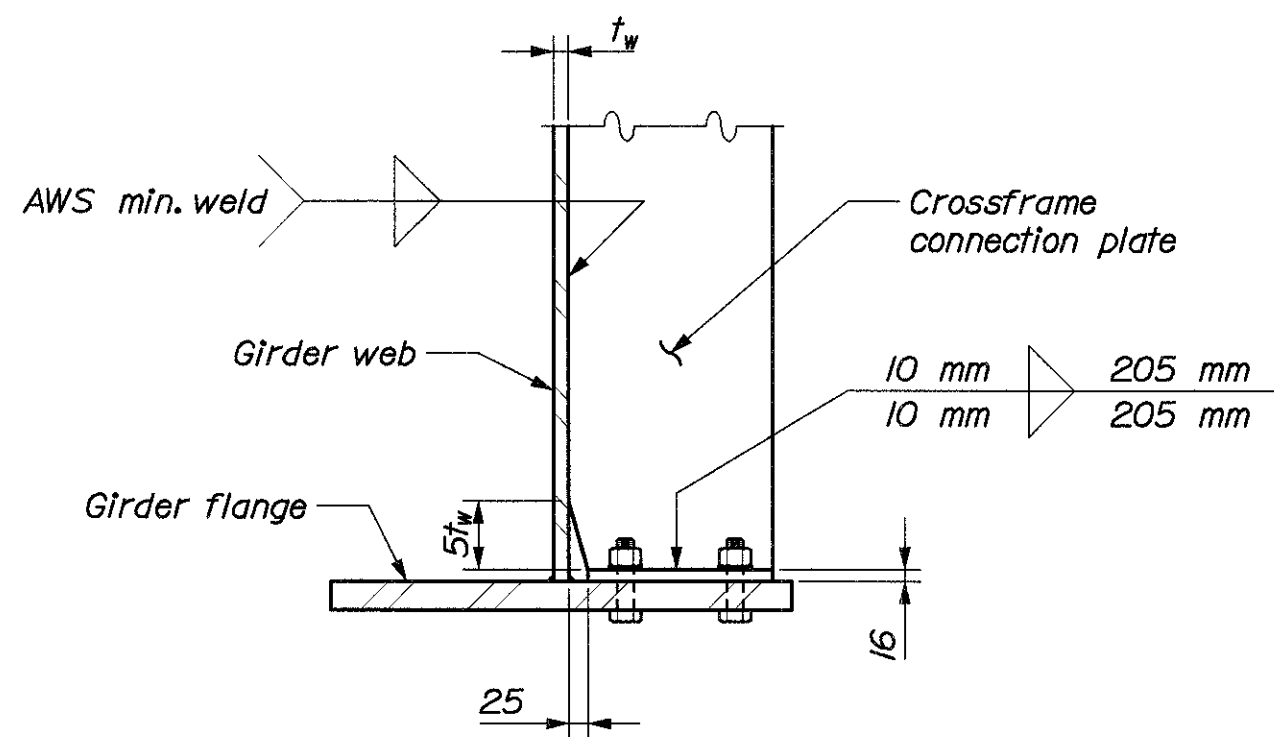
CROSSFRAME TYPE "S"



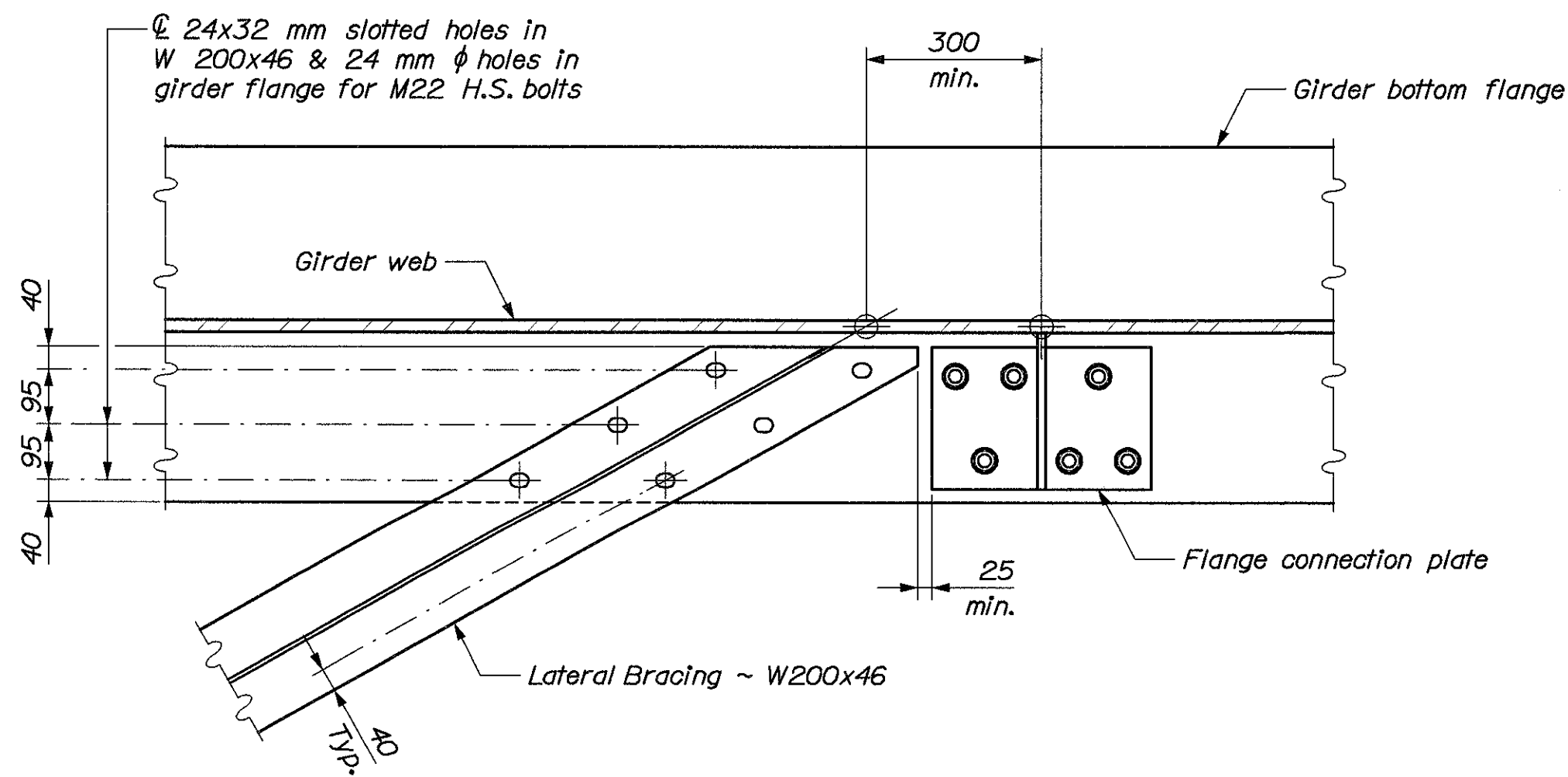
CROSSFRAME TYPE "T"



FLANGE CONNECTION PLATE



SECTION A-A



LATERAL BRACING DETAIL

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

|                   |       |                |           |              |
|-------------------|-------|----------------|-----------|--------------|
| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
| 1                 | MAINE | NH-0556(220)E  | 78        | 105          |

000556.22

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**CROSSFRAMES & BRACING**

Date:26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ...\\079\_superstructure\_slab.dgn

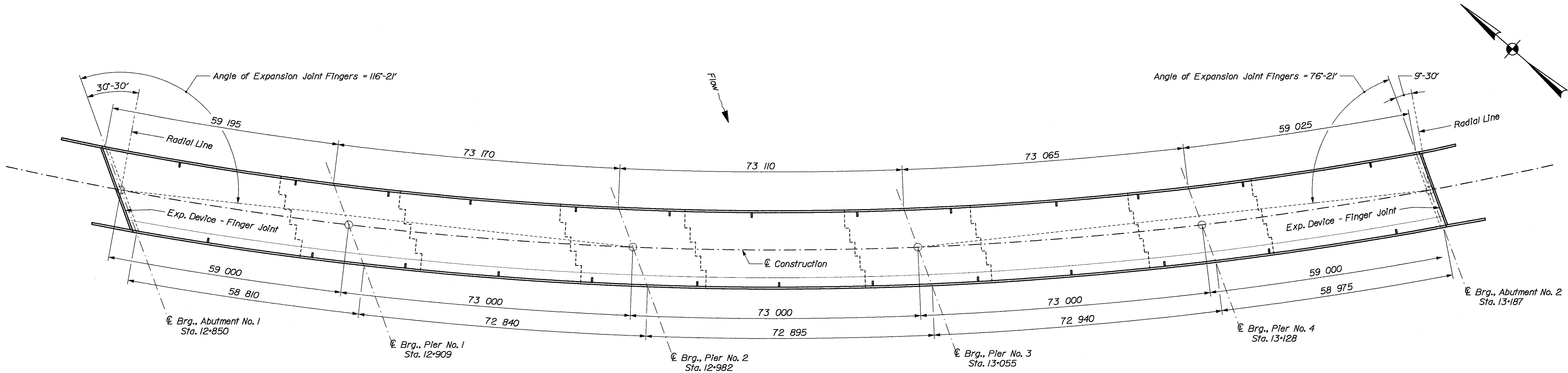
|                                                                                     |           |           |
|-------------------------------------------------------------------------------------|-----------|-----------|
| PROJECT DESIGN ENGINEER<br>DESIGN-DETAILED<br>CHECKED<br>REVISIONS<br>FIELD CHANGES | BY        | DATE      |
|                                                                                     | J. Folsom | T. Damren |
|                                                                                     |           |           |
|                                                                                     |           |           |
| PLANS                                                                               |           |           |

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

|                      |       |                |              |                 |
|----------------------|-------|----------------|--------------|-----------------|
| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
| 1                    | MAINE | NH-0556(220)E  | 79           | 105             |

000556.22



SUPERSTRUCTURE SLAB PLAN

SUPERSTRUCTURE NOTES

1. Form a 30 mm V - groove on the fascias at the horizontal joint between the curb and slab.  
2. Reinforcing steel shall have a minimum cover of 50 mm unless otherwise shown.  
3. Adjust reinforcing steel to fit around the bridge drains as shown and in a manner approved by the Engineer. Do not cut transverse reinforcing bars.  
4. Provide one Expansion Device - Finger Joint (Type "C") at each end of the superstructure slab in accordance with the Standard Details and the Supplemental Standard Details Section 521.  
5. Unless the superstructure slab concrete is placed in one continuous operation, placement shall follow one of the two options shown on the 'Slab Placement Sequence' sheet. The placement sequence shall be approved by the Engineer. A minimum of five days shall elapse between successive partial placements.  
6. If the superstructure slab concrete is placed in one continuous operation, the concrete shall be kept plastic one complete span behind the span being placed.

7. At the option of the Contractor, Precast Deck Panels may be used in place of the full depth cast - in - place deck slab in accordance with Special Provision Section 502, Structural Concrete - Precast Deck Panels, and in accordance with the Standard Details Book, Section 502. Design dimensions shall be as follows:

Slab "T" = 250 mm  
Panel "P" = 100 mm  
Number of strands required = 27

8. Payment for all reinforcing steel in the superstructure slab and curbs for either the cast - in - place or the precast options will be considered incidental to the structural concrete pay items. The cast - in - place deck option contains approximately 253 600 kg of reinforcing steel.

9. Coordinates and/or chord offsets to the fascias will be provided to the Contractor upon written request to the Engineer.

10. In order to provide surface drainage prior to placement of the wearing surface, a 50 mm  $\phi$  Schedule 40

galvanized steel pipe drain shall be installed at each curb line at the Abutment No. 2 end of the deck. Refer to details on the "Bridge Drains" drawing.

11. Curbs shall be constructed using the "Top of Curb Elevations" shown. Elevations are given at three meter intervals along the curve of the fascias beginning at their intersection with the centerline of bearing in each span and proceeding toward Abutment No. 2.

12. The non - shrink mortar located between the bridge drains and the face of curb shall be placed as shown on the "Bridge Drains" sheet. Payment will be considered incidental to contract items.

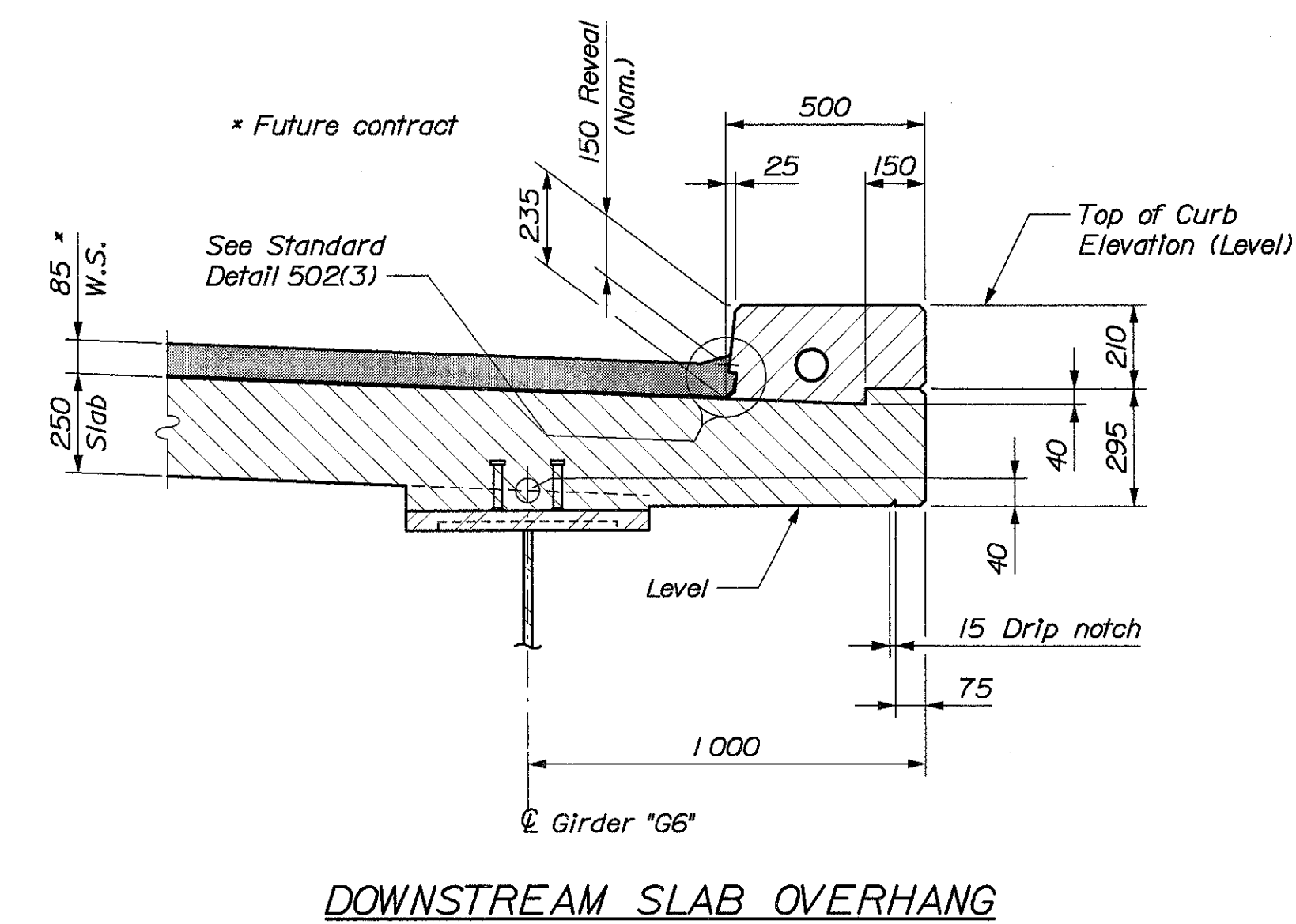
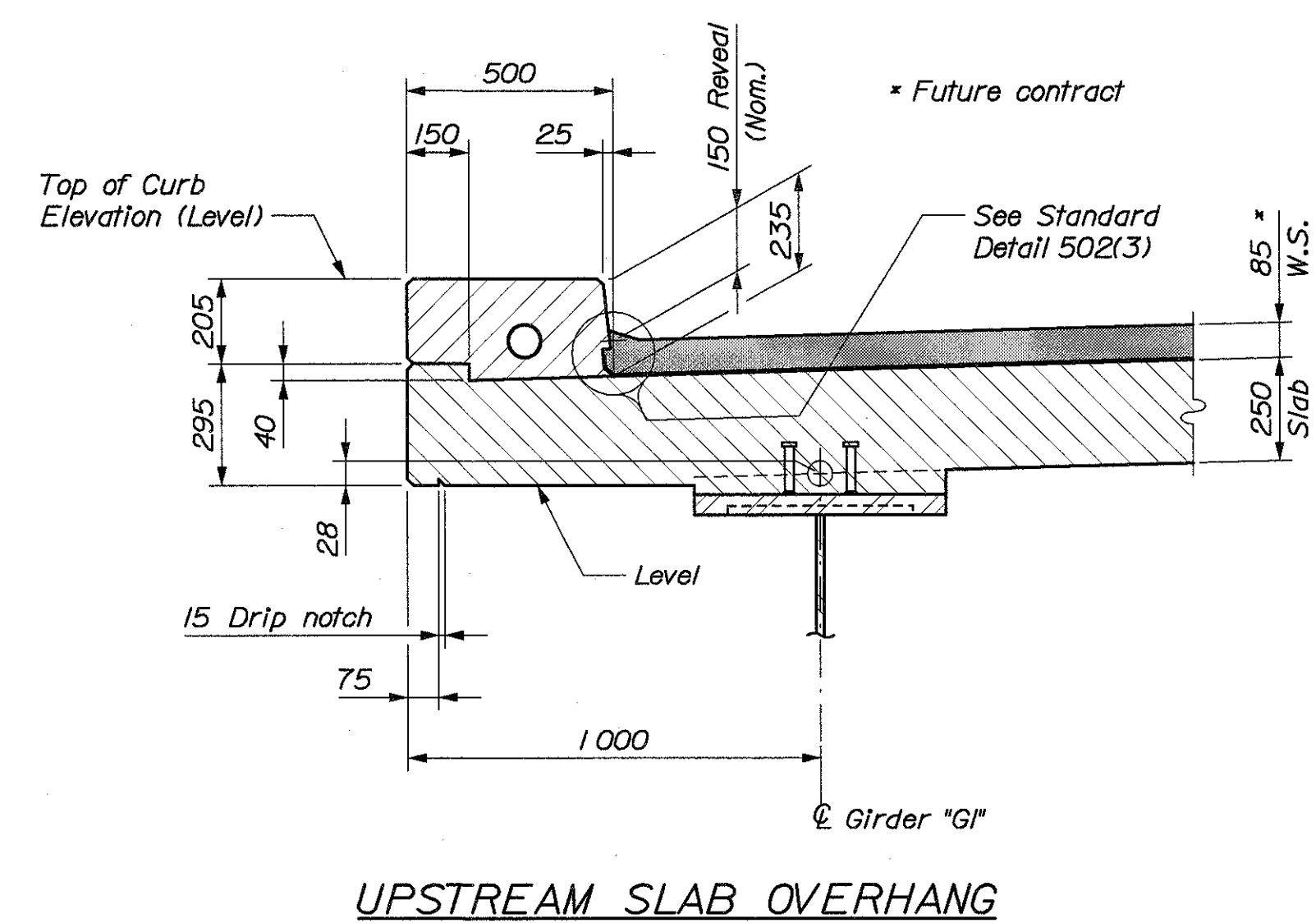
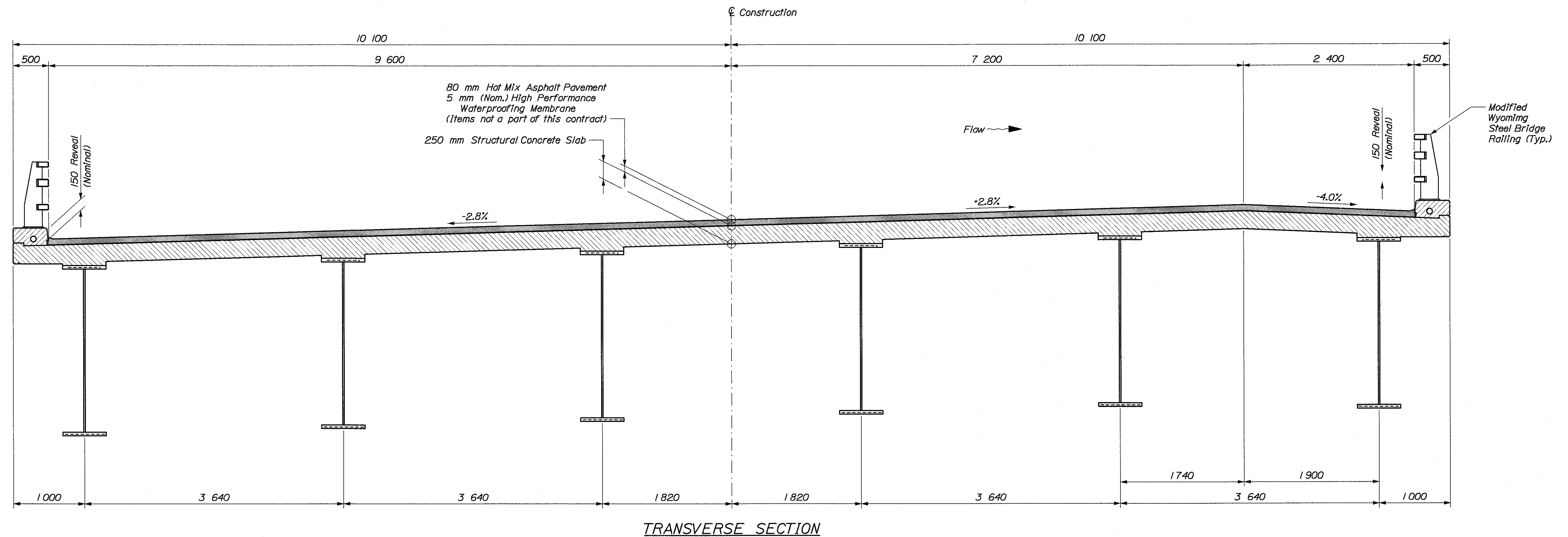
13. The completed deck shall be prepared to receive future membrane by cleaning it of all foreign materials, such as oil or grease, and removing any sharp protrusions. Potholes and spalls shall be filled with a product from the Department's Prequalified List of Deck Patching Materials applied in accordance with the Department's approved procedures. Payment for all work and materials will be considered incidental to the structural concrete pay items.

14. Protective Coating for Concrete Surfaces shall be applied to the face and top of concrete curbs, fascias down to the drip notch, and the permanently exposed strips of concrete adjacent to the expansion devices.

| TOP OF CURB ELEVATIONS |                     |                                                                   |        |        |        |        |        |        |        |        |        |        |        |        |            |
|------------------------|---------------------|-------------------------------------------------------------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|------------|
| Span No. 1             |                     |                                                                   |        |        |        |        |        |        |        |        |        |        |        |        |            |
| Curb                   | ℄ Brg., Abut. No. 1 | Elevations at 3 m intervals along fascia from ℄ Brg., Abut. No. 1 |        |        |        |        |        |        |        |        |        |        |        |        | Curb       |
| Upstream               | 40.583              | 40.532                                                            | 40.480 | 40.429 | 40.377 | 40.326 | 40.274 | 40.222 | 40.171 | 40.119 | 40.068 | 40.016 | 39.965 | 39.913 | Upstream   |
| Downstream             | 40.756              | 40.706                                                            | 40.655 | 40.605 | 40.554 | 40.504 | 40.453 | 40.403 | 40.352 | 40.302 | 40.252 | 40.201 | 40.151 | 40.100 | Downstream |
| Span No. 2             |                     |                                                                   |        |        |        |        |        |        |        |        |        |        |        |        |            |
| Curb                   | ℄ Brg., Pier No. 1  | Elevations at 3 m intervals along fascia from ℄ Brg., Pier No. 1  |        |        |        |        |        |        |        |        |        |        |        |        | Curb       |
| Upstream               | 39.566              | 39.514                                                            | 39.463 | 39.411 | 39.360 | 39.308 | 39.257 | 39.205 | 39.153 | 39.102 | 39.050 | 38.999 | 38.947 | 38.896 | Upstream   |
| Downstream             | 39.767              | 39.717                                                            | 39.666 | 39.616 | 39.565 | 39.515 | 39.464 | 39.414 | 39.363 | 39.313 | 39.263 | 39.212 | 39.162 | 39.111 | Downstream |
| Span No. 3             |                     |                                                                   |        |        |        |        |        |        |        |        |        |        |        |        |            |
| Curb                   | ℄ Brg., Pier No. 2  | Elevations at 3 m intervals along fascia from ℄ Brg., Pier No. 2  |        |        |        |        |        |        |        |        |        |        |        |        | Curb       |
| Upstream               | 38.308              | 38.257                                                            | 38.205 | 38.154 | 38.102 | 38.050 | 37.999 | 37.947 | 37.896 | 37.844 | 37.793 | 37.741 | 37.690 | 37.638 | Upstream   |
| Downstream             | 38.542              | 38.492                                                            | 38.441 | 38.391 | 38.340 | 38.290 | 38.239 | 38.189 | 38.139 | 38.088 | 38.038 | 37.987 | 37.937 | 37.886 | Downstream |
| Span No. 4             |                     |                                                                   |        |        |        |        |        |        |        |        |        |        |        |        |            |
| Curb                   | ℄ Brg., Pier No. 3  | Elevations at 3 m intervals along fascia from ℄ Brg., Pier No. 3  |        |        |        |        |        |        |        |        |        |        |        |        | Curb       |
| Upstream               | 37.052              | 37.000                                                            | 36.949 | 36.897 | 36.846 | 36.794 | 36.742 | 36.691 | 36.639 | 36.588 | 36.536 | 36.485 | 36.433 | 36.381 | Upstream   |
| Downstream             | 37.316              | 37.266                                                            | 37.215 | 37.165 | 37.115 | 37.064 | 37.014 | 36.963 | 36.913 | 36.862 | 36.812 | 36.761 | 36.711 | 36.660 | Downstream |
| Span No. 5             |                     |                                                                   |        |        |        |        |        |        |        |        |        |        |        |        |            |
| Curb                   | ℄ Brg., Pier No. 4  | Elevations at 3 m intervals along fascia from ℄ Brg., Pier No. 4  |        |        |        |        |        |        |        |        |        |        |        |        | Curb       |
| Upstream               | 35.796              | 35.744                                                            | 35.693 | 35.641 | 35.590 | 35.538 | 35.487 | 35.435 | 35.383 | 35.332 | 35.280 | 35.229 | 35.177 | 35.126 | Upstream   |
| Downstream             | 36.090              | 36.039                                                            | 35.989 | 35.938 | 35.888 | 35.837 | 35.787 | 35.737 | 35.686 | 35.636 | 35.585 | 35.535 | 35.484 | 35.434 | Downstream |

BRIDGE NO. 6400  
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
SUPERSTRUCTURE SLAB

000556.22



BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

TRANSVERSE SECTION

SHEET OF AUGUSTA, MAINE

Filename: ...\\081\_sup\_slab span no. 1.dgn

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

| PROJECT DESIGN ENGINEER |  | BY        | DATE |
|-------------------------|--|-----------|------|
| DESIGN-DETAILED         |  | J. Folsom |      |
| CHECKED                 |  |           |      |
| REVISIONS               |  |           |      |
| FIELD CHANGES           |  |           |      |

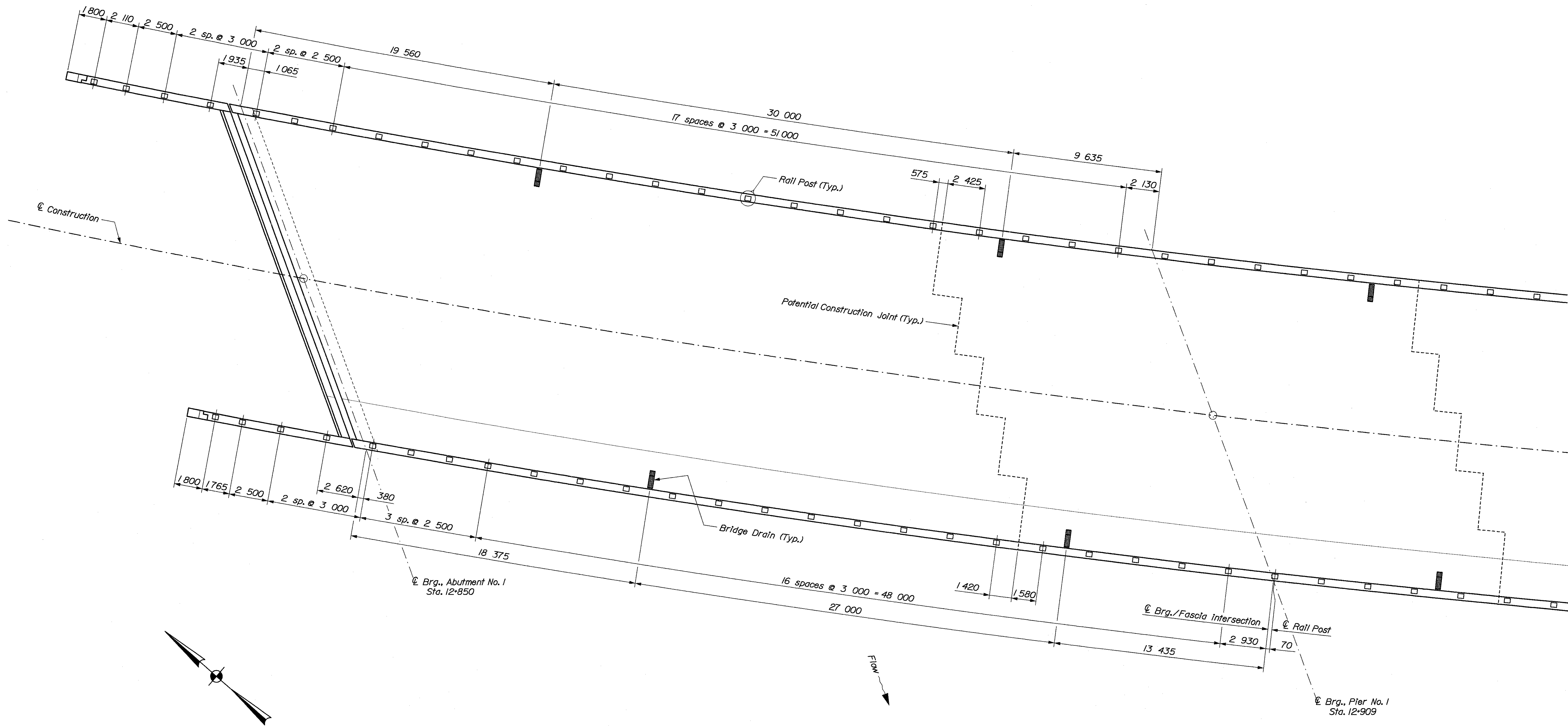
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| FHWA<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|------------------|-------|----------------|--------------|-----------------|
| 1                | MAINE | NH-0556(220)E  | 81           | 105             |

000556.22



SUPERSTRUCTURE SLAB ~ SPAN NO. 1

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
SPR. SLAB ~ SPAN NO. 1

SHEET OF AUGUSTA, MAINE

Filename: ...\\062\_sup\_slab span no. 2.dgn

Division: BRIDGE

Username: Dana.Damren

Date: 26 NOV 2002

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

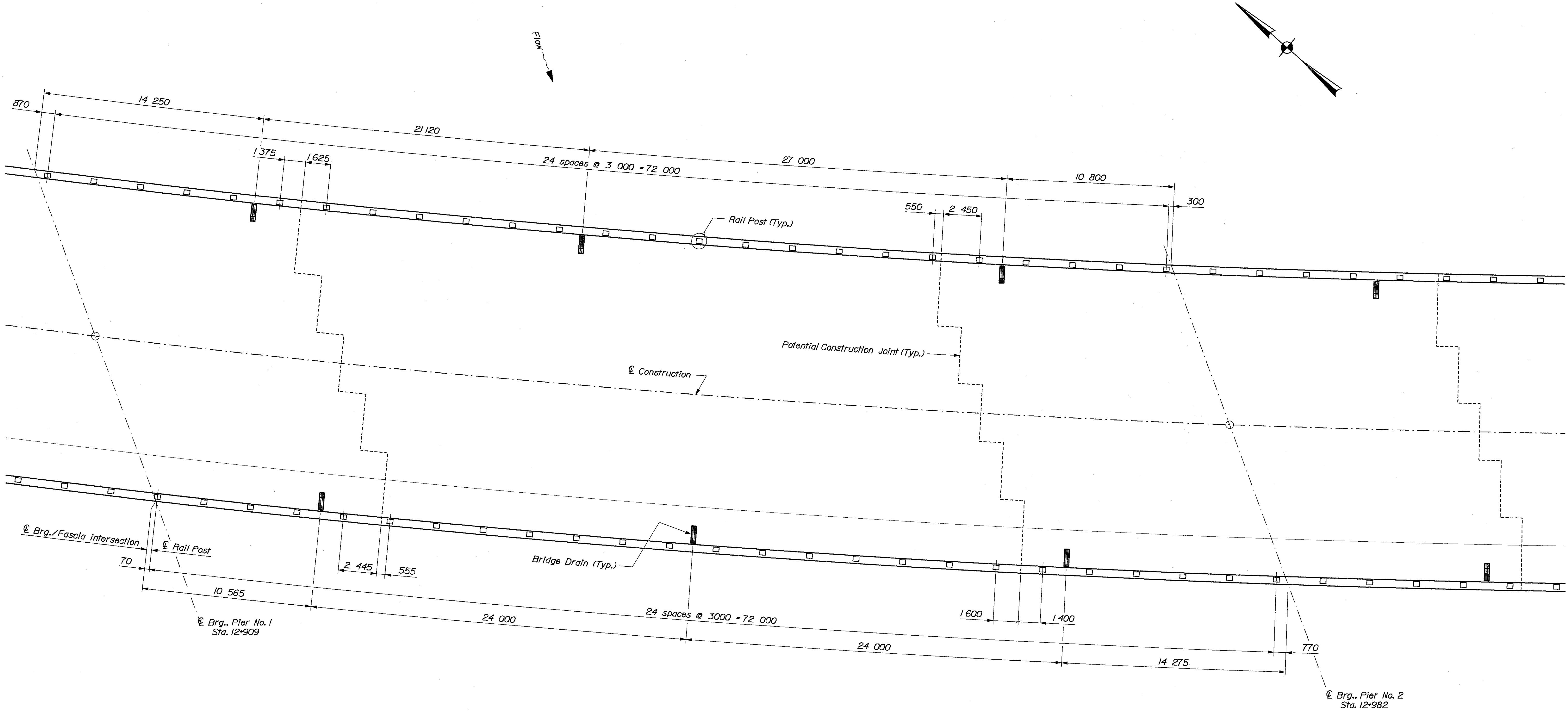
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|-----------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 82        | 105             |

000556.22



SUPERSTRUCTURE SLAB ~ SPAN NO. 2

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
SPR. SLAB ~ SPAN NO. 2

SHEET OF AUGUSTA, MAINE

Filename: ...\\063\_sup\_slab span no. 3.dgn

Date: 26 NOV 2002

Username: Dana.Damren

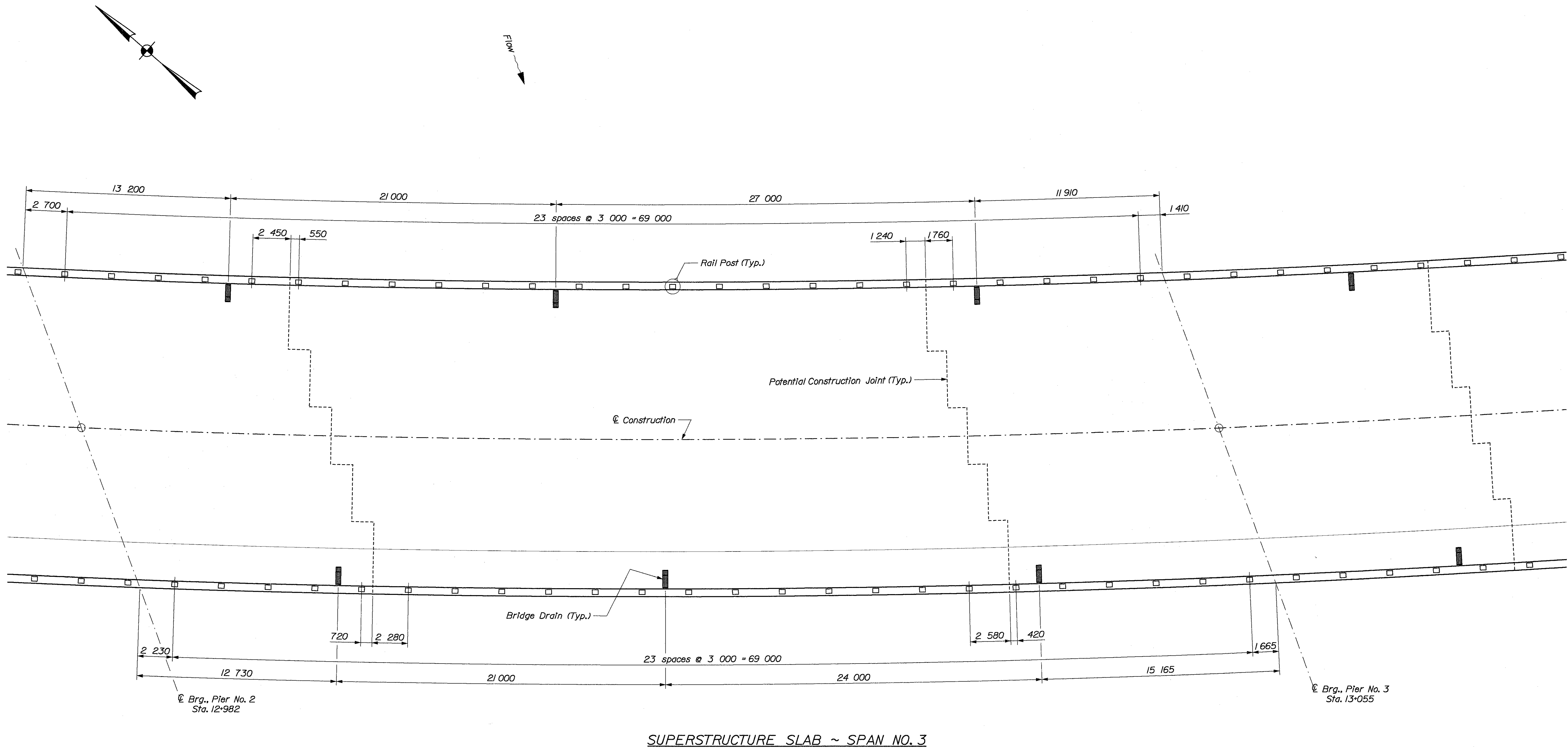
Division: BRIDGE

| PROJECT DESIGN ENGINEER | BY        | DATE      |
|-------------------------|-----------|-----------|
| DESIGN-DETAILED         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

PLANS

|        |                                                                                                               |      |       |                |           |              |
|--------|---------------------------------------------------------------------------------------------------------------|------|-------|----------------|-----------|--------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. | FHWA | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|        |                                                                                                               | 1    | MAINE | NH-0556(220)E  | 83        | 105          |

000556.22



SUPERSTRUCTURE SLAB ~ SPAN NO. 3

|                                                                                                                          |
|--------------------------------------------------------------------------------------------------------------------------|
| BRIDGE NO. 6400                                                                                                          |
| STATE OF MAINE<br>DEPARTMENT OF TRANSPORTATION                                                                           |
| STATE ROUTE 3<br>OVER<br>KENNEBEC RIVER<br>IN THE CITY OF<br>AUGUSTA<br>KENNEBEC COUNTY<br><u>SPR. SLAB ~ SPAN NO. 3</u> |
| SHEET OF<br>AUGUSTA, MAINE                                                                                               |

Filename: ... \084\_sup\_slab span no. 4.dgn

Date: 26 NOV 2002

Username: Dana.Damron

Division: BRIDGE

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damron |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

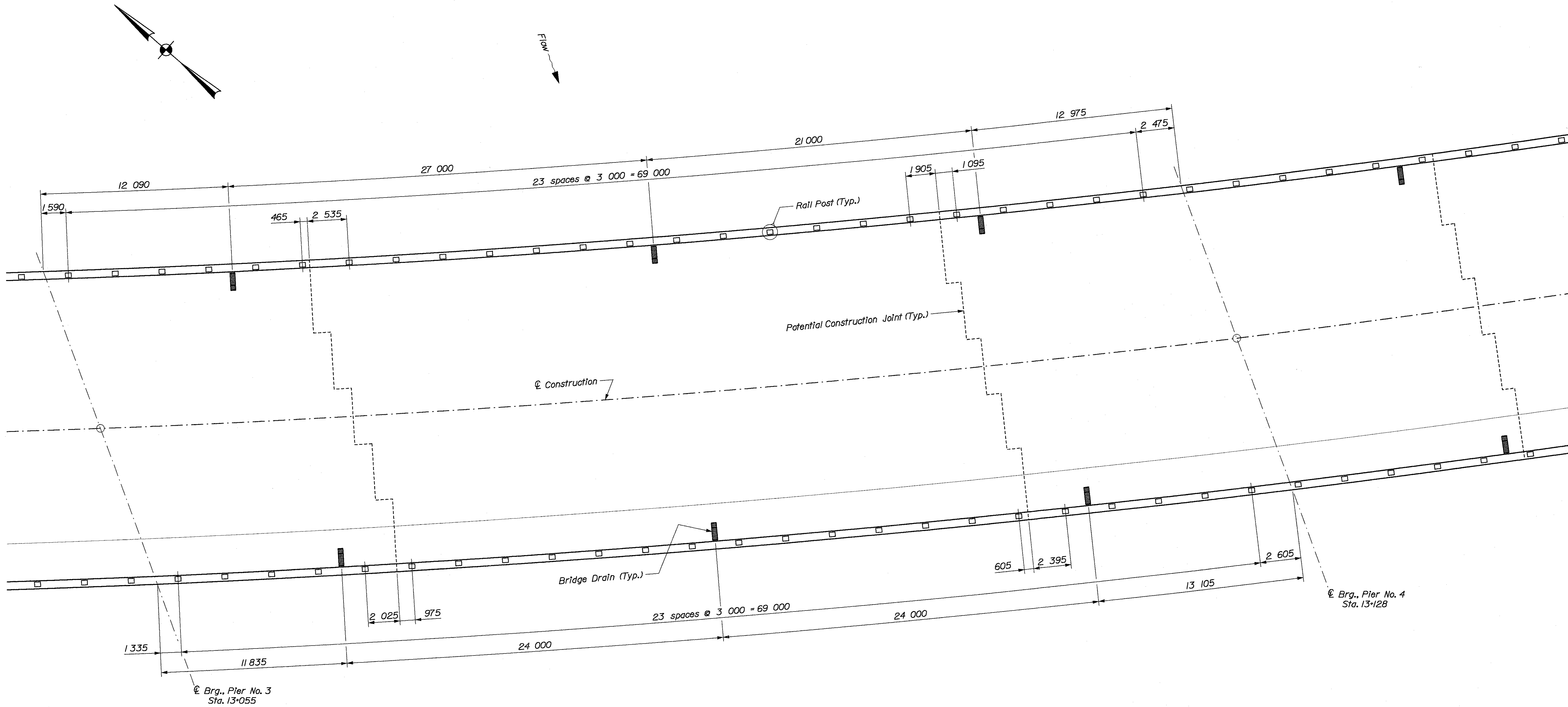
PLANS

METRIC

1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A. RES. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 84        | 105          |

000556.22



SUPERSTRUCTURE SLAB ~ SPAN NO. 4

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
SPR. SLAB ~ SPAN NO. 4

SHEET OF AUGUSTA, MAINE

Filename: ...\\085\_sup\_slab span no. 5.dgn

Division: BRIDGE

Username: Dana.Damren

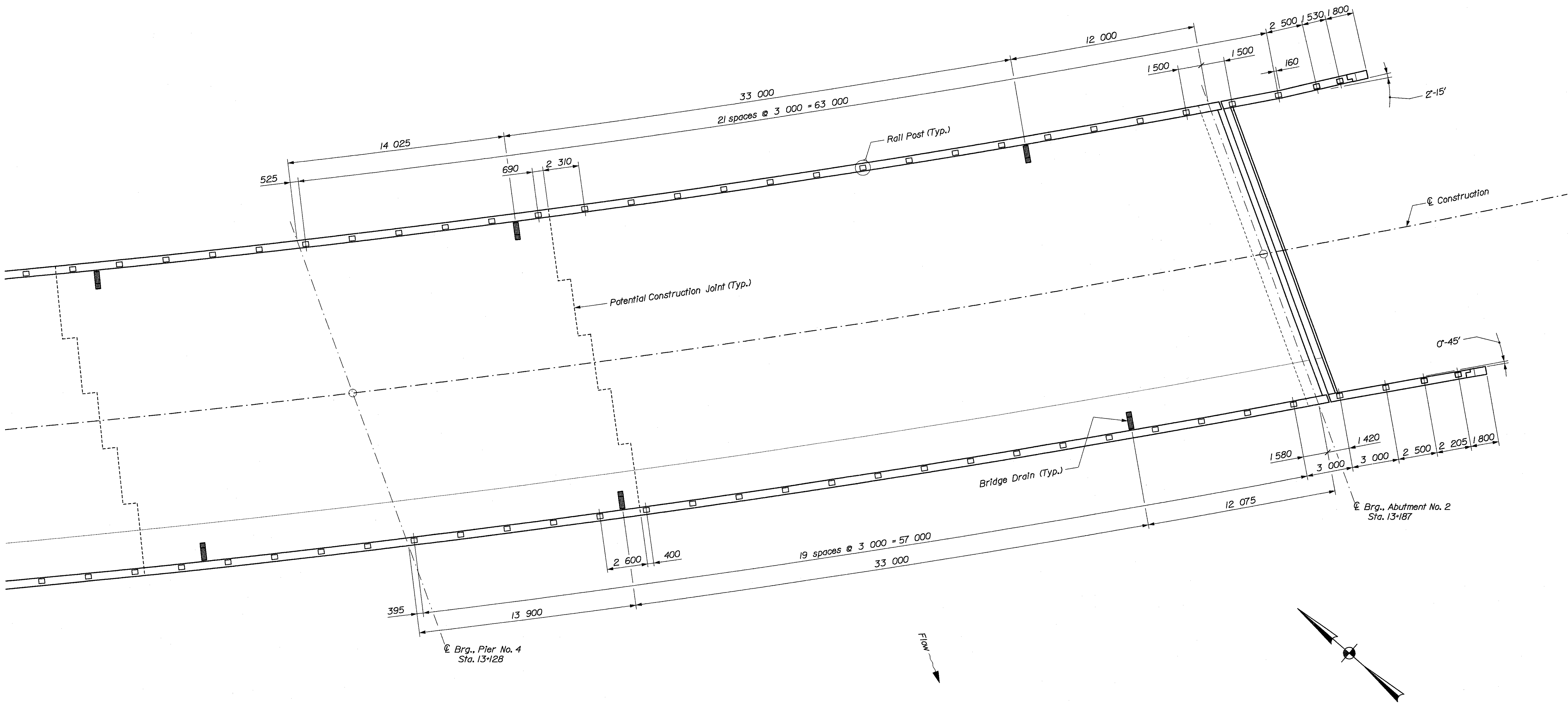
Date: 26 NOV 2002

|                         |           |      |
|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | J. Forsom |      |
| CHECKED                 |           |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS

|        |                                                              |  |                      |       |                |           |              |
|--------|--------------------------------------------------------------|--|----------------------|-------|----------------|-----------|--------------|
| METRIC | 1. All dimensions are in millimeters unless otherwise noted. |  | F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|        | 2. All elevations and stations are in meters.                |  | 1                    | MAINE | NH-0556(220)E  | 85        | 105          |

000556.22



SUPERSTRUCTURE SLAB ~ SPAN NO. 5

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

SPR. SLAB ~ SPAN NO. 5

SHEET OF

AUGUSTA, MAINE

Filename: ...\\086\_sup\_reinf\_span no.1.dgn

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

PLANS

METRIC

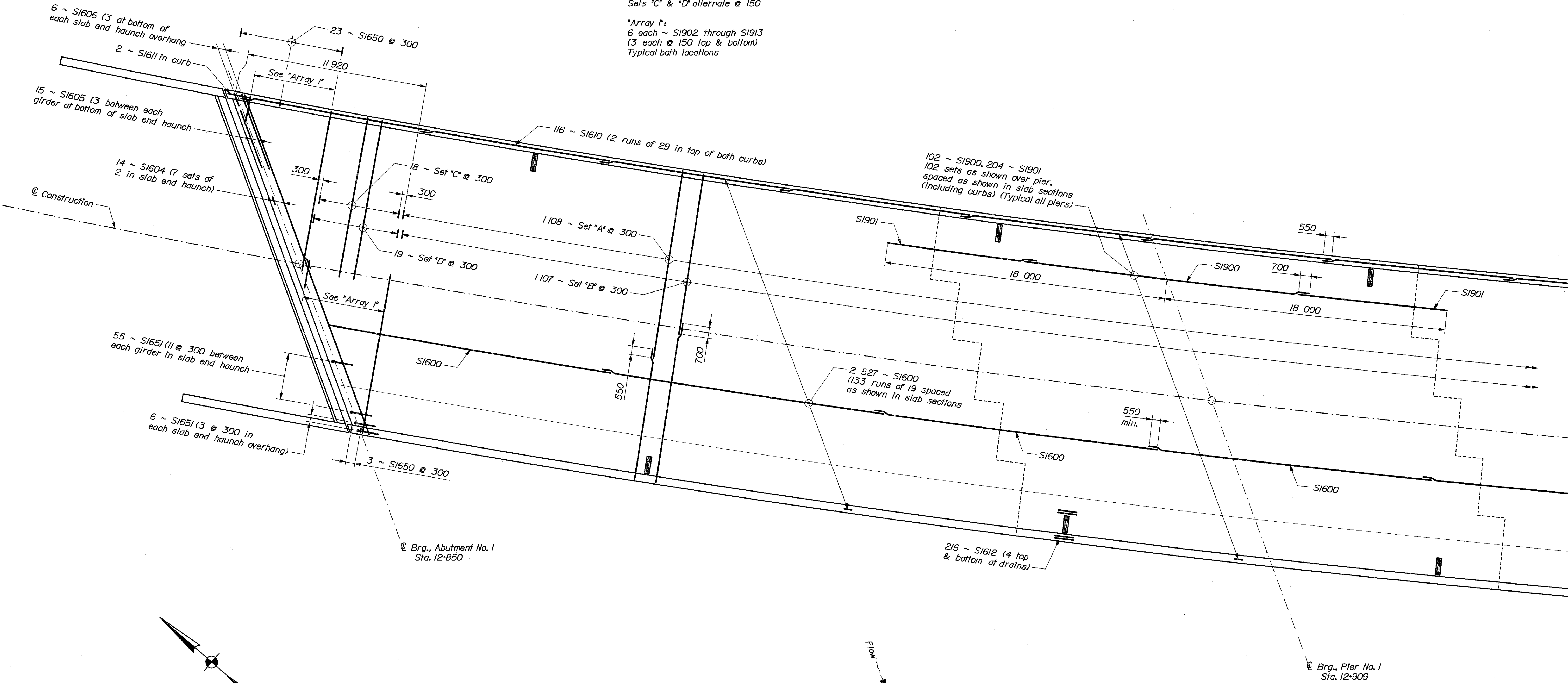
1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|-----------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 86        | 105             |

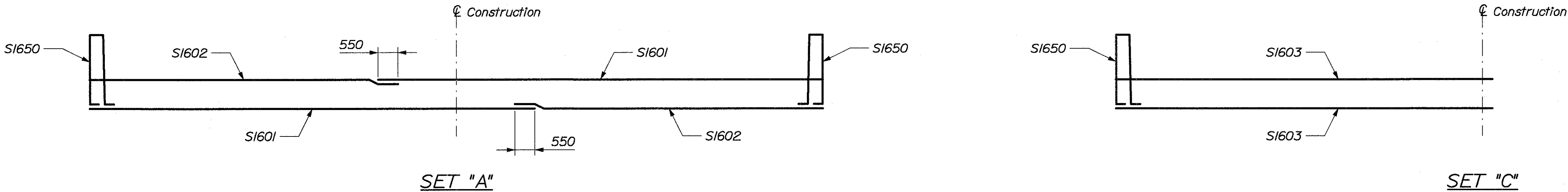
000556.22

Bar Placement Note:  
Sets "A" & "B" alternate @ 150  
Sets "C" & "D" alternate @ 150

\*Array I\*:  
6 each ~ S1902 through S1913  
(3 each @ 150 top & bottom)  
Typical both locations



SLAB REINFORCING PLAN ~ SPAN NO. 1



BRIDGE NO. 6400

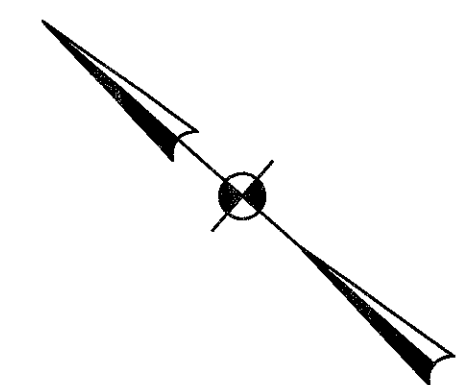
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
SLAB REINFORCING PLAN

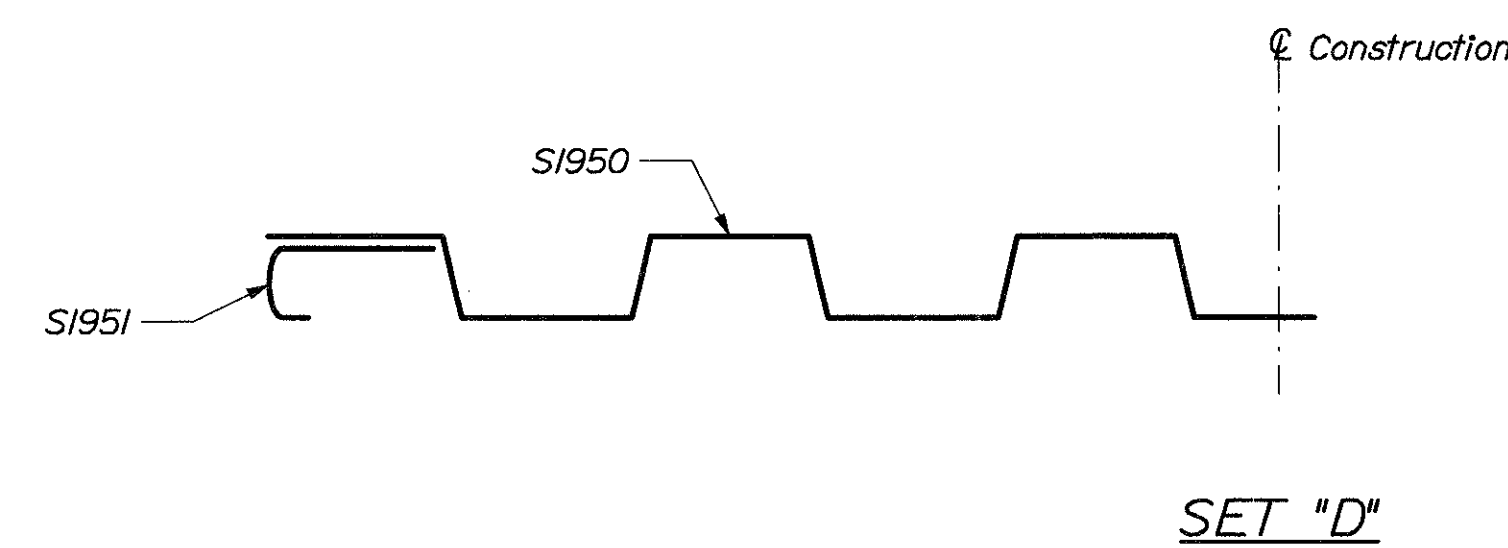
SHEET OF AUGUSTA, MAINE

000556.22

"Array 2":  
2 each ~ S1902 through S1912  
(1 each @ 150 top & bottom)  
Typical both locations



Flow



BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**SLAB REINFORCING PLAN**

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER | BY        | DATE      |
|-------------------------|-----------|-----------|
| DESIGN-DETAILED         | J. Folsom | D. Dornen |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ...\\088\_sup\_sections.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folom  |      |
| CHECKED                 | D. Damren |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

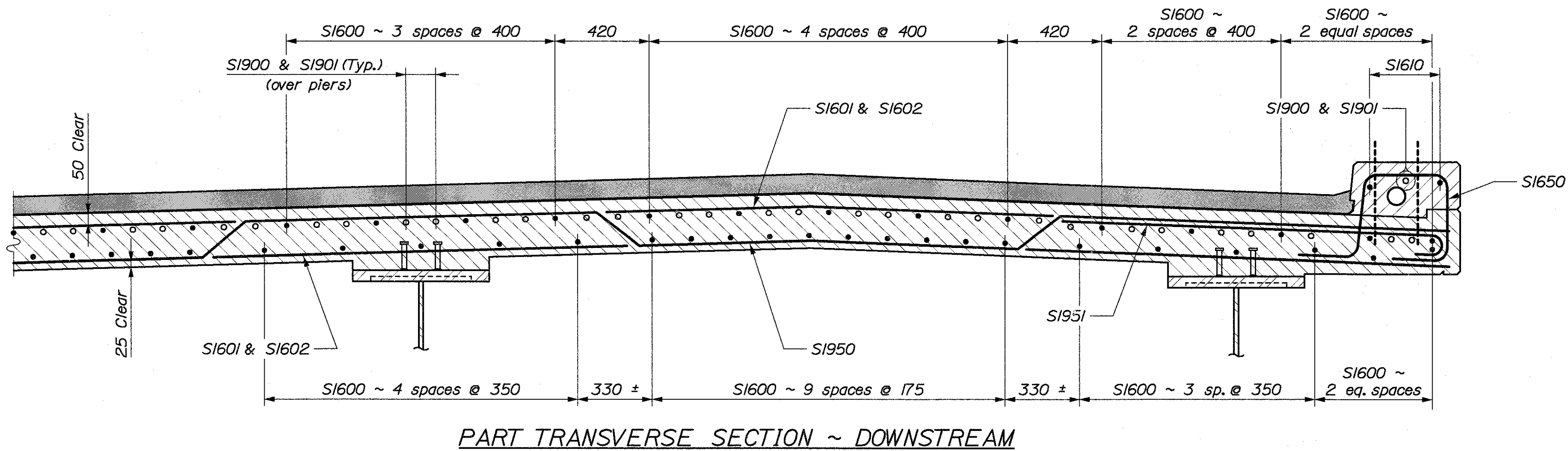
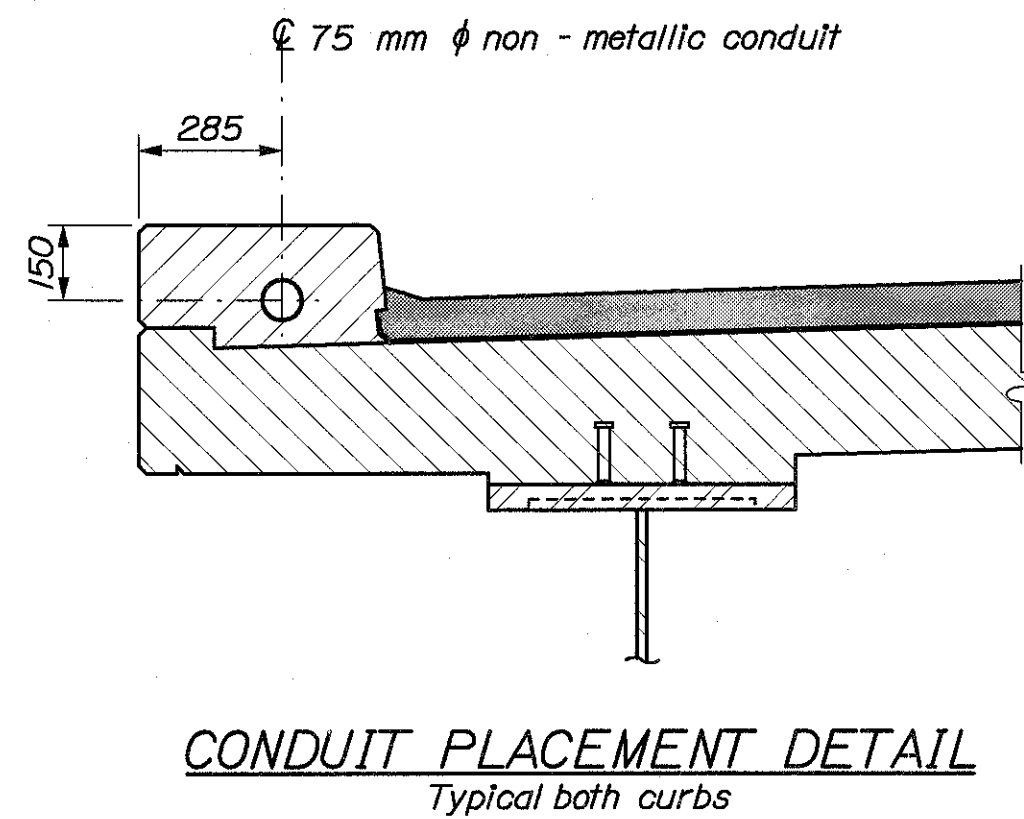
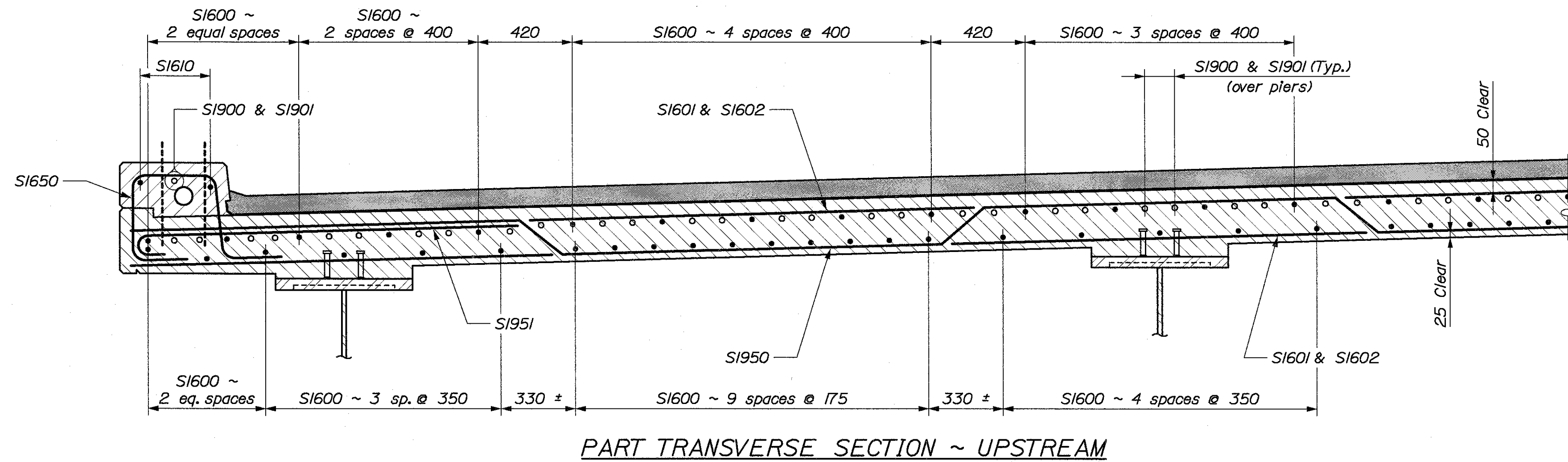
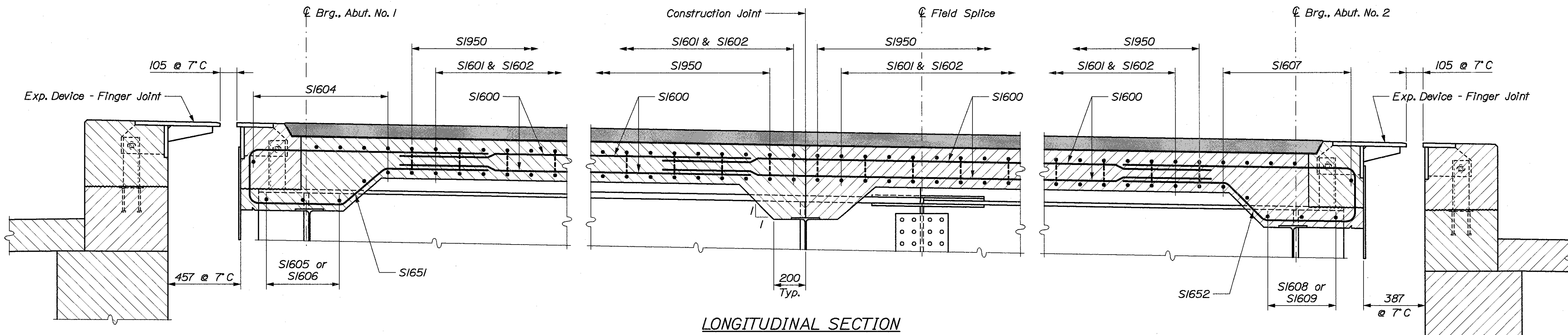
|       |
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| PLANS |
|-------|

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|-----------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 88        | 105             |

000556.22



BRIDGE NO. 6400  
STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
SPR. SLAB SECTIONS

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

Username: Dana.Darren

Division: BRIDGE

Filename: ...\\089\_placement sequence.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | J. Folsom |      |
| CHECKED                 |           |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

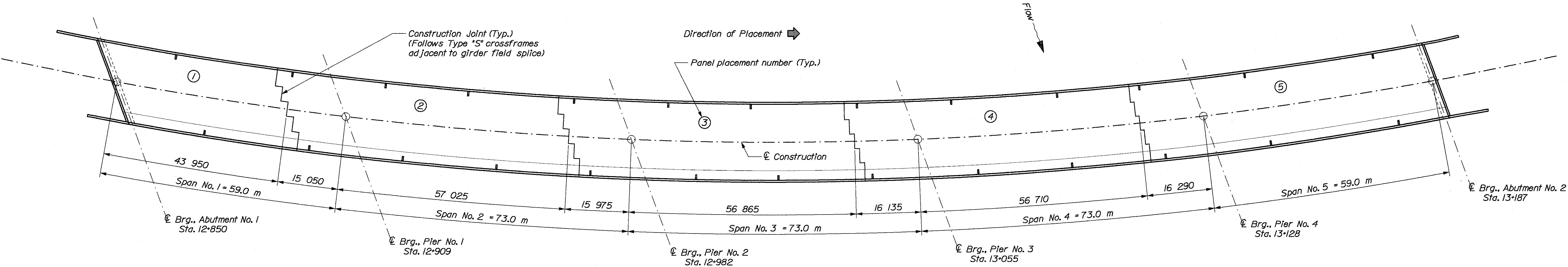
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|-------|
| PLANS |
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METRIC

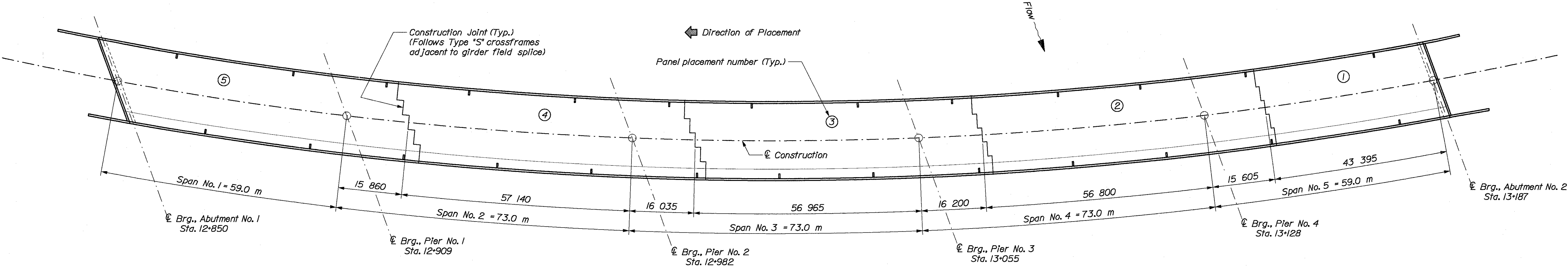
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 89           | 105             |

000556.22



SLAB PLACEMENT SEQUENCE ~ OPTION I



SLAB PLACEMENT SEQUENCE ~ OPTION II

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**SLAB PLACEMENT SEQUENCE**

SHEET OF AUGUSTA, MAINE

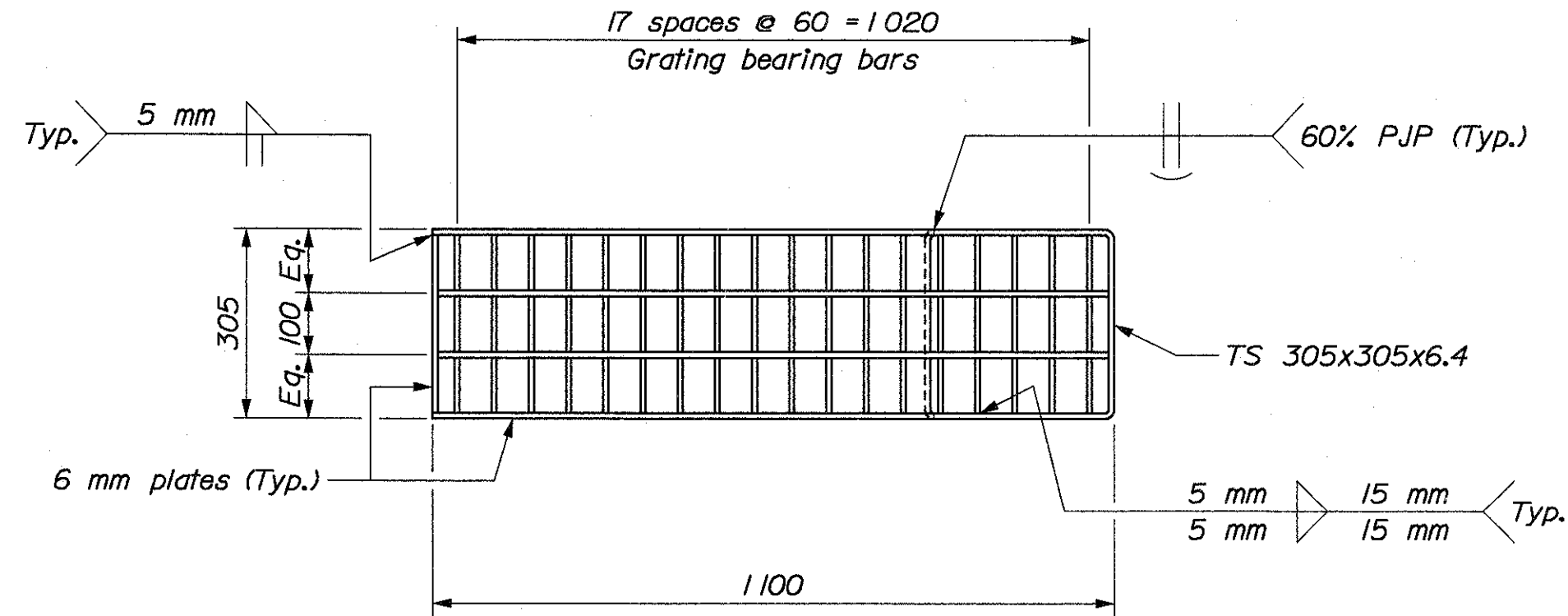
Date:26 NOV 2002

Username: Dana.Damren

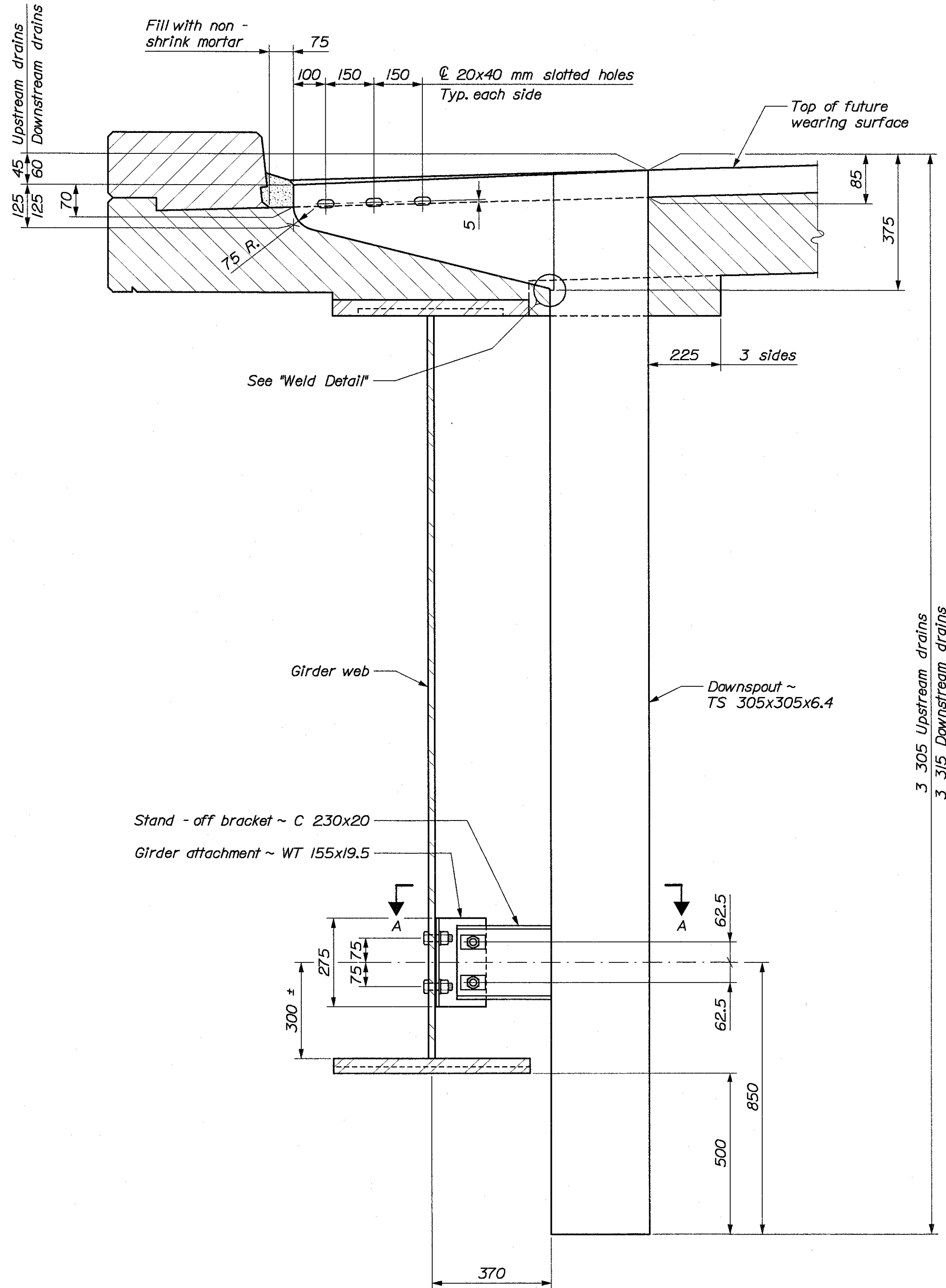
Division: BRIDGE

Filename: ...090\_drains.dgn

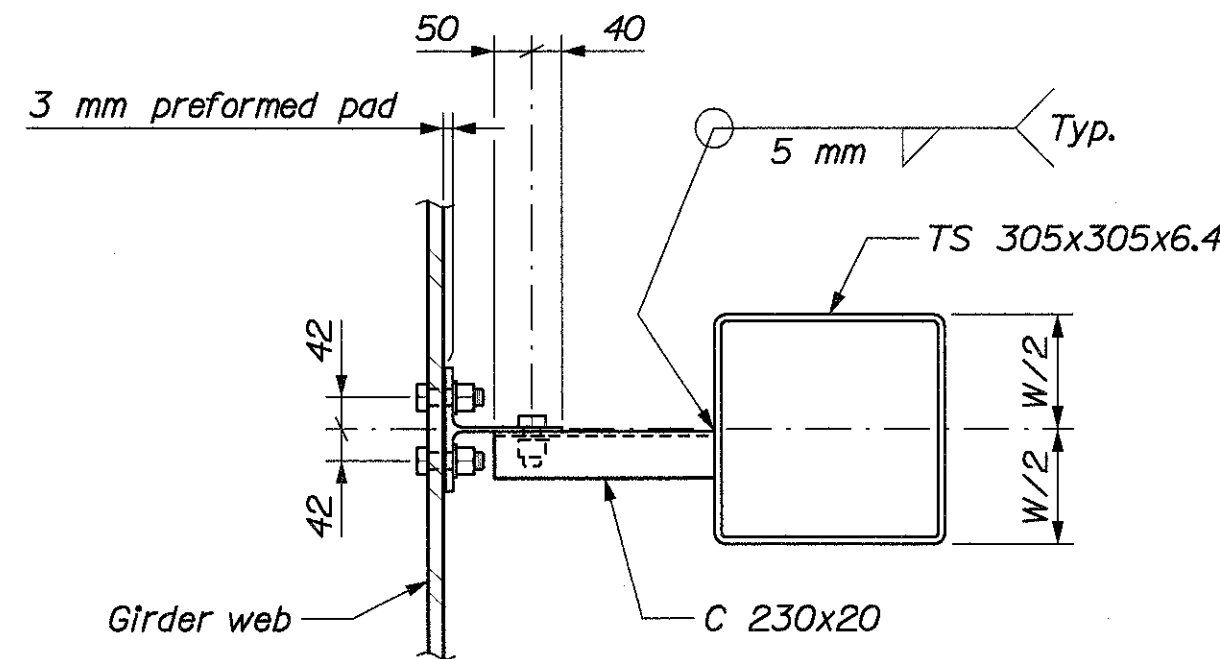
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|-------------------------|-----------|------|
| PROJECT DESIGN ENGINEER | BY        | DATE |
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |
| PLANS                   |           |      |



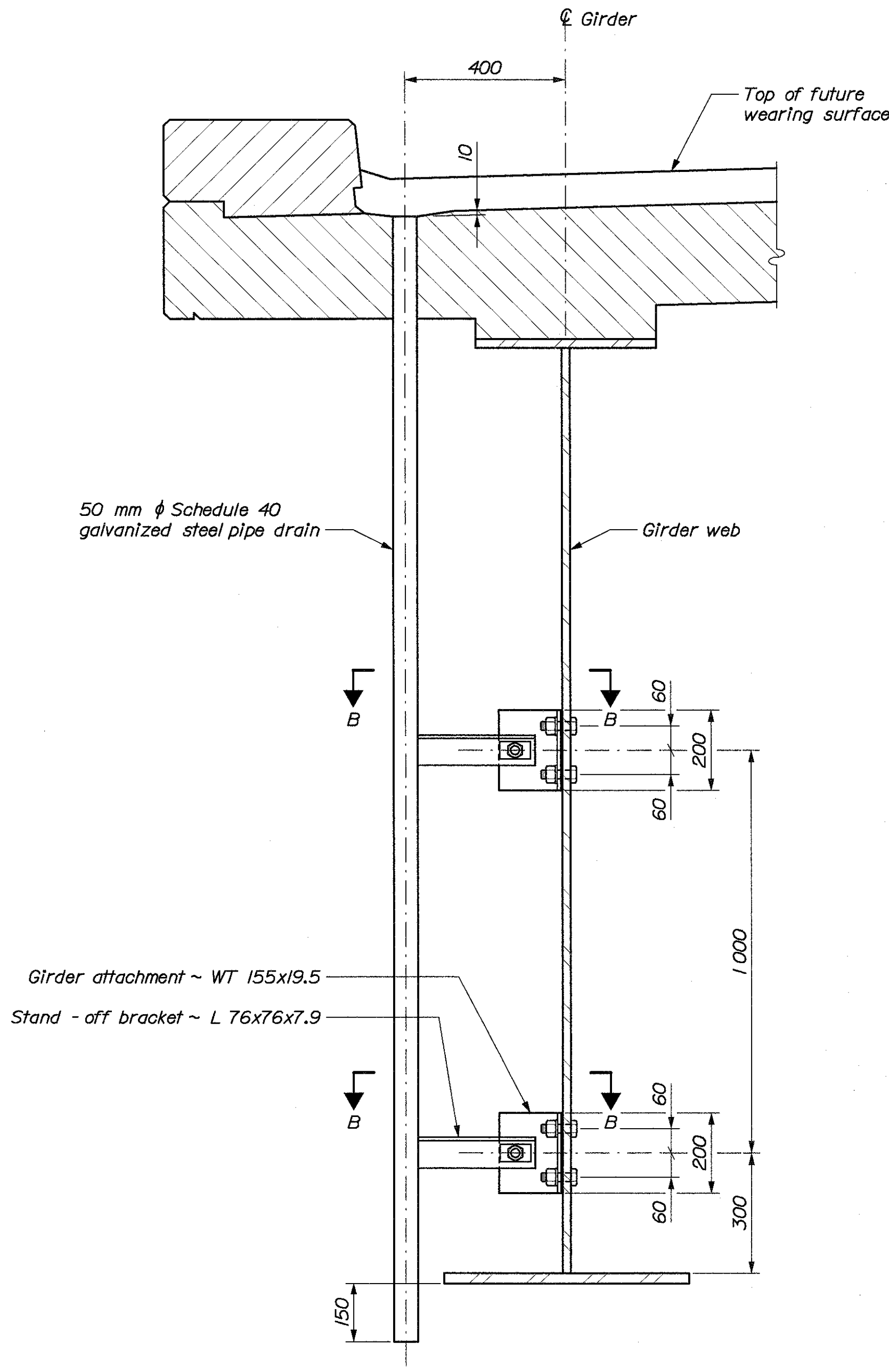
BRIDGE DRAIN PLAN



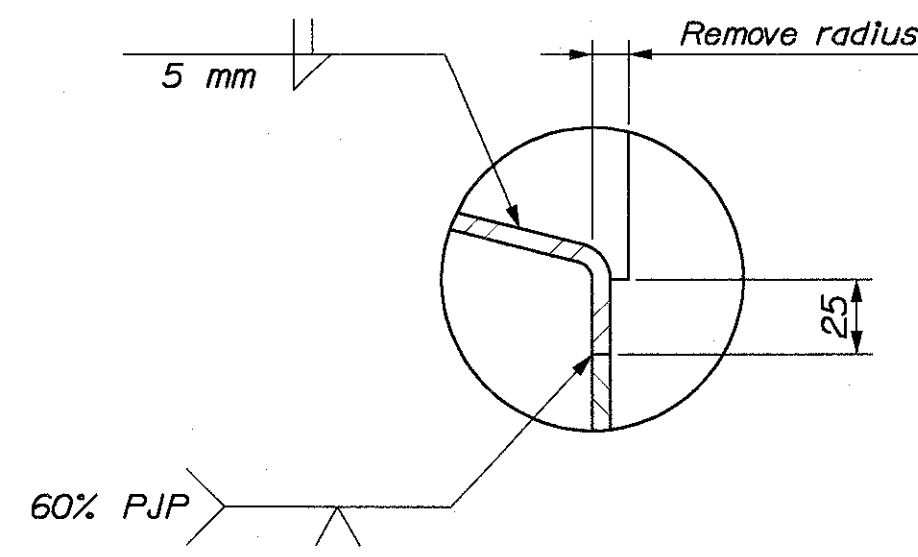
BRIDGE DRAIN ELEVATION



SECTION A-A



PIPE DRAIN ELEVATION



WELD DETAIL

METRIC

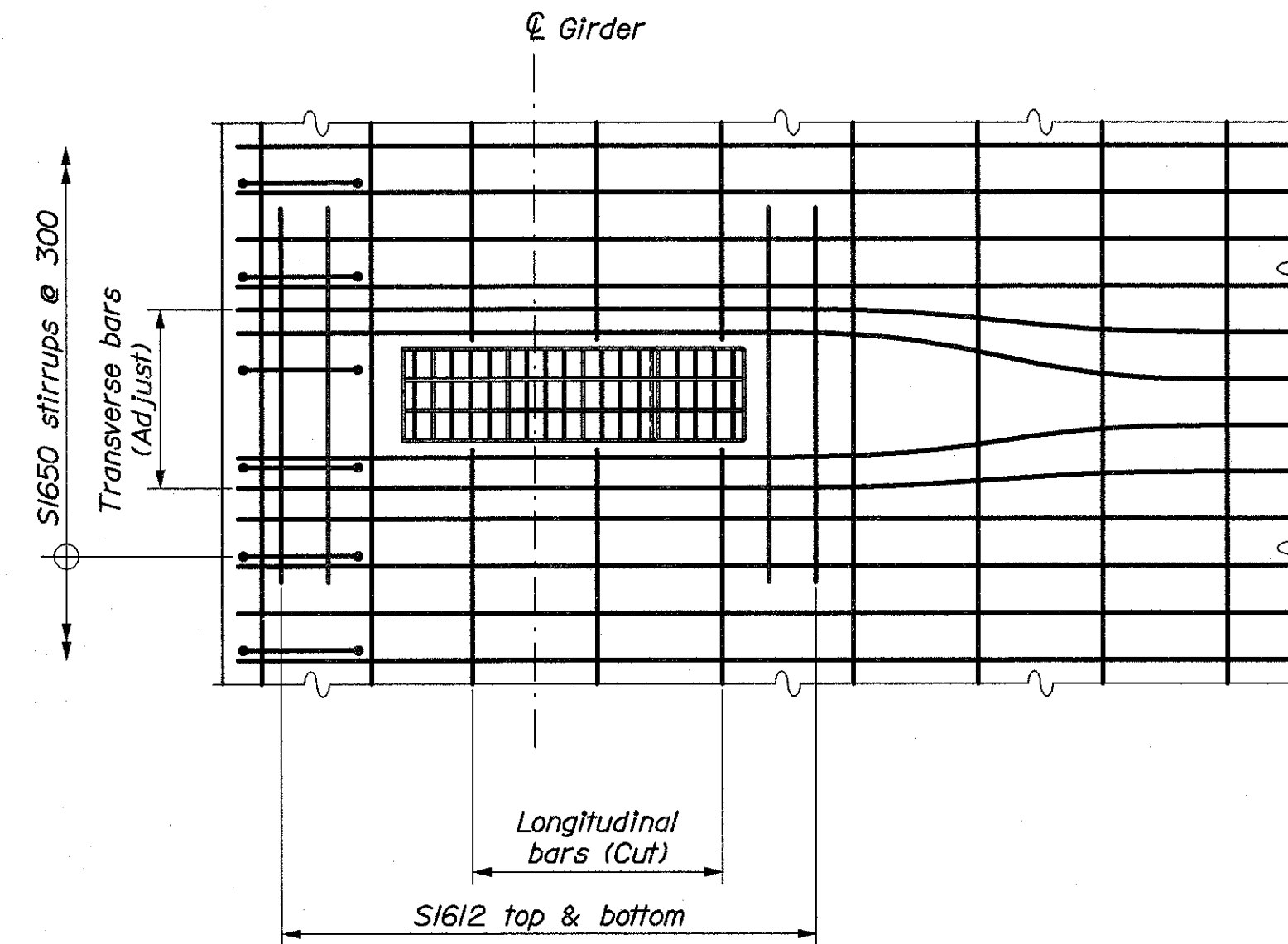
1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F-111WA<br>RES. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|---------------------|-------|----------------|--------------|-----------------|
| 1                   | MAINE | NH-0556(220)E  | 90           | 105             |

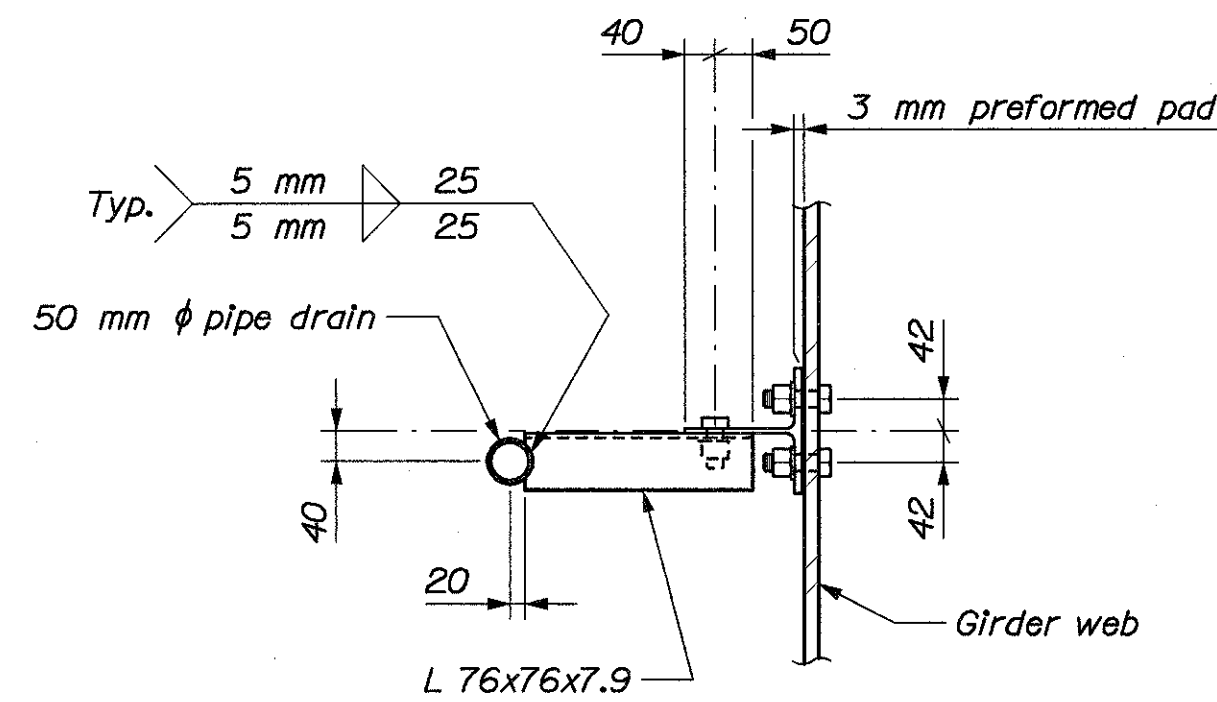
000556.22

BRIDGE DRAIN NOTES

1. Bridge and pipe drains shall meet the requirements of Subsection 711.04 of the Standard Specifications except as noted.
2. Locate pipe drains at both curbs at the Abutment No. 2 end of the deck slab, placed so as to clear bridge seats by a minimum of 300 mm.
3. Grating shall be a commercial heavy-duty grating with 38x8 mm bearing bars and 10 mm  $\phi$  cross bars.
4. Preformed pads shall meet the requirements of Subsection 703.13 of the Standard Specifications.
5. The WT 155x19.5 girder attachment shall conform to ASTM A 709/A 709M, Grade 250 and shall be cleaned and galvanized to the same specifications as the remainder of the bridge drain.
6. Bolted connections shall be made using M22 galvanized H.S. bolts. To facilitate erection, holes shall be slotted vertically in the "WT" girder attachment and horizontally in the "C" and "L" stand-off brackets. Plate washers shall be provided for both sides of slotted hole connections.
7. Payment for all bridge drains, including pipe drains, will be in accordance with Subsection 502.19 of the Supplemental Specifications.



SLAB REINFORCING AT DRAIN



SECTION B-B

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
BRIDGE DRAINS

SHEET OF AUGUSTA, MAINE

Date: 26 NOV 2002

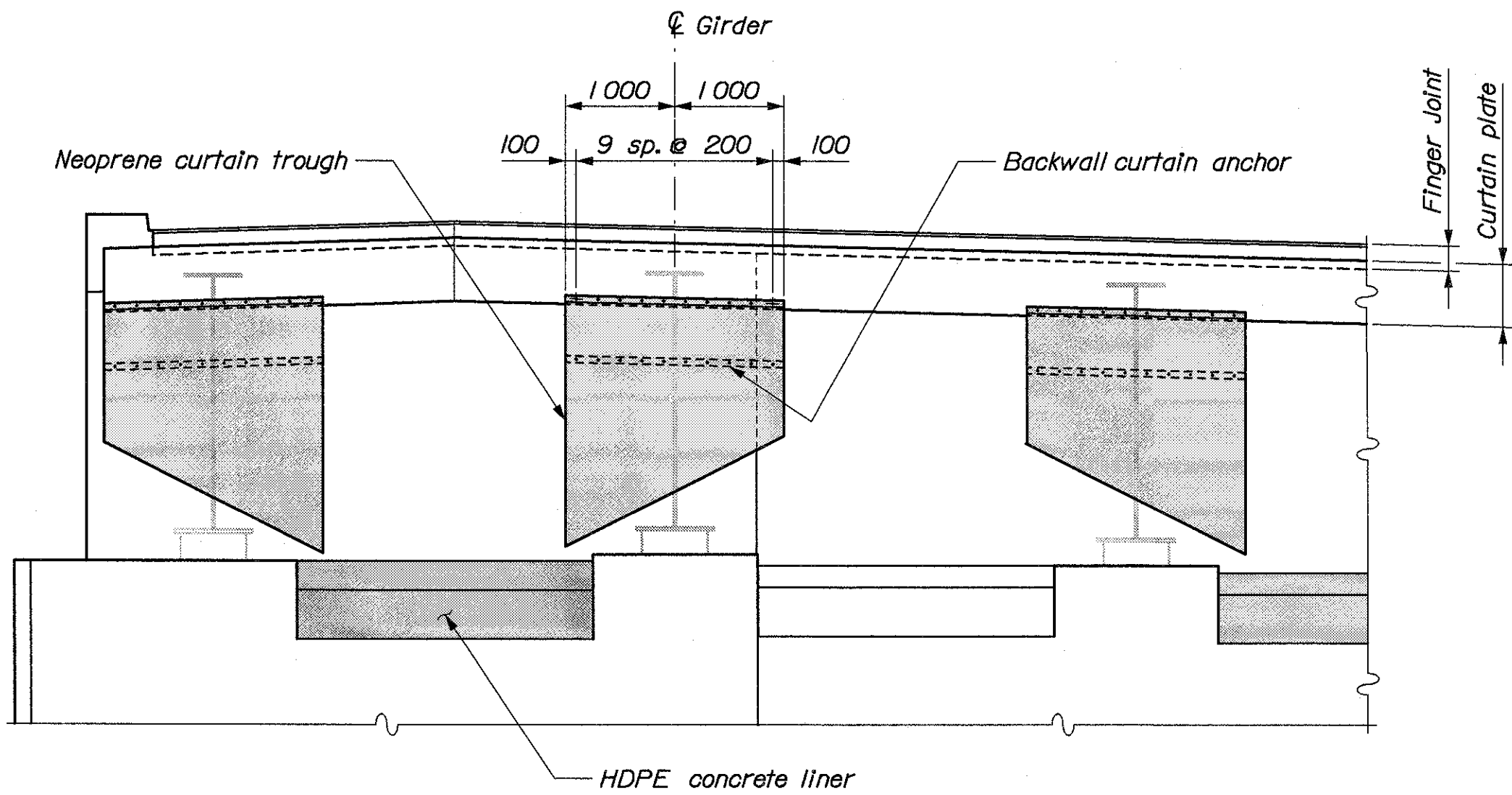
Username: Dana.Darren

Division: BRIDGE

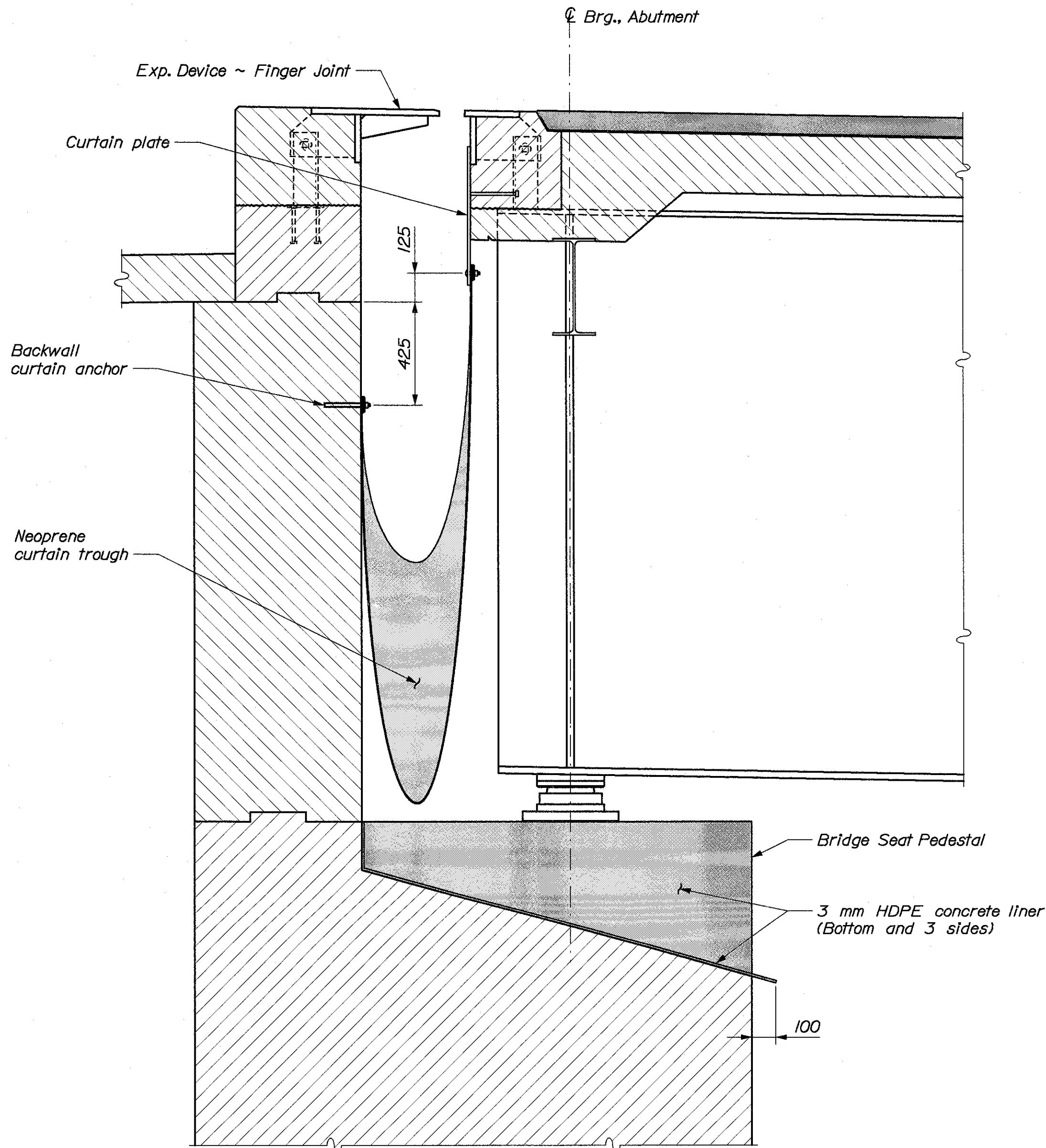
Filename: ... \091\_curtain details.dgn

| PROJECT DESIGN ENGINEER | BY           | DATE |
|-------------------------|--------------|------|
| DESIGN-DETAILED         | ADN / DMD    |      |
| CHECKED                 | J. J. Folsom |      |
| REVISIONS               |              |      |
| FIELD CHANGES           |              |      |

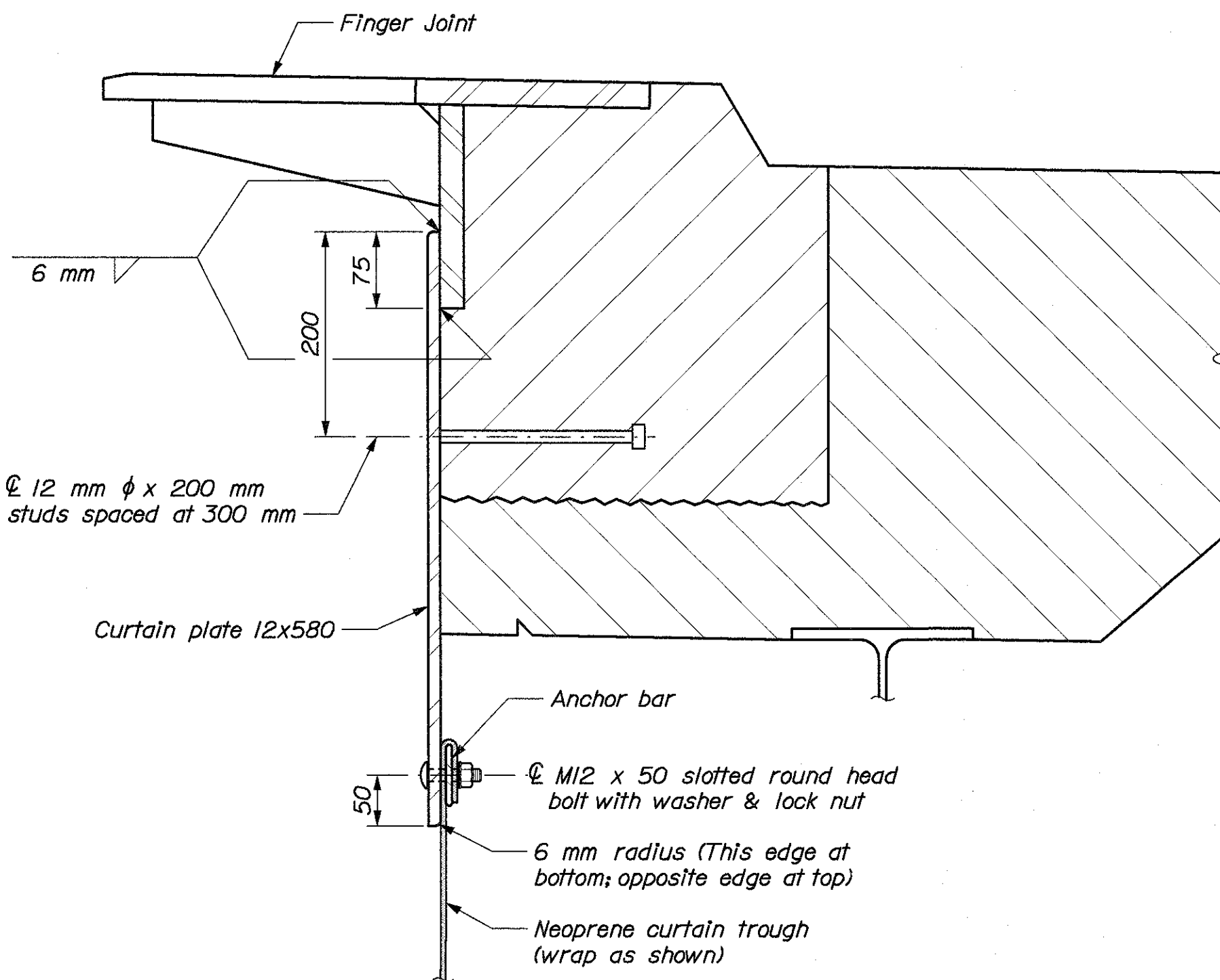
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| PLANS |
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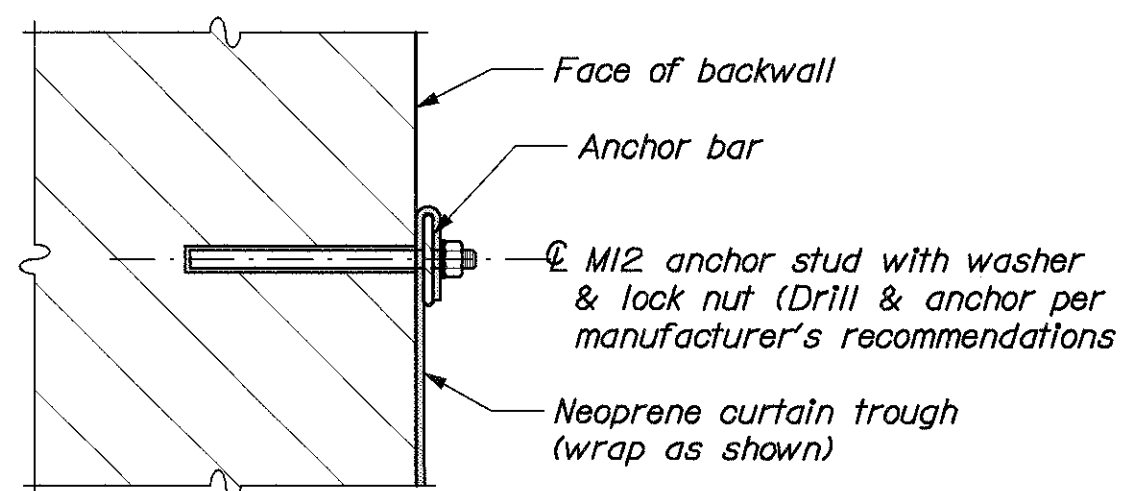
CURTAIN TROUGH ELEVATION



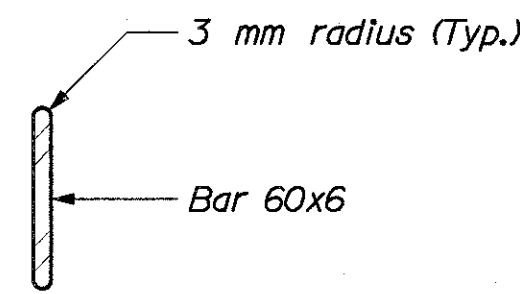
SECTION AT EXPANSION JOINT



CURTAIN PLATE DETAIL



CURTAIN ANCHOR DETAIL



ANCHOR BAR

METRIC

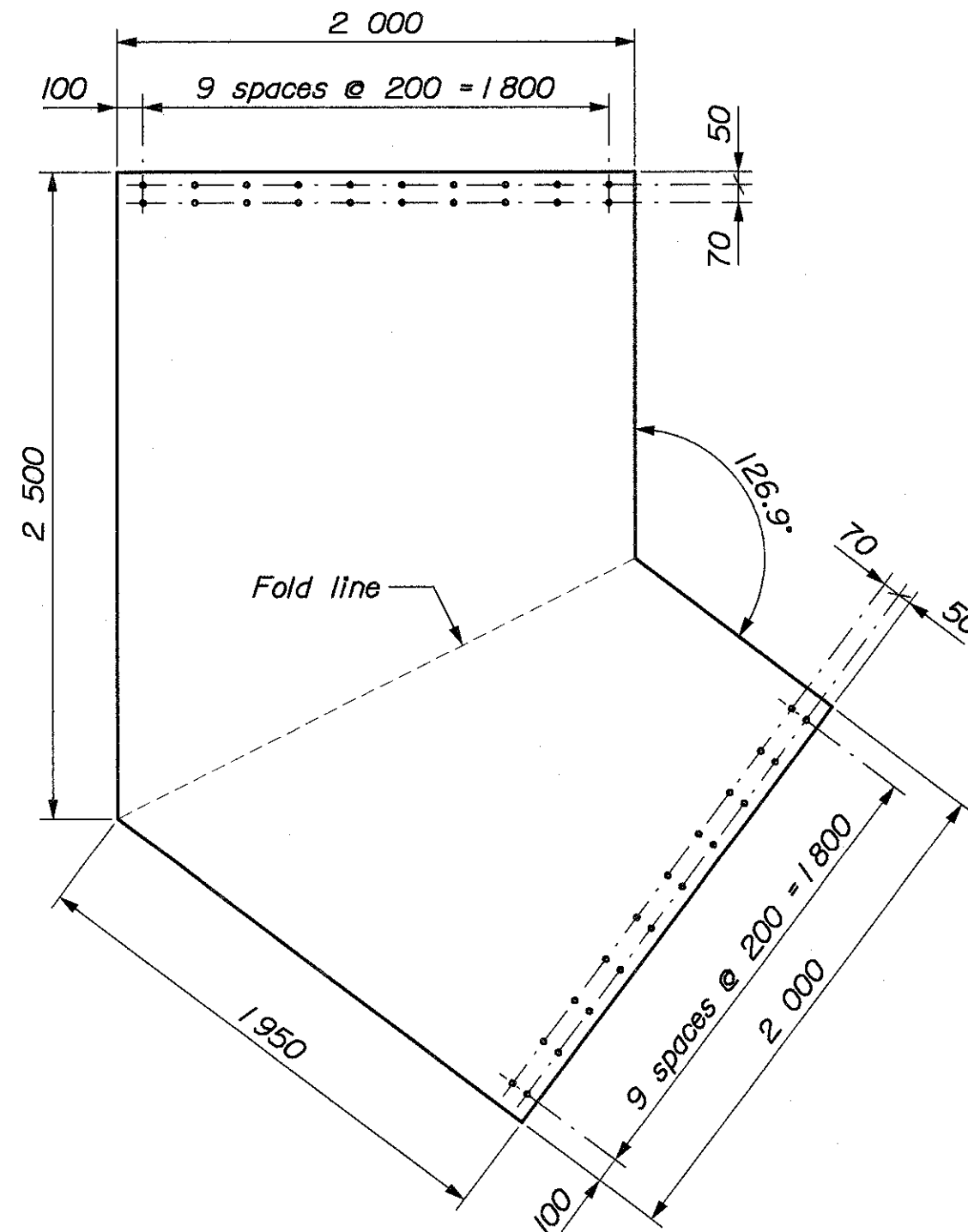
1. All dimensions are in millimeters unless otherwise noted.
2. All elevations and stations are in meters.

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 91        | 105          |

000556.22

CURTAIN TROUGH NOTES

1. HDPE liners shall be installed between alternate pairs of bridge seat pedestals (three at each abutment). Sheets shall be extruded with a large number of anchor studs. All joints shall be sealed by thermal welding or other methods as approved by the Engineer. Payment for HDPE liners will be considered incidental to the expansion joint pay items.
2. Fabrication and materials for the curtain trough, including galvanization of steel components, shall be in accordance with the provisions of Standard Specifications Section 521, Finger Joint and Fabric Trough.
3. The neoprene sheets shall be fabricated in one piece with no seams.
4. Curtain troughs shall be installed no less than two days after the placement of concrete.
5. Payment for curtain plates will be considered incidental to the respective Expansion Device - Finger Joint pay item.
6. Payment for one set of six (6) curtain troughs per each, including anchor bars and hardware, will be made under Item No. 521.32, Fabric Trough for Finger Joint.



NEOPRENE SHEET

Cut top edges and position holes to follow appropriate cross - slope

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
CURTAIN TROUGHS

SHEET OF AUGUSTA, MAINE

Date:26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ... \092\_sup\_lighting.dgn

| PROJECT DESIGN ENGINEER | BY        | DATE |
|-------------------------|-----------|------|
| DESIGN-DETAILED         | D. Damren |      |
| CHECKED                 | J. Folsom |      |
| REVISIONS               |           |      |
| FIELD CHANGES           |           |      |

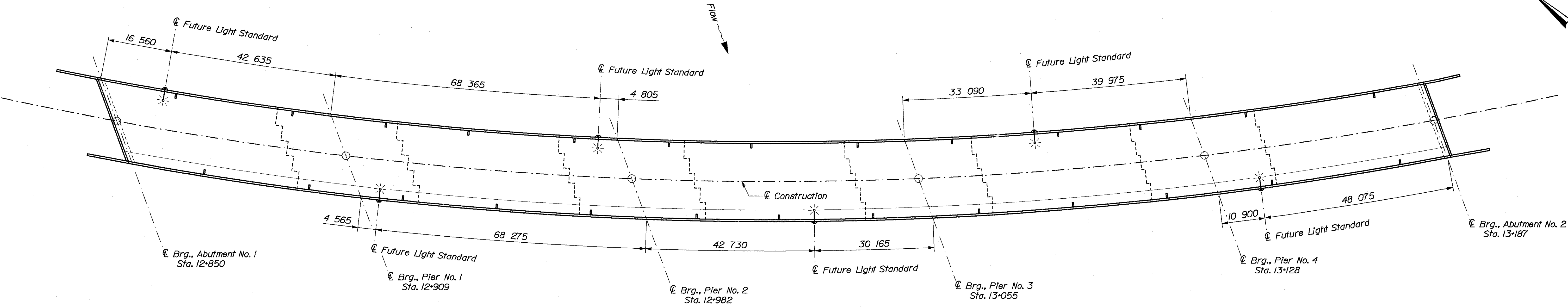
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|       |

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 92           | 105             |

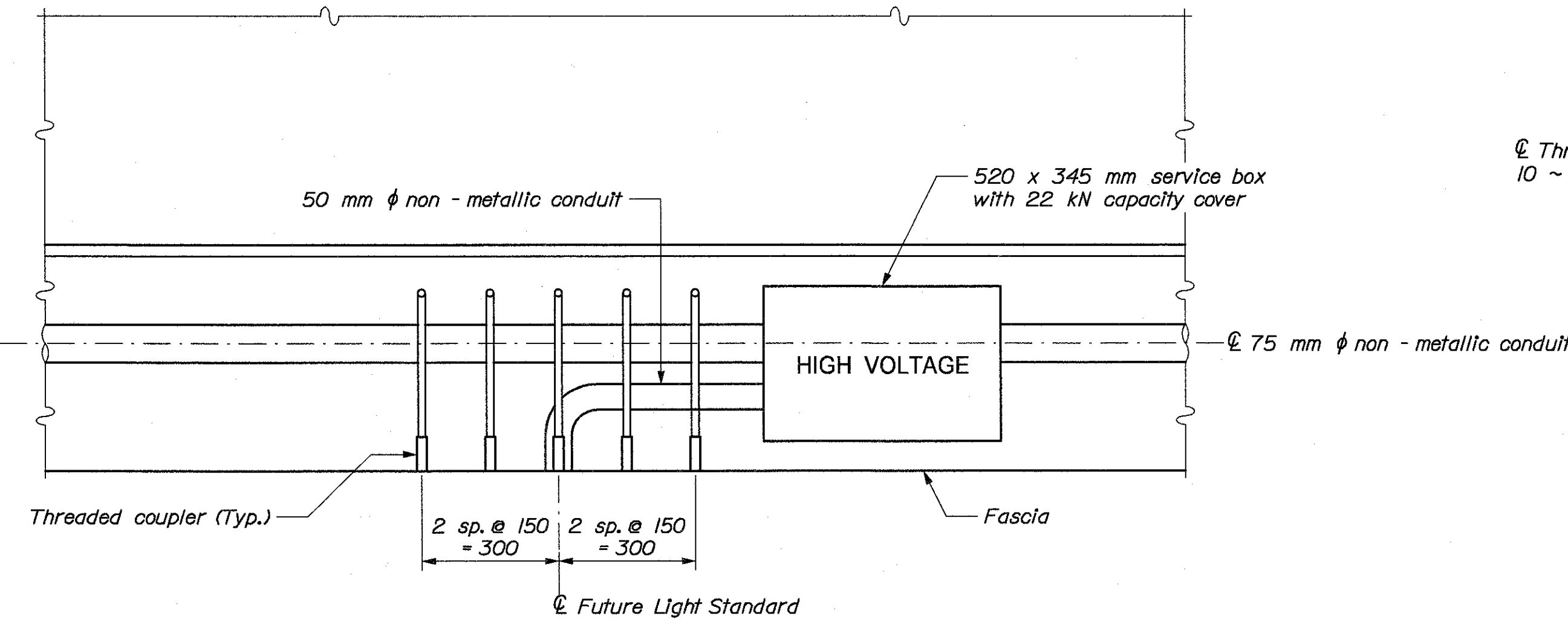
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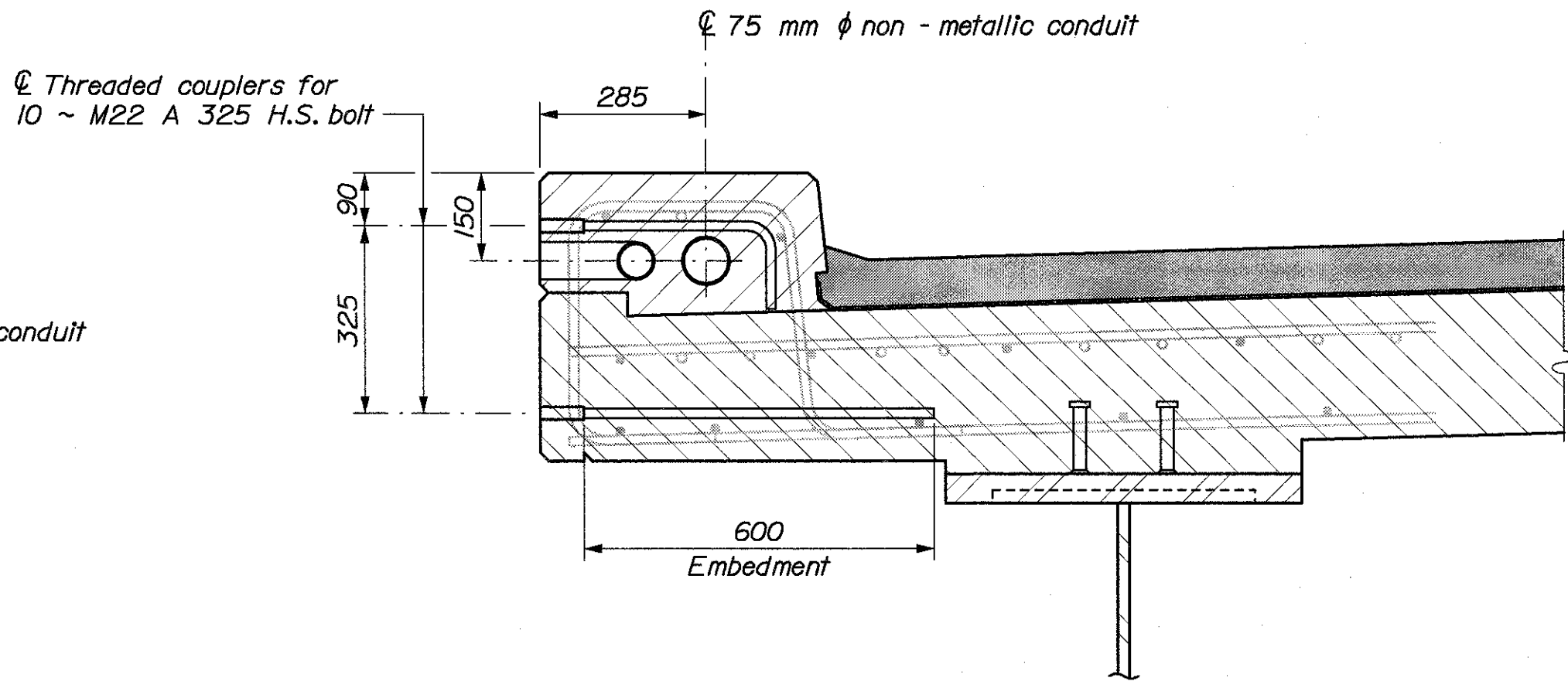
LIGHTING PLAN

BRIDGE LIGHTING NOTES

1. Bridge lighting anchorages are located so as to be positioned midway between adjacent rail posts.
2. Provide a waterproof expansion coupling in the conduit every three (3) meters.
3. Install a service box in each curb near the Abutment No. 2 end of the superstructure.
4. Provision shall be made to accommodate movement between the superstructure and Abutment No. 2 and to drain the conduit as approved by the Engineer.



LIGHT ANCHORAGE PLAN



LIGHT ANCHORAGE SECTION

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

**LIGHTING DETAILS**

SHEET OF AUGUSTA, MAINE

Date:26 NOV 2002

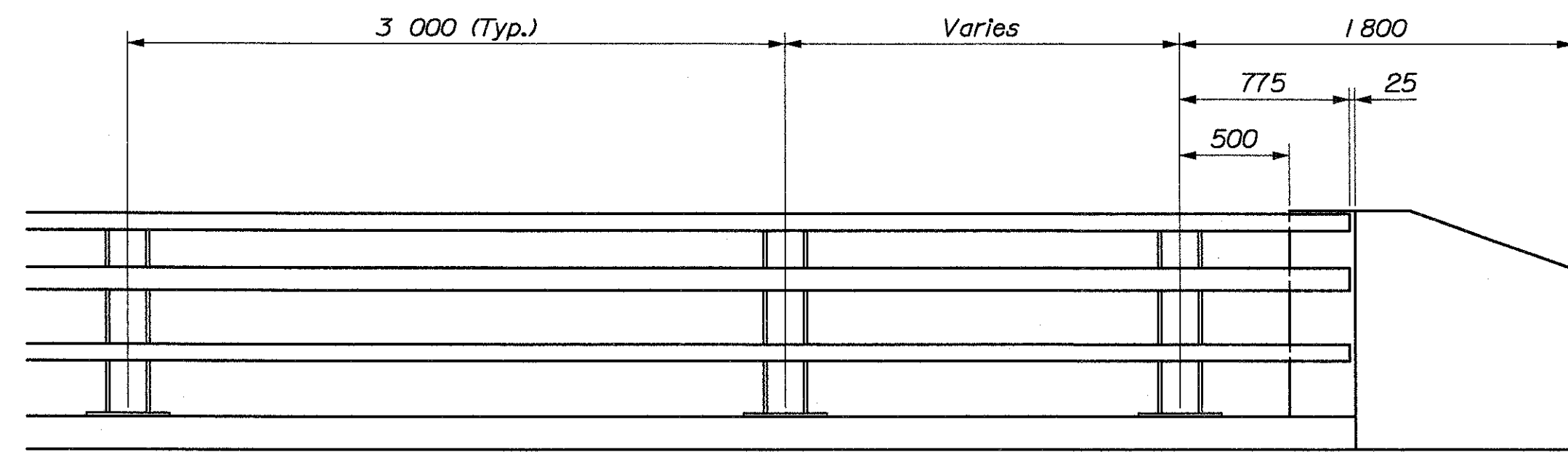
Username: Dana.Damren

Division: BRIDGE

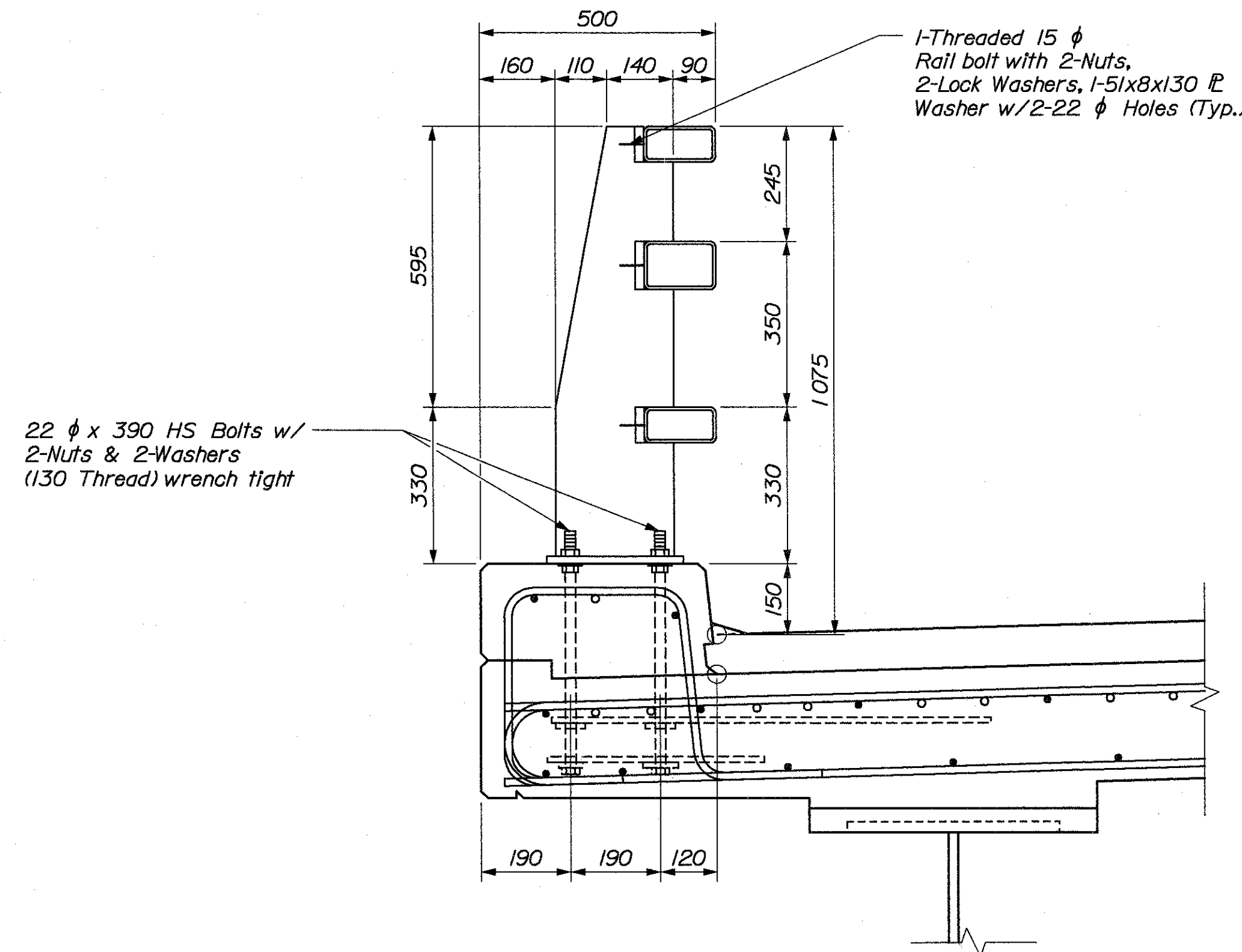
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| PROJECT DESIGN ENGINEER | BY         | DATE |
|-------------------------|------------|------|
| DESIGN-DETAILED         | B. Nichols |      |
| CHECKED                 | J. Folsom  |      |
| REVISIONS               |            |      |
| FIELD CHANGES           |            |      |

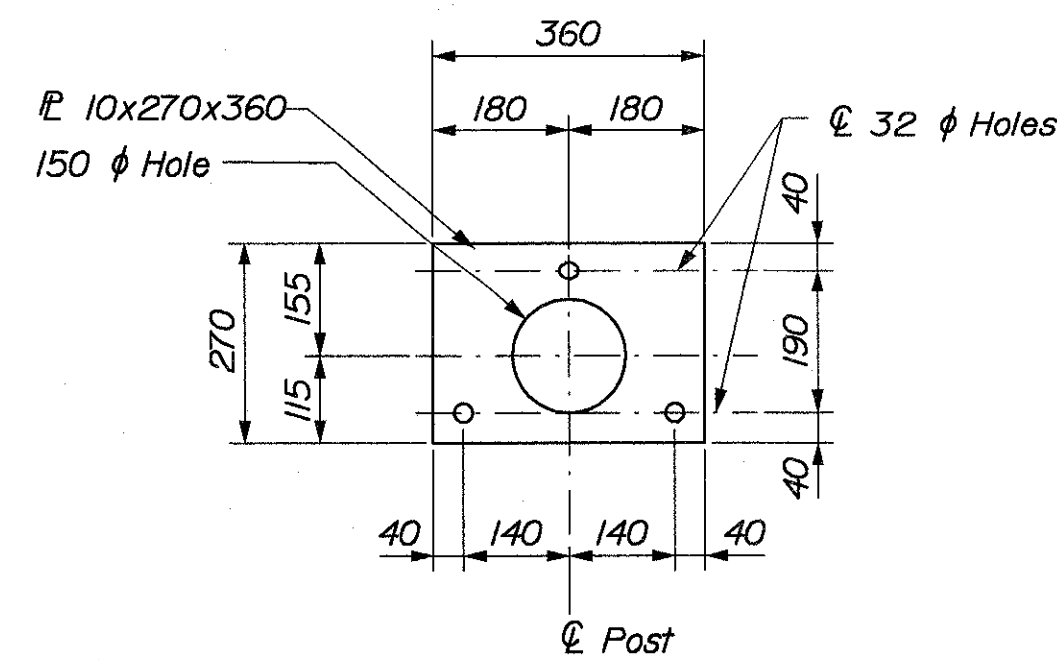
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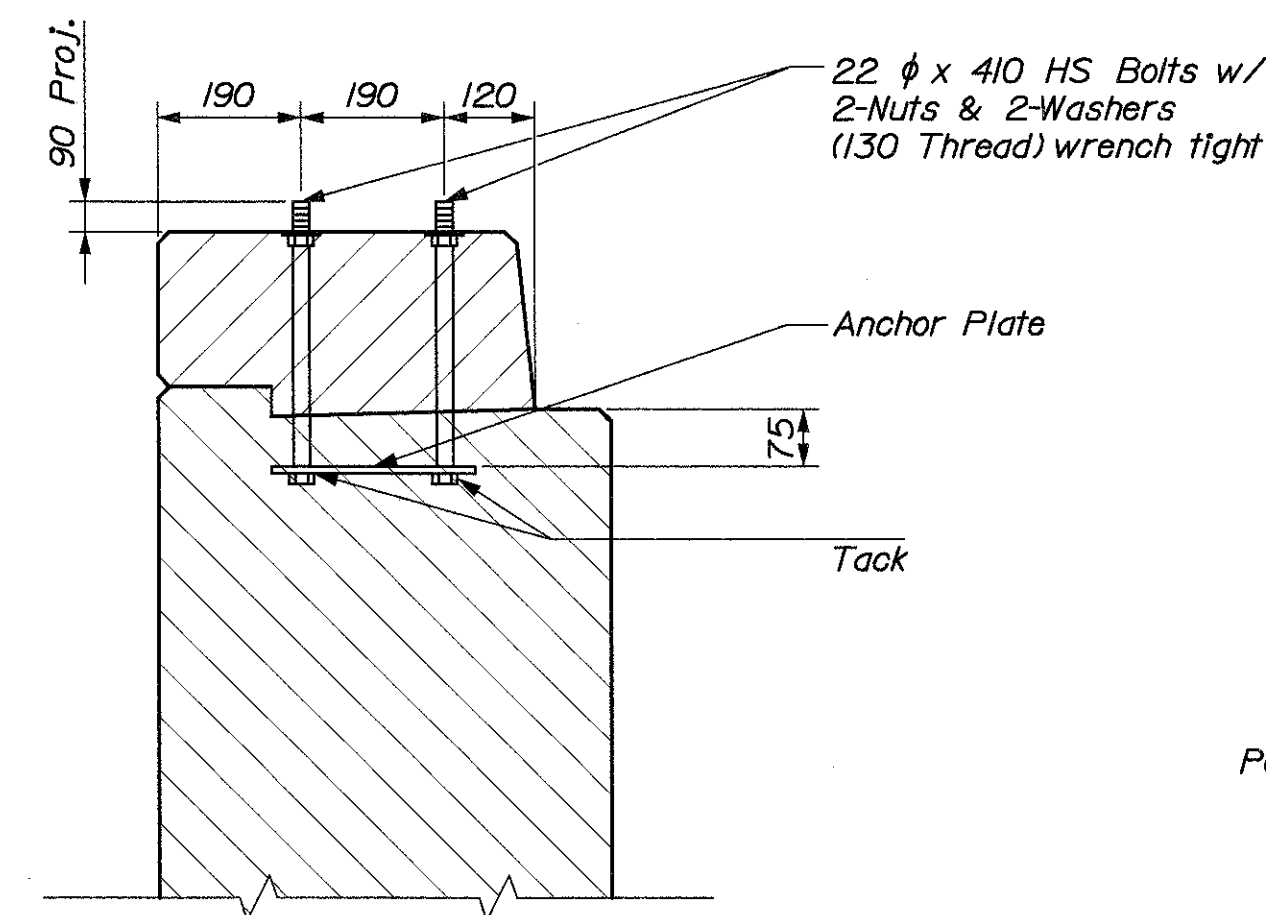
RAIL ELEVATION



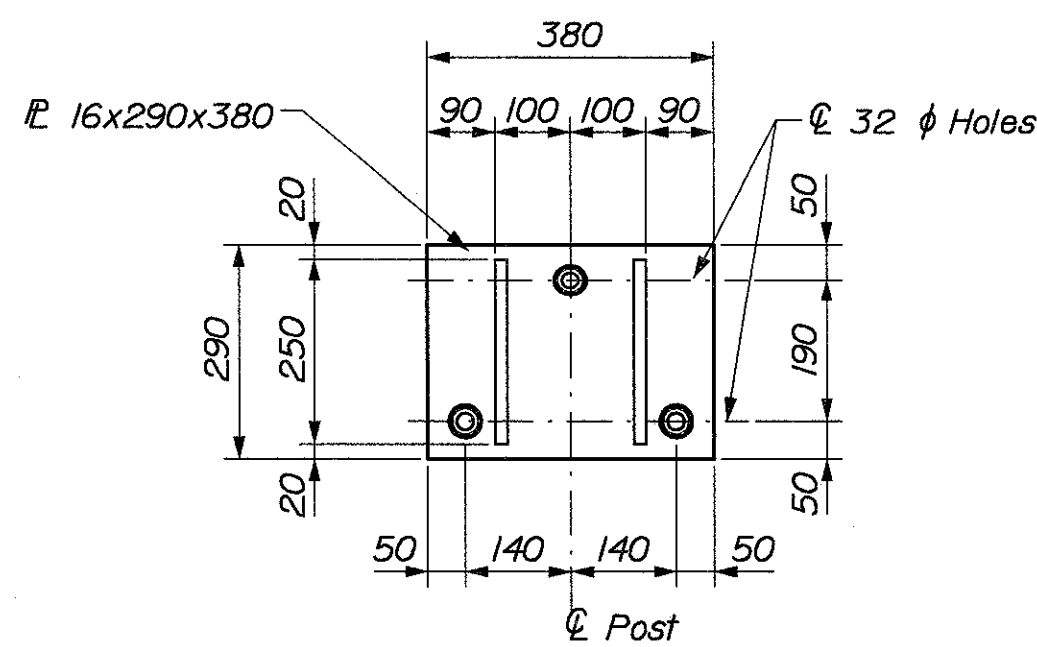
TYPICAL RAIL SECTION



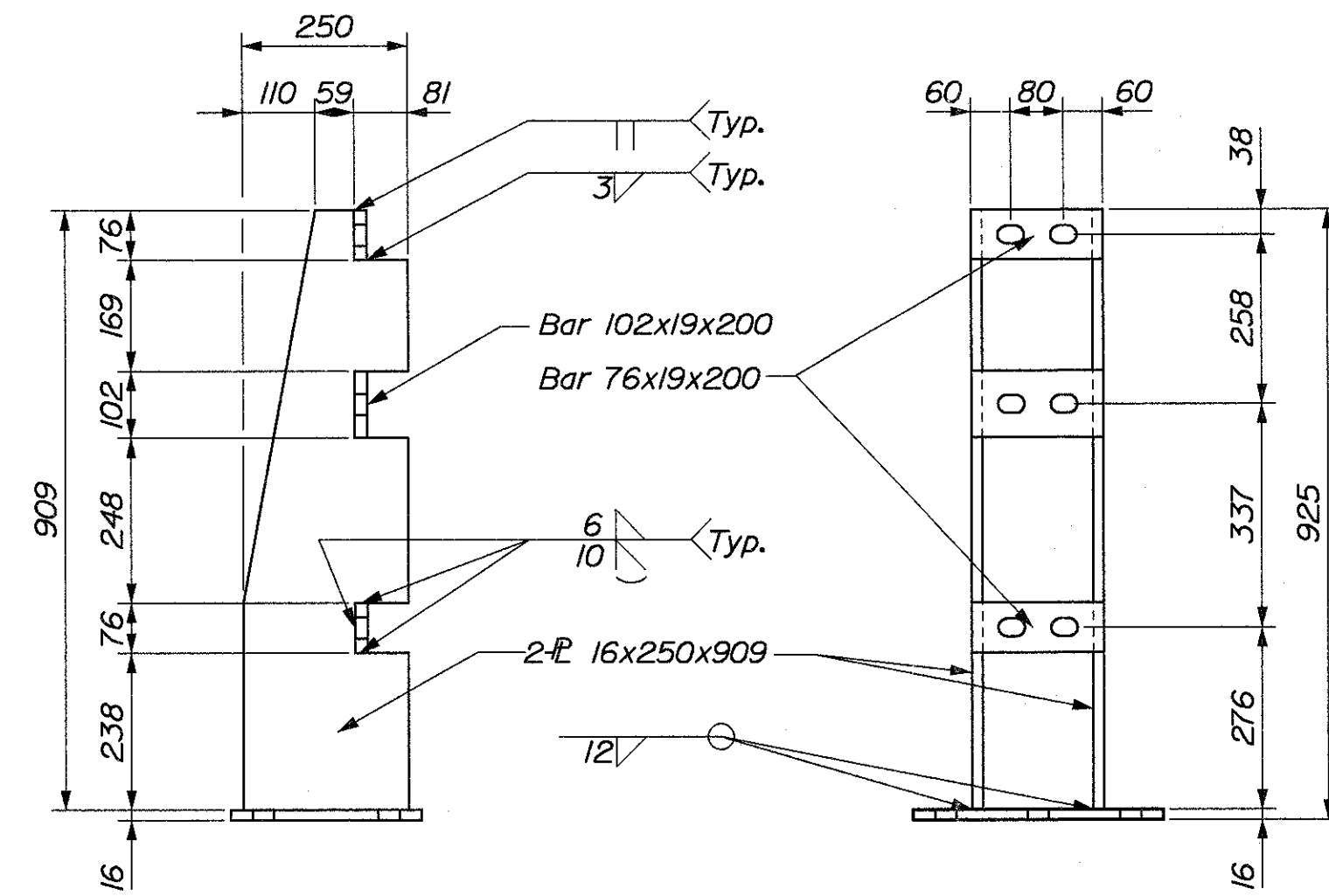
RETURN WING ANCHOR PLATE



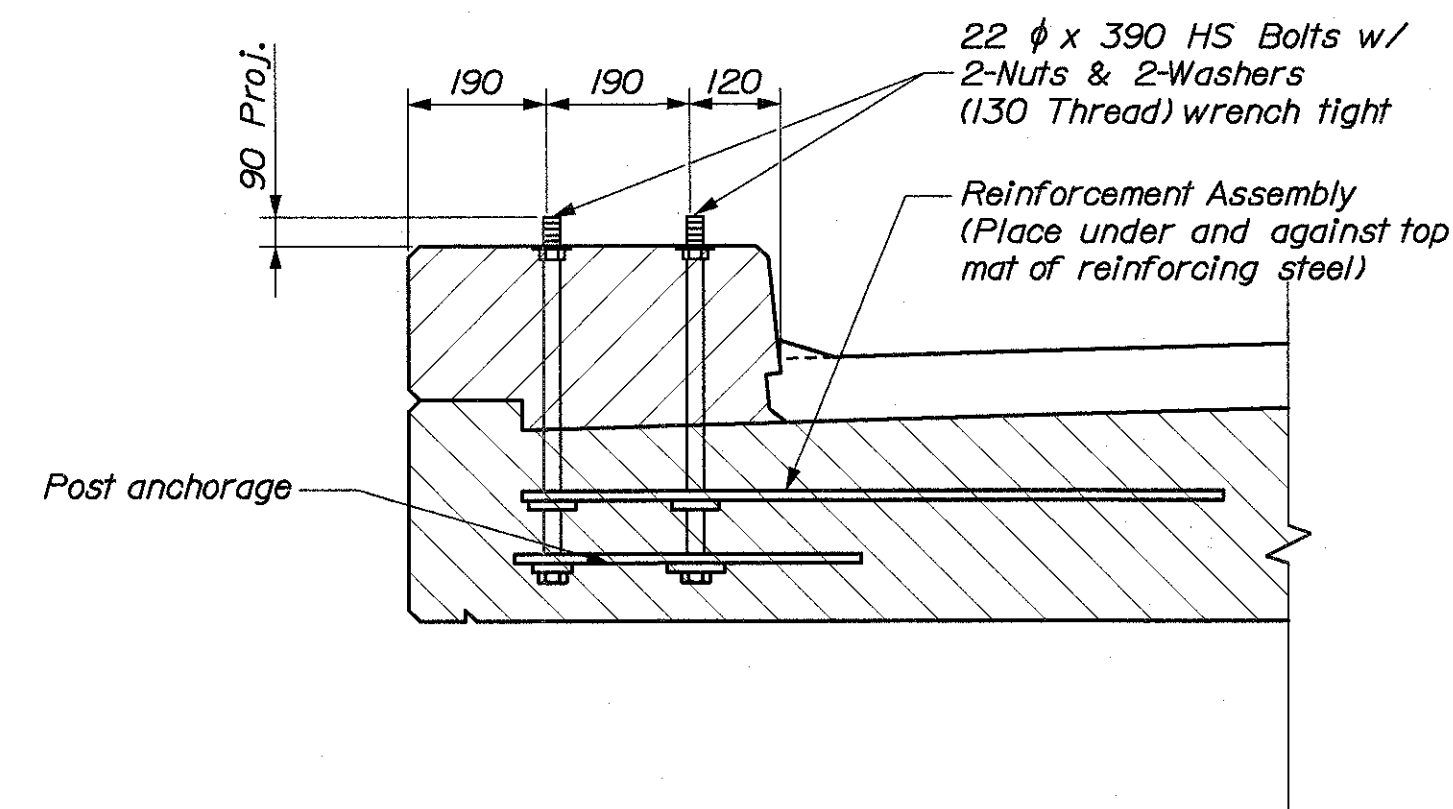
POST ANCHORAGE AT RETURN WING



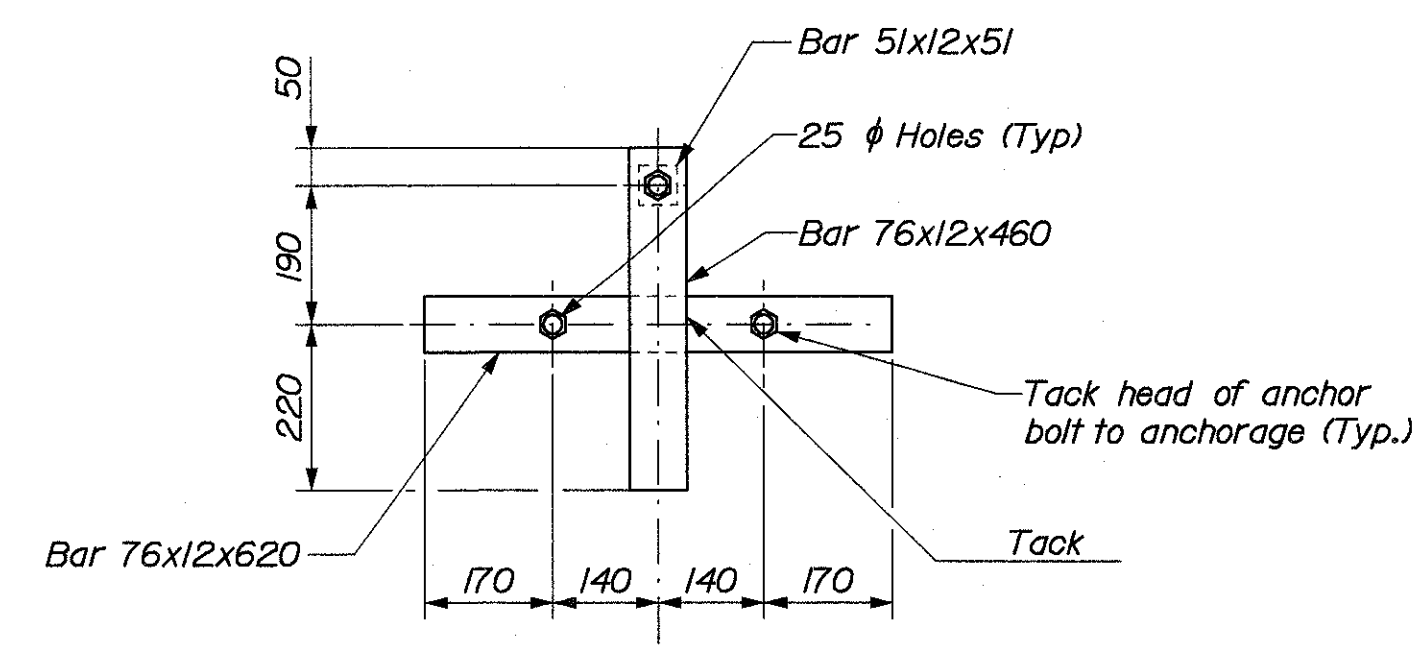
POST BASE



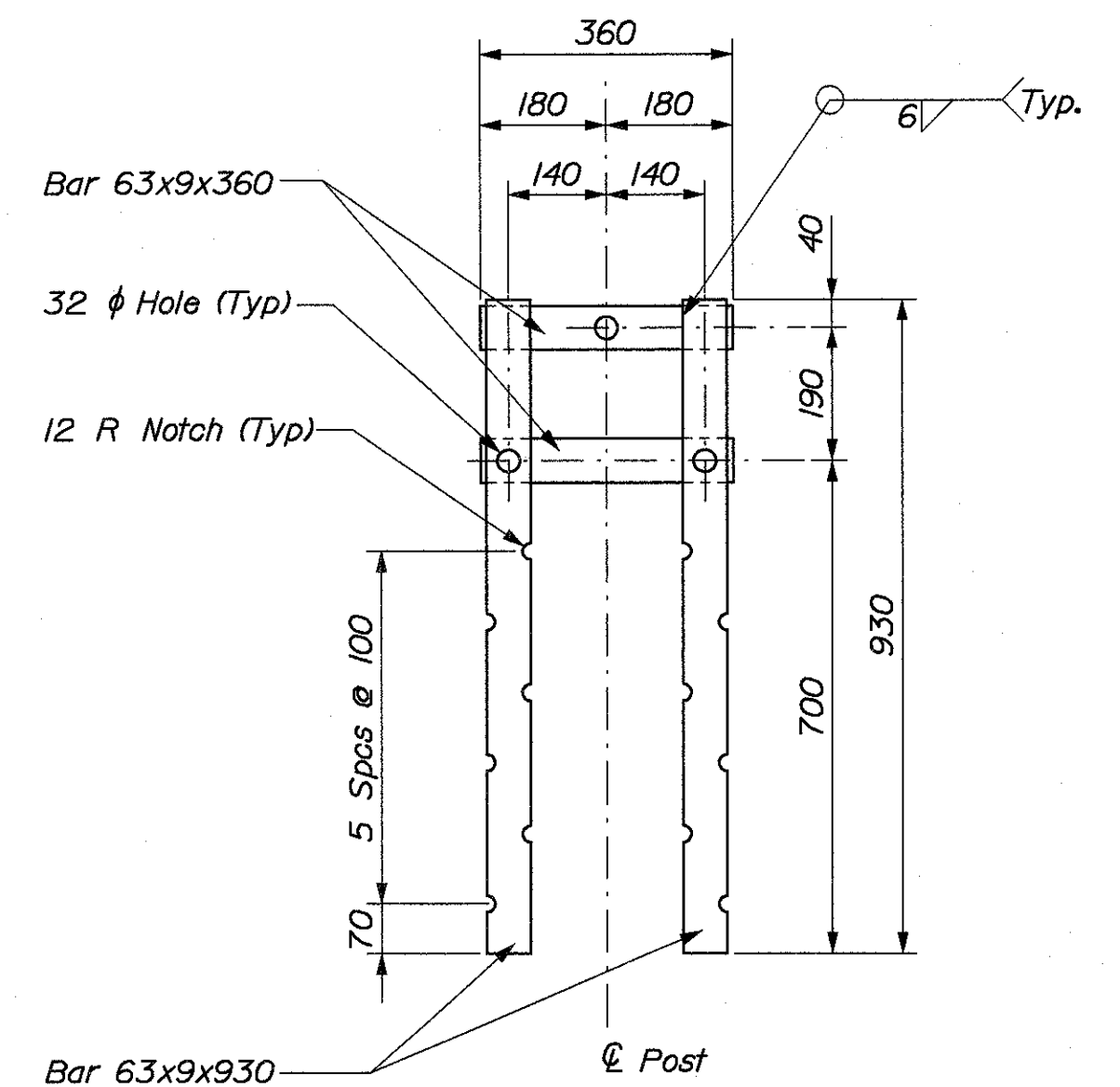
POST DETAILS



CURB POST ANCHORAGE



POST ANCHORAGE



REINFORCEMENT ASSEMBLY

**METRIC** 1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 93        | 105          |

000556.22

**BRIDGE RAIL NOTES**

1. Venting and pick-up holes in rails and sleeves shall be shown on Fabricator's shop plans.
2. Anchor bolts may be tack welded to anchorage.
3. Post base plates shall be flat after fabrication.
4. Rails shall not be shop spliced.
5. Rails shall be shop or field drilled 32φ to receive rail bolts.
6. After installation of rail, the exposed rail bolt threads shall be painted with two coats of Zinc-rich paint.
7. Slots may be omitted in standard sleeves where bolts are required for one side of splice only.
8. All testing of post to base welds shall conform to AWS D1.5, article 6.7.2.
9. All exposed cut or sheared edges shall be rounded and free of burrs.
10. Rail post shall be set normal to grade.
11. Lengths of rail shall be attached to a minimum of two (2) rail posts and to at least four (4) whenever possible.
12. Rail bar expansion joints shall be provided in any rail bay spanning a superstructure expansion joint. Expansion joint width shall be 100 mm at 7 degrees Celsius and will be adjusted in the field by the Engineer.
13. All parts except the post anchorage, the reinforcement assembly, and the anchor plate shall be galvanized after fabrication.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

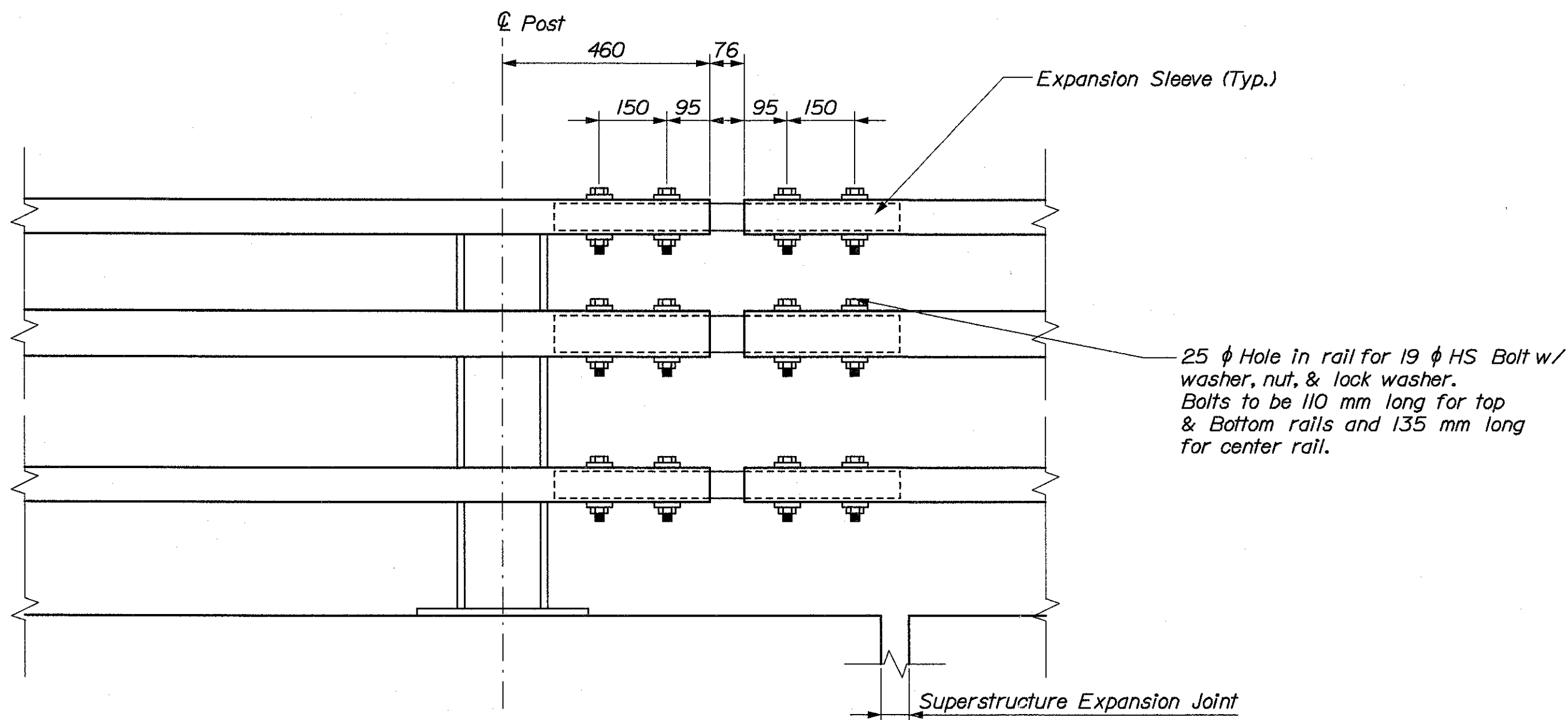
**BRIDGE RAIL**

SHEET OF AUGUSTA, MAINE

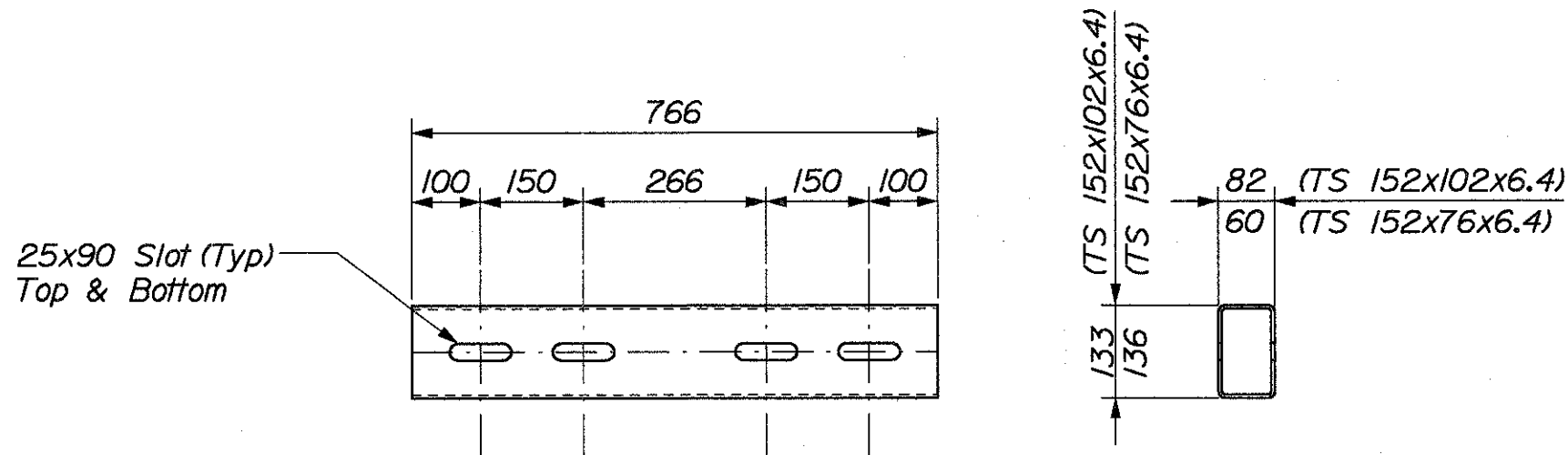
| PROJECT DESIGN ENGINEER | BY           | DATE |
|-------------------------|--------------|------|
| DESIGN-DETAILED         | B. Nichols   |      |
| CHECKED                 | J. F. Osborn |      |
| REVISIONS               |              |      |
| FIELD CHANGES           |              |      |
| PLANS                   |              |      |

| METRIC | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. | F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEET IS |
|--------|---------------------------------------------------------------------------------------------------------------|----------------------|-------|----------------|--------------|-------------------|
|        |                                                                                                               | 1                    | MAINE | NH-0556(220)E  | 94           | 105               |

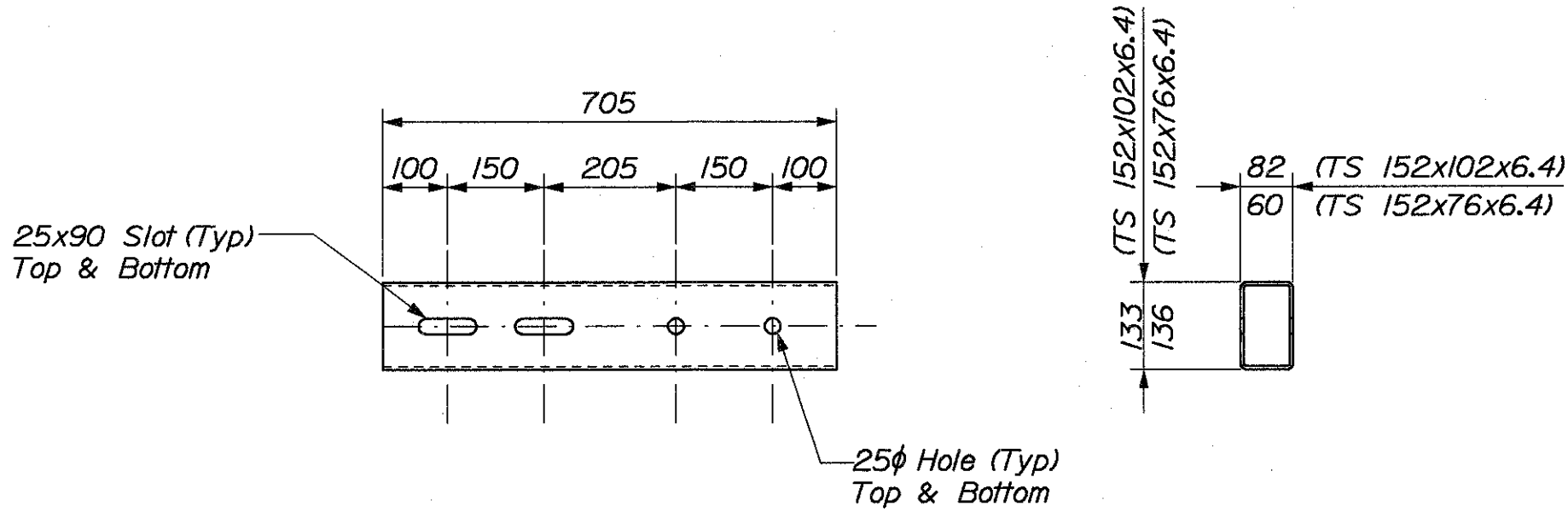
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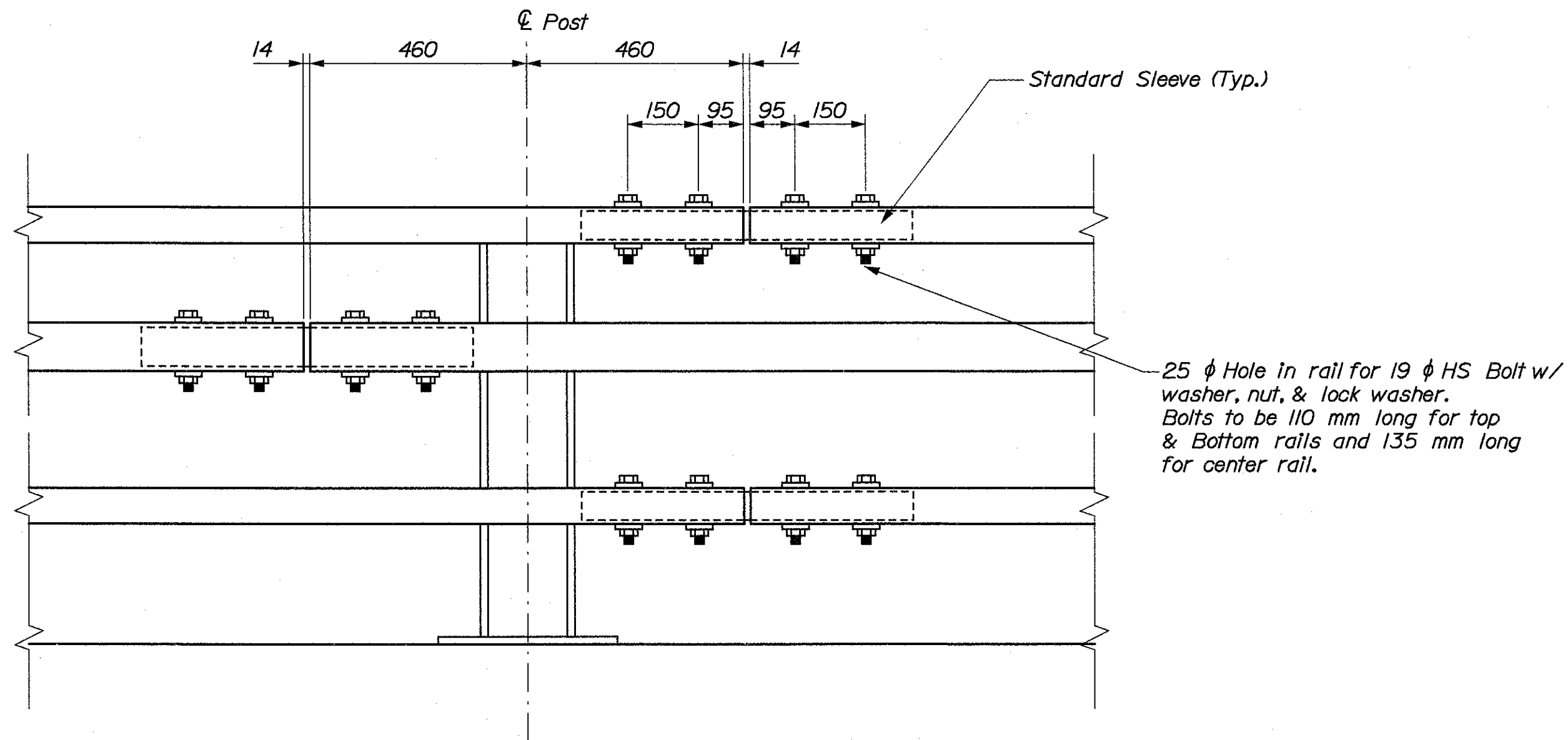
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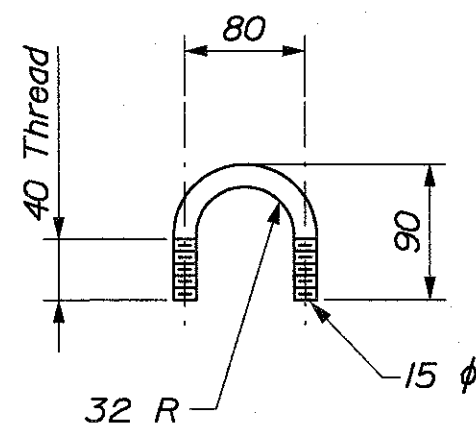
EXPANSION SPLICE



STANDARD SPLICE



STANDARD SPLICE



RAIL BOLT DETAIL

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**BRIDGE RAIL**

Date:26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ...095\_rebar\_abutments.dgn

REINFORCING STEEL SCHEDULE

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

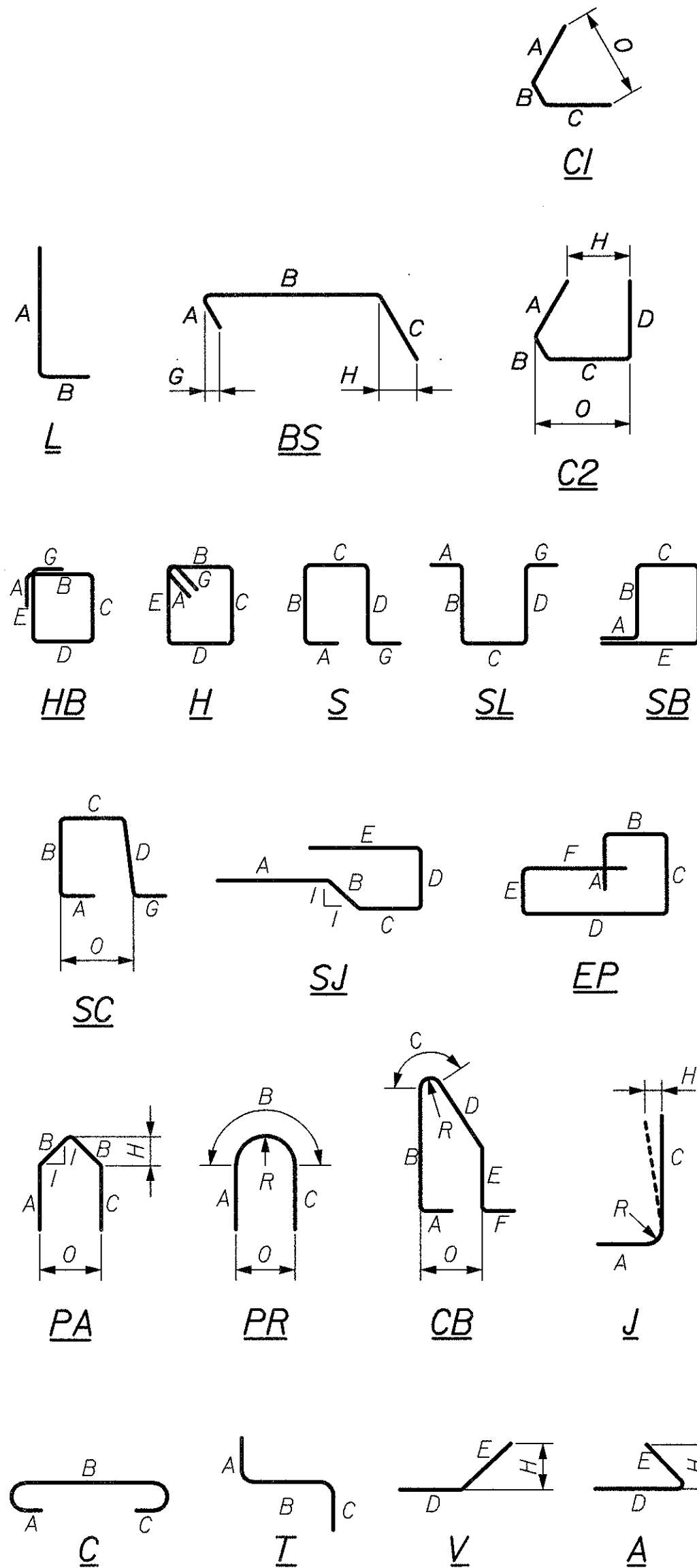
| F.H.W.A.<br>REF. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|-----------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 95        | 105             |

000556.22

| STRAIGHT BARS                  |      |        |                                    |                                |      |        |                                    |
|--------------------------------|------|--------|------------------------------------|--------------------------------|------|--------|------------------------------------|
| MARK                           | QTY. | LENGTH | LOCATION                           | MARK                           | QTY. | LENGTH | LOCATION                           |
| ABUTMENT NO. 1                 |      |        |                                    | ABUTMENT NO. 2                 |      |        |                                    |
| A/600                          | 75   | 3 500  | Footing - Breastwall               | B/600                          | 69   | 3 550  | Footing - Breastwall               |
| A/601                          | 10   | 3 000  | Footing - Breastwall               | B/601                          | 4    | 3 000  | Footing - Breastwall               |
| A/900                          | 24   | 18 300 | Footing - Breastwall               | B/900                          | 24   | 18 300 | Footing - Breastwall               |
| A/901                          | 24   | 7 000  | Footing - Breastwall               | B/901                          | 24   | 3 800  | Footing - Breastwall               |
| A2200                          | 75   | 3 500  | Footing - Breastwall               | B2200                          | 69   | 3 550  | Footing - Breastwall               |
| A2201                          | 10   | 3 500  | Footing - Breastwall               | B2201                          | 4    | 3 000  | Footing - Breastwall               |
|                                |      |        |                                    |                                |      |        |                                    |
| A/902                          | 14   | 9 800  | Footing - North Wingwall           | B/902                          | 14   | 8 500  | Footing - North Wingwall           |
| A/903                          | 12   | 10 900 | Footing - North Wingwall           | B/903                          | 12   | 8 200  | Footing - North Wingwall           |
| A/904                          | 14   | 9 500  | Footing - South Wingwall           | B/904                          | 14   | 9 300  | Footing - South Wingwall           |
| A/905                          | 14   | 8 300  | Footing - South Wingwall           | B/905                          | 12   | 9 700  | Footing - South Wingwall           |
| A2202                          | 29   | 3 750  | Footing - North Wingwall           | B2202                          | 24   | 3 850  | Footing - North Wingwall           |
| A2203                          | 25   | 3 850  | Footing - South Wingwall           | B2203                          | 28   | 3 850  | Footing - South Wingwall           |
| A2204                          | 8    | 3 000  | Footing - Wingwalls                | B2204                          | 4    | 3 000  | Footing - Wingwalls                |
| A2500                          | 57   | 3 750  | Footing - North Wingwall           | B2500                          | 47   | 3 850  | Footing - North Wingwall           |
| A2501                          | 49   | 3 850  | Footing - South Wingwall           | B2501                          | 55   | 3 850  | Footing - South Wingwall           |
| A2502                          | 14   | 3 000  | Footing - Wingwalls                | B2502                          | 6    | 3 000  | Footing - Wingwalls                |
|                                |      |        |                                    |                                |      |        |                                    |
| A/610                          | 158  | 850    | Vertical - Breastwall & Wingwalls  | B/610                          | 144  | 850    | Vertical - Breastwall & Wingwalls  |
| A/611                          | 77   | 2 800  | Vertical - Breastwall              | B/611                          | 68   | 2 900  | Vertical - Breastwall              |
| A/612                          | 74   | 1 200  | Vertical - Backwall                | B/612                          | 65   | 1 200  | Vertical - Backwall                |
| A/620                          | 47   | 11 050 | Horizontal - Breastwall & Backwall | B/620                          | 47   | 9 450  | Horizontal - Breastwall & Backwall |
| A/621                          | 10   | 5 650  | Horizontal - Breastwall            | B/621                          | 11   | 5 400  | Horizontal - Breastwall            |
| A/622                          | 11   | 6 100  | Horizontal - Breastwall            | B/622                          | 12   | 5 225  | Horizontal - Breastwall            |
| A/623                          | 20   | 6 225  | Horizontal - Breastwall            | B/623                          | 21   | 4 625  | Horizontal - Breastwall            |
| A/624                          | 21   | 4 700  | Horizontal - Breastwall            | B/624                          | 22   | 5 075  | Horizontal - Breastwall            |
| A/625                          | 12   | 6 500  | Horizontal - Backwall              | B/625                          | 12   | 5 175  | Horizontal - Backwall              |
| A/626                          | 12   | 5 500  | Horizontal - Backwall              | B/626                          | 12   | 5 450  | Horizontal - Backwall              |
| A/627                          | 3    | 6 250  | Horizontal - Backwall              | B/627                          | 3    | 4 650  | Horizontal - Backwall              |
| A/628                          | 3    | 4 900  | Horizontal - Backwall              | B/628                          | 3    | 5 050  | Horizontal - Backwall              |
| A/629                          | 20   | 2 650  | Top of Breastwall                  | B/629                          | 20   | 2 050  | Top of Breastwall                  |
|                                |      |        |                                    |                                |      |        |                                    |
| A/613                          | 5    | 3 450  | Vertical - North Wingwall          | B/613                          | 5    | 3 675  | Vertical - North Wingwall          |
| A/614                          | 5    | 3 650  | Vertical - South Wingwall          | B/614                          | 5    | 4 000  | Vertical - South Wingwall          |
| A/615                          | 35   | 5 650  | Vertical - North Wingwall          | B/615                          | 32   | 5 650  | Vertical - North Wingwall          |
| A/616                          | 36   | 5 825  | Vertical - South Wingwall          | B/616                          | 34   | 5 975  | Vertical - South Wingwall          |
| A/617                          | 34   | 3 375  | Vertical - North Wingwall          | B/617                          | 29   | 3 325  | Vertical - North Wingwall          |
| A/618                          | 33   | 3 550  | Vertical - South Wingwall          | B/618                          | 32   | 3 650  | Vertical - South Wingwall          |
| A/619                          | 65   | 3 650  | Vertical - Wingwalls               | B/619                          | 59   | 3 650  | Vertical - Wingwalls               |
| A/630                          | 12   | 12 000 | Horizontal - North Wingwall        | B/630                          | 13   | 10 825 | Horizontal - North Wingwall        |
| A/631                          | 13   | 12 025 | Horizontal - South Wingwall        | B/631                          | 14   | 11 425 | Horizontal - South Wingwall        |
| A/632                          | 20   | 10 125 | Horizontal - North Wingwall        | B/632                          | 21   | 8 825  | Horizontal - North Wingwall        |
| A/633                          | 21   | 9 850  | Horizontal - South Wingwall        | B/633                          | 22   | 9 600  | Horizontal - South Wingwall        |
| A/634                          | 11   | 10 375 | Horizontal - North Wingwall        | B/634                          | 11   | 9 350  | Horizontal - North Wingwall        |
| A/635                          | 11   | 10 625 | Horizontal - South Wingwall        | B/635                          | 11   | 9 975  | Horizontal - South Wingwall        |
| A/636                          | 3    | 10 250 | Horizontal - North Wingwall        | B/636                          | 3    | 9 000  | Horizontal - North Wingwall        |
| A/637                          | 3    | 10 075 | Horizontal - South Wingwall        | B/637                          | 3    | 9 750  | Horizontal - South Wingwall        |
| A/640                          | 1    | 9 375  | Horizontal - North Wingwall        | B/640                          | 1    | 8 350  | Horizontal - North Wingwall        |
| A/641                          | 1    | 9 600  | Horizontal - South Wingwall        | B/641                          | 1    | 8 950  | Horizontal - South Wingwall        |
| A/642                          | 1    | 9 650  | Horizontal - North Wingwall        | B/642                          | 1    | 8 300  | Horizontal - North Wingwall        |
| A/643                          | 1    | 9 425  | Horizontal - South Wingwall        | B/643                          | 1    | 9 025  | Horizontal - South Wingwall        |
| APPROACH SLAB - ABUTMENT NO. 1 |      |        |                                    | APPROACH SLAB - ABUTMENT NO. 2 |      |        |                                    |
| AS/600                         | 16   | 12 200 | Transverse                         | BS/600                         | 16   | 12 200 | Transverse                         |
| AS/601                         | 16   | 10 250 | Transverse                         | BS/601                         | 16   | 7 500  | Transverse                         |
| AS/900                         | 147  | 4 600  | Longitudinal                       | BS/900                         | 127  | 4 600  | Longitudinal                       |
|                                |      |        |                                    |                                |      |        |                                    |
|                                |      |        |                                    |                                |      |        |                                    |
|                                |      |        |                                    |                                |      |        |                                    |
|                                |      |        |                                    |                                |      |        |                                    |
|                                |      |        |                                    |                                |      |        |                                    |
| MARK                           | QTY. | LENGTH | LOCATION                           | MARK                           | QTY. | LENGTH | LOCATION                           |

| BENT BARS      |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|----------------|------|--------|------|-------|-------|-------|-------|-----|----|-----|------|-------|----|---------------------------------|
| MARK           | QTY. | LENGTH | TYPE | A     | B     | C     | D     | E   | F  | G   | H    | O     | R  | LOCATION                        |
| ABUTMENT NO. 1 |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
| A/650          | 74   | 2 800  | S    | 0     | 2 125 | 500   | 175   | --  | -- | 0   | --   | --    | -- | Vertical - Backwall             |
| A/651          | 74   | 4 400  | S    | 0     | 1125  | 350   | 2 925 | --  | -- | 0   | --   | --    | -- | Vertical - Backwall             |
| A/652          | 45   | 3 300  | BS   | 200   | 1975  | 1125  | --    | --  | -- | 60  | 340  | --    | -- | Top of Breastwall               |
| A/653          | 28   | 3 850  | S    | 0     | 175   | 1900  | 1775  | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/654          | 8    | 4 400  | S    | 0     | 1525  | 1350  | 1525  | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/655          | 2    | 4 700  | SC   | 0     | 1525  | 1400  | 1775  | --  | -- | 0   | --   | 2 330 | -- | Pedestals                       |
| A/656          | 2    | 5 150  | C2   | 1525  | 225   | 1875  | 1525  | --  | -- | --  | 1215 | 1985  | -- | Pedestals                       |
| A/657          | 20   | 3 200  | S    | 0     | 900   | 1400  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/658          | 1    | 3 400  | S    | 0     | 900   | 1600  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/659          | 1    | 3 575  | S    | 0     | 900   | 1775  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/660          | 1    | 3 750  | S    | 0     | 900   | 1950  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/661          | 1    | 3 925  | S    | 0     | 900   | 2 125 | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/662          | 1    | 3 800  | S    | 0     | 900   | 2 000 | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/663          | 1    | 3 625  | S    | 0     | 900   | 1825  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/664          | 1    | 3 450  | S    | 0     | 900   | 1650  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A/665          | 1    | 3 275  | S    | 0     | 900   | 1475  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| A2250          | 74   | 5 600  | L    | 5 225 | 375   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - Breastwall           |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
| A/670          | 55   | 2 300  | S    | 0     | 1625  | 500   | 175   | --  | -- | 0   | --   | --    | -- | Vertical - Wingwalls            |
| A/671          | 55   | 2 800  | SC   | 0     | 1875  | 350   | 575   | --  | -- | 440 | --   | --    | -- | Vertical - Wingwalls            |
| A/672          | 6    | 3 750  | S    | 0     | 1625  | 500   | 1625  | --  | -- | 0   | --   | --    | -- | Vertical - Wingwalls            |
| A/673          | 4    | 2 575  | S    | 0     | 1200  | 175   | 1200  | --  | -- | 0   | --   | --    | -- | Transition Barriers             |
| A/674          | 12   | 2 775  | S    | 0     | 1200  | 375   | 1200  | --  | -- | 0   | --   | --    | -- | Transition Barriers             |
| A/680          | 46   | 1200   | V    | --    | --    | --    | 600   | 600 | -- | --  | 520  | --    | -- | Horiz. - North Wingwall Corners |
| A/681          | 13   | 1425   | CI   | 600   | 225   | 600   | --    | --  | -- | --  | 840  | --    | -- | Horiz. - South Wingwall Corners |
| A/682          | 35   | 1200   | A    | --    | --    | --    | 600   | 600 | -- | --  | 515  | --    | -- | Horiz. - South Wingwall Corners |
| A2950          | 34   | 4200   | L    | 3725  | 475   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - North Wingwall       |
| A2951          | 33   | 4200   | L    | 3725  | 475   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - South Wingwall       |
| A2952          | 55   | 2700   | L    | 2225  | 475   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - Wingwalls            |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
| ABUTMENT NO. 2 |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
| B/650          | 65   | 2 800  | S    | 0     | 2 125 | 500   | 175   | --  | -- | 0   | --   | --    | -- | Vertical - Backwall             |
| B/651          | 65   | 4 400  | S    | 0     | 1125  | 350   | 2 925 | --  | -- | 0   | --   | --    | -- | Vertical - Backwall             |
| B/652          | 35   | 3 650  | BS   | 200   | 2 025 | 1425  | --    | --  | -- | 60  | 420  | --    | -- | Top of Breastwall               |
| B/653          | 28   | 4 175  | S    | 0     | 175   | 1950  | 2 050 | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/654          | 8    | 4 400  | S    | 0     | 1525  | 1350  | 1525  | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/655          | 2    | 4 900  | SC   | 0     | 1575  | 1725  | 1600  | --  | -- | 0   | --   | 1460  | -- | Pedestals                       |
| B/656          | 2    | 4 700  | SC   | 0     | 1575  | 1525  | 1600  | --  | -- | 0   | --   | 1790  | -- | Pedestals                       |
| B/657          | 20   | 3 200  | S    | 0     | 900   | 1400  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/658          | 1    | 3 500  | S    | 0     | 900   | 1700  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/659          | 1    | 3 450  | S    | 0     | 900   | 1650  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/660          | 1    | 3 400  | S    | 0     | 900   | 1600  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/661          | 1    | 3 350  | S    | 0     | 900   | 1550  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/662          | 1    | 3 400  | S    | 0     | 900   | 1600  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/663          | 1    | 3 450  | S    | 0     | 900   | 1650  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/664          | 1    | 3 500  | S    | 0     | 900   | 1700  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B/665          | 1    | 3 550  | S    | 0     | 900   | 1750  | 900   | --  | -- | 0   | --   | --    | -- | Pedestals                       |
| B2250          | 69   | 5 900  | L    | 5 525 | 375   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - Breastwall           |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
| B/670          | 60   | 2 300  | S    | 0     | 1625  | 500   | 175   | --  | -- | --  | --   | --    | -- | Vertical - Wingwalls            |
| B/671          | 60   | 2 800  | SC   | 0     | 1875  | 350   | 575   | --  | -- | 440 | --   | --    | -- | Vertical - Wingwalls            |
| B/672          | 6    | 3 750  | S    | 0     | 1625  | 500   | 1625  | --  | -- | --  | --   | --    | -- | Vertical - Wingwalls            |
| B/673          | 4    | 2 575  | S    | 0     | 1200  | 175   | 1200  | --  | -- | 0   | --   | --    | -- | Transition Barriers             |
| B/674          | 12   | 2 775  | S    | 0     | 1200  | 375   | 1200  | --  | -- | 0   | --   | --    | -- | Transition Barriers             |
| B/680          | 48   | 1200   | A    | --    | --    | --    | 600   | 600 | -- | --  | 590  | --    | -- | Horiz. - North Wingwall Corners |
| B/681          | 50   | 1200   | V    | --    | --    | --    | 600   | 600 | -- | --  | 590  | --    | -- | Horiz. - South Wingwall Corners |
| B2950          | 29   | 4200   | L    | 3725  | 475   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - North Wingwall       |
| B2951          | 32   | 4200   | L    | 3725  | 475   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - South Wingwall       |
| B2952          | 59   | 2700   | L    | 2225  | 475   | --    | --    | --  | -- | --  | --   | --    | -- | Vertical - Wingwalls            |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
|                |      |        |      |       |       |       |       |     |    |     |      |       |    |                                 |
| MARK           | QTY. | LENGTH | TYPE | A     | B     | C     | D     | E   | F  | G   | H    | O     | R  | LOCATION                        |

TYPE - BENDING DIAGRAMS



All dimensions are out-to-out of bar.

Bending details and hooks shall conform to the recommendations of the current revision of ACI Standard 318.

Reinforcing Bar: ASTM A615/A615M, Grade 420

GENERAL NOTES

- The first two digits following the letter(s) of the mark indicate the size of the bar:  
Mark "A1602" = bar size #16  
Mark "P2501" = bar size #25  
Mark "S1950" = bar size #19
- Each crank bar, Type B, may be replaced by two (2) straight bars (one top and one bottom) of the same bar size as the crank bar. Payment in either case shall be based on crank bars as schedule on the plans.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
**ABUTMENT REINF. SCHEDULE**

SHEET OF AUGUSTA, MAINE

### REINFORCING STEEL SCHEDULE

METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

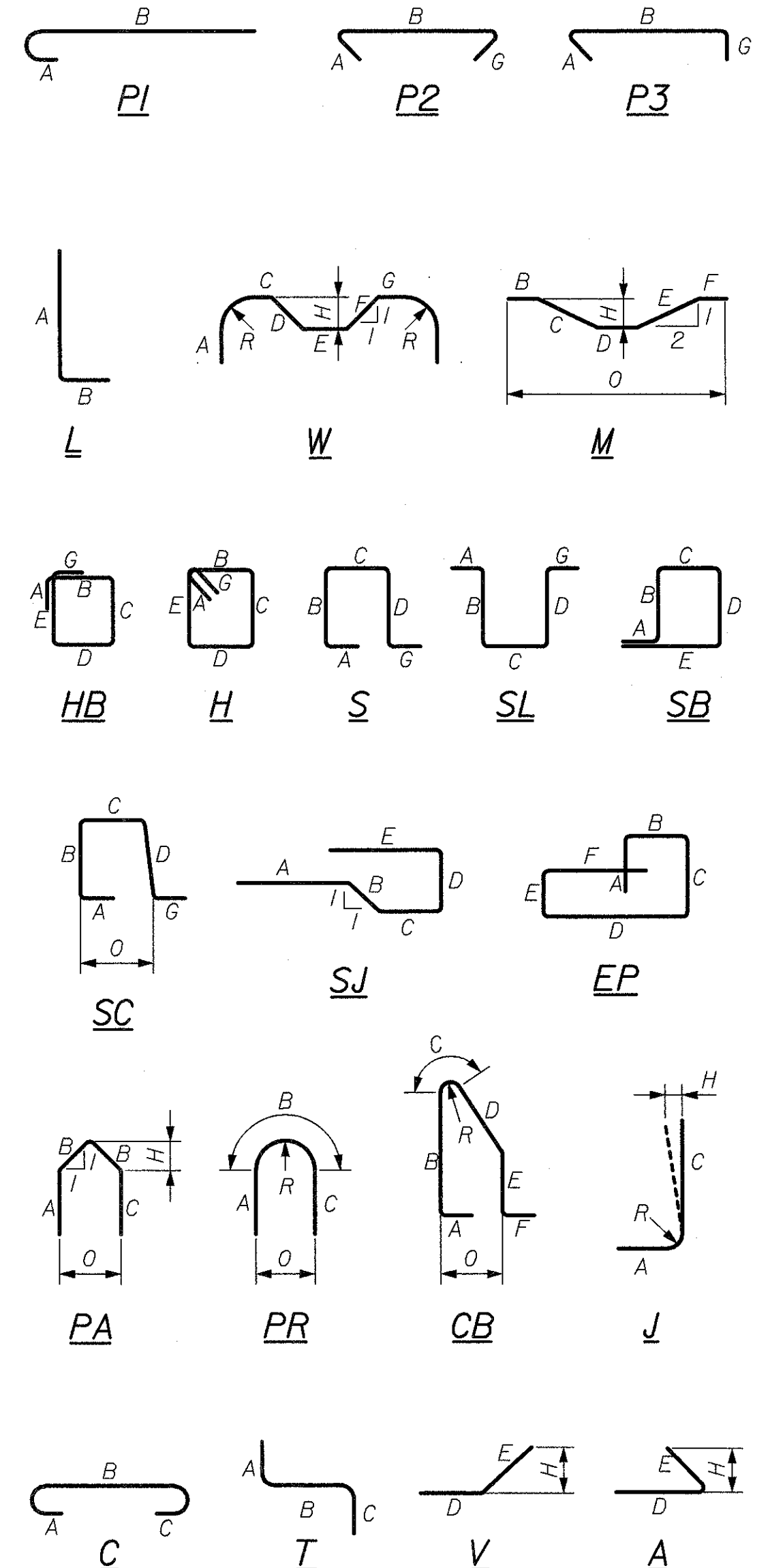
| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 96           | 105             |
| 000556.22            |       |                |              |                 |

[illegible]

| BENT BARS |      |        |      |   |     |      |   |   |     |                         |
|-----------|------|--------|------|---|-----|------|---|---|-----|-------------------------|
| MARK      | QTY. | LENGTH | TYPE | A | B   | C    | D | E | G   | LOCATION                |
| PIER NO.1 |      |        |      |   |     |      |   |   |     |                         |
| 1Q/950    | 40   | 2 300  | S    | 0 | 600 | 1100 | 0 | — | 600 | Pedestals G1 through G4 |
| 1Q/951    | 20   | 2 900  | S    | 0 | 900 | 1100 | 0 | — | 900 | Pedestals G5 & G6       |
| MARK      | QTY. | LENGTH | TYPE | A | B   | C    | D |   | G   | LOCATION                |

| BENT BARS  |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|------------|------|--------|------|-------|--------|-------|-------|-------|----|-----|-------|-------|------|--------------------------------|--|
| MARK       | QTY. | LENGTH | TYPE | A     | B      | C     | D     | E     | F  | G   | H     | O     | R    | LOCATION                       |  |
| PIER NO. 1 |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP3650     | 89   | 6 800  | C    | 475   | 5 850  | 475   | --    | --    | -- | --  | --    | --    | --   | Footing                        |  |
| IP3651     | 54   | 11 900 | C    | 475   | 10 950 | 475   | --    | --    | -- | --  | --    | --    | --   | Footing                        |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1350     | 26   | 9 000  | PR   | 2 700 | 3 600  | 2 700 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "IC"               |  |
| IP1351     | 26   | 8 700  | PR   | 2 550 | 3 600  | 2 550 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "ID"               |  |
| IP1352     | 26   | 8 350  | PR   | 2 375 | 3 600  | 2 375 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "IE"               |  |
| IP1353     | 20   | 8 050  | PR   | 2 225 | 3 600  | 2 225 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "IF"               |  |
| IP1354     | 20   | 7 800  | PR   | 2 100 | 3 600  | 2 100 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "IG"               |  |
| IP1355     | 20   | 7 550  | PR   | 1975  | 3 600  | 1975  | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "IH"               |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1360     | 26   | 3 850  | PI   | 150   | 3 700  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "IC"               |  |
| IP1361     | 26   | 3 700  | PI   | 150   | 3 550  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "ID"               |  |
| IP1362     | 26   | 3 525  | PI   | 150   | 3 375  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "IE"               |  |
| IP1363     | 20   | 3 375  | PI   | 150   | 3 225  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "IF"               |  |
| IP1364     | 20   | 3 250  | PI   | 150   | 3 100  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "IG"               |  |
| IP1365     | 20   | 3 125  | PI   | 150   | 2 975  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "IH"               |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1370     | 414  | 1150   | PI   | 150   | 1000   | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Sets "IC" through "IH" |  |
| IP1371     | 138  | 2 000  | P2   | 125   | 1750   | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets "IC" through "IH" |  |
| IP1372     | 138  | 2 425  | P2   | 125   | 2 175  | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets "IC" through "IH" |  |
| IP1373     | 138  | 2 550  | P2   | 125   | 2 300  | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets "IC" through "IH" |  |
| IP1374     | 705  | 2 550  | P3   | 125   | 2 300  | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets "IC" through "IH" |  |
| IP1375     | 276  | 1675   | P3   | 125   | 1550   | --    | --    | --    | -- | 0   | --    | --    | --   | Shaft - Sets "IC" through "IH" |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1950     | 56   | 9 600  | PR   | 3 000 | 3 600  | 3 000 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "IA"               |  |
| IP1951     | 56   | 9 350  | PR   | 2 875 | 3 600  | 2 875 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set "IB"               |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1960     | 56   | 4 200  | PI   | 200   | 4 000  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "IA"               |  |
| IP1961     | 56   | 4 075  | PI   | 200   | 3 875  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set "IB"               |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1970     | 336  | 1200   | PI   | 200   | 1000   | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Sets "IA" & "IB"       |  |
| IP1971     | 112  | 2 150  | P2   | 200   | 1750   | --    | --    | --    | -- | 200 | --    | --    | --   | Shaft - Sets "IA" & "IB"       |  |
| IP1972     | 112  | 2 575  | P2   | 200   | 2 175  | --    | --    | --    | -- | 200 | --    | --    | --   | Shaft - Sets "IA" & "IB"       |  |
| IP1973     | 112  | 2 700  | P2   | 200   | 2 300  | --    | --    | --    | -- | 200 | --    | --    | --   | Shaft - Sets "IA" & "IB"       |  |
| IP1974     | 784  | 2 800  | P3   | 200   | 2 300  | --    | --    | --    | -- | 300 | --    | --    | --   | Shaft - Sets "IA" & "IB"       |  |
| IP1975     | 224  | 1750   | P3   | 200   | 1550   | --    | --    | --    | -- | 0   | --    | --    | --   | Shaft - Sets "IA" & "IB"       |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP3660     | 150  | 5 150  | L    | 600   | 4 550  | --    | --    | --    | -- | --  | --    | --    | --   | Vertical - Shaft               |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1980     | 38   | 7 950  | S    | 0     | 2 800  | 2 350 | 2 800 | --    | -- | 0   | --    | --    | --   | Cap - Set "IL"                 |  |
| IP1981     | 16   | 6 550  | S    | 0     | 2 800  | 950   | 2 800 | --    | -- | 0   | --    | --    | --   | Cap - Set "IM"                 |  |
| IP1982     | 16   | 6 150  | S    | 0     | 2 600  | 950   | 2 600 | --    | -- | 0   | --    | --    | --   | Cap - Set "IN"                 |  |
| IP1983     | 16   | 5 750  | S    | 0     | 2 400  | 950   | 2 400 | --    | -- | 0   | --    | --    | --   | Cap - Set "IP"                 |  |
| IP1984     | 16   | 5 350  | S    | 0     | 2 200  | 950   | 2 200 | --    | -- | 0   | --    | --    | --   | Cap - Set "IQ"                 |  |
| IP1985     | 16   | 4 950  | S    | 0     | 2 000  | 950   | 2 000 | --    | -- | 0   | --    | --    | --   | Cap - Set "IR"                 |  |
| IP1986     | 16   | 4 550  | S    | 0     | 1800   | 950   | 1800  | --    | -- | 0   | --    | --    | --   | Cap - Set "IS"                 |  |
| IP1987     | 4    | 4 075  | S    | 0     | 1650   | 775   | 1650  | --    | -- | 0   | --    | --    | --   | Cap - Set "IT"                 |  |
| IP1988     | 4    | 3 975  | S    | 0     | 1600   | 775   | 1600  | --    | -- | 0   | --    | --    | --   | Cap - Set "IU"                 |  |
| IP1989     | 4    | 3 700  | S    | 0     | 1550   | 600   | 1550  | --    | -- | 0   | --    | --    | --   | Cap - Set "IV"                 |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP1990     | 16   | 4 800  | PR   | 600   | 3 600  | 600   | --    | --    | -- | --  | --    | 2 300 | 1150 | Horizontal - Ends of Cap       |  |
| IP1991     | 8    | 6 900  | S    | 0     | 2 800  | 1300  | 2 800 | --    | -- | 0   | --    | --    | --   | Cap - Set "IM"                 |  |
| IP1992     | 8    | 6 500  | S    | 0     | 2 600  | 1300  | 2 600 | --    | -- | 0   | --    | --    | --   | Cap - Set "IN"                 |  |
| IP1993     | 8    | 6 100  | S    | 0     | 2 400  | 1300  | 2 400 | --    | -- | 0   | --    | --    | --   | Cap - Set "IP"                 |  |
| IP1994     | 8    | 5 700  | S    | 0     | 2 200  | 1300  | 2 200 | --    | -- | 0   | --    | --    | --   | Cap - Set "IQ"                 |  |
| IP1995     | 8    | 5 300  | S    | 0     | 2 000  | 1300  | 2 000 | --    | -- | 0   | --    | --    | --   | Cap - Set "IR"                 |  |
| IP1996     | 8    | 4 900  | S    | 0     | 1800   | 1300  | 1800  | --    | -- | 0   | --    | --    | --   | Cap - Set "IS"                 |  |
| IP1997     | 2    | 4 600  | S    | 0     | 1650   | 1300  | 1650  | --    | -- | 0   | --    | --    | --   | Cap - Set "IT"                 |  |
| IP1998     | 2    | 4 500  | S    | 0     | 1600   | 1300  | 1600  | --    | -- | 0   | --    | --    | --   | Cap - Set "IU"                 |  |
| IP1999     | 2    | 4 400  | S    | 0     | 1550   | 1300  | 1550  | --    | -- | 0   | --    | --    | --   | Cap - Set "IV"                 |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP3670     | 14   | 18 300 | PI   | 475   | 17 825 | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| IP3671     | 2    | 8 125  | PI   | 475   | 7 650  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| IP3672     | 2    | 8 075  | PI   | 475   | 7 600  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| IP3673     | 2    | 7 975  | PI   | 475   | 7 500  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| IP3674     | 2    | 7 800  | PI   | 475   | 7 325  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| IP3675     | 2    | 7 525  | PI   | 475   | 7 050  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| IP3676     | 2    | 7 150  | PI   | 475   | 6 675  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| IP3677     | 2    | 6 425  | PI   | 475   | 5 950  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| IP3680     | 4    | 12 900 | V    | --    | --     | --    | 4 625 | 8 275 | -- | --  | 2 620 | --    | --   | Horizontal - Cap               |  |
| IP3681     | 4    | 12 725 | V    | --    | --     | --    | 4 625 | 8 100 | -- | --  | 2 565 | --    | --   | Horizontal - Cap               |  |
| IP3682     | 4    | 12 000 | V    | --    | --     | --    | 4 625 | 7 375 | -- | --  | 2 335 | --    | --   | Horizontal - Cap               |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |      |        | </   |       |        |       |       |       |    |     |       |       |      |                                |  |

### TYPE - BENDING DIAGRAMS



All dimensions are out-to-out of bar.

Bending details and hooks shall conform to the recommendations of the current revision of ACI Standard 318.

Reinforcing Bar: ASTM A615/A615M, Grade 420

## GENERAL NOTES

1. The first two digits following the letter(s) of the mark indicate the size of the bar:  
  
Mark "A1602" = bar size #16  
Mark "P2501" = bar size #25  
Mark "S1950" = bar size #19
2. Each crank bar, Type B, may be replaced by two (2) straight bars (one top and one bottom) of the same bar size as the crank bar. Payment in either case shall be based on crank bars as schedule on the plans.

```
Mark "A1602" = bar size #16
Mark "P2501" = bar size #25
Mark "S1950" = bar size #19
```

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
***PIER NO. 1 REINF. SCHEDULE***

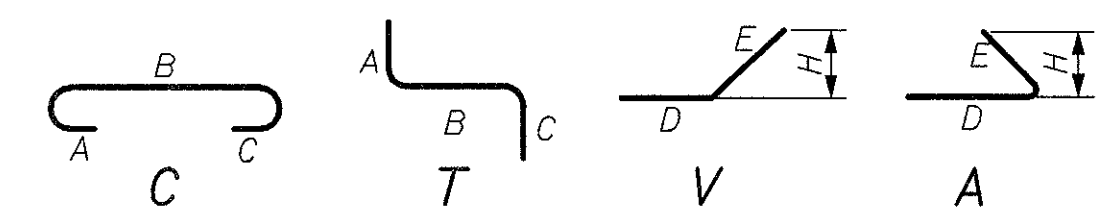
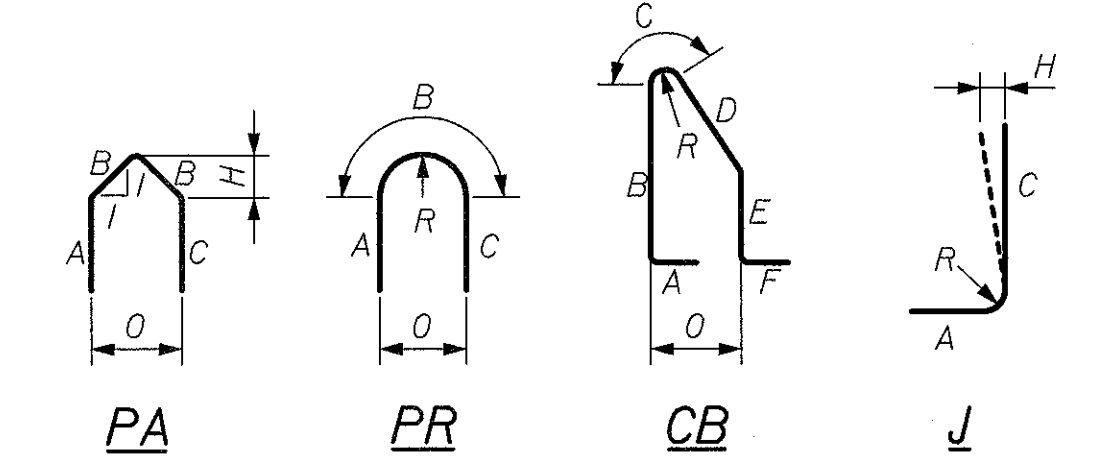
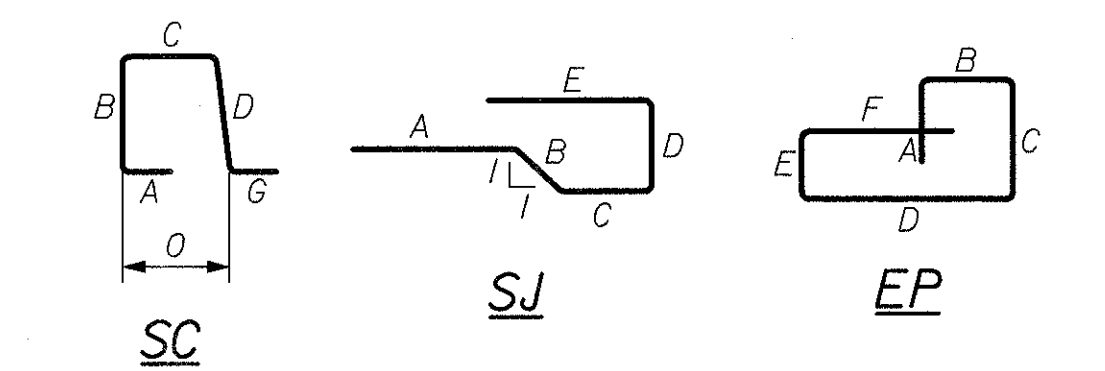
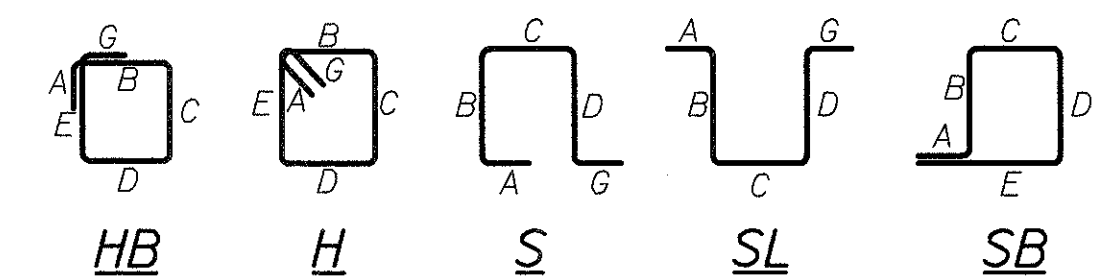
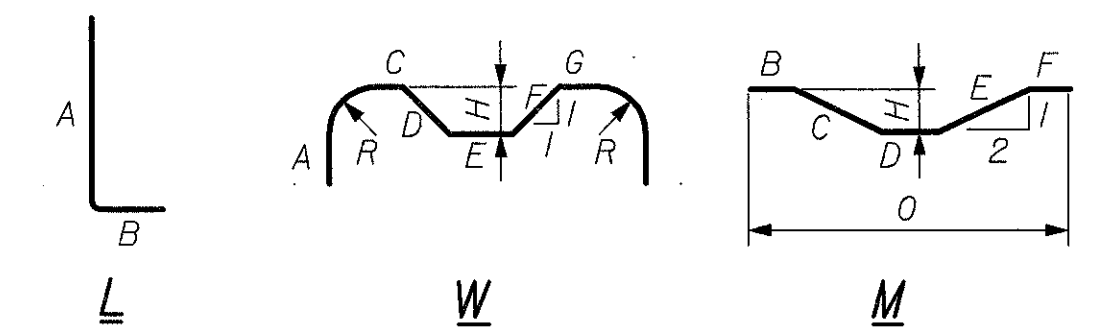
SHEET OF AUGUSTA, MAINE

## METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 97           | 105             |

000556.22



All dimensions are out-to-out of bar.

Bending details and hooks shall conform to the recommendations of the current revision of ACI Standard 318.

Reinforcing Bar: ASTM A615/A615M, Grade 420

1. The first two digits following the letter(s) of the mark indicate the size of the bar:

Mark "A1602" = bar size #16  
Mark "P2501" = bar size #25  
Mark "S1950" = bar size #19

2. Each crank bar, Type B, may be replaced by two (2) straight bars (one top and one bottom) of the same bar size as the crank bar. Payment in either case shall be based on crank bars as schedule on the plans.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY

PIER NO. 2 REINF. SCHEDULE

SHEET OF AUGUSTA, MAINE

[illegible]

| BENT BARS  |      |        |      |   |     |      |   |    |     |                         |
|------------|------|--------|------|---|-----|------|---|----|-----|-------------------------|
| MARK       | QTY. | LENGTH | TYPE | A | B   | C    | D | E  | G   | LOCATION                |
| PIER NO. 2 |      |        |      |   |     |      |   |    |     |                         |
| 20/950     | 20   | 2 300  | S    | 0 | 600 | 1/00 | 0 | -- | 600 | Pedestals G1 & G2       |
| 20/951     | 40   | 2 900  | S    | 0 | 900 | 1/00 | 0 | -- | 900 | Pedestals G3 through G6 |
| MARK       | QTY. | LENGTH | TYPE | A | B   | C    | D |    | G   | LOCATION                |

### BENT BARS

| BENT BARS  |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
|------------|------|--------|------|-------|--------|-------|-------|-------|-----|----|-------|-------|------|--------------------------------|--|
| MARK       | QTY. | LENGTH | TYPE | A     | B      | C     | D     | E     | F   | G  | H     | O     | R    | LOCATION                       |  |
| PIER NO. 2 |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P3650     | 95   | 7 400  | C    | 475   | 6 450  | 475   | --    | --    | --  | -- | --    | --    | --   | Distribution Slab              |  |
| 2P3651     | 55   | 12 300 | C    | 475   | 11 350 | 475   | --    | --    | --  | -- | --    | --    | --   | Distribution Slab              |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1350     | 24   | 9 700  | PR   | 3 050 | 3 600  | 3 050 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2C"               |  |
| 2P1351     | 24   | 9 400  | PR   | 2 900 | 3 600  | 2 900 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2D"               |  |
| 2P1352     | 24   | 9 100  | PR   | 2 750 | 3 600  | 2 750 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2E"               |  |
| 2P1353     | 24   | 8 800  | PR   | 2 600 | 3 600  | 2 600 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2F"               |  |
| 2P1354     | 24   | 8 500  | PR   | 2 450 | 3 600  | 2 450 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2G"               |  |
| 2P1355     | 24   | 8 200  | PR   | 2 300 | 3 600  | 2 300 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2H"               |  |
| 2P1356     | 24   | 7 900  | PR   | 2 150 | 3 600  | 2 150 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2I"               |  |
| 2P1357     | 24   | 7 600  | PR   | 2 000 | 3 600  | 2 000 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2K"               |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1360     | 24   | 4 200  | PI   | 150   | 4 050  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2C"               |  |
| 2P1361     | 24   | 4 050  | PI   | 150   | 3 900  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2D"               |  |
| 2P1362     | 24   | 3 900  | PI   | 150   | 3 750  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2E"               |  |
| 2P1363     | 24   | 3 750  | PI   | 150   | 3 600  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2F"               |  |
| 2P1364     | 24   | 3 600  | PI   | 150   | 3 450  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2G"               |  |
| 2P1365     | 24   | 3 450  | PI   | 150   | 3 300  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2H"               |  |
| 2P1366     | 24   | 3 300  | PI   | 150   | 3 150  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2I"               |  |
| 2P1367     | 24   | 3 150  | PI   | 150   | 3 000  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2K"               |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1370     | 576  | 1150   | PI   | 150   | 1000   | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Sets "2C" through "2K" |  |
| 2P1371     | 192  | 2 000  | P2   | 125   | 1750   | --    | --    | --    | 125 | -- | --    | --    | --   | Shaft - Sets "2C" through "2K" |  |
| 2P1372     | 192  | 2 425  | P2   | 125   | 2 175  | --    | --    | --    | 125 | -- | --    | --    | --   | Shaft - Sets "2C" through "2K" |  |
| 2P1373     | 192  | 2 550  | P2   | 125   | 2 300  | --    | --    | --    | 125 | -- | --    | --    | --   | Shaft - Sets "2C" through "2K" |  |
| 2P1374     | 1056 | 2 550  | P3   | 125   | 2 300  | --    | --    | --    | 125 | -- | --    | --    | --   | Shaft - Sets "2C" through "2K" |  |
| 2P1375     | 384  | 1675   | P3   | 125   | 1550   | --    | --    | --    | 0   | -- | --    | --    | --   | Shaft - Sets "2C" through "2K" |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1950     | 66   | 10 350 | PR   | 3 375 | 3 600  | 3 375 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2A"               |  |
| 2P1951     | 66   | 10 050 | PR   | 3 225 | 3 600  | 3 225 | --    | --    | --  | -- | --    | 2 300 | 1150 | Shaft - Set "2B"               |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1960     | 66   | 4 575  | PI   | 200   | 4 375  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2A"               |  |
| 2P1961     | 66   | 4 425  | PI   | 200   | 4 225  | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Set "2B"               |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1970     | 396  | 1200   | PI   | 200   | 1000   | --    | --    | --    | --  | -- | --    | --    | --   | Shaft - Sets "2A" & "2B"       |  |
| 2P1971     | 132  | 2 150  | P2   | 200   | 1750   | --    | --    | --    | 200 | -- | --    | --    | --   | Shaft - Sets "2A" & "2B"       |  |
| 2P1972     | 132  | 2 575  | P2   | 200   | 2 175  | --    | --    | --    | 200 | -- | --    | --    | --   | Shaft - Sets "2A" & "2B"       |  |
| 2P1973     | 132  | 2 700  | P2   | 200   | 2 300  | --    | --    | --    | 200 | -- | --    | --    | --   | Shaft - Sets "2A" & "2B"       |  |
| 2P1974     | 1056 | 2 800  | P3   | 200   | 2 300  | --    | --    | --    | 300 | -- | --    | --    | --   | Shaft - Sets "2A" & "2B"       |  |
| 2P1975     | 264  | 1750   | P3   | 200   | 1550   | --    | --    | --    | 0   | -- | --    | --    | --   | Shaft - Sets "2A" & "2B"       |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P3660     | 162  | 5 150  | L    | 600   | 4 550  | --    | --    | --    | --  | -- | --    | --    | --   | Vertical - Shaft               |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1980     | 38   | 7 850  | S    | 0     | 2 750  | 2 350 | 2 750 | --    | --  | 0  | --    | --    | --   | Cap - Set "2L"                 |  |
| 2P1981     | 16   | 6 450  | S    | 0     | 2 750  | 950   | 2 750 | --    | --  | 0  | --    | --    | --   | Cap - Set "2M"                 |  |
| 2P1982     | 16   | 6 050  | S    | 0     | 2 550  | 950   | 2 550 | --    | --  | 0  | --    | --    | --   | Cap - Set "2N"                 |  |
| 2P1983     | 16   | 5 650  | S    | 0     | 2 350  | 950   | 2 350 | --    | --  | 0  | --    | --    | --   | Cap - Set "2P"                 |  |
| 2P1984     | 16   | 5 250  | S    | 0     | 2 150  | 950   | 2 150 | --    | --  | 0  | --    | --    | --   | Cap - Set "2Q"                 |  |
| 2P1985     | 16   | 4 850  | S    | 0     | 1950   | 950   | 1950  | --    | --  | 0  | --    | --    | --   | Cap - Set "2R"                 |  |
| 2P1986     | 12   | 4 450  | S    | 0     | 1750   | 950   | 1750  | --    | --  | 0  | --    | --    | --   | Cap - Set "2S"                 |  |
| 2P1987     | 4    | 4 075  | S    | 0     | 1650   | 775   | 1650  | --    | --  | 0  | --    | --    | --   | Cap - Set "2T"                 |  |
| 2P1988     | 4    | 3 975  | S    | 0     | 1600   | 775   | 1600  | --    | --  | 0  | --    | --    | --   | Cap - Set "2U"                 |  |
| 2P1989     | 4    | 3 700  | S    | 0     | 1550   | 600   | 1550  | --    | --  | 0  | --    | --    | --   | Cap - Set "2V"                 |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P1990     | 16   | 4 800  | PR   | 600   | 3 600  | 600   | --    | --    | --  | -- | --    | 2 300 | 1150 | Horizontal - Ends of Cap       |  |
| 2P1991     | 8    | 6 800  | S    | 0     | 2 750  | 1300  | 2 750 | --    | --  | 0  | --    | --    | --   | Cap - Set "2M"                 |  |
| 2P1992     | 8    | 6 400  | S    | 0     | 2 550  | 1300  | 2 550 | --    | --  | 0  | --    | --    | --   | Cap - Set "2N"                 |  |
| 2P1993     | 8    | 6 000  | S    | 0     | 2 350  | 1300  | 2 350 | --    | --  | 0  | --    | --    | --   | Cap - Set "2P"                 |  |
| 2P1994     | 8    | 5 600  | S    | 0     | 2 150  | 1300  | 2 150 | --    | --  | 0  | --    | --    | --   | Cap - Set "2Q"                 |  |
| 2P1995     | 8    | 5 200  | S    | 0     | 1950   | 1300  | 1950  | --    | --  | 0  | --    | --    | --   | Cap - Set "2R"                 |  |
| 2P1996     | 6    | 4 800  | S    | 0     | 1750   | 1300  | 1750  | --    | --  | 0  | --    | --    | --   | Cap - Set "2S"                 |  |
| 2P1997     | 2    | 4 600  | S    | 0     | 1650   | 1300  | 1650  | --    | --  | 0  | --    | --    | --   | Cap - Set "2T"                 |  |
| 2P1998     | 2    | 4 500  | S    | 0     | 1600   | 1300  | 1600  | --    | --  | 0  | --    | --    | --   | Cap - Set "2U"                 |  |
| 2P1999     | 2    | 4 400  | S    | 0     | 1550   | 1300  | 1550  | --    | --  | 0  | --    | --    | --   | Cap - Set "2V"                 |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P3670     | 14   | 18 300 | PI   | 475   | 17 825 | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
| 2P3671     | 2    | 7 400  | PI   | 475   | 6 925  | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
| 2P3672     | 2    | 7 350  | PI   | 475   | 6 875  | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
| 2P3673     | 2    | 7 250  | PI   | 475   | 6 775  | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
| 2P3674     | 2    | 7 075  | PI   | 475   | 6 600  | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
| 2P3675     | 2    | 6 800  | PI   | 475   | 6 325  | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
| 2P3676     | 2    | 6 425  | PI   | 475   | 5 950  | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
| 2P3677     | 2    | 5 700  | PI   | 475   | 5 225  | --    | --    | --    | --  | -- | --    | --    | --   | Horizontal - Cap               |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| 2P3680     | 4    | 12 525 | V    | --    | --     | --    | 4 625 | 7 900 | --  | -- | 2 500 | --    | --   | Horizontal - Cap               |  |
| 2P3681     | 4    | 12 350 | V    | --    | --     | --    | 4 625 | 7 725 | --  | -- | 2 445 | --    | --   | Horizontal - Cap               |  |
| 2P3682     | 4    | 11 625 | V    | --    | --     | --    | 4 625 | 7 000 | --  | -- | 2 215 | --    | --   | Horizontal - Cap               |  |
|            |      |        |      |       |        |       |       |       |     |    |       |       |      |                                |  |
| MARK       | QTY. | LENGTH | TYPE | A     | B      | C     | D     | E     | F   | G  | H     | O     | R    | LOCATION                       |  |

| MARK | QTY. | LENGTH | TYPE | A | B | C | D | E | F | G | H | O | R | LOCATION |
|------|------|--------|------|---|---|---|---|---|---|---|---|---|---|----------|
|------|------|--------|------|---|---|---|---|---|---|---|---|---|---|----------|

| PROJECT DESIGN ENGINEER | BY        | DATE      |
|-------------------------|-----------|-----------|
| DESIGN-DETAILED         | J. Folsom | D. Damren |
| CHECKED                 |           |           |
| REVISIONS               |           |           |
| FIELD CHANGES           |           |           |

Date: 26 NOV 2002

Username: Dana.Damren

Division: BRIDGE

Filename: ... \097\_rebar\_pier 2.dgn

### REINFORCING STEEL SCHEDULE

|               |                                                                                                               |
|---------------|---------------------------------------------------------------------------------------------------------------|
| <b>METRIC</b> | 1. All dimensions are in millimeters unless otherwise noted.<br>2. All elevations and stations are in meters. |
|---------------|---------------------------------------------------------------------------------------------------------------|

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 98           | 105             |

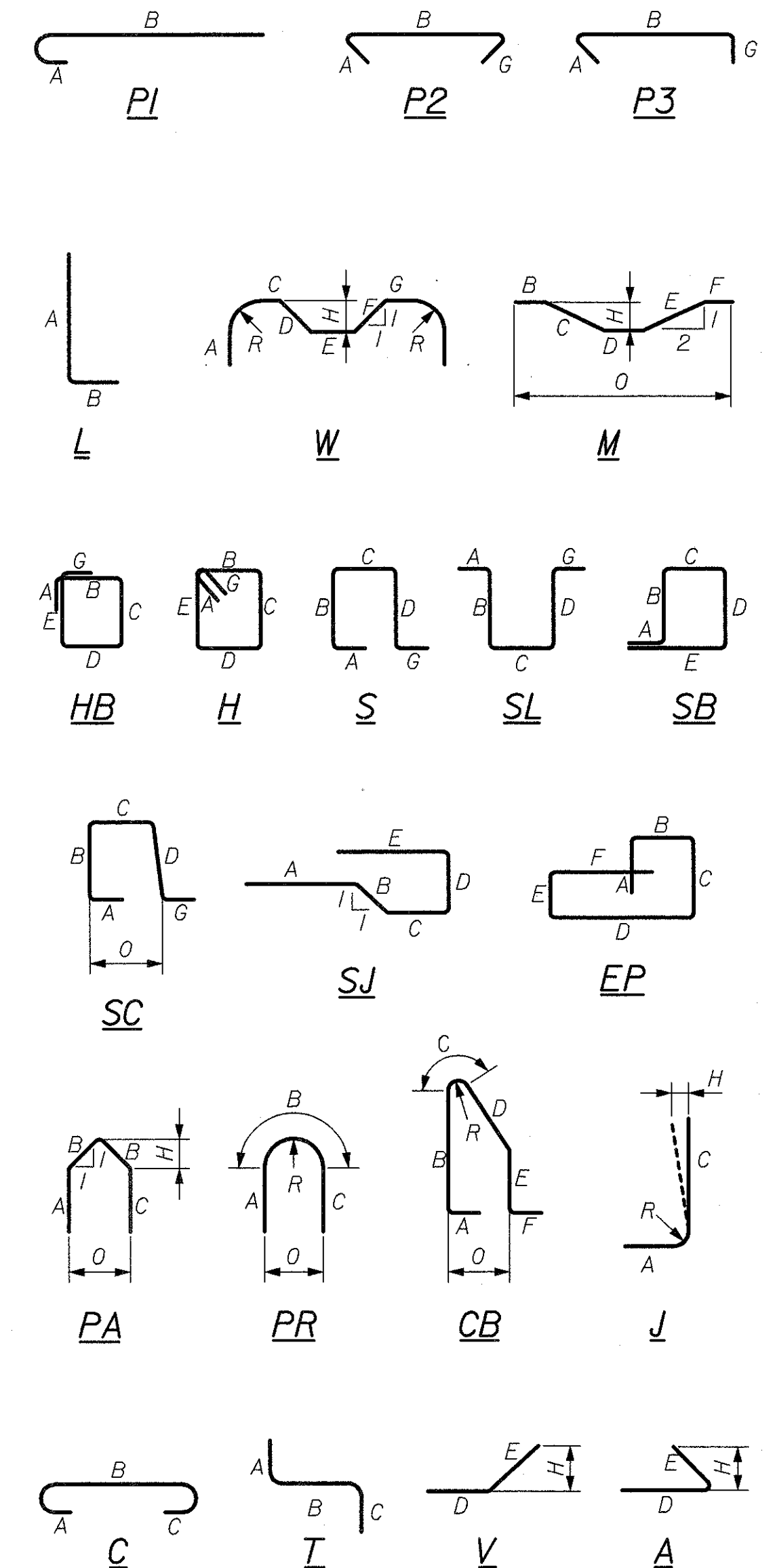
[illegible]

| BENT BARS  |      |        |      |   |     |       |   |   |     |                         |
|------------|------|--------|------|---|-----|-------|---|---|-----|-------------------------|
| MARK       | QTY. | LENGTH | TYPE | A | B   | C     | D | E | G   | LOCATION                |
| PIER NO. 3 |      |        |      |   |     |       |   |   |     |                         |
| 30/950     | 20   | 2 300  | S    | 0 | 600 | 1 100 | 0 | — | 600 | Pedestals G1 & G2       |
| 30/951     | 40   | 2 900  | S    | 0 | 900 | 1 100 | 0 | — | 900 | Pedestals G3 through G6 |
| MARK       | QTY. | LENGTH | TYPE | A | B   | C     | D |   | G   | LOCATION                |

## BENT BARS

| BENT BARS  |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|------------|-------|--------|------|-------|--------|-------|-------|-------|----|-----|-------|-------|------|--------------------------------|--|
| MARK       | QTY.  | LENGTH | TYPE | A     | B      | C     | D     | E     | F  | G   | H     | O     | R    | LOCATION                       |  |
| PIER NO. 3 |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3P3650     | 93    | 7 100  | C    | 475   | 6 150  | 475   | --    | --    | -- | --  | --    | --    | --   | Distribution Slab              |  |
| 3P3651     | 52    | 12 100 | C    | 475   | 11 150 | 475   | --    | --    | -- | --  | --    | --    | --   | Distribution Slab              |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI350     | 24    | 9 600  | PR   | 3 000 | 3 600  | 3 000 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3C*               |  |
| 3PI351     | 24    | 9 300  | PR   | 2 850 | 3 600  | 2 850 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3D*               |  |
| 3PI352     | 24    | 9 000  | PR   | 2 700 | 3 600  | 2 700 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3E*               |  |
| 3PI353     | 24    | 8 700  | PR   | 2 550 | 3 600  | 2 550 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3F*               |  |
| 3PI354     | 24    | 8 400  | PR   | 2 400 | 3 600  | 2 400 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3G*               |  |
| 3PI355     | 24    | 8 100  | PR   | 2 250 | 3 600  | 2 250 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3H*               |  |
| 3PI356     | 24    | 7 800  | PR   | 2 100 | 3 600  | 2 100 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3I*               |  |
| 3PI357     | 16    | 7 500  | PR   | 1950  | 3 600  | 1950  | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3K*               |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI360     | 24    | 4 150  | PI   | 150   | 4 000  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3C*               |  |
| 3PI361     | 24    | 4 000  | PI   | 150   | 3 850  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3D*               |  |
| 3PI362     | 24    | 3 850  | PI   | 150   | 3 700  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3E*               |  |
| 3PI363     | 24    | 3 700  | PI   | 150   | 3 550  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3F*               |  |
| 3PI364     | 24    | 3 550  | PI   | 150   | 3 400  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3G*               |  |
| 3PI365     | 24    | 3 400  | PI   | 150   | 3 250  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3H*               |  |
| 3PI366     | 24    | 3 250  | PI   | 150   | 3 100  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3I*               |  |
| 3PI367     | 16    | 3 100  | PI   | 150   | 2 950  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3K*               |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI370     | 552   | 1 150  | PI   | 150   | 1 000  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Sets *3C* through *3K* |  |
| 3PI371     | 184   | 2 000  | P2   | 125   | 1 750  | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets *3C* through *3K* |  |
| 3PI372     | 184   | 2 425  | P2   | 125   | 2 175  | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets *3C* through *3K* |  |
| 3PI373     | 184   | 2 550  | P2   | 125   | 2 300  | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets *3C* through *3K* |  |
| 3PI374     | 1 028 | 2 550  | P3   | 125   | 2 300  | --    | --    | --    | -- | 125 | --    | --    | --   | Shaft - Sets *3C* through *3K* |  |
| 3PI375     | 368   | 1 675  | P3   | 125   | 1 550  | --    | --    | --    | -- | 0   | --    | --    | --   | Shaft - Sets *3C* through *3K* |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI950     | 66    | 10 250 | PR   | 3 325 | 3 600  | 3 325 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3A*               |  |
| 3PI951     | 66    | 9 950  | PR   | 3 175 | 3 600  | 3 175 | --    | --    | -- | --  | --    | 2 300 | 1150 | Shaft - Set *3B*               |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI960     | 66    | 4 475  | PI   | 200   | 4 325  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3A*               |  |
| 3PI961     | 66    | 4 325  | PI   | 200   | 4 175  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Set *3B*               |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI970     | 396   | 1 200  | PI   | 200   | 1 000  | --    | --    | --    | -- | --  | --    | --    | --   | Shaft - Sets *3A* & *3B*       |  |
| 3PI971     | 132   | 2 150  | P2   | 200   | 1 750  | --    | --    | --    | -- | 200 | --    | --    | --   | Shaft - Sets *3A* & *3B*       |  |
| 3PI972     | 132   | 2 575  | P2   | 200   | 2 175  | --    | --    | --    | -- | 200 | --    | --    | --   | Shaft - Sets *3A* & *3B*       |  |
| 3PI973     | 132   | 2 700  | P2   | 200   | 2 300  | --    | --    | --    | -- | 200 | --    | --    | --   | Shaft - Sets *3A* & *3B*       |  |
| 3PI974     | 1 056 | 2 800  | P3   | 200   | 2 300  | --    | --    | --    | -- | 300 | --    | --    | --   | Shaft - Sets *3A* & *3B*       |  |
| 3PI975     | 264   | 1 750  | P3   | 200   | 1 550  | --    | --    | --    | -- | 0   | --    | --    | --   | Shaft - Sets *3A* & *3B*       |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3P3660     | 162   | 5 150  | L    | 600   | 4 550  | --    | --    | --    | -- | --  | --    | --    | --   | Vertical - Shaft               |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI980     | 38    | 7 750  | S    | 0     | 2 700  | 2 350 | 2 700 | --    | -- | 0   | --    | --    | --   | Cap - Set *3L*                 |  |
| 3PI981     | 16    | 6 350  | S    | 0     | 2 700  | 950   | 2 700 | --    | -- | 0   | --    | --    | --   | Cap - Set *3M*                 |  |
| 3PI982     | 16    | 5 950  | S    | 0     | 2 500  | 950   | 2 500 | --    | -- | 0   | --    | --    | --   | Cap - Set *3N*                 |  |
| 3PI983     | 16    | 5 550  | S    | 0     | 2 300  | 950   | 2 300 | --    | -- | 0   | --    | --    | --   | Cap - Set *3P*                 |  |
| 3PI984     | 16    | 5 150  | S    | 0     | 2 100  | 950   | 2 100 | --    | -- | 0   | --    | --    | --   | Cap - Set *3Q*                 |  |
| 3PI985     | 12    | 4 750  | S    | 0     | 1900   | 950   | 1900  | --    | -- | 0   | --    | --    | --   | Cap - Set *3R*                 |  |
| 3PI986     | 12    | 4 450  | S    | 0     | 1750   | 950   | 1750  | --    | -- | 0   | --    | --    | --   | Cap - Set *3S*                 |  |
| 3PI987     | 4     | 4 075  | S    | 0     | 1650   | 775   | 1650  | --    | -- | 0   | --    | --    | --   | Cap - Set *3T*                 |  |
| 3PI988     | 4     | 3 975  | S    | 0     | 1600   | 775   | 1600  | --    | -- | 0   | --    | --    | --   | Cap - Set *3U*                 |  |
| 3PI989     | 4     | 3 700  | S    | 0     | 1550   | 600   | 1550  | --    | -- | 0   | --    | --    | --   | Cap - Set *3V*                 |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3PI990     | 16    | 4 800  | PR   | 600   | 3 600  | 600   | --    | --    | -- | --  | --    | 2 300 | 1150 | Horizontal - Ends of Cap       |  |
| 3PI991     | 8     | 6 700  | S    | 0     | 2 700  | 1300  | 2 700 | --    | -- | 0   | --    | --    | --   | Cap - Set *3M*                 |  |
| 3PI992     | 8     | 6 300  | S    | 0     | 2 500  | 1300  | 2 500 | --    | -- | 0   | --    | --    | --   | Cap - Set *3N*                 |  |
| 3PI993     | 8     | 5 900  | S    | 0     | 2 300  | 1300  | 2 300 | --    | -- | 0   | --    | --    | --   | Cap - Set *3P*                 |  |
| 3PI994     | 8     | 5 500  | S    | 0     | 2 100  | 1300  | 2 100 | --    | -- | 0   | --    | --    | --   | Cap - Set *3Q*                 |  |
| 3PI995     | 6     | 5 100  | S    | 0     | 1900   | 1300  | 1900  | --    | -- | 0   | --    | --    | --   | Cap - Set *3R*                 |  |
| 3PI996     | 6     | 4 800  | S    | 0     | 1750   | 1300  | 1750  | --    | -- | 0   | --    | --    | --   | Cap - Set *3S*                 |  |
| 3PI997     | 2     | 4 600  | S    | 0     | 1650   | 1300  | 1650  | --    | -- | 0   | --    | --    | --   | Cap - Set *3T*                 |  |
| 3PI998     | 2     | 4 500  | S    | 0     | 1600   | 1300  | 1600  | --    | -- | 0   | --    | --    | --   | Cap - Set *3U*                 |  |
| 3PI999     | 2     | 4 400  | S    | 0     | 1550   | 1300  | 1550  | --    | -- | 0   | --    | --    | --   | Cap - Set *3V*                 |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3P3670     | 14    | 18 300 | PI   | 475   | 17 825 | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| 3P3671     | 2     | 6 850  | PI   | 475   | 6 375  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| 3P3672     | 2     | 6 800  | PI   | 475   | 6 325  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| 3P3673     | 2     | 6 700  | PI   | 475   | 6 225  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| 3P3674     | 2     | 6 525  | PI   | 475   | 6 050  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| 3P3675     | 2     | 6 250  | PI   | 475   | 5 775  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| 3P3676     | 2     | 5 875  | PI   | 475   | 5 400  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
| 3P3677     | 2     | 5 150  | PI   | 475   | 4 675  | --    | --    | --    | -- | --  | --    | --    | --   | Horizontal - Cap               |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| 3P3680     | 4     | 12 525 | V    | --    | --     | --    | 4 625 | 7 600 | -- | --  | 2 405 | --    | --   | Horizontal - Cap               |  |
| 3P3681     | 4     | 12 350 | V    | --    | --     | --    | 4 625 | 7 425 | -- | --  | 2 350 | --    | --   | Horizontal - Cap               |  |
| 3P3682     | 4     | 11 625 | V    | --    | --     | --    | 4 625 | 6 700 | -- | --  | 2 120 | --    | --   | Horizontal - Cap               |  |
|            |       |        |      |       |        |       |       |       |    |     |       |       |      |                                |  |
| MARK       | QTY.  | LENGTH | TYPE | A     | B      | C     | D     | E     | F  | G   | H     | O     | R    | LOCATION                       |  |

TYPE - BENDING DIAGRAMS



All dimensions are out-to-out of bar.

Bending details and hooks shall conform to the recommendations of the current revision of ACI Standard 318.

Reinforcing Bar: ASTM A615/A615M, Grade 420

## GENERAL NOTES

1. The first two digits following the letter(s) of the mark indicate the size of the bar:
- Mark 'A1602' = bar size #16  
Mark 'P2501' = bar size #25  
Mark 'S1950' = bar size #19
2. Each crank bar, Type B, may be replaced by two (2) straight bars (one top and one bottom) of the same bar size as the crank bar. Payment in either case shall be based on crank bars as schedule on the plans.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
*PIER NO. 3 REINF. SCHEDULE*

SHEET OF AUGUSTA, MAINE

## METRIC

| F.H.W.A.<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|----------------------|-------|----------------|--------------|-----------------|
| 1                    | MAINE | NH-0556(220)E  | 99           | 105             |
| 000556.22            |       |                |              |                 |

| BENT BARS  |      |        |      |   |     |      |   |   |     |                         |
|------------|------|--------|------|---|-----|------|---|---|-----|-------------------------|
| MARK       | QTY. | LENGTH | TYPE | A | B   | C    | D | E | G   | LOCATION                |
| PIER NO. 1 |      |        |      |   |     |      |   |   |     |                         |
| 4Q/950     | 40   | 2 500  | S    | 0 | 700 | 1100 | 0 | — | 700 | Pedestals G1 through G4 |
| 4Q/951     | 20   | 3 000  | S    | 0 | 950 | 1100 | 0 | — | 950 | Pedestals G5 & G6       |
| MARK       | QTY. | LENGTH | TYPE | A | B   | C    | D |   | G   | LOCATION                |

| BENT BARS  |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|------------|------|--------|------|-------|--------|-------|-------|-------|----|-----|-------|-------|-------|------------------------------|--|
| MARK       | QTY. | LENGTH | TYPE | A     | B      | C     | D     | E     | F  | G   | H     | O     | R     | LOCATION                     |  |
| PIER NO. 4 |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P3650     | 90   | 7 100  | C    | 475   | 6 150  | 475   | --    | --    | -- | --  | --    | --    | --    | Footing                      |  |
| 4P3651     | 52   | 11 600 | C    | 475   | 10 650 | 475   | --    | --    | -- | --  | --    | --    | --    | Footing                      |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P350      | 26   | 9 150  | PR   | 2 775 | 3 600  | 2 775 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4C              |  |
| 4P351      | 26   | 8 800  | PR   | 2 600 | 3 600  | 2 600 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4D              |  |
| 4P352      | 26   | 8 500  | PR   | 2 450 | 3 600  | 2 450 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4E              |  |
| 4P353      | 22   | 8 150  | PR   | 2 275 | 3 600  | 2 275 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4F              |  |
| 4P354      | 22   | 7 900  | PR   | 2 150 | 3 600  | 2 150 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4G              |  |
| 4P355      | 22   | 7 600  | PR   | 2 000 | 3 600  | 2 000 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4H              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P360      | 26   | 3 925  | PI   | 150   | 3 775  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4C              |  |
| 4P361      | 26   | 3 750  | PI   | 150   | 3 600  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4D              |  |
| 4P362      | 26   | 3 600  | PI   | 150   | 3 450  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4E              |  |
| 4P363      | 22   | 3 425  | PI   | 150   | 3 275  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4F              |  |
| 4P364      | 22   | 3 300  | PI   | 150   | 3 150  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4G              |  |
| 4P365      | 22   | 3 150  | PI   | 150   | 3 000  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4H              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P370      | 432  | 1 150  | PI   | 150   | 1 000  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Sets *4C through *4H |  |
| 4P371      | 144  | 2 000  | P2   | 125   | 1 750  | --    | --    | --    | -- | 125 | --    | --    | --    | Shaft - Sets *4C through *4H |  |
| 4P372      | 144  | 2 425  | P2   | 125   | 2 175  | --    | --    | --    | -- | 125 | --    | --    | --    | Shaft - Sets *4C through *4H |  |
| 4P373      | 144  | 2 550  | P2   | 125   | 2 300  | --    | --    | --    | -- | 125 | --    | --    | --    | Shaft - Sets *4C through *4H |  |
| 4P374      | 730  | 2 550  | P3   | 125   | 2 300  | --    | --    | --    | -- | 125 | --    | --    | --    | Shaft - Sets *4C through *4H |  |
| 4P375      | 288  | 1 675  | P3   | 125   | 1 550  | --    | --    | --    | -- | 0   | --    | --    | --    | Shaft - Sets *4C through *4H |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P1950     | 56   | 9 700  | PR   | 3 050 | 3 600  | 3 050 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4A              |  |
| 4P1951     | 56   | 9 450  | PR   | 2 925 | 3 600  | 2 925 | --    | --    | -- | --  | --    | 2 300 | 1 150 | Shaft - Set *4B              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P1960     | 56   | 4 250  | PI   | 200   | 4 050  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4A              |  |
| 4P1961     | 56   | 4 125  | PI   | 200   | 3 925  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Set *4B              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P1970     | 336  | 1 200  | PI   | 200   | 1 000  | --    | --    | --    | -- | --  | --    | --    | --    | Shaft - Sets *4A & *4B       |  |
| 4P1971     | 112  | 2 150  | P2   | 200   | 1 750  | --    | --    | --    | -- | 200 | --    | --    | --    | Shaft - Sets *4A & *4B       |  |
| 4P1972     | 112  | 2 575  | P2   | 200   | 2 175  | --    | --    | --    | -- | 200 | --    | --    | --    | Shaft - Sets *4A & *4B       |  |
| 4P1973     | 112  | 2 700  | P2   | 200   | 2 300  | --    | --    | --    | -- | 200 | --    | --    | --    | Shaft - Sets *4A & *4B       |  |
| 4P1974     | 784  | 2 800  | P3   | 200   | 2 300  | --    | --    | --    | -- | 300 | --    | --    | --    | Shaft - Sets *4A & *4B       |  |
| 4P1975     | 224  | 1 750  | P3   | 200   | 1 550  | --    | --    | --    | -- | 0   | --    | --    | --    | Shaft - Sets *4A & *4B       |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P3660     | 150  | 5 150  | L    | 600   | 4 550  | --    | --    | --    | -- | --  | --    | --    | --    | Vertical - Shaft             |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P1980     | 38   | 7 700  | S    | 0     | 2 675  | 2 350 | 2 675 | --    | -- | 0   | --    | --    | --    | Cap - Set *4L                |  |
| 4P1981     | 16   | 6 300  | S    | 0     | 2 675  | 950   | 2 675 | --    | -- | 0   | --    | --    | --    | Cap - Set *4M                |  |
| 4P1982     | 16   | 5 900  | S    | 0     | 2 475  | 950   | 2 475 | --    | -- | 0   | --    | --    | --    | Cap - Set *4N                |  |
| 4P1983     | 16   | 5 500  | S    | 0     | 2 275  | 950   | 2 275 | --    | -- | 0   | --    | --    | --    | Cap - Set *4P                |  |
| 4P1984     | 12   | 5 100  | S    | 0     | 2 075  | 950   | 2 075 | --    | -- | 0   | --    | --    | --    | Cap - Set *4Q                |  |
| 4P1985     | 12   | 4 800  | S    | 0     | 1 925  | 950   | 1 925 | --    | -- | 0   | --    | --    | --    | Cap - Set *4R                |  |
| 4P1986     | 12   | 4 500  | S    | 0     | 1 775  | 950   | 1 775 | --    | -- | 0   | --    | --    | --    | Cap - Set *4S                |  |
| 4P1987     | 4    | 4 075  | S    | 0     | 1 650  | 775   | 1 650 | --    | -- | 0   | --    | --    | --    | Cap - Set *4T                |  |
| 4P1988     | 4    | 3 975  | S    | 0     | 1 600  | 775   | 1 600 | --    | -- | 0   | --    | --    | --    | Cap - Set *4U                |  |
| 4P1989     | 4    | 3 700  | S    | 0     | 1 550  | 600   | 1 550 | --    | -- | 0   | --    | --    | --    | Cap - Set *4V                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P1990     | 16   | 4 800  | PR   | 600   | 3 600  | 600   | --    | --    | -- | --  | --    | 2 300 | 1 150 | Horizontal - Ends of Cap     |  |
| 4P1991     | 8    | 6 650  | S    | 0     | 2 675  | 1 300 | 2 675 | --    | -- | 0   | --    | --    | --    | Cap - Set *4M                |  |
| 4P1992     | 8    | 6 250  | S    | 0     | 2 475  | 1 300 | 2 475 | --    | -- | 0   | --    | --    | --    | Cap - Set *4N                |  |
| 4P1993     | 8    | 5 850  | S    | 0     | 2 275  | 1 300 | 2 275 | --    | -- | 0   | --    | --    | --    | Cap - Set *4P                |  |
| 4P1994     | 6    | 5 450  | S    | 0     | 2 075  | 1 300 | 2 075 | --    | -- | 0   | --    | --    | --    | Cap - Set *4Q                |  |
| 4P1995     | 6    | 5 150  | S    | 0     | 1 925  | 1 300 | 1 925 | --    | -- | 0   | --    | --    | --    | Cap - Set *4R                |  |
| 4P1996     | 6    | 4 850  | S    | 0     | 1 775  | 1 300 | 1 775 | --    | -- | 0   | --    | --    | --    | Cap - Set *4S                |  |
| 4P1997     | 2    | 4 600  | S    | 0     | 1 650  | 1 300 | 1 650 | --    | -- | 0   | --    | --    | --    | Cap - Set *4T                |  |
| 4P1998     | 2    | 4 500  | S    | 0     | 1 600  | 1 300 | 1 600 | --    | -- | 0   | --    | --    | --    | Cap - Set *4U                |  |
| 4P1999     | 2    | 4 400  | S    | 0     | 1 550  | 1 300 | 1 550 | --    | -- | 0   | --    | --    | --    | Cap - Set *4V                |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P3670     | 14   | 18 300 | PI   | 475   | 17 825 | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
| 4P3671     | 2    | 6 425  | PI   | 475   | 5 950  | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
| 4P3672     | 2    | 6 375  | PI   | 475   | 5 900  | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
| 4P3673     | 2    | 6 275  | PI   | 475   | 5 800  | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
| 4P3674     | 2    | 6 100  | PI   | 475   | 5 625  | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
| 4P3675     | 2    | 5 825  | PI   | 475   | 5 350  | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
| 4P3676     | 2    | 5 450  | PI   | 475   | 4 975  | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
| 4P3677     | 2    | 4 725  | PI   | 475   | 4 250  | --    | --    | --    | -- | --  | --    | --    | --    | Horizontal - Cap             |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
| 4P3680     | 4    | 12 025 | V    | --    | --     | --    | 4 625 | 7 400 | -- | --  | 2 340 | --    | --    | Horizontal - Cap             |  |
| 4P3681     | 4    | 11 850 | V    | --    | --     | --    | 4 625 | 7 225 | -- | --  | 2 285 | --    | --    | Horizontal - Cap             |  |
| 4P3682     | 4    | 11 125 | V    | --    | --     | --    | 4 625 | 6 500 | -- | --  | 2 055 | --    | --    | Horizontal - Cap             |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
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|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |
|            |      |        |      |       |        |       |       |       |    |     |       |       |       |                              |  |

2. Each crank bar, Type B, may be replaced by two (2) straight bars (one top and one bottom) of the same bar size as the crank bar. Payment in either case shall be based on crank bars as schedule on the plans.

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
*PIER NO. 4 REINF. SCHEDULE*

SHEET OF AUGUSTA, MAINE

| PROJECT DESIGN ENGINEER |                 | BY       | DATE |
|-------------------------|-----------------|----------|------|
| PLANS                   | DESIGN-DETAILED | J. Fosom |      |
|                         | CHECKED         |          |      |
|                         | REVISIONS       |          |      |
|                         | FIELD CHANGES   |          |      |

### REINFORCING STEEL SCHEDULE

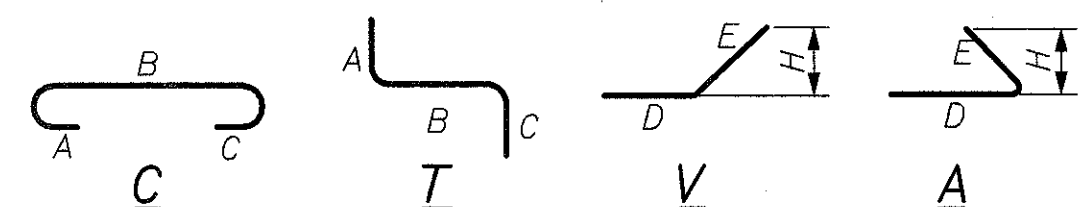
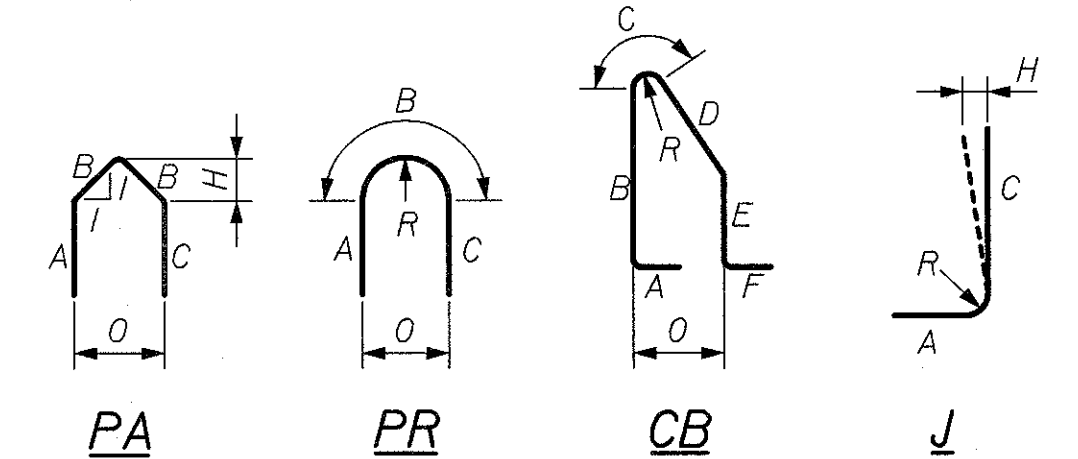
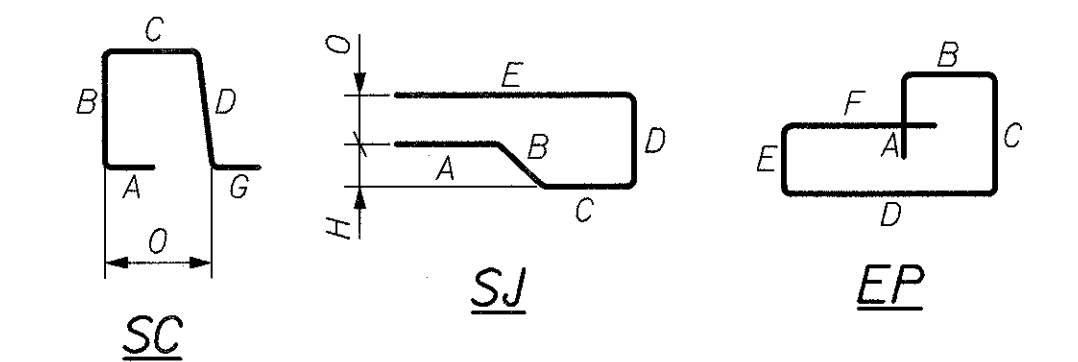
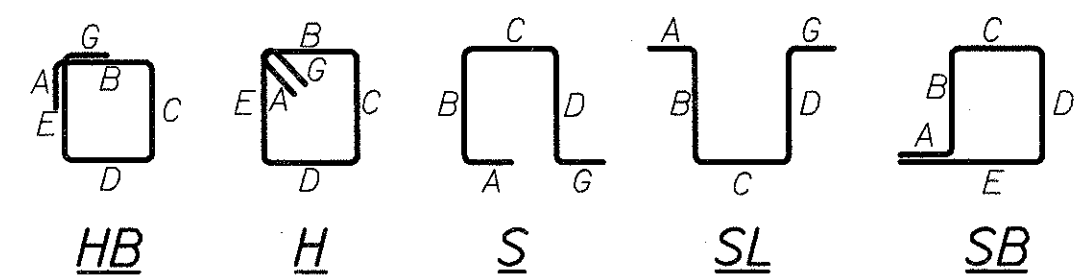
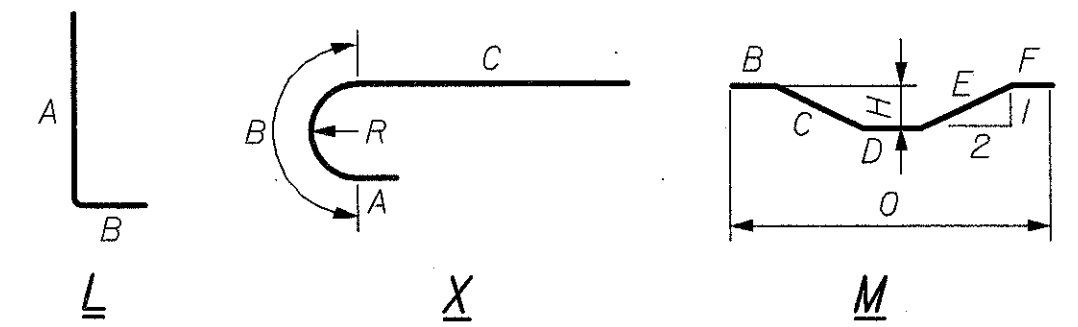
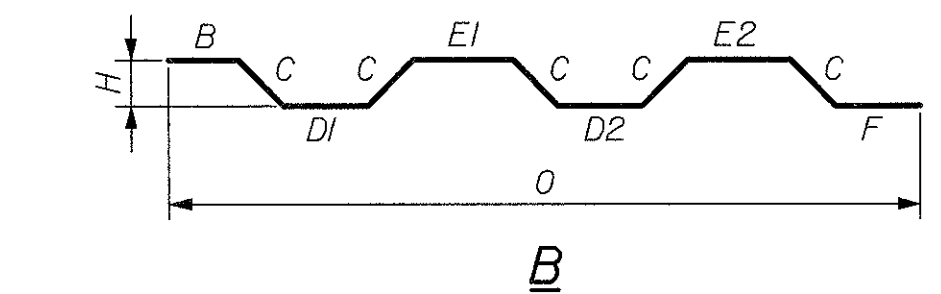
METRIC

1. All dimensions are in millimeters unless otherwise noted.  
2. All elevations and stations are in meters.

| F.H.W.A<br>REG. NO. | STATE | PROJECT NUMBER | SHEET<br>NO. | TOTAL<br>SHEETS |
|---------------------|-------|----------------|--------------|-----------------|
| 1                   | MAINE | NH-0556(220)E  | 100          | 105             |

[illegible][illegible]

### TYPE - BENDING DIAGRAMS



All dimensions are out-to-out of bar.

Bending details and hooks shall conform to the recommendations of the current revision of ACI Standard 318.

Reinforcing Bar: ASTM A615/A615M, Grade 420

## GENERAL NOTES

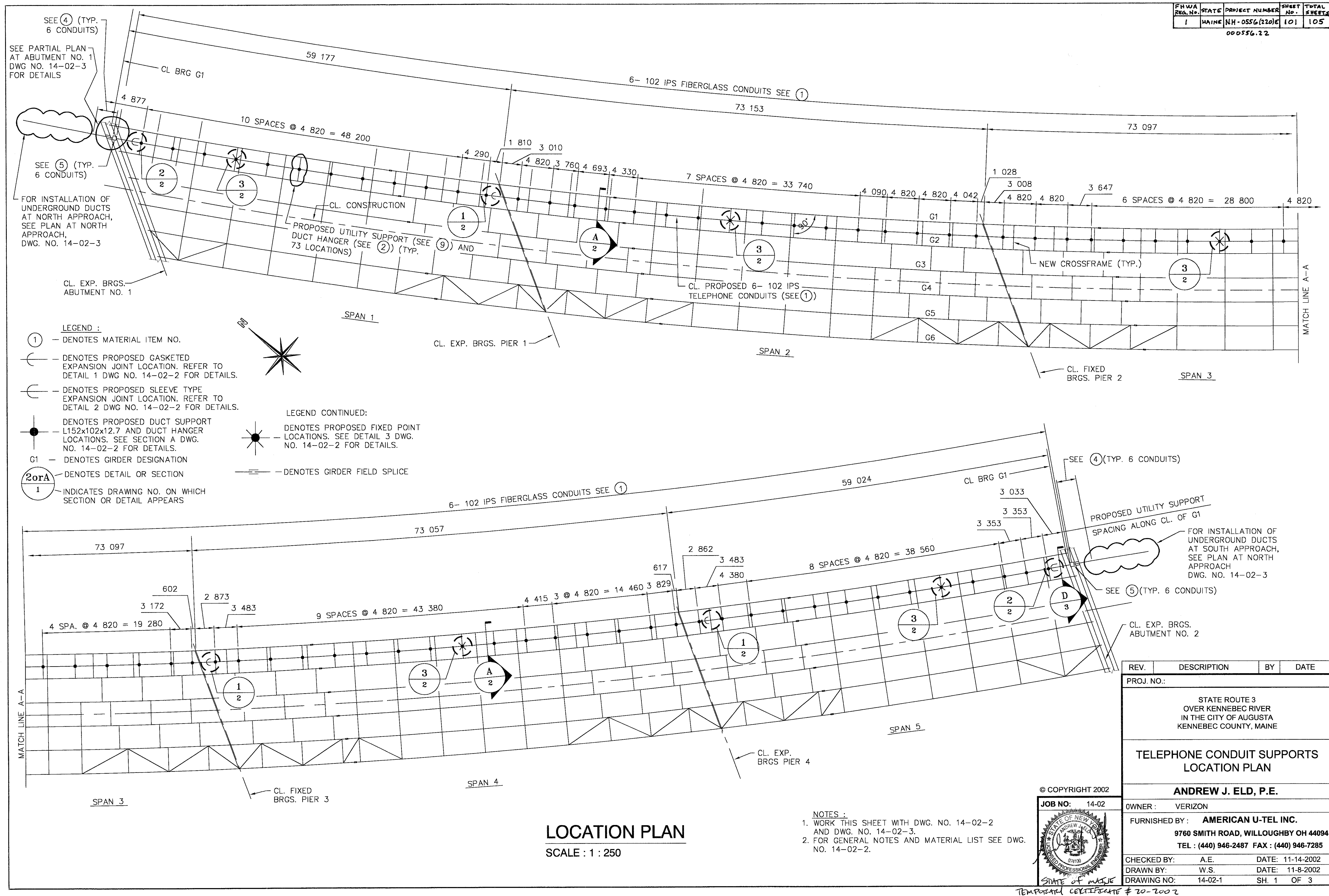
1. The first two digits following the letter(s) of the mark indicate the size of the bar:  
  
Mark 'A1602' = bar size #16  
Mark 'P2501' = bar size #25  
Mark 'S1950' = bar size #19
2. Each crank bar, Type B, may be replaced by two (2) straight bars (one top and one bottom) of the same bar size as the crank bar. Payment in either case shall be based on crank bars as schedule on the plans.

BRIDGE NO. 6400

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION

STATE ROUTE 3  
OVER  
KENNEBEC RIVER  
IN THE CITY OF  
AUGUSTA  
KENNEBEC COUNTY  
*SPR. SLAB REINF. SCHEDULE*

SHEET OF AUGUSTA, MAINE

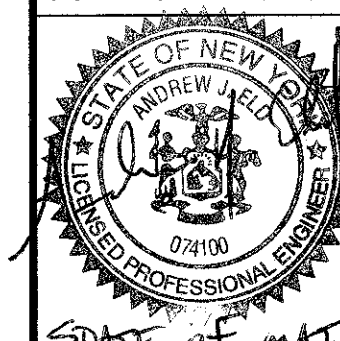


# LOCATION PLAN

SCALE : 1 : 250

© COPYRIGHT 2002

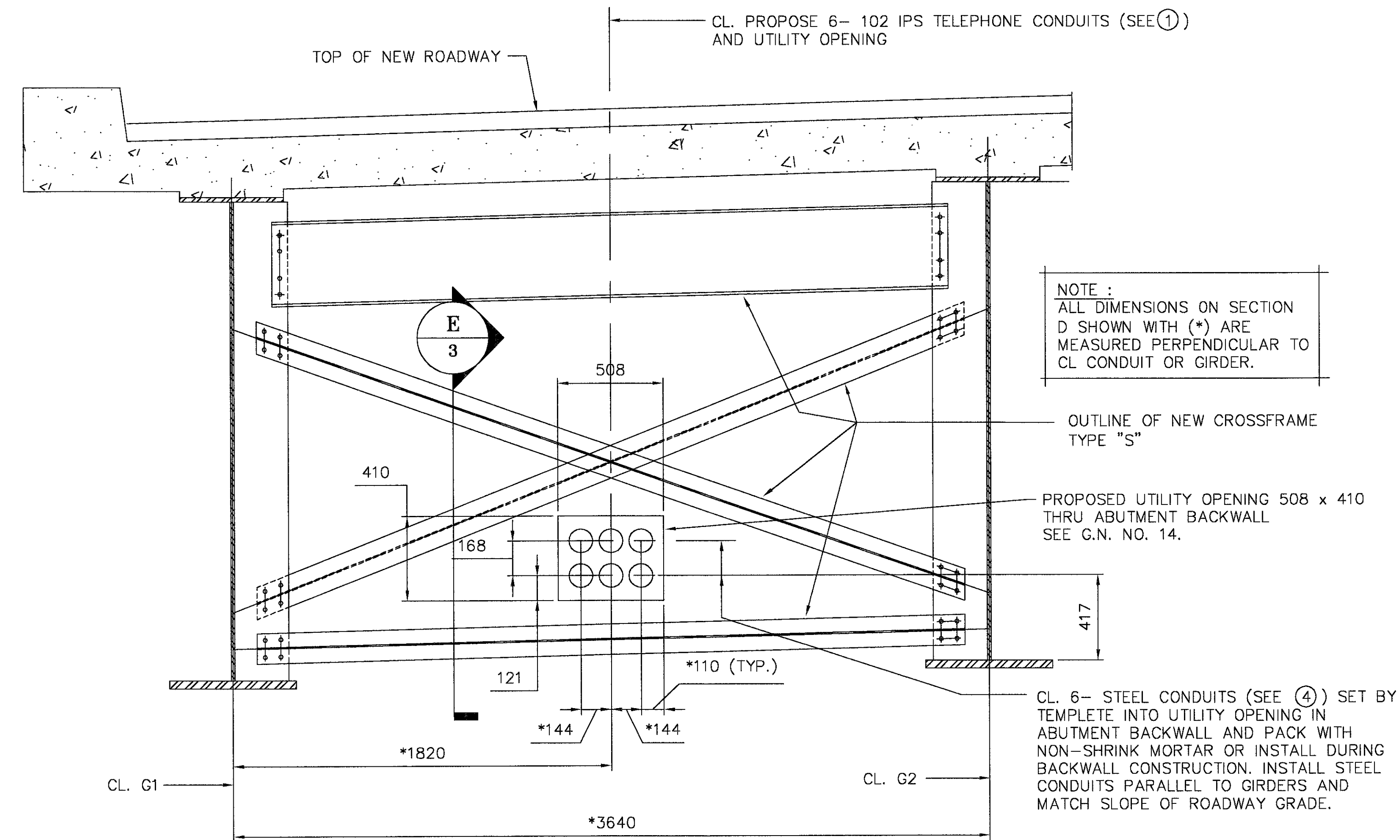
JOB NO: 14-02



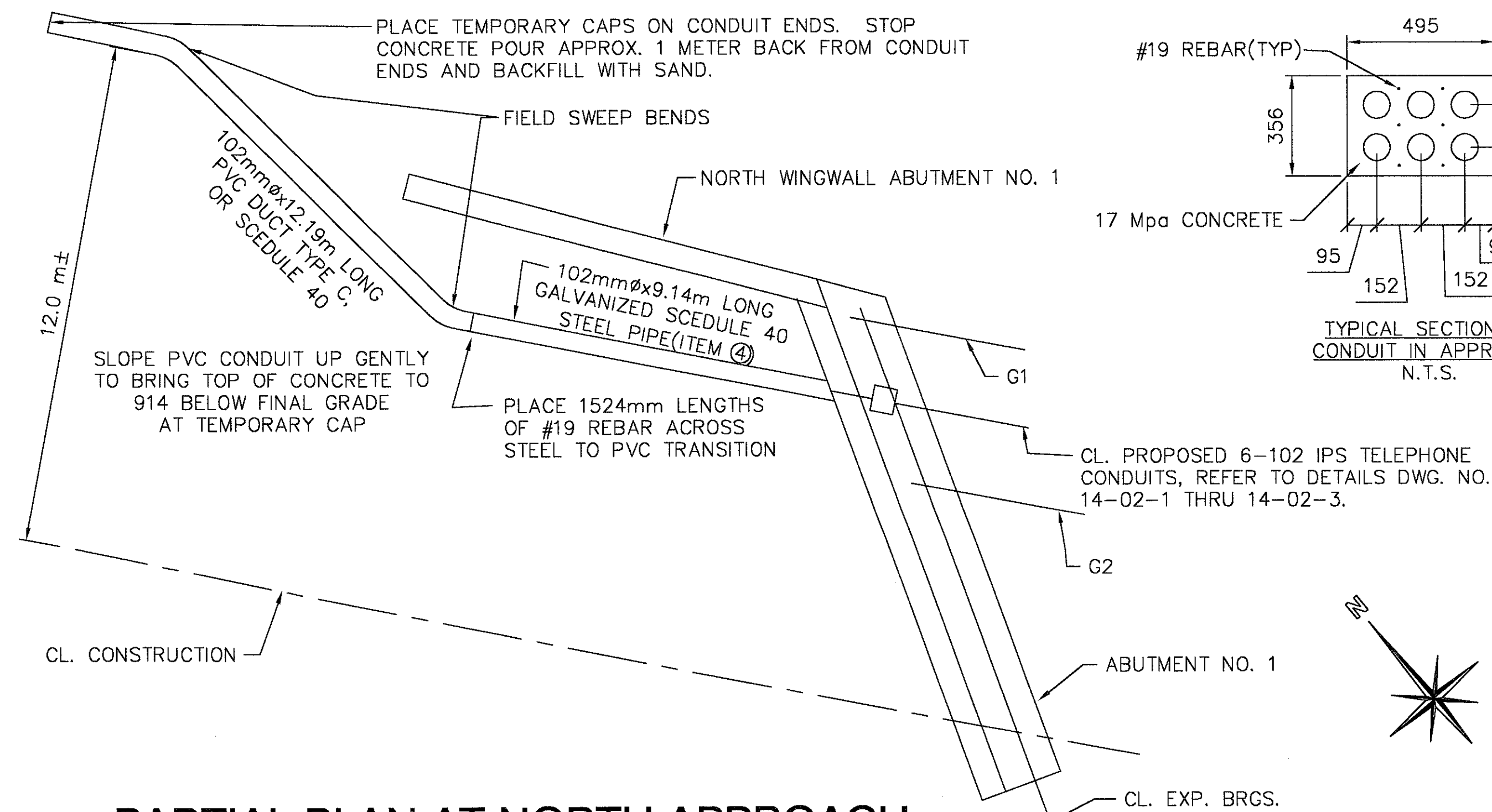
TEMPORARY CERTIFICATE # 20-2002

| REV.                                                                                     | DESCRIPTION | BY | DATE |
|------------------------------------------------------------------------------------------|-------------|----|------|
| PROJ. NO.:                                                                               |             |    |      |
| STATE ROUTE 3<br>OVER KENNEBEC RIVER<br>IN THE CITY OF AUGUSTA<br>KENNEBEC COUNTY, MAINE |             |    |      |
| TELEPHONE CONDUIT SUPPORTS<br>LOCATION PLAN                                              |             |    |      |
| ANDREW J. ELD, P.E.                                                                      |             |    |      |
| OWNER: VERIZON                                                                           |             |    |      |
| FURNISHED BY: AMERICAN U-TEL INC.                                                        |             |    |      |
| 9760 SMITH ROAD, WILLOUGHBY OH 44094                                                     |             |    |      |
| TEL : (440) 946-2487 FAX : (440) 946-7285                                                |             |    |      |
| CHECKED BY: A.E. DATE: 11-14-2002                                                        |             |    |      |
| DRAWN BY: W.S. DATE: 11-8-2002                                                           |             |    |      |
| DRAWING NO: 14-02-1 SH. 1 OF 3                                                           |             |    |      |

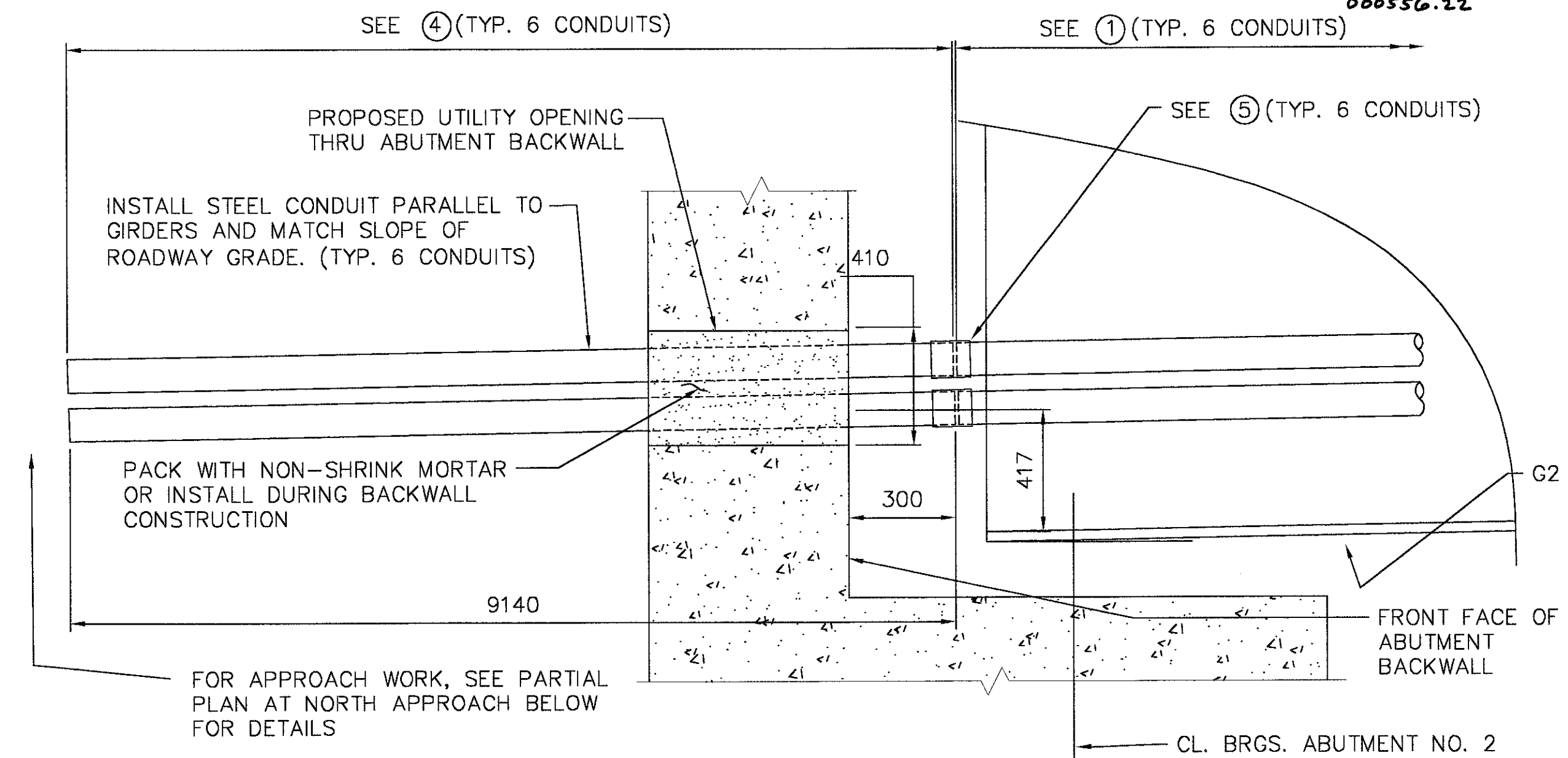




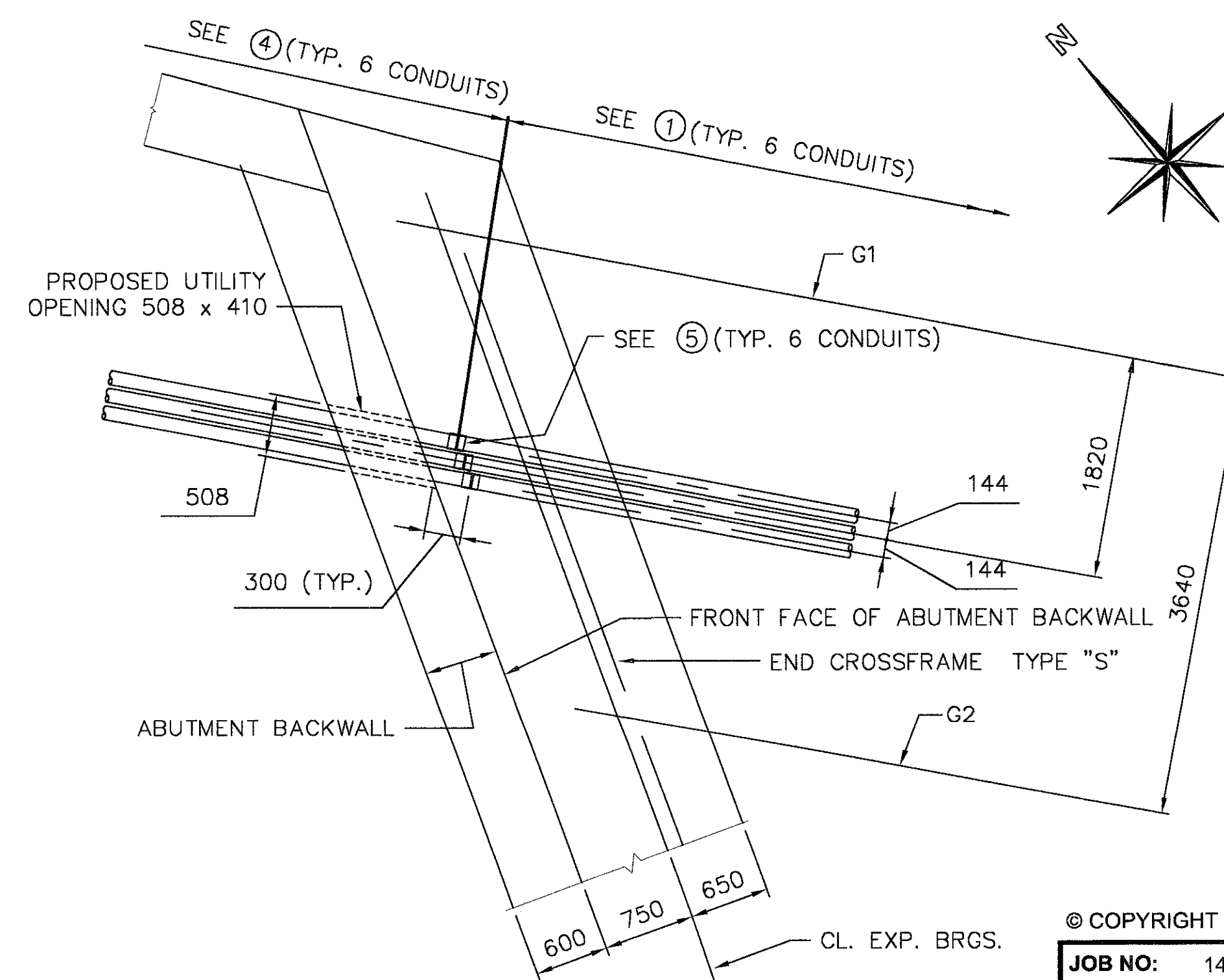
**D SECTION**  
1  
SCALE : 1 : 16  
(ABUTMENT NO. 2 SHOWN, ABUTMENT NO. 1 SIMILAR.)



**PARTIAL PLAN AT NORTH APPROACH**  
SCALE : 1 : 100  
(ABUTMENT NO. 1 SHOWN, USE SIMILAR REVERSED LAYOUT FOR ABUTMENT NO. 2)



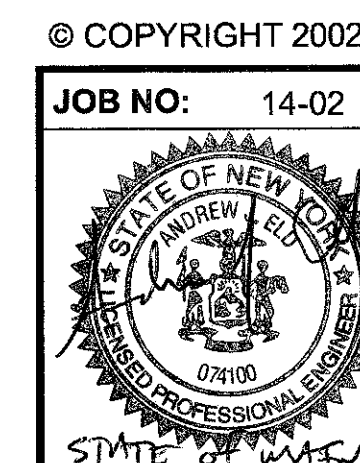
**E SECTION**  
3  
SCALE : 1 : 16  
ABUTMENT NO. 2 SHOWN, ABUTMENT NO. 1 SIMILAR.)



**PARTIAL PLAN AT ABUTMENT NO. 1**  
SCALE : 1 : 40  
(ABUTMENT NO. 1 SHOWN, ABUTMENT NO. 2 SIMILAR.)

- NOTES :
1. WORK THIS SHEET WITH DWG. NO. 14-02-1 AND 14-02-2.
  2. FOR GENERAL NOTES AND MATERIAL LIST SEE DWG. NO. 14-02-2.

| REV.                                                                                                                    | DESCRIPTION | BY | DATE |
|-------------------------------------------------------------------------------------------------------------------------|-------------|----|------|
| PROJ. NO.:                                                                                                              |             |    |      |
| STATE ROUTE 3<br>OVER KENNEBEC RIVER<br>IN THE CITY OF AUGUSTA<br>KENNEBEC COUNTY, MAINE                                |             |    |      |
| TELEPHONE CONDUIT SUPPORTS<br>ABUTMENT NO. 1 & 2 DETAILS                                                                |             |    |      |
| ANDREW J. ELD, P.E.                                                                                                     |             |    |      |
| OWNER : VERIZON                                                                                                         |             |    |      |
| FURNISHED BY : AMERICAN U-TEL INC.<br>9760 SMITH ROAD, WILLOUGHBY OH 44094<br>TEL : (440) 946-2487 FAX : (440) 946-7285 |             |    |      |
| CHECKED BY: A.E. DATE: 11-14-2002                                                                                       |             |    |      |
| DRAWN BY: W.S. DATE: 11-8-2002                                                                                          |             |    |      |
| DRAWING NO: 14-02-3 SH. 3 OF 3                                                                                          |             |    |      |



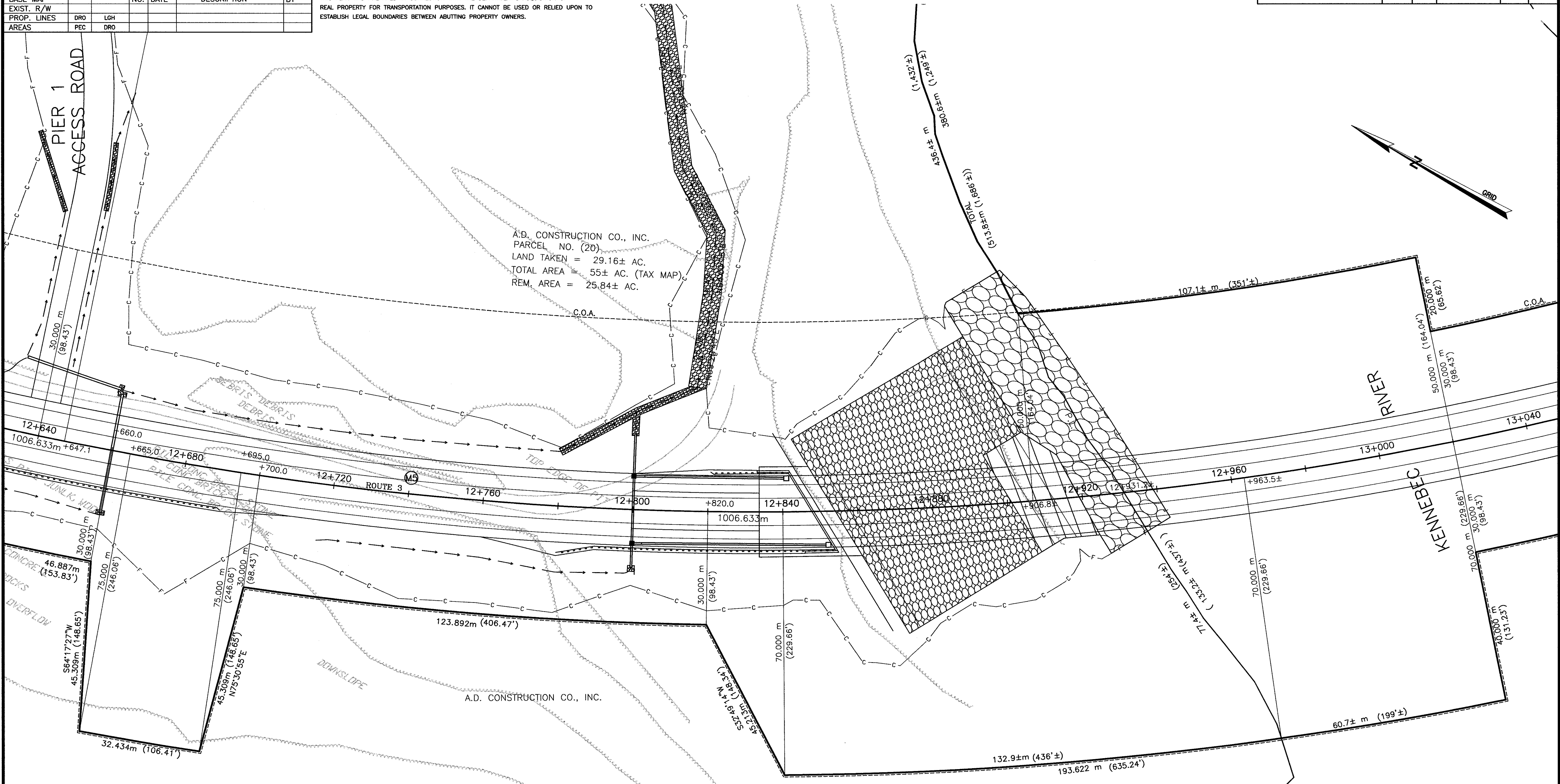
TEMPERATURE CERTIFICATE # 20-2002

|                         |      |
|-------------------------|------|
| PROJECT DESIGN ENGINEER | DATE |
| DESIGN-DETAILED         |      |
| CHECKED                 |      |
| REVISIONS               |      |
| FIELD CHANGES           |      |
| PLANS                   |      |

| ITEM        | TECH | CHECKED | REVISIONS | NO. | DATE | DESCRIPTION | BY |
|-------------|------|---------|-----------|-----|------|-------------|----|
| BASE MAP    |      |         |           |     |      |             |    |
| EXIST. R/W  |      |         |           |     |      |             |    |
| PROP. LINES | DRO  | LGH     |           |     |      |             |    |
| AREAS       | PEC  | DRO     |           |     |      |             |    |

THIS PLAN WAS PREPARED IN CONNECTION WITH THE DEPARTMENT'S ACQUISITION OF REAL PROPERTY FOR TRANSPORTATION PURPOSES. IT CANNOT BE USED OR RELIED UPON TO ESTABLISH LEGAL BOUNDARIES BETWEEN ADJUTING PROPERTY OWNERS.

|        |                   |               |                |           |              |
|--------|-------------------|---------------|----------------|-----------|--------------|
| METRIC | F.H.W.A. REG. NO. | STATE         | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
| 1      | MAINE             | NH-0556(220)E | 104            | 105       |              |



A.D. CONSTRUCTION CO., INC.  
PARCEL NO. (20)  
LAND TAKEN = 29.16± AC.  
TOTAL AREA = 55± AC. (TAX MAP)  
REM. AREA = 25.84± AC.

A.D. CONSTRUCTION CO., INC.

P.I.N. 000556.20

#### BASELINE CURVE DATA

PI STA = 13+174.259  
 $\Delta = 62^\circ-21'-08.0''$  LT.  
R = 925.000 m  
L = 1006.633 m  
T = 559.673 m  
E = 156.138 m

|                                                                                                                                                    |
|----------------------------------------------------------------------------------------------------------------------------------------------------|
| CONTROLLED ACCESS LIMIT                                                                                                                            |
| CONTROLLED ACCESS LIMIT LINE                                                                                                                       |
| ALONG R/W LINE                                                                                                                                     |
| R/W LINE                                                                                                                                           |
| CONTROLLED ACCESS LIMIT LINE NOT ON R/W LINE                                                                                                       |
| NO ACCESS WILL BE PERMITTED TO OR FROM STATE HIGHWAY "28-SPUR" AND ADJOINING PROPERTIES WITHIN BOUNDARIES DEFINED BY CONTROLLED ACCESS LIMIT LINES |

| SYMBOLS                                       |  | ○ - RAILROAD SPIKE                                      |       | △ - CONTROL MONUMENTS   |        |
|-----------------------------------------------|--|---------------------------------------------------------|-------|-------------------------|--------|
| IP (IRON PIPE or PIN)                         |  | MAINE DEPARTMENT OF TRANSPORTATION - CENTERLINE CONTROL |       |                         |        |
| S.T. (SEPTIC TANK)                            |  | MAINE STATE COORDINATE SYSTEM - ZONE                    |       |                         |        |
| WATER LINE                                    |  | CENTERLINE CONTROL MONUMENTS                            |       | TRAVERSE CONTROL POINTS |        |
| GAS LINE                                      |  | STATION                                                 | NORTH | EAST                    | NUMBER |
| ELECTRIC LINE                                 |  |                                                         |       |                         |        |
| TELEPHONE LINE                                |  |                                                         |       |                         |        |
| SEWER LINE                                    |  |                                                         |       |                         |        |
| PROPERTY LINE                                 |  |                                                         |       |                         |        |
| LIMITS OF WROUGHT PORTION                     |  |                                                         |       |                         |        |
| EXISTING RIGHT OF WAY                         |  |                                                         |       |                         |        |
| NEW RIGHT OF WAY                              |  |                                                         |       |                         |        |
| NEW RIGHT OF WAY WITHIN EXISTING RIGHT OF WAY |  |                                                         |       |                         |        |
| CONTROL OF ACCESS                             |  |                                                         |       |                         |        |

|                                                |                               |
|------------------------------------------------|-------------------------------|
| STATE OF MAINE                                 |                               |
| DEPARTMENT OF TRANSPORTATION                   |                               |
| 16 STATE HOUSE STATION, AUGUSTA, ME 04333-0016 |                               |
| RIGHT OF WAY MAP                               |                               |
| STATE HIGHWAY "28-SPUR"                        |                               |
| AUGUSTA                                        | KENNEBEC COUNTY               |
| FEDERAL AID PROJECT NO. STP - 0556 (20)        |                               |
| DATE: SEPTEMBER 2001                           | PART 2                        |
| SCALE: 1 = 500                                 | JOHN G. MELROSE               |
| SHEET NO. 16 OF 28 SHEETS                      | COMMISSIONER                  |
| D.O.T. FILE NO. 6-382                          | JOHN E. DORITY CHIEF ENGINEER |

ARCHITECTS ENGINEERS PLANNERS  
HNTB

| PLAN FILED IN PLAN BOOK |                          | PAGE         |         |
|-------------------------|--------------------------|--------------|---------|
| NO.                     | GRANTOR                  | INSTRUMENT   | DATE    |
|                         | PART 2 - PARCEL NUMBERS: | CONDEMNATION | 4/16/02 |
|                         | (20)                     |              | 6877    |
|                         |                          |              | 28      |

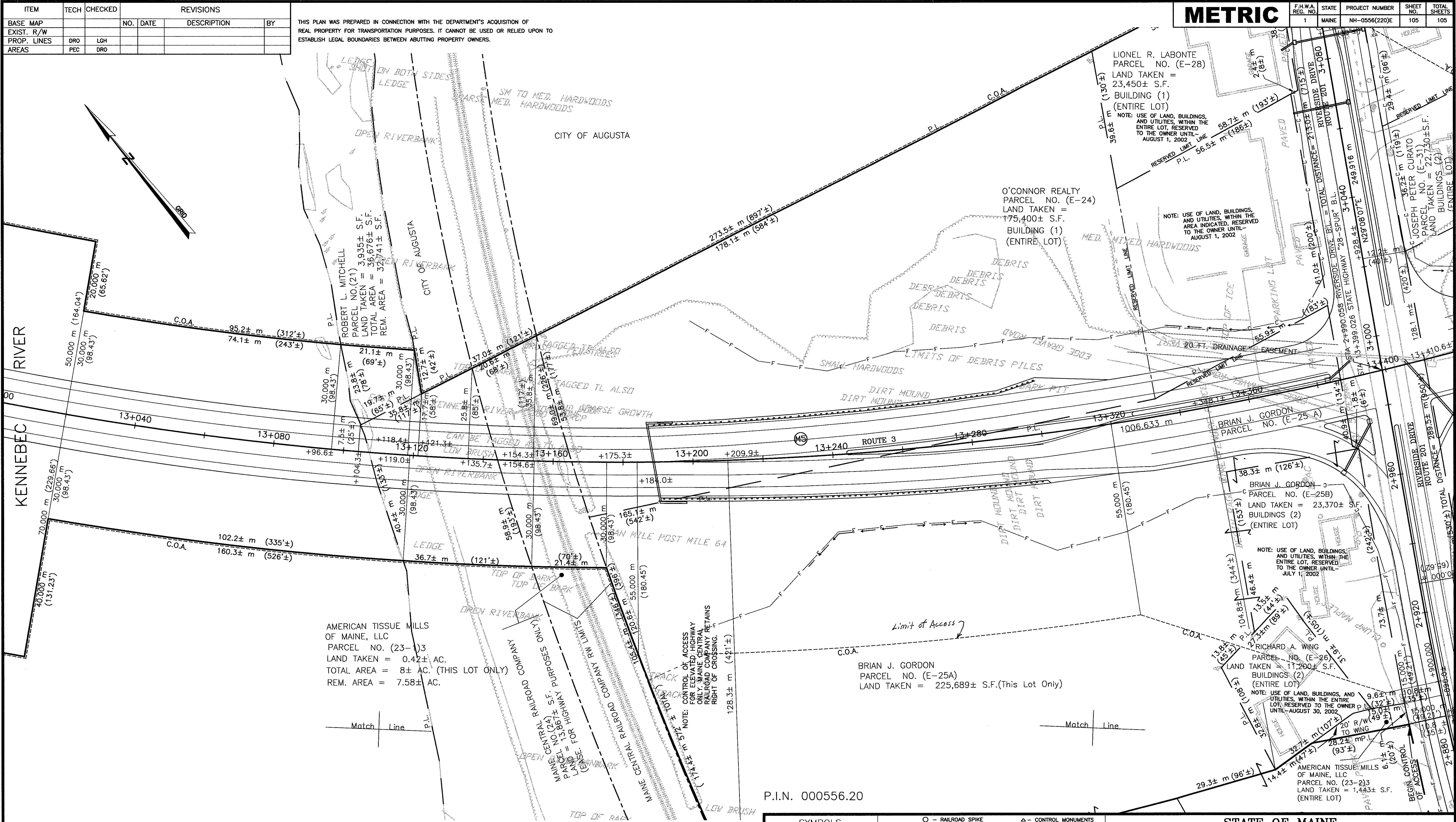
|                         |      |
|-------------------------|------|
| PROJECT DESIGN ENGINEER | DATE |
| DESIGN-DETAILED         | BY   |
| CHECKED                 |      |
| REVISIONS               |      |
| FIELD CHANGES           |      |
| PLANS                   |      |

| ITEM        | TECH | CHECKED | NO. | DATE | DESCRIPTION | BY |
|-------------|------|---------|-----|------|-------------|----|
| BASE MAP    |      |         |     |      |             |    |
| EXIST. R/W  |      |         |     |      |             |    |
| PROP. LINES | DRO  | LGH     |     |      |             |    |
| AREAS       | PEC  | DRO     |     |      |             |    |

THIS PLAN WAS PREPARED IN CONNECTION WITH THE DEPARTMENT'S ACQUISITION OF REAL PROPERTY FOR TRANSPORTATION PURPOSES. IT CANNOT BE USED OR RELIED UPON TO ESTABLISH LEGAL BOUNDARIES BETWEEN ADJUTING PROPERTY OWNERS.

METRIC

| F.H.W.A. REG. NO. | STATE | PROJECT NUMBER | SHEET NO. | TOTAL SHEETS |
|-------------------|-------|----------------|-----------|--------------|
| 1                 | MAINE | NH-0556(220)E  | 105       | 105          |



P.I.N. 000556.20

BASELINE CURVE DATA

PI STA = 13+174.259  
 $\Delta = 62^\circ-21'-08.0''$  LT.  
R = 925.000 m  
L = 1006.633 m  
T = 559.673 m  
E = 156.138 m

RIVERSIDE DRIVE - SEE SHC FILE NO 6-368

| NO. | GRANTOR                                          | INSTRUMENT   | DATE    | BOOK | PAGE |
|-----|--------------------------------------------------|--------------|---------|------|------|
|     | PART 2 - PARCEL NUMBERS:                         | CONDEMNATION | 4/18/02 | 8877 | 28   |
|     | (E-24), (E-25A), (E-25B), (E-26), (E-28), (E-31) |              |         |      |      |
|     | PART 4 - PARCEL NUMBERS:                         | CONDEMNATION |         |      |      |
|     | (21), (23-1), (23-2), (24)                       |              |         |      |      |

|                                                                                                                                                   |
|---------------------------------------------------------------------------------------------------------------------------------------------------|
| CONTROLLED ACCESS LIMIT                                                                                                                           |
| CONTROLLED ACCESS LIMIT LINE ALONG R/W LINE                                                                                                       |
| R/W LINE                                                                                                                                          |
| CONTROLLED ACCESS LIMIT LINE NOT ON R/W LINE                                                                                                      |
| NO ACCESS WILL BE PERMITTED TO OR FROM STATE HIGHWAY "28-SPUR" AND ADJUTING PROPERTIES WITHIN BOUNDARIES DEFINED BY CONTROLLED ACCESS LIMIT LINES |

| SYMBOLS                                       | MAINE DEPARTMENT OF TRANSPORTATION - CENTERLINE CONTROL |
|-----------------------------------------------|---------------------------------------------------------|
| IP (IRON PIPE OR PIN)                         | MAINE STATE COORDINATE SYSTEM - ZONE                    |
| S.T. (SEPTIC TANK)                            | CENTERLINE CONTROL MONUMENTS                            |
| WATER LINE                                    | TRAVERSE CONTROL POINTS                                 |
| GAS LINE                                      |                                                         |
| ELECTRIC LINE                                 |                                                         |
| TELEPHONE LINE                                |                                                         |
| SEWER LINE                                    |                                                         |
| PROPERTY LINE                                 |                                                         |
| LIMITS OF WROUGHT PORTION                     |                                                         |
| EXISTING RIGHT OF WAY                         |                                                         |
| NEW RIGHT OF WAY                              |                                                         |
| NEW RIGHT OF WAY WITHIN EXISTING RIGHT OF WAY |                                                         |
| CONTROL OF ACCESS                             |                                                         |

STATE OF MAINE  
DEPARTMENT OF TRANSPORTATION  
16 STATE HOUSE STATION, AUGUSTA, ME 04333-0016  
RIGHT OF WAY MAP

|                                         |                              |
|-----------------------------------------|------------------------------|
| AUGUSTA                                 | KENNEBEC COUNTY              |
| FEDERAL AID PROJECT NO. STP - 0556 (20) | PARTS 2 & 4                  |
| DATE: SEPTEMBER 2001                    | JOHN G. MELROSE              |
| SCALE: 1 = 500                          | COMMISSIONER                 |
| SHEET NO. 19 OF 28 SHEETS               |                              |
| D.O.T. FILE NO. 6-382                   | JOHN E. DORTY CHIEF ENGINEER |