| # | Question | Response | Amendment |
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| 1 | When does the Bidder submit their Prequal material? | Bidders are encouraged to submit their prequalification information before submitting their bids, however prequalification information submission can occur during or after bid submission. Bids will be received from all Bidders; however, the contract will only be awarded to Bidders passing the qualification requirements as stated in the Notice to Contractors. Apparent low bidders not prequalified at the time of bid opening can expect a two to three week delay in the award process while their application is processed and also run the risk of not meeting prequalification standards. For more information, please see Prequalification - MaineDOT Contracts Section or email Project Manager Sean Ledwin at Sean.M.Ledwin@maine.gov. | 1 |
| 5 | Are there temporary in-water impacts allowed for construction access? The only temporary impacts listed on the plans are for the cofferdams. | Temporary environmental impacts in accordance with permits in Exhibit E are allowed within the in-water work period, which is now fully unrestricted per agency concurrence, and must be described in the Work Plan submittal required after Contract execution. However, impacts to Woodland Pulp LLC's operations, such as powerhouse unit shutdowns or reductions, may be heavily limited and must be coordinated with Woodland Pulp LLC. Bidders shall include in their bids any temporary impact timeframes proposed for such reductions or shutdowns of generation from the hydro station to facilitate activities such as cofferdam installations. Bidders shall minimize their proposed outage timeframes to limit impacts to Woodland Pulp LLC operations to the extent practicable. | 3 |
| 6 | Has the alternative In-water TOY Restriction been approved and made formal to only restrict In-water work May 15-June30? | Yes, Maine DMR has received final concurrence from all required agencies. The in-water work period is now fully unrestricted per recent correspondence from USACE enclosed with this Amendment and incorporated into Exhibit E by reference. The USACE permit is valid until October 15, 2026, as noted in the correspondence, but Bidders shall assume the permit will be reissued or extended up to the completion date in the Contract Agreement, Offer & Award with the same unrestricted in-water work provisions. | 3 |
| 7 | Will both sets of overhead power lines on/off the island substation remain energized the entire project? | The power lines on/off the island substation will remain energized with the exception of pre-approved construction windows of limited duration. Bidders shall include in their bids their proposed outage plan and timeline(s) for these electric power distribution cables. Bidders shall minimize their proposed outage timeframes to limit impacts to Woodland Pulp LLC operations to the extent practicable. | 3 |
| 8 | Where is the intended location for the relocated substation guy wires, and what is the intended support structure? | The Contractor shall be responsible for submitting a proposed relocation of the guy wires in-kind as much as feasible for review and approval by the Engineer and Woodland Pulp LLC. Existing drawings are not available for the substation at this time, so Bidders shall assume relocation must be in close proximity to existing and with similar anchorage capacity. | 3 |
| 9 | Is there access to the island through the dam powerhouse/generator room? | Access through the powerhouse will be pedestrian traffic only with very limited equipment permitted at the discretion of Woodland Pulp LLC. The Contractor must coordinate requests for small equipment access through the powerhouse with Woodland Pulp LLC but shall not assume such requests will be approved. | 3 |
| 10 | Fish Lift Isolation Gate: Please confirm there is a suspended concrete cut-off wall in front of the Isolation Gate. This is shown on M-100 but not the Structural plans. Also, do the Isolation Gate and Stop Log bondouts require steel embeds? If so, please provide details. | There is a suspended concrete cut-off wall in front of the isolation gate as shown on M-100. The isolation gate will require embeds into the concrete channel. Bidders shall assume the gate manufacturer is responsible for designing and supplying the gate guides and embeds. Refer to Specification Section 35 20 10 for fabricated gate design and submittal requirements. | 3 |
| 16 | Drawings S-303 & 304 reference a slab on deck at elevation 132'-6", but there is not other reference to this elevation on the framing plans. Please provide the limits. Are there any other slabs withing this structure? | The 9'-3 ¼" x 8'-6 ½" concrete slab limits are shown between gridlines B and C of Sections A and B and between gridlines 1 and 2 of Section E on S-304. There are no other slabs within this structure. | 3 |
| 17 | Please provide details of the Access Road Retaining Wall. | Retaining wall structural details are provided in the Approach Ramp Section, Section D of S-185, which is the portion of the access road approaching Access Bridge Abutment 2 shown on S-180. | 3 |
| 18 | It is stated on all paint specs that the color is to be selected by Woodland Pulp. Please provide a list of all paint colors for each application. | This will be a Contractor submittal item for WPLLC approval in general accordance with Specification 09 90 00. Verdantas has included color choices where specific performance requirements apply, such as "beige or off-white" under 2.03-C-3 for the Steel Trough and Flume Interior Lining to protect against UV exposure. Where selection by Woodland Pulp is noted, the Specification language also identifies color family (i.e. light for which colors such as off-white, beige, or light grey are expected to be acceptable) that Bidders shall use for pricing purposes. | 6 |
| 19 | Please provide alloy, Linear feet and fitting quantities for the 16" viewing tank pipe. | Plan and profile for the viewing tank pipe are provided on C-304. 16" OD ASME B36.10M steel pipe, standard weight. Refer to Spec Section 40 20 55 – 2.01 for allowable pipe grades. Pipe length is approximately 53 feet long with 6 flanged joints. | 6 |
| 20 | Please provide the specifications on Rotork IQT 90 Degree Motor Operator. | Refer to Specification 11 10 00 in which the 90-degree motor operator is identified as "Quarter-Turn". | 6 |

| # | Question | Response | Amendment |
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| 21 | How tall is the viewing tank? Drawing M-123 only shows diameter being 6' with (2) drains, crowder screen and pump. | The viewing tank is 3 feet tall per Elevation A of M-123. | 6 |
| 23 | Please clarify who is supplying the (4) Harrington hoist models? | The Contractor shall supply the hoists. | 6 |
| 24 | What is the duration for the contractor to create a security gate and staff it. Will the contractor be required to remove at completion of the project? | The Contractor shall install the security gate and provide staffing during mobilization prior to frequent access by its personnel within the WPLLC fence perimeter for staging/storage or construction purposes. Contractor access for scoping of utilities, installation of fence components, and other preparatory purposes may be coordinated with WPLLC before the Contractor's gate is fully functional. Bidders shall assume the gate remains permanently in place for WPLLC future use and shall turn over all security lock keys and/or codes to WPLLC upon demobilization. | 6 |
| 25 | Please provide details on the tank fill pipe (size, alloy, LF, fittings, supports). | The holding tank fill pipe called out on M-123 shall be flexible, 4-inch-diameter hose. The Contractor shall include in its tank shop drawings submittal the proposed flexible fill hose suitable for outdoor use under UV exposure, fittings, and suitable length and routing to avoid creating tripping hazards. The Contractor shall also include a proposed means of retrieving the submersible pump for winter storage. | 6 |
| 26 | Please clarify the drain location for the (3) winterization drains on the air burst system. | The air burst system manufacturer shall specify the proposed drain locations. | 6 |
| 27 | Please confirm all of the lifting motor operators are to be Rotork IQT 90 degree. The make/model has not been specified on the drawings or specifications. | Confirmed that the gate lifting motor operators identified on M-102 and M-124 are IQ T 90-degree (Quarter-Turn). Refer to EE-BA-001 of the Electrical Design Plans for model information on all IQ and IQ T series Rotork motor operators. | 6 |
| 28 | Please provide the specifications and preferred manufacturer for wire rope drum & motor operator for the Fish Lift Entrance Gate EG2. | Acceptable suppliers for the Hopper Electric Wire Rope Hoist are included in Specification Section 41 22 00. The Contractor shall be responsible for proposing the manufacturer for the wire rope drum and motor operator suitable for the outdoor environment in its required submittal. | 6 |
| 29 | How many Hamilton wheels are needed to move the floor? Drawing M-127 shows (4), please confirm. | Section A on M-128 shows five (5) wheels per L4x4 vertical leg for a total of 20 required. | 6 |
| 32 | The water level probes will require supports for the various lengths of SS pipe. Please advise as to the size of these supports. | The Contractor shall propose supports spaced no greater than 4 feet on-center in its shop drawing submittal for the Still Well Water Level Probes. | 6 |
| 35 | Please clarify the length of the (9) W8x18 screen supports shown on DWG S-124 | Tops of concrete for the flume walls and floor are Elevation 146.36 feet and Elevation 135.4 feet, respectively, as shown on Section D of S-122. Accounting for the top 1 ½" grating and the base ½" grout and ½" plate, the length of each W8x18 is approximately 10.75 feet. | 6 |
| 37 | Please confirm viewing room and counting room are the same building | Confirmed. | 6 |
| 38 | Please confirm all pipe flanges are FF | Assuming FF denotes flat-faced, then it is confirmed that all pipe flanges are FF. | 6 |
| 42 | Do you have specifications for the curtain, depths, types, and quantity in lengths? | It is assumed that "curtain" refers to the Curtain Wall shown on Sheet S-101. Refer to the Stoplog Schedule on M-001 requiring nine (9) 12"-high stoplogs approximately 14 feet long. If this is not the intent of this question, please submit another RFI with more detail and reference to applicable drawing(s) and/or specification(s). | 6 |

| # | Question | Response | Amendment |
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| 45 | Addendum #3 response to questions 5 & 7 indicate bidders shall include in their bids their proposed outage plan and timeline(s) for impacts to hydro and/or electric power distribution. Please clarify the intent. Is a written narrative of our work plan and/or clarifications required with the bid, or is the intent to price in the cost of the shutdown time? If the latter, what are the rates for and allowable durations for shutdown impacts? | Bidders shall assume the following key hydro turbine and high-voltage power outages to prepare bids and may disregard the implication of a written work plan provided prior to award. If deviation from any of these is proposed, submit an RFI detailing the proposed alternate outage(s) and include anticipated time-of-year for WPLLC's consideration of flow conditions and operational constraints.<br><br>Bidders are reminded that, after award and contracting, the Contractor is required to submit its detailed work plan and schedule per the Design Specifications that shall clearly identify outage types, durations, and dates within the work sequencing to be reviewed for acceptance by the Engineer, Maine DMR, and WPLLC. Activities and submittals identified below as the responsibility of the "Contractor" or the "awarded Contractor" shall be interpreted to occur after the construction contract is fully executed.<br><br>Note that hydro station outages cannot overlap with any proposed outage of the 34kV overhead lines, and the Contractor's onsite crews shall be scheduled to work double shifts through each outage period to minimize respective duration and subsequent impacts to WPLLC operations.<br><br>**Hydro Station Outages**<br>1. Outage of two hydro units for up to 4 weeks for installation of the cofferdam within the tailrace.<br>2. Outage of two hydro units for up to 3 weeks for removal of the cofferdam within the tailrace.<br>3. Outage of two units for 3 to 5 days to slow water flow and reduce turbulence in the tailrace sufficiently to move the access bridge for installation.<br>4. The awarded Contractor must coordinate closely with WPLLC to sequence hydro unit outages in 3 phases for trash rack replacement and bypass work to allow some generation at the hydro station at all times. Double shifts are required unless the work is scheduled to occur during seasonal low-flow periods when full generation is not feasible.<br><br>**Overhead 34kV Line Outages**<br>5. Outage of the 34kV overhead distribution lines for two shifts to install insulating jackets on the lines, which will facilitate crane placement and picks and delineation of the wires for added safety.<br>6. Up to 20 additional 12-hours shifts are considered acceptable for critical work activities in close proximity to the overhead lines.<br>A. The Contractor shall propose additional protection measures to be deployed during the outages permitted by WPLLC to avoid or limit further outage for work on the new fish ladder due to its low overhead clearance to the lines in select locations.<br><br>**Cable Duct Outage**<br>7. A limited-duration outage of the cable duct for relocation during construction may be considered acceptable. If so elected, the awarded Contractor shall submit a detailed plan for review and approval by WPLLC that includes duration, method, materials, and route proposed for the relocation. | 7 |
| 46 | Do the micropile cased lengths within the micropile schedules on drawings S-455, S-456, and S-457 include the portion of the casing that is embedded in the pile cap? | No, the cased length shown in the micropile schedule starts at the bottom of the footing and does not include the embedded length within the pile cap. | 7 |
| 47 | Specification Section 31 63 33 Mircropiles and drawing S-458 state threaded casing joints/connections are not permitted, which is unusual. What is the rationale for this? Note 10 on drawing S-458 requires micropiles to be able to extend 200% deeper than expected. Threaded casing connections are required to allow this flexibility. Will threaded casing connections be permitted? | Given the height of the structure and seismicity of the area, there is a relatively high bending stress on the casing. Consideration of corrosion loss indicates there is insufficient steel section at the joints. Threaded casing joints within the upper 10 feet are therefore not permitted unless the joints are reinforced, thicker casing is proposed, or an inner casing is used and staggered so that the joints are not overlapping. Also note that "200 percent depth" in Note 10 will be corrected to clarify double the expected depth since 200% of depth can be inferred as quadruple. | 7 |
| 48 | Is compression testing required for micropiles that are to be verification tested or proof tested? | Yes. Refer to Notes 12 and 13 of Sheet S-458. | 7 |

| # | Question | Response | Amendment |
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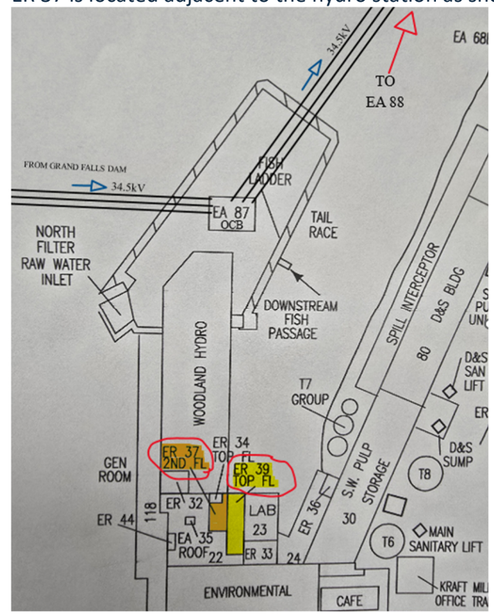
| 54 | Please provide the location of Primary Mill Source ER37 to include available existing raceways. | ER 37 is located adjacent to the hydro station as shown in the map below:   | 7 |
| 58 | Please provide manufacturer and part # of V-Gate limit switches VG3AZS & VG3BZS. | The Contractor shall propose the manufacturer and part numbers for the limit switches with its shop drawing submittal for the V-Gate. Refer to Question 57. | 7 |
| 59 | Please provide manufacturer and part # of Water Level Sensors 1 thru 8. | The Contractor shall propose manufacturer and part numbers in its shop drawing submittal for the Still Well Water Level Probes. The sensors shall be pressure transducers with accuracy tolerance no greater than +/-0.1%, operating temperature range of at least +/- 10 deg C beyond historic water temperature fluctuations, IP68 or better rating, stainless steel or titanium body, sampling rate capability of 2 seconds or less, overpressure protection, and temperature compensation. Examples include the YSI Model WL-450 and the Campbell Scientific CS451, but the Contractor may propose any equivalent meeting these requirements. | 7 |
| 60 | Is contractor responsible for programing Fish lift Control Panel? If so please provide order of operation. | Yes, the Contractor shall be responsible for programming inclusive of order of operation. The Contractor shall submit an operation sequencing diagram and description to the Engineer for review and acceptance as part of its testing plan or as a separate submittal in advance of commencing programming as needed. | 7 |
| 63 | The foundation schedule on sheet S-450 provides the top of concrete elevations for the foundations. The provided elevations do not match the elevations provided on sheets C-142, C-143, and C144. Please confirm which elevations govern. | The bedrock profiles shown in the civil plans are approximate and may vary. Bidders shall refer to the top-of-concrete elevations shown in the Foundation Schedule of S-450. | 7 |
| 64 | Please provide the number of Fiberoptic pairs required for Multimode Fiber Optic Cables. | All Fiber shall be no less than 48 strands when pulled from building to building per WPLLC requirements. Refer to the enclosed WPLLC Networking Installation Specification for further details on which the electrical design was based. | 7 |
| 67 | Note 3 states, "Existing Storm Drain System. Replace In-Kind. Coordinate with Woodland Pulp." Please provide details on what this work entails. | Bidders shall assume the existing drains will be protected and re-established in their existing locations after completion of the access road. The awarded Contractor shall submit a proposed means of extending the drains up to the new finished access road elevation. | 7 |

| # | Question | Response | Amendment |
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| 69 | Please provide a detail or explanation of the bottom stiffener plates for the steel trough. | Assuming this is the bypass trough on S-160, there are (5) ½" x 6" steel bottom stiffeners. The stiffeners are spaced at 2'-0" on center. There are 14 segments. Refer to S-163 and S-164.<br><br>Refer to S-167, S-169, S-171, and S-173 for the steel transition flume (1-4). The connection plates are ½" x 6". The back stiffeners vary in thickness depending on location; refer to Bypass Lower Plan View and the Detail 3 Back Stiffener (PL-4 to PL-6). The additional back stiffeners are ½" x 6"; refer to Bypass Lower Plan View and the Detail 4 Additional Back Stiffener (PL-8 to PL-11). | 7 |
| 70 | The quantity of the supports needed on the interior wall of the steel trough are unclear. Please advise. | Assuming this is the bypass trough on S-160, there are 4 supports per segment. There are 14 segments. On steel transition flume (1-4), there are 4 supports per segment. Total there are 72 total supports. | 7 |
| 72 | Several vendors are having difficult procuring large bore HDG pipe domestically, would a substitute for SS be acceptable? | We assume this question refers to the galvanized steel casing shown in Section B of Sheet S-459. Due to strength requirements, stainless steel is not an acceptable alternative. Non-domestic galvanized steel pipe is acceptable for this application. | 7 |
| 74 | Drawing G-121 shows removal of berm. At the site visit it was unclear whether the material was contaminated or not. Please clarify. Also does is it the intent for the berm to be rebuilt? | The response to this question will be provided no later than July 16, 2025.<br><br>The Contractor shall be responsible for sampling and testing of the berm soil and for providing the results to WPLLC. The Contractor shall also provide proper documentation to WPLLC if the Contractor intends to remove the berm soil for offsite disposal and will be responsible for selection of the disposal site and the associated costs. Bidders may instead assume the Contractor can stockpile the soil on site, the location of which shall be coordinated with WPLLC, until it can be restored to its original location by the Contractor. | 7 |
| 75 | Please provide all site overhead line voltages for the determination and maintaining safe working distances. | Incoming and outgoing overhead lines are 34.5kV. Refer to the map provided under Question 54. | 7 |
| 77 | Micropile drawing S-459, detail D shows a galvanized plate that is welded to the micropile. The welding of galvanized material concerns us. Would either of the 2 approached be allowed if the contractor can provide design calculations to support?<br>2 approaches would resolve this:<br>1. We would suggest either the plates and stiffeners are uncoated bare steel.<br>2. Thicken the 10"x10" galv plate, eliminate the welding and stiffeners. (typical approach) | Either option is acceptable. The plate and stiffeners are in concrete. | 7 |
| 78 | Micropile drawing S-455, S456, S457 require micropile casing lengths of 12.5 to 22.5 feet. S-458 Note16 and Section 31 63 33 Micropile 3.04 C4 state threaded pipe casing joint are not permitted. Please confirm casing joints are not allowed, or would either of the approach be allowed if the contractor can provide design calculations to support? Please consider the information below from a micropile installer's point of view – thank you.A standard micropile rig will install casing in 10 foot flush joint threaded sections. This allows the drill to adjust the cased length with uneven rock elevation. The following 2 approaches will allow standard equipment to be used. 1. The joints can be drilled such that they are a minimum of 9 foot below cut off (10 foot + room for tolerances). That joint would be adequately confined but ~ 8 feet of soil. 2. The joint could be reinforced with an inner-casing meeting or exceeding the allowable bending.If no threaded joints are allowed, the micropile will be much more costly to install and be "out-of-the-ordinary". Possible solutions are:• Mobilize a long stroke drill (into a small area), start with longer casing, and hit rock where plans say +/-• Propose welding procedure for field welding a full-pen 80ksi pipe., impractical.• Use temporary oversized sectional casing, practical but expensive. | Refer to Question 47. Standard micropile rig and threaded joints are agreeable provided the Contractor submits design calculations to support for the reinforced inner casing for review and acceptance by the Engineer. | 7 |
| 79 | Sheet S-458, Note 12 specifies 5 proof tests on production pile.<br>Sheet S-458, Note 13 specifies 5 verification tests on sacrificial pile.<br>Section 31 63 33 Micropile 3.06 specifies compression, tension, and lateral load testing.<br>Typically all proof and verification testing of micropile are in tension to eliminate end bearing contribution and compression tests are 200% more costly. Please specify if compression tests and/or lateral test are required and how many of each. | Five proof tests and five verification compression tests are required given the poor rock quality encountered. | 7 |

| # | Question | Response | Amendment |
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| 80 | Addenda 2 lists 3 types of rock anchors (see below)<br>Will all rock anchor need to be load tested?<br>Fish Lift Rock Anchors - #8 75 ksi Galvanized Steel Bar x LF 156<br>Access Bridge Rock Anchors - 2" Hollow Core 85 ksi Galvanized Steel Bar x LF 85<br>Access Bridge Rock Anchors - #8 Dowels x LF 196 | The #8 dowels will not need to be load tested. The Fish Lift Rock Anchors and 2” hollow core anchors at the bridge will need to be load tested; refer to Note 5 of Sheet S-182. | 7 |
| 82 | Butterfly Valves are called out as an AWWA spec, are we able to use LUG style due to the availability? | The use of 24” and 30” lug style butterfly valves on the attraction water system pipes may be an option. The option exists to use a ductile or steel bodied lug style valve provided a variety of conditions are met. For the lug style butterfly valve to be acceptable, it would need to be manufactured in accordance with acceptable standards (AWWA, API, ASME, etc). Furthermore, the valves would need to be manufactured by a long-standing and reputable company (DeZurik, Pratt, etc). Should contractors have follow up questions related to the use of lug style valves including design, installation, manufacturing, and manufacturers, submit additional detailed RFIs. | 8 |
| 84 | It does not appear a specification for the mechanical indicators for actuator stems has been provided. Is it acceptable to have a clear view of the stem with open/close notches? | Rotork IQ3 electric actuators shall be supplied with the IQ Mechanical Position Indicator. See snippet below.<br> | 8 |
| 85 | 6" Butterfly Valves for the eel bypass are set for IQTM125 actuators which have a maximum closing speed of 20sec. The spec calls for a 3min minimum closing speed. Is the contractor required to program a delayed timer for the 6" valves? | The 20-second maximum closing speed of the IQTM125 actuators shall apply. There shall be neither 3-minute minimum closing speed nor respective programming of a delayed timer required. | 8 |
| 86 | Butterfly Valve operation in the spec book only calls out closing speed due to water hammering, is there a specific speed you would like to achieve for opening the valve as well? | Electrically actuated butterfly valve controls shall be programmed so that the opening time from fully closed to fully open is approximately three (3) minutes, minimum. | 8 |
| 87 | In multiple sections of the specification, it calls out structural steel and miscellaneous metals to follow AISC & AWS standards. Will ASME certifications, qualifications and procedures will be accepted in lieu of AISC, AWS or any other standards specified? | The response to this question will depend on the specification, material, and proposed application to be addressed on a case-by-case basis in submittals after award and contracting. Acceptance by the Engineer of a proposed alternate standard cannot be guaranteed. | 8 |
| 88 | Please provide a support detail for the manifolds required on the air burst pipe system at each wedge wire screen | The Contractor will be responsible for providing support details in the shop drawing submittal from the air burst system manufacturer. | 8 |
| 89 | Please provide LF and Fitting quantity for the air burst system. Sheet M-133 appears to indicate only one pipe run per wedge wire. Will additional piping be required? | Linear footage and fitting quantity are dependent upon and will be determined as part of the air burst system design to be completed by the manufacturer. | 8 |
| 92 | Please elaborate on line F in spec section 40 20 55 | It is assumed that hydrostatic leakage test water can be returned to the St. Croix River if the Contractor secures the proper licensing and permits from the Maine Department of Environmental Protection. | 8 |
| 96 | Drawing S-123 calls out polypropylene manhole rungs from lane international. Please provide the quantity and spacing required. | Rungs shall be equally spaced at 12” +/- 2” on center in accordance with OSHA requirements; spacing requirements also apply to any step up or step down to a surface from an end rung. The quantity of rungs for access to various structures including the fish lift flume and fish ladder will therefore be dependent upon the spacing. | 8 |
| 97 | Please confirm if the existing gates for the fish ladder are operational and capable of blocking water flow to the fish ladder. | Confirmed, but there is leakage present when the gates are closed. Bidders shall assume sandbagging and pump dewatering or approved alternative method are required to manage leakage and maintain workable conditions. | 8 |
| 98 | There is approximately a 5 foot discrepancy in the top-of-rock elevation on the civil drawings (C-002 and C-003) and the top-of-rock elevations shown on the borings in that general area. What elevation should be used for the post-tension rock anchor design? | Bedrock profiles on the civil plans were approximated. Bidders are recommended to assume the most conservative top-of-bedrock elevations where discrepancies exist between the drawings and the boring logs. | 8 |

| # | Question | Response | Amendment |
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| 99 | Please confirm detail for fish ladder walkway #3 is the remaining fish ladder section south of FL-H not shown on drawing S-413. If not, please provide detail. | There is not a fish ladder walkway 3. Fish ladder walkways 1 and 2 are the remining fish ladder section south of FL-H. | 9 |
| 100 | Will the knife gate valves that are to be installed below the viewing tank on drawing M-123 require a valve handle wheel routed above the exit flume? | Yes. | 9 |
| 101 | Would it be allowable for the 4" and 2" PVC pipe be Sch-80 for UV resistance on the holding tank on drawing M-123; or does the pipe need to be coated? | Schedule 80 PVC pipe may be used as an alternative to coated schedule 40 PVC pipe. | 9 |
| 102 | Please confirm that MEDOT concrete specifications and requirements do not apply. | MEDOT concrete specifications and requirements do not apply to this project. The bid documents include project-specific concrete specifications and requirements in Division 03. | 9 |
| 103 | The specifications call for National Ready Mix Concrete Associate certification. Can MEDOT certification be submitted in lieu of NRMCA certification. | MEDOT certification is acceptable. | 9 |
| 104 | Please confirm that concrete time limits and revolution requirements can be waived through the proper use of hydration stabilizers. Hydration stabilizer is assumed to be used per MEDOT requirements. | Hydration controlling admixtures shall meet the requirements of Section 03 30 00, 2.01-H. Concrete time limits shall be per Table 03 30 00-3. Limit drum revolutions to 300 unless otherwise recommended by the ready-mix supplier. | 9 |
| 105 | The concrete specification calls for Type IL cement. Is Type I/II acceptable? | Type I/II cement is also acceptable. | 9 |
| 106 | The specifications state that slag cement will be permitted if Class F fly ash is not available. Is up to 50% replacement of slag acceptable as allowed by MEDOT for ASR mitigation and thermal management? | Up to 50% replacement of slag is acceptable for ASR mitigation and thermal management. | 9 |
| 107 | The maximum specified percent alkalies in cement is 0.6%. Please confirm the intent is for all cement to be low alkali cement. | The “low-alkali cement” designation is not required; however, cement with low alkali content is needed to minimize alkali-silica reaction (ASR). | 9 |
| 108 | Please specify the color required for duct bank concrete. | Red. | 9 |
| 110 | The specifications call for flowable fill to be composed on 20% - 25% coarse aggregate measured by weight. Please confirm flowable fill with fine aggregate but no coarse aggregate is acceptable. | This is acceptable. | 9 |
| 111 | Detail E on DWG 200-04 states to remove/replace tube strut and severely corroded steel if needed on Gate 12. Is this referring to just gate 14 or all gates that were completed in phase 1? This work is included in a lump sum bid item. The contractor cannot quantify the replacement of members based on the current information provided. Unless the work will be considered a change order, the contractor will have to assume all members will need replacement. Please confirm removal and replacement of severely corroded members identified in consultation with the engineer during the project will be considered a change order. | This refers to all intake bent frames (these members are not part of the gates). The intent is to remove steel that is severely corroded to prevent material from becoming dislodged and going through the generating units. It is anticipated that only the top struts from the original intake frame [TOS ~EL. 138.8 (NAVD88)] will need to be removed based on the previous observations during dewatering in front of Unit No. 8 and 9; refer to dewatered intake inspection photographs provided via Amendment No. 5. These top struts also coincide with the original headpond elevation of the station and were subjected to more frequent wetting/drying cycles. The new steel has been designed to span top to bottom; therefore, there is no need to replace this steel member. However, Bidders shall estimate per-bent unit pricing for removal of the struts and associated supports. The Contractor shall perform a detailed inspection and verify the need for corroded steel removal with the Engineer by submitting a Request for Information. The Price Component Schedule will be revised to add this new pricing item, to be awarded at DMR’s option, and distributed to Bidders via Amendment. | 10 |
| 112 | Section 36 of Exhibit B-Woodland Pulp LLC General Requirements states, "The Contractor agrees to be the generator of any hazardous waste that results from his activity and agrees and is required to comply fully with all federal, state and/or local statues, regulations and ordinances." Please confirm that Woodland Pulp LLC will be considered the generator of any contaminated or hazardous waste which is currently owned or part of the property owned by Woodland Pulp LLC. For example, Woodland Pulp LLC would be considered the generator of lead waste if it was determined that paint on existing steel to be removed or blasted and painted was lead-based. Similarly, if the soil in the berm shown for removal on the drawings is found to contain contaminated or hazardous material, Woodland Pulp LLC would be considered the generator of such waste. Please confirm the contractor is only responsible as the generator for materials and consumables brought on site by the contractor to perform the contracted scope of work. | WPLLC is not generating the hazardous waste resulting from the activity but is responsible to track the disposal, and the WPLLC General Requirements section referenced in this question shall therefore be assumed accurate. WPLLC will work with the awarded Contractor to ensure that the appropriate documentation is selected. The Contractor is responsible for hazardous materials testing and abatement by a licensed contractor with proper documentation. Refer to Division 100 General Conditions Section 105.8.4. | 11 |

| # | Question | Response | Amendment |
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| 113 | Trash rack drawing 300-02 states to field confirm the presence of a concrete shelf and retainers in place at bent 0. If either item needs to be repaired or replaced, please confirm this be performed as a change order. | Bidders shall estimate replacement of the shelf on a time-and-materials basis should field verification find extensive deterioration or the absence of a shelf. The Contractor shall perform a detailed inspection and verify the need for repair or replacement with the Engineer by submitting a Request for Information. The Price Component Schedule will be revised to add this new pricing item and distributed to Bidders via Amendment. A steel bracket detail will be added to retain the stoplog. | 12 |
| 114 | Please confirm the contractor only needs to provide (60) Type 2 stop logs for the trash rack. Type 1 & 3 are already fabricated with no need for modifications. See table on drawing 300-10 for reference. | That is mostly correct. There will need to be modifications to (3) of the existing stoplogs to accommodate the eel bypass piping; refer to the note below the table on 300-10 and the details on 500-04. This assumes the Contractor does not need to dewater the intake in front of both Unit 7/10 and Unit 8/9 simultaneously. The stoplogs already fabricated can dewater either Unit 8/9 or Unit 7/10 (using the same stoplogs) assuming there are not deviations in bent spacing in front of Unit 7/10. The new Type 2 stoplogs are intended to dewater in front of Unit 4/5/6. | 10 |
| 115 | Please provide further clarification on which part of the trash rack requires a new concrete footing? Is the footing only required at "bents" 0-15 or the entire trash rack? | The footing needs to be placed along the entire trash rack. This was not completed in front of Units 8/9 during the prior work. | 10 |
| 116 | Please provide more detail on the "3 phase" outage for trash rack install. Please clarify how many gates will be allowed to be closed out of service to facilitate this work. Does the engineer anticipate this work, including the new concrete footing will be performed in the dry? | Closure walls called out on the Kleinschmidt design plans were installed in 2022 and allow each of the three segments of unit intakes (Units 4/5/6, Units 7/10, and Units 8/9) to be isolated once the closure plate is installed (refer to Section A of Sheet 300-04)[1]. It is assumed all intake gates within each segment will be closed and locked out and that only one segment will be isolated and dewatered at a time after the closure plate is installed.<br><br>This work is anticipated to be performed in the dry but may be supported by divers as part of the means and methods determined by Bidders and proposed in the written work plan by the awarded Contractor. Prior work by WPLLC maintained adequate dewatering of leakage with 2” trash pumps. | 10 |
| 118 | Please provide the weight of embedded C15 on dwg 300-04 | It is assumed that this question refers to the existing channel embedded in the sill. If this is correct, historical drawings indicate that it is a 33# channel. | 10 |
| 120 | On Sheet C-183 the Access Roads Section Plan depicts the eastern retaining wall ending at station 1+05. On Sheet C-184 the Access Road Section for station 1+25 and 1+31.27 shows the retaining wall continues Until 1+31.27. Please clarify what station the eastern wall ends at. | Extend both walls to Station 1+31.27. | 10 |
| 121 | Drawing 400-02 section B references new concrete between bents 1-12. This call out contradicts other drawings in package. Please clarify which bents require new concrete. | Section B is not applicable at Bents beyond Bent 6 as new concrete ends between Bents 6 and 7; refer to the darker shading of the concrete surface on Sheets 300-01, 300-10, and 400-01 for extents of new concrete. The plan view on Sheet 400-02 was delineated this way because it was the halfway point, allowing retention of the plan on a single sheet at the same/largest drawing scale possible. | 10 |
| 122 | Drawing 300-09 shows a concrete footing that needs to be installed behind the new trash rack and within the footprint of the existing trash rack. Is it the intent of the design for this concrete to be installed in the dry? If so, what is the engineer’s intent for dewatering the area. | Yes, the intent is to install the footing in the dry. Historical drawings indicate the existing intake column (24I) is attached to the embedded channel and anchored to the sill. The intent is to use the base of the existing column to support the lateral hydrostatic load of the new stoplogs and W24 in the interim prior to casting the concrete footing. This was performed by Demo Salvage in front of Unit No. 8 and 9 during the 2022 work. The concrete footing is intended to provide support for the new trash raker as well as long term lateral support for hydrostatic loads on the intake. | 10 |

[1] Note that red denotes text added since the response was issued by Amendment under the previous bid opportunity.

| # | Question | Response | Amendment |
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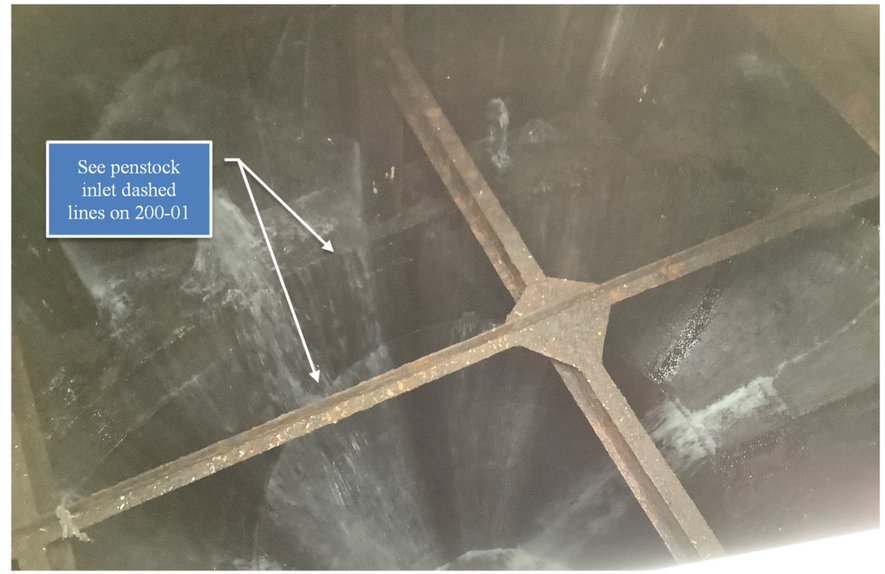
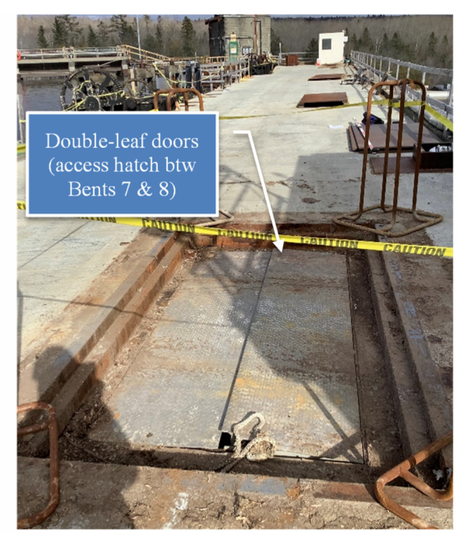
| 123 | All fish bypass piping in the trash rack shows the need to core drill 2 holes with a void spanning 18'9". This "void" isn't labeled, please provide details of the area and information on what the access will be to stage the area for access to the work. | The "void" acts as an inner forebay chamber between the intake gates and open penstock inlets. The first photograph below shows a representative penstock inlet, denoted by dashed lines on Sheet 200-01, with the inner forebay chamber in a dewatered state. Dewatering occurs through a drain valve in the penstock, and trash pumps discharging into the penstock opening can be used to maintain workable conditions. The Contractor will be responsible for safety measures if proposing to work within the inner forebay in the dry, such as scaffolding, ladders, stairways, man baskets, and/or temporary railing around the penstock inlet.<br>  <br><br>Refer to the access hatch and vents noted on C-160 and 200-002 for the Contractor's access into the chamber, which are also shown in the following photographs. The vents are comprised of timber supports topped with steel cover plates. The access hatch has double-leaf hinged doors.<br> <br><br>  <br> | 10 |

| # | Question | Response | Amendment |
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| 125 | Sheet S-102, Note 1, Can the Contractor elect to not install any rock anchors (post tensioned or otherwise) to support the existing wall? If not, please provide the existing loading on the wall so the contractor can design the stabilization of the existing wall. | Yes, but only as long as the wall is stable during construction. | 11 |
| 127 | Sheet S-181, Rock Anchor Note 5, please confirm that Grade 75 thread bar or hollow bar is acceptable in lieu of Grade 60 deformed bar. | A substitution must be approved by the Engineer and may be proposed as value engineering after award. | 11 |
| 128 | Sheet S-182, Note 4, please clarify that this is a typographic error and domestic steel is allowed to be used from any legit supplier. | The size would have to be reduced from 52 mm to 51 mm diameter.<br>**B7X1 Domestic Hollow Injection Bar**<br>BAR DIAMETER / AVERAGE INNER DIAMETER / MINIMUM NET AREA THROUGH THREADS / MINIMUM ULTIMATE STRENGTH / MINIMUM YIELD STRENGTH / NOMINAL WEIGHT / PART NUMBER<br>32 mm (1-1/4") / 0.787" (20.0 mm) / 0.556 in² (359 mm²) / 58.4 kips (260 kN) / 47.2 kips (210 kN) / 2.1 lbs/ft (3.1 kg/m) / B7X1-032N<br>32S mm (1-1/4") / 0.626" (15.9 mm) / 0.776 in² (501 mm²) / 81.5 kips (363 kN) / 66.0 kips (294 kN) / 2.7 lbs/ft (4.0 kg/m) / B7X1-032S<br>38 mm (1-1/2") / 0.830" (21.1 mm) / 1.067 in² (688 mm²) / 112 kips (498 kN) / 90.7 kips (404 kN) / 3.76 lbs/ft (5.6 kg/m) / B7X1-038N<br>51 mm (2") / 1.187" (30.1 mm) / 1.795 in² (1158 mm²) / 188 kips (837 kN) / 152 kips (677 kN) / 6.26 lbs/ft (9.3 kg/m) / B7X1-051N<br>76 mm (3") / 1.890" (48.0 mm) / 3.880 in² (2503 mm²) / 407 kips (1811 kN) / 329 kips (1466 kN) / 13.79 lbs/ft (20.5 kg/m) / B7X1-076N<br>**B7Y1 Non-Domestic Hollow Injection Bar**<br>BAR DESIGNATION & OUTER DIAMETER / AVERAGE INNER DIAMETER / MINIMUM NET AREA THROUGH THREADS / MINIMUM ULTIMATE STRENGTH / MINIMUM YIELD STRENGTH / NOMINAL WEIGHT / PART NUMBER<br>T30S – 30 mm (1.18") / 11 mm (0.43") / 0.662 in² (427 mm²) / 71.9 kips (320 kN) / 58.5 kips (260 kN) / 2.42 lbs/ft (3.6 kg/m) / B7Y1-030S<br>T40N – 40 mm (1.57") / 20 mm (0.79") / 1.046 in² (675 mm²) / 121 kips (539 kN) / 96.7 kips (430 kN) / 4.23 lbs/ft (6.3 kg/m) / B7Y1-040N<br>T40S – 40 mm (1.57") / 16 mm (0.63") / 1.222 in² (788 mm²) / 148 kips (660 kN) / 118 kips (525 kN) / 4.84 lbs/ft (7.2 kg/m) / B7Y1-040S<br>T52N – 52 mm (2.05") / 24 mm (0.94") / 1.874 in² (1209 mm²) / 209 kips (929 kN) / 164 kips (730 kN) / 6.92 lbs/ft (10.3 kg/m) / B7Y1-052N<br>* T64 – 64 mm (2.52") / 40 mm (1.57") / 2.4 in² (1548 mm²) / 261 kips (1160 kN) / 209 kips (930 kN) / 8.74 lbs/ft (13 kg/m) / B7Y1-064N *<br>* T73N – 73 mm (2.87") / 53 mm (2.08") / 2.5 in² (1615 mm²) / 260 kips (1160 kN) / 218 kips (970 kN) / 8.9 lbs/ft (13.2 kg/m) / B7Y1-073N *<br>T76S – 76 mm (2.99") / 45 mm (1.77") / 3.891 in² (2510 mm²) / 427 kips (1900 kN) / 337 kips (1500 kN) / 13.23 lbs/ft (19.7 kg/m) / B7Y1-076S<br>* T103N – 103 mm (4.06") / 78 mm (3.07") / 4.87 in² (3140 mm²) / 510 kips (2270 kN) / 405 kips (1800 kN) / 17.0 lbs/ft (25.3 kg/m) / B7Y1-103N *<br>* T130N – 130 mm (5.12") / 60 mm (2.36") / 16.2 in² (10452 mm²) / 1785 kips (7940 kN) / 1180 kips (5250 kN) / 50.4 lbs/ft (78 kg/m) / B7Y1-130N * | 11 |
| 129 | Sheet S-182 Note 6, Is the independent testing agency being hired by the Owner? Does the rock anchor installer need to provide anything to this testing agency? (testing frame, equipment, personnel, or are they truly independent?) | The Contractor is responsible for hiring the testing agency and for preparation and submittal of a comprehensive Quality Control Plan. Refer to 106.4 of Division 100 General Conditions and Specifications Section 01 40 00. | 11 |
| 130 | Sheet S-450, please confirm Foundation 18 is micropile supported. | Foundation 18 will have micropiles. | 11 |
| 133 | Addendum 7, response to question 47 indicates that there are large bending moments in the pile, but only axial loads are provided on the drawings. Please indicate lateral load demand for the piles. | The unfactored lateral loads applied to each micropile is provided below, broken out by structure:<br>• Pipe Supports: 11,900 pounds<br>• Fish Ladder: 10,590 pounds<br>• Exit Flume Column Support Foundations: 8,000 pounds | 11 |

| # | Question | Response | Amendment |
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| 134 | Compression testing for the micropiles will require hold down anchors, please confirm hold down anchors can be designed by the contractor and the contractor has free range of material type, location and that anchors used for testing are allowed to be abandoned in place. If not, please provide a design or location for the required micropile testing hold down anchors, or reconsider allowing tension testing only. Tension testing is more economical and provides more conservative results. Tension testing would be beneficial to this project. | Hold down anchors can be designed by the Contractor including material types, locations and types of anchors used. Anchors may be abandoned in place, as long as not in conflict with any existing or proposed structures. Value engineering proposals such as alternate testing methods may be submitted following award. | 11 |
| 135 | Please confirm that no lateral load testing of the micropiles is required. | Lateral load testing will not be required. | 11 |
| 136 | Is the contractor allowed to redesign the micropiles (Contractor design)? | This approach is desirable assuming it will offer cost reduction, but there is insufficient time remaining in the bidding process to address this and ensure the redesign is acceptable to the Engineer. Bidders shall therefore bid on the provided micropile design, and the awarded Contractor shall submit its redesign as a value engineering proposal. | 11 |
| 137 | For the micropiles, what is the pile deflection/rotation criteria for the lateral loads (if any)? | The design deflection is intended to be less than 1 inch. | 11 |
| 138 | If there are lateral loads and they are provided - Is a combined stress ratio calculation to demonstrate the combination of axial and flexure loading at a potential joint location acceptable? | Yes. Also, see responses to Questions 47, 78, and 133. | 11 |
| 139 | Is the contractor free to eliminate the joint restriction within the upper 10 feet of the pile if the Contractor's design indicates the casing and joint can handle the required loading? Assuming the lateral loading is provided to the contractor. | Yes. Refer to responses to Questions 78, 133, and 136. | 11 |
| 140 | Would a higher geotechnical resistance factor be allowed per AASHTO with a verification test? | The micropiles were designed using an allowable stress design following Sabatini et. al. (2005). "Micropile Design and Construction (Reference Manual for NHI Course 132078)," National Highway Institute, Washington D.C. However, consideration can be given to the assumed grout-ground bond strength and associated factor of safety following completion of the verification load testing program. | 11 |
| 143 | Is the contractor free to redesign the micropile bond length? | This is an acceptable approach to redesign, but see response to Question 136. | 11 |
| 144 | Can the bond length be reduced, both diameter and length, based on the Contractor's design? | This is an acceptable approach to redesign, but see response to Question 136. | 11 |
| 145 | What is the maximum demand bending moment in the pile? | This varies depending on location, but the maximum bending moment was calculated to be 446,151 inch-pounds at a depth of 4.5 feet below ground surface. | 11 |
| 146 | Can the contractor adjust the casing size such as outside diameter, and wall thickness based on their design? | This is an acceptable approach to redesign, but see response to Question 136. | 11 |
| 147 | Through the Contractor's design, is the contractor allowed to reduce the number of piles? | A reduced number of piles can be submitted with supporting calculations for review and acceptance by the Engineer. However, see the response to Question 136. | 11 |
| 149 | Please confirm the corrosion protection. The micropile specifications do not indicate any specific corrosion protection, but note 3C on sheet S-458 and detail B on Sheet S-459 indicate an epoxy coated bar and galvanized steel casing. Please provide requirements and specifications for these coatings if a coating is required. | Since corrosion loss was assumed, the hot-dip galvanizing of the casing shall be considered an optional alternate to carbon steel. This also amends the response to Question 72 so that carbon steel is acceptable, though stainless steel remains unacceptable. | 11 |
| 150 | Please also provide the rational for providing 1/16" corrosion loss on the casing while also making the casing galvanized? This is extremely rare, and overly conservative. Is the 1/16" corrosion loss on the wall thickness, or the diameter as a whole? (i.e. 1/32 on each wall) | The 1/16" corrosion loss on the wall thickness was assumed in design. The hot-dip galvanized casing should have been shown as an alternative. Carbon steel is acceptable for the casing. | 11 |
| 151 | Please consider bare steel for the casing with a 1/16" corrosion loss only. | Carbon steel is acceptable. See responses to Questions 149 and 150. | 11 |
| 152 | Please provide corrosivity data for the site soils. | There is no known corrosivity data available to provide. Corrosivity was assumed as part of the design in the absence of field test results on which to rely. | 11 |

| # | Question | Response | Amendment |
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| 153 | Please clarify Note 6 on sheet S-458. Should the micropile contractor carry up to 200% of the neat volume of the micropile? How will the contractor be compensated for grouting time and volume? Please consider making excess grouting time and material a unit price to reduce risk to all parties. | The Contractor shall be responsible for material procurement in sufficient quantities to maintain its schedule and drilling progress, including accounting for variability in depth to bedrock encountered in the field up to the limit per Note 6. Unit pricing for grout take is amenable and included in the attached revised Price Component Schedule. Additional grouting requirements have been included in the attached revised Specifications Sections 31 63 33 and 31 68 00. | 11 |
| 154 | Based on the contractor's micropile design, can the contractor elect to install a longer bond length instead of pulling back with casing? Pulling back casing within rock has unique challenges and risks and it is unclear how the contractor will get compensated if tooling gets locked in the rock. | This is an acceptable approach to redesign, but see response to Question 136. | 11 |
| 156 | It is unclear if the micropile casing and associated parts and rock anchors are required to be domestic steel. Please confirm if this steel needs to be domestic. Understood that there is a waiver for certain BABA requirements, but it is unclear to the bidders what these waivers include or do not include. | The steel is not required to be domestic. Refer to the following:<br>Multi-Agency Tribal BABA Waiver.pdf<br>DOC Tribal Consultations Waiver Issuance Memo CFO ASA v2.pdf | 11 |
| 157 | Micropile Specification, 2.03 A – please confirm that this is a typographical error and confirm that Portland Cement Type I or Type II, or Type I/II is acceptable. Type IL can be challenging, and this is an odd requirement. | Portland Cement Type I or Type II, or Type I/II is acceptable. | 11 |
| 165 | New trash rack drawings on amendment 7 do not provide connection details showing how the trash racks attach to the panels. Please provide detail. | This question is assumed to refer to how the trash rack panels (e.g. 500-03) are attached to the support frames (e.g. 500-02). The trash rack panels are slid into the support frames. Two retainer clip angles will be added to the top of each trash rack panel that will be welded to the trash rack panels and bolted to the top member of the support frames. | 11 |
| 166 | Please provide the specification for the 3/4" threaded rod for the trash racks. | The 3/4" DIA. threaded rod should be ASTM A36 steel. | 11 |
| 167 | Drawing D-100 detail 1 states to relocate the duct work around the bypass flume. This will require more material, please provide information on the size and type of ductwork along with information of a new connection point if one is required. | The following photograph depicts the duct work to be relocated, which supplies shower water for WPLLC's north filter screens and is shut down for winter. The awarded Contractor shall submit a request for information during construction following field verification to propose relocation and reconnection for acceptance by the Engineer and WPLLC. Bidders may assume pipe diameter is up to 6 inches and has insulation and pipe sleeve to be extended and/or restored after relocation as needed.<br> | 11 |

| # | Question | Response | Amendment |
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| 168 | Specification 31 23 19 0 Water Control Measures, Subsection 1.07.D, states the dewatering plan shall include details for discharging water and measures for monitoring & limiting turbidity. The permits discuss the requirement for turbidity curtains, but do not appear to specifically mention monitoring. Please clarify what turbidity monitoring measures are required. | The permit requirements shall control. Additional compliance requirements, if any, will be negotiated with the Contractor. | 11 |
| 169 | Specification Section 32 34 00 Fabricated Bridges, part 2.4 Engineering, paragraph 7 Bridge Pedestrian Safety Rails outlines very specific design requirements. In order to achieve the pedestrian loading design requirements, a custom 5-foot cantilever walkway attached to the modular bridge with a specially designed rail picket system will be necessary. Please confirm that this was the intent of the specification and that this is acceptable. | A custom walkway is not intended. Pedestrians are expected to traverse the vehicle pathway. | 11 |
| 170 | Specification Section 32 34 00 Fabricated Bridges, part 2.3 Geometry, paragraph D vehicular guardrail, states that a 42-inch minimum rail height is required. Modular bridge systems do not come with a standard 42-inch rail height, this would have to be a custom-made rail system that will come at additional cost. Please confirm that a 42-inch vehicular guard rail is required. | The intent is to add handrail anchored into the guardrail support posts to achieve a total height of 42 inches for pedestrian safety. This is not expected to be the total height of the vehicular guardrail. | 11 |
| 175 | Drawing C-180 shows existing pipes which may impact the construction of Abutment 2 and its approach. Please provide drawings showing the as-built condition of the pipes and pipe supports | No as-built drawings are currently available for these pipes. The Contractor shall implement measures to protect the pipe per notes on C-181. | 11 |
| 179 | Specifications Section 32 34 00 Fabricated Bridges, part 1.8 Warranty, paragraph A, calls for the warrant of materials and installation for 10 years after Woodland Pulp and Maine DMR's acceptance of work. Acrow does not provide a warranty on installation as Acrow does not control the installation. Secondly, Acrow does not warrant materials. | Warranty exclusively by the bridge manufacturer is not what is specified. Some warranty provisions from the bridge manufacturer are assumed, but it is the Contractor's responsibility to ensure the full extent of the warranty requirement is met. | 11 |
| 180 | Specifications Section 32 34 00 Fabricated Bridges, part 1.5 Submittals, paragraph A, states that the bridge design shall be performed by a PE who is an employee of the Bridge Manufacturer and that the bridge design engineer shall provide signed and sealed drawings and calculations. All Acrow Engineers are licensed Professional Engineers. However, Acrow's internal policy requires 3rd party Professional Engineers to review and sign our work. Please confirm this is acceptable. | Confirmed. Include detailed contact information for both the Acrow lead engineering representative and the third-party professional engineer responsible for bridge design with the shop drawing submittal. The engineer sealing the design plans shall be registered in the State of Maine. | 11 |
| 181 | Specifications Section 32 34 00 Fabricated Bridges, part 1.7 Quality Assurance, paragraph A.2, calls for visual and ultrasonic testing of all welds in accordance with AWS D1.1. Acrow is a bridge manufacturer, which differs from a bridge fabricator due to our pre-engineered modular components. Our manufacturing process includes a visual inspection of all welds per AWS D1.1. We also do NDT (Non-Destructive Testing) on a percentage of parts produced. Please confirm this is acceptable. | Confirmed acceptable. | 11 |
| 182 | Specifications Section 32 34 00 Fabricated Bridges, part 2.3 Geometry, paragraph D, says in addition to vehicular guard rail, bridges shall include fencing or railing to allow for pedestrians to safely use the bridge. Paragraph 1 says minimum railing height shall be 42 inches above the finished deck. The standard Acrow vehicular rail height for a 13.6' wide Acrow 700XS Bridge (TL-2, not crash tested) is 2' - 8 11/16" from top of epoxy to top of guard rail. Changing this height will require a custom designed rail system, adding cost. Please confirm the 42" height requirement refers to the handrail only and the standard Acrow vehicle guardrail height for vehicles is acceptable. | Confirmed. The 42-inch height requirement applies to pedestrian handrail only. Refer to the response to Question 170. | 11 |
| 183 | Specifications Section 32 34 00 Fabricated Bridges, part 2.4 Engineering, paragraph 6, calls for bridge rail design to meet the forces of TL-2 (not crash tested). Acrow 700XS Bridge Rail for a 13.6 foot wide bridge is 2' - 8 11/16" from top of epoxy to top of guard rail. Please confirm this is acceptable as shown on the plans. | Confirmed acceptable. Refer to the response to Question 181. | 11 |

| # | Question | Response | Amendment |
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| 184 | Specifications Section 32 34 00 Fabricated Bridges, part 2.4 Engineering, paragraph 7 (a and b), calls for bridge pedestrian safety rails, picket system, or fencing shall be designed for infill loading of 200 lbs applied horizontally at right angles to one square foot of the system and 50 lbs per lineal foot or 200 lbs point load (whichever is greater), applied in any direction at any point along the top chord or at the top of the safety system. These loading requirements will require a specially designed 5' cantilevered footwalk off the side of the bridge with a specially designed pedestrian rail system. This will add cost, weight, and overall out to out dimensions of the Acrow bridge. Please confirm these loading requirements are required and this is the intent. | This is not the intent. Refer to the responses to Questions 169, 170, and 181. | 11 |
| 185 | Specifications Section 32 34 00 Fabricated Bridges, part 2.5 Materials, paragraph A, calls for bridges to be fabricated from self-weathering structural steel tubing. Acrow Bridge components are hot dipped galvanized. Please confirm that the following Acrow specifications is acceptable - all major components shall be galvanized to AASHTO M111-ASTM A123, all bolts are hot dipped galvanized, and pins are electro galvanized. | Confirmed galvanized is acceptable. | 11 |
| 186 | Specifications Section 32 34 00 Fabricated Bridges, part 2.7 Finish, paragraph B, calls for exposed surfaces and components of the bridge not constructed of self-weathering steel shall be finished in accordance with requirements of Specification Section 09 90 00 Painting and Coating. Acrow bridges are hot dipped galvanized. Acrow does not require additional surface treatment of their components. Please clarify what is required in addition to the HDG treatment. | Confirmed galvanized is acceptable with no additional treatment. | 11 |
| 188 | A few questions relating to seals:<br>Are there safety-critical parts included in this RFQ? In other words, for the purposes of the rubber seals, would a leak constitute an emergency (threat of injury, equipment damage, and/or contamination of hazardous materials into the surrounding environment)?<br>a. A Critical Safety Item (CSI) is a part, assembly, or support equipment whose failure could cause loss of life, permanent disability or major injury, loss of a system, or significant equipment damage.<br>b. A Critical Item is a part, assembly, or equipment whose failure, malfunction, or absence could cause a catastrophic or critical failure, leading to loss of life, serious injury, significant equipment damage or unacceptable | This response supersedes the response provided in Amendment No. 11. The question was previously misinterpreted to apply to existing gate seals and working conditions; the question instead has been clarified to relate to Design Specification Section 35 20 17, Rubber Seals, for the exit flume expansion joints. These rubber seals are not considered safety-critical and only prevent water leakage. Failure of a seal is expected to result in some, mostly minor, leakage but to pose no immediate personnel safety or equipment damage risk. | 11,14 |
| 189 | Are there flowdowns, DPAS ratings, or other requirements attached to this request? | The Contractor is responsible for subcontracts and the provisions contained therein unless specified in Division 100 General Conditions; refer to Section 104.5.7. No DPAS ratings apply to this Contract. | 11 |
| 190 | Is the end user a government entity? If so, who? | Woodland Pulp LLC is the end user and will operate the facilities after commissioning. | 11 |
| 191 | Are there specifications for the Section 2 work, or just what is noted on the drawings? Specifically the Trash Rack, but other work as well. | Refer only to the notes for the Kleinschmidt intake design drawings, Exhibit B6. The Design Specifications of Exhibit C apply to design plans in Exhibits B1 through B5 by Verdantas and PIC; refer to "Bid Supplement" labeling of select drawing sheets and specifications for other tasks incorporated into Section 2 per the list of updates provided in Amendment No. 2. | 11 |
| 192 | The primary and alternate mill source breakers are listed as 800A trip. Should this be 400A? Reference drawing EE-BA-001. | Correct. Both feed breakers should be 400A. | 12 |
| 193 | Is Woodland Pulp supplying the primary and alternate breakers? Reference drawing EE-BA-001. | WPLLC engineers provided instructions on how to tie into existing loadcenter C, so the Contractor shall provide the breakers at both locations.<br>Source C172 is intended to be connected to a spare size 3 starter slot in a GE MCC. A new bucket will be needed for this.<br>Source B86 is intended to be connected to B8-TX-6. | 12 |
| 194 | Drawing EE-CJ-009 shows rigid conduit sleeves extending to the cable tray above the equipment. How are the IAC cables in the tray supposed to terminate into this sleeve? No code-compliant method comes to mind. | Rigid conduit sleeves are to act as supports for the IAC cables as needed for the drop from the cable tray to the MCC. The armored jacket is to be terminated on the top of the MCC with killark MC series "Clencher" connectors. | 12 |
| 195 | The finish schedule indicates Trash Rack Framing to be galvanized steel. Does this pertain to the entire trash rack system, or can the panels themselves be composite? | Please use the steel notes and coating schedule shown on 100-01 for the intake steel (Kleinschmidt scope). | 12 |

| # | Question | Response | Amendment |
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| 196 | Project IFB Specifications do not specify the allowable spacing requirements for 4" SS Sch-40 pipe. Please provide. | This question is assumed to refer to the embedded stilling wells for the water level sensors. Still well locations are shown on Sheet C-100 per Key Note 5 and align with water level sensor locations on Sheet M-002; there is no applicable standard spacing. | 12 |
| 198 | Section 03 70 00 Mass Concrete states that structural mass concrete requirements shall only apply to the large concrete sections of the fish lift concrete flume. Please confirm that mass concrete requirements do not apply to other concrete elements that may meet the dimensional definitions of mass concrete such as bridge pier or abutments. | Confirmed. Mass concrete only applies to the large concrete sections of the fish lift concrete flume. | 12 |
| 200 | The existing drawings that have been provided to date do not clearly detail the access to the interior of the intake structure where penetrations are required for the fish and eel bypass piping included in Bid Option. Please provide additional detail for access to the locations of the pipe penetrations. It appears that walls with 3' diameter openings separate the penstocks at the bent locations. It does not appear that there is clear access from the access hatch to all of the interior of the intake structure. Please clarify the access within the intake structure and between penstocks. | Refer to the responses to Question 123 in Amendment No. 10 showing the access hatch and vents between Bents 7 and 23 and to Question 91 in Amendment No. 8. Vents 1, 2, and 3 shown on Sheet 200-01 provide access from the deck to Units 4/5/6 chambers, each separated by the noted isolation walls. Historical intake drawings were provided within the SharePoint folder shared with Bidders from Amendment No. 5, and they indicate the locations of dividing walls for each Units 4/5/6 and of dividing walls between the penstock inlets of the inner forebay for Units 7/10 and Units 8/9. Refer to the following list of Intake Drawings that were provided in the Existing Drawings – Amendment 8 SharePoint folder. | 12 |
| 201 | Please confirm the embedded steel shown on drawing 600-01 of the Kleinschmidt drawing package is to be painted in accordance with the specification on drawing 100-02. | The embedded steel shown in the 600 series does not need to be painted and can be left plain. | 12 |
| 202 | Please provide details for the anchor studs shown on drawing 600-01 for both the C12x20.7 channel and vent angle frames. | Please use ¾" diameter by 10" long anchor studs for both the channel and vent angle frames. | 12 |
| 203 | Please provide strap anchor detail for the footing of the W24x84 guide frames on the trash rack shown on drawing 300-09 | The strap anchor size, length, thickness, and weld details have been provided on 300-09 (lower left corner). Please clarify if additional information is desired. | 12 |
| 204 | Please confirm only bents 0-15 will need the new trash-raker support beam, W8 strut and embedded C12x20.7. | The trash raker support beam is required the full length of the intake. Refer to Drawing 200-02 for steel that has already been installed. Bents 0 – 14 require all new steel. Bents 15 – 23 already have both the W24 inclined column and W8 strut except at bent 22 where the existing tube strut needs to be replaced with a W8. Bents 15 – 23 also need the new W12 above the W8 strut to support the longitudinal trash raker supports. The embedded C12 is only required for the extents shown on 600-01 (bents 0 – 6). | 12 |
| 206 | Is the project tax exempt? | Bidders shall exclude State of Maine sales tax from their bids since this project is considered eligible for the refund provision of 36 MRS § 2014 (Title 36, §2014: Fish passage facilities). | 15 |
| 208 | How is the Bid Supplement Unit Price for Repair or Replacement of Concrete Shelf to be priced? If this work is to be done on a T&M basis is a price required as a Subtotal on the bid form? If so, in what units shall it be priced: crew hour, daily rate, lump sum, or other? | Bidders are asked to provide an estimated subtotal only on this line for replacement of the shelf to be paid on a T&M basis per Note A below the associated table. Units, unit pricing, and estimated quantity cells were greyed out in the Price Component Schedule and are not requested for this item. The extents, means, and methods for repair or replacement will be mutually agreed between Maine DMR, the Engineer, and the Contractor following inspection/field verification. Once established, the repair or replacement is expected to be paid based on the Contractor's labor and equipment rate schedule required under Part II. Any labor or equipment needed and not contained in the rate schedule will be negotiated. | 16 |