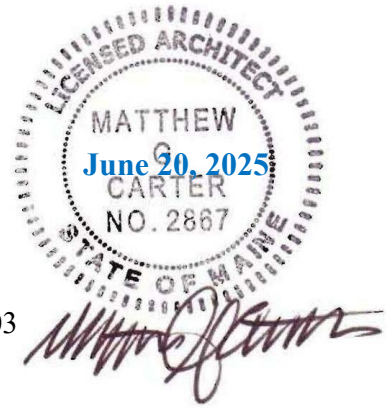
ADDENDUM #1 Rebid



Date: June 20, 2025

Re: Connor School Renovations
Connor Consolidated School
1581 Van Buren Road, Connor Township, Maine

From: Haley Ward, Inc.
One Merchant Plaza
Suite 701
Bangor, Maine 04401

Owner Project No: 3403

A/E Project No.: 10377.028

To: Perspective Bidders

This Addendum forms a part of the Contract Documents and modifies the original bidding documents dated June 13, 2025. Please acknowledge receipt of this Addendum #1 in the space provided on the Bid Form; failure to do so may subject the Bidder to disqualification. The original conditions govern all work unless specifically exempted or modified herein.

This Addendum consists of **eleven (11)** pages and is being issued for revised drawings and clarification.

REPLACE THE REVISED DRAWING SHEETS

- S001 – Structural Notes
- S002 – Structural Notes
- S003 – Special Inspections
- S101 – Foundation and Slab Plans
- S102 – Roof Framing Plans
- S200 – Structural Front Elevation
- S300 – Structural Section
- S301 – Structural Section
- S302 – Structural Section
- S303 – Structural Section

CLARIFICATIONS

1. *We are seeing a few discrepancies between some of the structural drawings; it appears we are framing wood walls but in other areas it appears the CMU/brick veneer is pasted in. Can we revise this, so we look at this properly with our masonry contractor?*

   Please see revised structural drawings.

SAWN LUMBER NOTES:

1. ALL WOOD FRAMING MEMBERS INCLUDING BUT NOT LIMITED TO WALL STUDS AND JOISTS, ARE INTENDEDTO ACT AS A SYSTEM AS DETAILED IN THE STRUCTURAL DRAWINGS AND ONCE CONSTRUCTION IS COMPLETE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE SAFETY AND STABILITY OF WOOD FRAMING SYSTEMS (I.E. TEMPORARY BRACING IF REQUIRED) DURING CONSTRUCTION AS A RESULT OF CONSTRUCTION METHODS AND SEQUENCES. REFERENCE ARCHITECTURAL DRAWINGS FOR ALL SAWN LUMBER FINISH REQUIREMENTS.

2. ALL SAWN LUMBER SHALL CONFORM TO THE WESTERN WOOD PRODUCTS ASSOCIATION OR THE WEST COAST LUMBER INSPECTION BUREAU GRADING RULES. LUMBER SHALL BE OF THE SPECIES AND GRADE SHOWN BELOW:

MEMBER	GRADE
2x AND 4x FRAMING	DOUGLAS FIR-LARCH NO. 2
5x AND GREATER BEAMS	DOUGLAS FIR-LARCH NO. 1
POSTS/COLUMNS	DOUGLAS FIR-LARCH NO. 1

WOOD STUDS IN BEARING WALLS SHALL BE OF THE SIZE, GRADE, AND SPACING NOTED BELOW UNLESS NOTED OTHERWISE ON THE DRAWINGS. ALL STUD BEARING WALLS REQUIRE SHEATHING ON A MINIMUM OF ONE SIDE OF THE WALL. WHERE PLYWOOD SHEATHING DOES NOT EXIST AS NOTED IN THE DRAWINGS, THE SHEATHING MAY CONSIST OF 3/4" GYPSUM SHEATHING ATTACHED WITH #8 SCREWS AT 6" ON CENTER AT ALL PANEL EDGES AND AT 12" O.C. IN THE FIELD. WHERE ARCHITECTURAL FINISHES ARE APPLIED TO FLOORS ABOVE PRIOR TO THE MINIMUM SHEATHING REQUIREMENTS BEING INSTALLED ON THE STUD BEARING WALLS, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE ADEQUACY OF THE UNSHEATHED BEARING WALL AND TO PROVIDE BRACING AS REQUIRED.

MEMBER AND SPACING	GRADE	FLOOR LOCATION
REF. PLANS	DOUGLAS FIR-LARCH NO. 2	EXTERIOR AND INTERIOR BEARING WALLS

3. STORAGE OF ALL LUMBER AND TIMBER ON SITE SHALL BE KEPT OFF GROUND, UNDER COVER AND PROTECTED FROM DAMAGE

4. ALL DIMENSIONAL LUMBER SHALL BE CERTIFIED BY THE SUPPLIER IN WRITING TO BE KILN DRIED

5. STRUCTURE SHALL NOT BE ENCLOSED UNLESS LUMBER MOISTURE CONTENT HAS BEEN VERIFIED TO BE AT OR BELOW 15%. ANY SIGNS OF MOLD SHALL BE REMOVED AND TREATED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS OR INDUSTRY STANDARDS.

6. ALL LUMBER IN CONTACT WITH THE GROUND, CONCRETE SHALL BE PRESSURE TREATED. CONTRACTOR MAY SUBMIT FOR APPROVAL, A MOISTURE BARRIER IN-LIEU OF THE PRESSURE TREATED WOOD

7. FASTENERS FOR PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT-DIPPED ZINC-COATED GALVANIZED STEEL, OR STAINLESS STEEL AND SHALL FOLLOW CURRENT SIMPSON GUIDELINES BASED ON WEATHER EXPOSURE. WHERE STAINLESS STEEL CONNECTORS OR HOT DIPPED GALVANIZED CONNECTORS ARE SPECIFIED IN THE DRAWINGS, STAINLESS STEEL OR HOT DIPPED GALVANIZED FASTENERS SHALL BE USED TO MATCH THE CONNECTOR TYPE

8. ALL PLATES AND LEDGERS SHALL BE FASTENED WITH A MINIMUM (3) ANCHORS PER PIECE.

9. ALL METAL HARDWARE AND FRAMING ACCESSORIES SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY. SUBSTITUTIONS SHALL NOT BE MADE. ALL ITEMS SHALL BE INSTALLED PER THE SIMPSON'S INSTALLATION REQUIREMENTS. ALL NAIL HOLES SHALL BE FILLED WITH THE RECOMMENDED FASTENER UNLESS NOTED OTHERWISE ON THE DRAWINGS

10. WHERE FRAMING HANGERS OR WOOD CONNECTIONS ARE REQUIRED BUT HAVE NOT BEEN SPECIFIED ON THE STRUCTURAL DRAWINGS, PLEASE CONTACT EOR FOR APPROPRIATE WOOD CONNECTOR OR CONNECTION TO UTILIZE

11. ALL WALLS SHALL HAVE DOUBLE TOP PLATES AND SHALL BE SPLICED PER THE TYPICAL TOP PLATE SPLICE DETAIL, UNLESS NOTED OTHERWISE. TOP PLATES AT WALL INTERSECTIONS SHALL BE LAPPED AND NAILED WITH (3) 16d NAILS.

12. WHERE ROOF MEMBERS OR ROOF TRUSSES ARE CONNECTED TO EXTERIOR WALLS OR WALLS w/ PLYWOOD SHEATHING, THE SPECIFIED HURRICANE CLIP SPECIFIED SHALL BE PLACED ON THE SIDE OF THE WALL DOUBLE TOP PLATE WHICH ATTACHES TO THE PLYWOOD.

13. HOLES FOR BOLTS SHALL BE DRILLED WITH A BIT OF THE SAME NOMINAL DIAMETER AS THE BOLT + 1/16". LEAD HOLES FOR LAG SCREWS SHALL BE BORED PER NDS 11.1.3

14. ALL BOLTS, CARRIAGE BOLTS, LAG SCREWS, EXPANSION BOLTS AND EPOXY BOLTS SHALL BE INSTALLED WITH STANDARD CUT WASHERS UNDER THE BOLT HEADS AND NUTS THAT BEAR DIRECTLY ON THE WOOD. ALL NUTS SHALL BE TIGHTENED AT THE TIME OF INSTALLATION AND RE-TIGHTENED IF NECESSARY, DUE TO WOOD SHRINKAGE, PRIOR TO CLOSE-IN OR AT THE COMPLETION OF THE PROJECT. BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1. WOOD SCREWS SHALL CONFORM TO B18.6.1. ALL BOLTS SHALL CONFORM TO ASTM A307 GRADE A UNLESS NOTED OTHERWISE. THE MINIMUM STRENGTHS FOR LAG SCREWS AND WOOD SCREWS SHALL BE AS FOLLOWS:

WOOD SCREW DIAMETER - INCHES	MIN. BENDING YIELD STRENGTH (PSI)
0.138 (#6)	100,000
0.151 (#7)	90,000
0.164 (#8)	90,000
0.177 (#9)	90,000
0.190 (#10)	80,000
0.216 (#12)	80,000
0.246 (#14)	70,000

LAG SCREW DIAMETER - INCHES	MIN. BENDING YIELD STRENGTH (PSI)
1/4	70,000
5/16	60,000
3/8 AND GREATER	45,000

15. CUTTING AND NOTCHING OF SAWN LUMBER JOISTS, SAWN LUMBER RAFTERS AND STUDS SHALL BE IN CONFORMANCE WITH THE FOLLOWING CRITERIA:
    - A. JOISTS — NOTCHES AT THE ENDS OF JOISTS SHALL NOT EXCEED 1/5 THE JOIST DEPTH. HOLES BORED IN JOISTS SHALL NOT BE WITHIN 2-1/2 INCHES OF THE TOP OR BOTTOM OF THE JOIST, AND THE DIAMETER OF ANY SUCH HOLE SHALL NOT EXCEED 1/4 THE DEPTH OF THE JOIST. NOTCHES IN THE TOP OR BOTTOM OF THE JOISTS SHALL NOT EXCEED 1/6 THE DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE THIRD OF THE SPAN
    - B. RAFTERS — NOTCHING AT THE ENDS OF RAFTERS OR CEILING JOISTS SHALL NOT EXCEED 1/5 THE DEPTH. NOTCHES IN THE TOP OR BOTTOM OF THE RAFTER OR CEILING JOIST SHALL NOT EXCEED 1/6 THE DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE 1/3 OF THE SPAN, EXCEPT THAT A NOTCH NOT EXCEEDING 1/3 OF THE DEPTH IS PERMITTED IN THE TOP OF THE RAFTER OR CEILING JOIST NOT FURTHER FROM THE FACE OF THE SUPPORT THAN THE DEPTH OF THE MEMBER. HOLES BORED IN RAFTERS OR CEILING JOISTS SHALL NOT BE WITHIN 2-1/2 INCHES OF THE TOP AND BOTTOM AND THEIR DIAMETER SHALL NOT EXCEED 1/4 THE DEPTH OF THE MEMBER.
    - C. WALLS — MAXIMUM OF 2 1/4" DIAMETER NEATLY BORED HOLE MAY BE PLACED IN THE CENTER OF ALL BEARING 2x6 STUDS WITH NO ADDITIONAL REINFORCEMENT REQUIRED. REF. SHEET SXXX FOR ADDITIONAL INFORMATION ON STUDS AND POSTS

16. ALL NAILS FOR STRUCTURAL WORK SHALL BE COMMON WIRE NAILS UNLESS NOTED OR DETAILED OTHERWISE MEETING ASTM F1667. HOLES SHALL BE PRE-DRILLED WHERE NECESSARY TO PREVENT SPLITTING. NAILS SHALL HAVE THE MINIMUM PROPERTIES SPECIFIED IN THE TABLE BELOW:

NAIL TYPE	SHANK DIA. - INCHES	MIN. PENETRATION - INCHES	MIN. BENDING YIELD STRENGTH (PSI)
6d	0.113	1.13	100,000
8d	0.131	1.31	100,000
10d	0.148	1.48	90,000
12d	0.148	1.48	90,000
16d	0.162	1.63	90,000
20d	0.192	1.92	80,000

17. NAILING NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE PER THE APPLICABLE VERSION OF THE IBC NAILING SCHEDULE.

NAILING SCHEDULE NOTES:

1. ALL OTHER NAILING REQUIREMENTS NOT SHOWN ON DRAWINGS OR IN SCHEDULE ABOVE SHALL BE IN ACCORDANCE WITH 2021 INTERNATIONAL BUILDING CODE.

2. POWER DRIVEN OR PNEUMATIC NAILS OTHER THAN COMMON NAILS MAY BE USED IF DATA IS SUBMITTED TO THE ARCHITECT/ENGINEER FOR APPROVAL PRIOR TO USE.

3. MINIMUM NAIL LENGTHS SHALL BE SUFFICIENT TO ACHIEVE MINIMUM PENETRATION INTO MAIN MEMBER AS NOTED IN SCHEDULE ON NOTE #16.

STRUCTURAL STEEL NOTES:

1. ALL STRUCTURAL STEEL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION'S "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE "AISC CODE OF STANDARD PRACTICE."

2. ALL STRUCTURAL STEEL SHAPES AND PLATES SHALL CONFORM TO THE FOLLOWING:

W AND WT SHAPES	ASTM A992
S SHAPES, CHANNEL, ANGLE & PLATE	ASTM A36
HSS	ASTM A500, GRADE B
PIPES	ASTM A53, GRADE B

3. THE STRUCTURAL STEEL FABRICATOR SHALL BE AN AISC CERTIFIED FABRICATOR. MINIMUM CATEGORY "SBD" CERTIFICATION. (SUBMIT FABRICATOR CERTIFICATION FOR APPROVAL).

4. ALL SHOP INSPECTION SHALL BE COMPLETED BY THE FABRICATOR'S INSPECTOR. SHOP INSPECTION SHALL BE IN ACCORDANCE WITH AISC, AWS, AND AS OUTLINED IN THE CONTRACT DRAWINGS.

5. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION" (14TH EDITION) AND THE AISC"SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS (2005 EDITION).

6. ALL STEEL DETAILS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE AISC "MANUAL OF STEEL CONSTRUCTION" (14TH EDITION)

7. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACING OR GUYS TO PROVIDE LATERAL SUPPORT UNTIL THE PERMANENT LATERAL FORCE RESISTING SYSTEM IS INSTALLED.

8. THE CONTRACTOR SHALL COORDINATE BOTTOM OF BASE PLATE ELEVATION WITH THE TOP OF CONCRETE ELEVATION PLUS ALLOWANCE FOR GROUT BED. IN CASE OF CONFLICT THE CONTRACTOR SHALL MAKE ALLOWANCE IN THE BID FOR THE MORE STRINGENT REQUIREMENT.

9. CUTS OR BURNING OF HOLES IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED.

10. ALL NUTS INDICATED "FINGER TIGHT" SHALL BE HAND TIGHTENED AS REQUIRED TO INSTALL ELEMENTS. DO NOT TIGHTEN NUTS INDICATED AS "FINGER TIGHT" BY MECHANICAL MEANS. TACK WELD "FINGER TIGHT" NUTS IN PLACE OR PROVIDE JAM NUT TO PREVENT BACK OFF.

11. ALL STEEL TO STEEL CONNECTIONS SHALL BE SIMPLE SHEAR CONNECTIONS UNLESS NOTED OTHERWISE.

12. ALL SIMPLE SHEAR CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS UNLESS NOTED AS SLIP CRITICAL.

13. MINIMUM NUMBER OF BOLTS FOR ANY CONNECTION SHALL BE TWO.

14. ALL STEEL TO STEEL CONNECTIONS SHALL EXTEND AT LEAST 2/3 THE DEPTH OF THE SHALLOWEST MEMBER BEING CONNECTED.

15. SHOP CONNECTIONS NOT SPECIFICALLY INDICATED ON THE DRAWINGS MAY BE WELDED OR BOLTED. FIELD CONNECTIONS NOT SPECIFICALLY SHOWN ON THE DRAWINGS SHALL BE BOLTED.

16. ALL WELDING ELECTRODES SHALL BE E70 WITH A MINIMUM YIELD STRENGTH OF 58KSI, MINIMUM TENSILE STRENGTH OF 70 KSI, AND MINIMUM ELONGATION OF 22% IN ACCORDANCE WITH AWS A5.

17. ALL WELD MATERIAL SHALL HAVE A MINIMUM CVN TOUGHNESS OF 20 FT-LBS AT MINUS 10 DEGREES FAHRENHEIT AND 40 FT-LB AT 70° FAHRENHEIT.

18. 100% OF ALL SHOP FULL PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED AND ALL DEFECTS REPAIRED.

19. ALL STRUCTURAL STEEL SHALL BE SHOP PRIMED WITH FABRICATOR'S STANDARD LEAD AND CHROMATE FREE PRIMER UNO OR UNLESS STEEL IS TO BE FIREPROOFED OR IS INDICATED TO RECEIVE HIGH PERFORMANCE PRIMER AND TOP COAT. FABRICATOR SHALL COORDINATE PRIMER REQUIREMENTS WITH SLIP CRITICAL BOLTS

20. FABRICATOR SHALL SUBMIT METHOD FOR INSTALLING SLIP CRITICAL BOLTS FOR APPROVAL. ERECTOR SHALL SET UP PREINSTALLATION TESTING WITH THE OWNER'S SPECIAL INSPECTOR.

21. STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS FABRICATION OF ALL STRUCTURAL STEEL ELEMENTS. SHOP DRAWINGS SHALL INDICATE:
    - A. INCLUDE DETAILS OF CUTS, CONNECTIONS, SPLICES, CAMBER, HOLES AND OTHER PERTINENT DATA.
    - B. INCLUDED EMBEDMENT DRAWINGS.
    - C. INDICATE WELDS BY STANDARD AWS SYMBOLS, DISTINGUISHING BETWEEN SHOP AND FIELD WELDS, AND SHOW SIZE, LENGTH AND TYPE OF EACH WELD.
    - D. INDICATE TYPE, SIZE, AND LENGTH OF BOLTS, DISTINGUISHING BETWEEN SHOP AND FIELD BOLTS.
    - E. IDENTIFY PRETENSIONED AND SLIP CRITICAL HIGH STRENGTH BOLTS.

22. FIELD TESTING AND INSPECTION OF STRUCTURAL STEEL MATERIALS, AND STRUCTURAL STEEL INSTALLATION SHALL BE COMPLETED BY AN INDEPENDENT TESTING AGENCY (COMMISSIONED BY THE OWNER), AND SHALL BE IN ACCORDANCE WITH THE SCHEDULE OF SPECIAL INSPECTIONS.

WOOD STRUCTURAL PANEL NOTES:

1. STRUCTURAL WOOD PANELS SHALL CONFORM TO THE REQUIREMENTS OF ONE OF THE FOLLOWING STANDARD:
    - A. U.S. PRODUCT STANDARD PS1-95 FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD.
    - B. U.S. PRODUCT STANDARD PS2-92 PERFORMANCE STANDARD FOR WOOD BASED STRUCTURAL USE PANELS
    - C. APA PRP-108 PERFORMANCE STANDARDS
    - D. ANY CODE-APPROVED STANDARD OR PUBLICATION. APPROVAL MUST BE OBTAINED FROM STRUCTURAL ENGINEERS.

2. ROOF PANELS SHALL BE APA RATED, EXPOSURE 1, 5/8" (AS NOTED ON DRAWINGS), 5 PLY PLYWOOD OR HUBER ADVANTECH SHEATHING WITH A MIN. 40/20 SPAN RATING UNLESS NOTED OTHERWISE ON THE DRAWINGS. SHEATHING SHALL BE EXTERIOR GRADE WHERE EITHER SIDE OF SHEATHING IS PERMANENTLY EXPOSED TO WEATHER.

3. WALL PANELS SHALL BE APA RATED, EXPOSURE 1, 1/2"" (AS NOTED ON DRAWINGS), 5 PLY PLYWOOD OR HUBER ADVANTECH WITH A MIN. 32/16 SPAN RATING UNLESS NOTED OTHERWISE ON THE DRAWINGS. SHEATHING SHALL BE EXTERIOR GRADE WHERE EITHER SIDE OF SHEATHING IS PERMANENTLY EXPOSED TO WEATHER.

4. EXTERIOR WALL PANELS SHALL BE ZIP R-6 SHEATHING w/ 0.131"x3" NAILS @4" O.C. EDGE AND 12" O.C. FIELD.

5. ALL FLOOR AND ROOF SHEATHING SHALL BE INSTALLED WITH THE FACE GRAIN PERPENDICULAR TO THE SUPPORTS AND A 1/8" GAP AT ALL PANEL EDGES UNLESS RECOMMENDED OTHERWISE BY THE PANEL MANUFACTURER.

6. ALL SHEATHING PANELS SHALL BE INSTALLED WITH END JOINTS STAGGERED UNLESS NOTED OTHERWISE ON THE DRAWINGS

7. WHERE BLOCKING IS NOT SPECIFICALLY REQUIRED FOR THE ROOF SHEATHING, PLY CLIPS OR TONGUE AND GROOVE PLYWOOD SHALL BE USED.

8. SUB-FLOOR SHEATHING SHALL BE UNBLOCKED UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS. SUB-FLOOR SHEATHING SHALL BE GLUED DOWN TO THE SUPPORTING MEMBERS AND GLUED AT THE TONGUE AND GROOVE JOINTS.

9. ALL NAILS SHALL BE COMMON NAILS. ROOF SHEATHING SHALL UTILIZE RING SHANK NAILS. STAINLESS STEEL (TYPE 316) NAILS SHALL BE USED AT PERMANENTLY EXPOSED EXTERIOR AREAS. ALL NAILS THAT ARE NOT EXPOSED TO THE ELEMENTS BUT IN CONTACT WITH PRESERVATIVE TREATED LUMBER SHALL BE MINIMUM HOT-DIPPED GALVANIZED MEETING ASTM A153.

FOUNDATION NOTES:

1. DESIGN OF FOUNDATIONS IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 POUNDS PER SQUARE FOOT.

2. ALL DELETERIOUS MATERIALS FOUND WITHIN THE LIMITS OF THE STRUCTURE, AS DETERMINED BY THE TESTING AGENCY, SHALL BE REMOVED AND REPLACED WITH COMPACTED SELECT FILL.

3. NO FOUNDATIONS SHALL BE PLACED ON FROZEN GROUND OR IN WATER. ALL TRENCHES SHALL BE DEWATERED PRIOR TO PLACING CONCRETE.

4. FROST WALLS SHALL BE CURED FOR A MINIMUM OF 7 DAYS PRIOR TO BACKFILLING. THE BACKFILL MATERIAL SHALL BE BROUGHT UP TO GRADE EQUALLY ON BOTH SIDES OF FROST WALLS.

5. RETAINING WALLS AND FOUNDATION WALLS SHALL BE CURED TO MEET SPECIFIED STRENGTH PRIOR TO BACKFILLING.

6. SELECT FILL AND BACKFILL MATERIAL SHALL BE PLACED IN MAXIMUM 8" LIFTS. EACH LIFT SHALL BE COMPACTED TO 95% OF THE MAXIMUM DRY DENSITY PER ASTM D-1557, MODIFIED PROCTOR TEST.

7. SELECT FILL AND BACKFILL MATERIAL SHALL BE SCREENED OR CRUSHED GRAVEL OF HARD DURABLE PARTICLES FREE FROM VEGETABLE MATTER, LUMPS, BALLS OF CLAY AND OTHER DELETERIOUS SUBSTANCES. SELECT FILL SHALL CONFORM TO THE FOLLOWING GRADATION REQUIREMENTS:

SELECT FILL - GRADATION REQUIREMENTS	
SIEVE SIZE	PERCENT FINER BY WEIGHT
4 INCH	100
3 INCH	90 - 100
1/4 INCH	25 - 90
No. 40	0 - 30
No. 200	0 - 5

8. ON SITE MATERIALS GENERATED DURING EXCAVATION MAY BE USED AS BACKFILL MATERIAL PLACED ADJACENT TO FOUNDATION WALLS PROVIDED IT MEETS THE GRADATION REQUIREMENTS FOR SELECT FILL.

9. FIELD QUALITY CONTROL FOR SUBGRADE PREPARATION AND ALL OTHER ASSOCIATED FOUNDATION WORK SHALL BE COMPLETED BY AN INDEPENDENT TESTING AGENCY (COMMISSIONED BY THE OWNER), AND SHALL BE IN ACCORDANCE WITH THE SCHEDULE OF SPECIAL INSPECTIONS.

10. CONCRETE FOR FOUNDATIONS SHALL COMPLY WITH THE CONCRETE NOTES

11. COORDINATE PLUMBING AND FOUNDATION ELEVATIONS TO MINIMIZE INTERFERENCES. STEP FOOTINGS PER TYPICAL DETAILS WHERE INTERFERENCES OCCUR.

12. THE CONTRACTOR SHALL SUBMIT THE FOLLOWING INFORMATION TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW:
    - A. GRADATION OF MATERIAL TO BE USED AS SELECT FILL.
    - B. COMPACTION TEST RESULTS UNDER PAVEMENTS, SLABS ON GRADE AND FOUNDATIONS.

POST INSTALLED ANCHOR NOTES:

1. NOTED EMBEDMENT DEPTHS ARE FROM FACE OF CMU OR FACE OF CONCRETE

2. ALL INSTALLATION SHALL BE IN STRICT ACCORDANCE WITH THE MANUFACTURER'S DATA AND THE ASSOCIATED ICC REPORT.

3. ALL PERSONNEL INSTALLING ANCHORS SHALL HAVE ATTENDED INSTALLER TRAINING PER THE SPECIFICATIONS.

4. FIELD TESTING AND INSPECTION OF POST INSTALLED ANCHOR MATERIALS AND POST INSTALLED ANCHOR INSTALLATION SHALL BE COMPLETED BY AN INDEPENDENT TESTING AGENCY COMMISSIONED BY THE OWNER, AND SHALL BE IN ACCORDANCE WITH THE SCHEDULE OF SPECIAL INSPECTIONS.

CONCRETE AND REINFORCEMENT NOTES:

1. ALL CONCRETE SHALL CONFORM TO LATEST EDITIONS OF ACI 318 AND ACI 301.

2. CONCRETE NOT EXPOSED TO FREEZING AND THAWING SHAL BE AS FOLLOWS:

A.	MINIMUM COMPRESSIVE STRENGTH:	4000 PSI @28 DAYS
B.	CEMENT	ASTM C150 TYPE II
C.	AGGREGATE	ASTM C33 OR C330
D.	MAXIMUM AGGREGATE SIZE	1 1/2 INCH
E.	MAXIMUM WATER-CEMENT RATIO:	0.45
F.	SLUMP:	2 TO 4 INCHES
G.	WATER REDUCING ADMIXTURE:	ASTM C494

    1. ADMIXTURES SHALL BE USED IN ACCORDANCE WITH ACI AND THE MANUFACTURERS RECOMMENDATIONS.
    2. USE OF CALCIUM CHLORIDE, CHLORIDE IONS OR OTHER SALTS IS NOT PERMITTED.

3. CONCRETE EXPOSED TO FREEZING AND THAWING SHAL BE AS FOLLOWS:

A.	MINIMUM COMPRESSIVE STRENGTH:	5000 PSI @28 DAYS
B.	CEMENT	ASTM C150 TYPE II
C.	AGGREGATE	ASTM C33 OR C330
D.	MAXIMUM AGGREGATE SIZE	1 1/2 INCH
E.	MAXIMUM WATER-CEMENT RATIO:	0.40
F.	SLUMP:	2 TO 4 INCHES
G.	AIR ENTRAINMENT:	ASTM C260, 6.5% (± 1%)
H.	WATER REDUCING ADMIXTURE:	ASTM C494

    1. ADMIXTURES SHALL BE USED IN ACCORDANCE WITH ACI AND THE MANUFACTURERS RECOMMENDATIONS.
    USE OF CALCIUM CHLORIDE, CHLORIDE IONS OR OTHER SALTS IS NOT PERMITTED.

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60

2. REINFORCING STEEL SHALL BE SUPPORTED ON CHAIRS OR BOLSTERS.

3. ALL LAP SPLICES SHALL BE IN ACCORDANCE WITH THE TABLES BELOW (TYPE 2 MECHANICAL SPLICES MAY BE USED IN LIEU OF LAP SPLICES AT CONTRACTOR'S OPTION)

4. ALL LAP SPLICES SHALL BE ACI CLASS B SPLICES. THE FOLLOWING TABLES ARE BASED ON NORMAL WEIGHT CONCRETE WITH BARS 4 BAR DIAMETERS OR MORE APART AND CONCRETE COVER EQUAL TO 2 BAR DIAMETERS OR MORE. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE PLACED BELOW THE REINFORCEMENT.

4000/5000 PSI CONCRETE	GRADE 60 REINFORCING STEEL fy = 60,000 PSI								
BAR SIZE	#3	#4	#5	#6	#7	#8	#9	#10	#11
LAP (IN) - TOP BARS	15	20	24	29	48	60	74	91	109
LAP (IN) - OTHER BARS	12	15	19	22	37	47	57	70	84

5. REINFORCEMENT SHALL BE SECURELY ANCHORED IN POSITION WHILE PLACING CONCRETE. THE CONTRACTOR SHALL PROVIDE ADDITIONAL BARS OR STIRRUPS AS REQUIRED TO ANCHOR BARS IN THE PROPER POSITION

6. THE DESIGN AND CONSTRUCTION OF FORMS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. FORMS SHALL BE CONSTRUCTED TO SHAPE, FORMS, AND LINES INDICATED ON DRAWINGS. BRACING SHALL BE DESIGNED TO RESIST FORCES EXERTED BY FRESH CONCRETE.

7. QUALIFIED WORKMEN SHALL CONSTANTLY OBSERVE AND ADJUST FORMS AND SHORES AS REQUIRED DURING CONCRETE PLACEMENT.

8. ALL SHORING SHALL REMAIN IN PLACE UNTIL THE SUPPORTED CONCRETE HAS ATTAINED 75% OF THE REQUIRED 28 DAY COMPRESSIVE STRENGTH.

9. CONTRACTOR SHALL VERIFY DIMENSIONS AND LOCATIONS OF ALL SLOTS, PIPE SLEEVES, ANCHOR BOLTS, ETC. AS REQUIRED FOR ALL OTHER TRADES BEFORE CONCRETE IS POURED. THESE ITEMS SHALL BE INSTALLED AND VERIFIED BY THE CONTRACTOR.

10. CONCRETE COVER TO REINFORCING STEEL SHALL CONFORM TO ACI 318 AS FOLLOWS:

A.	CONCRETE CAST AGAINST EARTH:		3"
B.	CONCRETE EXPOSED TO EARTH OR WEATHER:	#3 THRU #5:	1 ½"
		#6 THRU #11:	2"
C.	CONCRETE NOT EXPOSED TO WEATHER OR PLACED IN CONTACT WITH GROUND:	SLABS/WALLS:	¾"
		BEAMS/COLUMNS:	1 ½"

11. FOOTING AND GRADE BEAM SIZES SHOWN ARE FOR FOOTINGS CONSTRUCTED WITH SIDE FORMS. IF EARTH FORMING IS USED FOUNDATION SIZES SHALL INCREASED IN WIDTH 1" IN EACH DIRECTION.

12. SHOP DRAWINGS FOR PLACEMENT SHALL BE SUBMITTED FOR REVIEW PRIOR TO REBAR FABRICATION.

13. ALL INSIDE CONCRETE WEARING SURFACES SHALL RECEIVE A SMOOTH STEEL TROWEL FINISH.

14. ALL OUTSIDE CONCRETE WEARING SURFACES SHALL RECEIVE A STEEL TROWEL AND A MEDIUM BROOM FINISH PERPENDICULAR TO THE TRAFFIC FLOW.

15. ALL CONCRETE SLABS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE FOLLOWING FLATNESS/LEVELNESS REQUIREMENTS:

SLAB CATEGORY	FLATNESS, Ff	LEVELNESS, Fl	DEVIATION
BULLFLOATED	15	13	½" IN 10'
STRAIGHTEDGED	20	15	5/16" IN 10'
FLAT	30	20	3/16" IN 10'
VERY FLAT	50	30	⅛" IN 10'
SUPERFLAT	100	50	<⅛" IN 10'

FLOOR FLATNESS / LEVELNESS TESTS SHALL BE CONDUCTED ACCORDING TO ASTM E1155

16. PROVIDE A 3/4" CHAMFER TO ALL EXPOSED CONCRETE EDGES

17. WET CURE ALL CONCRETE SLABS FOR A MINIMUM OF 3 DAYS.

18. SAW CUT CONTROL JOINTS SHALL BE 1/8"x1 1/2" DEEP CUT WITH AN EARLY ENTRY DRY-CUT SAW AS SOON AS THE CONCRETE IS SUFFICIENTLY HARD TO RESIST TEARING AND RAVELING (1 TO 4 HOURS AFTER FINISHING).

19. HORIZONTAL JOINTS IN FOOTINGS, GRADE BEAMS, AND TIE BEAMS WILL NOT BE PERMITTED.

20. DO NOT INSTALL PLUMBING SLEEVES IN GRADE BEAMS OR TIE BEAMS WITHOUT ENGINEER APPROVAL.

21. REINFORCING BARS SHALL NOT BE CUT TO ACCOMMODATE THE INSTALLATION OF ANCHORS, EMBEDS, OR OTHER ITEMS.

22. AT CHANGES IN DIRECTION OF CONTINUOUS CONCRETE ELEMENTS PROVIDE CORNER BARS OF SAME SIZE AND SPACING OF HORIZONTAL REINFORCEMENT.

23. PLACE CONCRETE PER ACI 304. USE INTERNAL MECHANICAL VIBRATION FOR ALL CONCRETE. LIMIT MAXIMUM FREE FALL DROP OF CONCRETE TO 6'-0". PRECAUTIONS SHALL BE TAKEN TO AVOID SEGREGATION DURING CONCRETE PLACEMENT.

24. FIELD TESTING AND INSPECTION OF ALL CONCRETE MATERIALS AND CONCRETE INSTALLATION SHALL BE CONDUCTED BY AN INDEPENDENT TESTING AGENCY (COMMISSIONED BY THE OWNER), AND SHALL BE IN ACCORDANCE WITH THE SCHEDULE OF SPECIAL INSPECTIONS.

GENERAL NOTES:

1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL CONTRACT DRAWINGS, AND ASSOCIATED SHOP DRAWING SUBMITTALS. CONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING AND COORDINATING DIMENSIONS, CLEARANCES, ETC. WITH WORK OF OTHER TRADES.

2. IN CASE OF CONFLICT BETWEEN VARIOUS STRUCTURAL DRAWINGS, OR STRUCTURAL PLANS AND DETAILS, THE MORE STRINGENT REQUIREMENT SHALL GOVERN

3. IN CASE OF CONFLICT BETWEEN DRAWINGS, NOTES, AND SPECIFICATIONS THE MORE STRINGENT REQUIREMENT SHALL GOVERN.

4. WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE REPEATED.

5. ALL DETAILS AND SECTIONS ARE INTENDED TO BE TYPICAL FOR THE GENERAL CONDITIONS INDICATED. ALL DETAILS SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION THROUGHOUT THE PROJECT EXCEPT WHERE A SEPARATE DETAIL IS INDICATED

6. REVIEW ALL PROJECT DOCUMENTS PRIOR TO FABRICATION AND START OF CONSTRUCTION. REPORT ANY DISCREPANCIES TO CONTRACTING OFFICER OR A/E PRIOR TO PROCEEDING WITH WORK.

7. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROTECT EXISTING FACILITIES, STRUCTURES, UTILITY LINES, ETC. FROM DAMAGE DURING CONSTRUCTION.

8. COORDINATE STRUCTURAL DRAWINGS WITH OTHER CONTRACT DRAWINGS FOR ANCHORED, EMBEDDED OR SUPPORTED ITEMS THAT MAY AFFECT THE STRUCTURAL DRAWINGS.

9. USE OF CONTRACT DRAWINGS REPRODUCED IN WHOLE OR IN ANY PART FOR SHOP DRAWING PRODUCTION SHALL NOT RELIEVE THE CONTRACTOR OR SUBCONTRACTOR FROM THE REQUIREMENT TO ACCURATELY LAYOUT, COORDINATE, DETAIL, FABRICATE AND INSTALL A COMPLETE STRUCTURE.

10. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE SUBCONTRACTOR AND CONTRACTOR FOR CONFORMANCE WITH CONTRACT DOCUMENTS, COMPLETENESS, AND TO RESPOND TO QUESTIONS RELATED TO CONTRACTOR INFORMATION PRIOR TO SUBMITTING FOR APPROVAL. ALL

MASONRY NOTES:

1. ALL MASONRY WORK SHALL BE IN COMPLIANCE WITH ACI 530, "BUILDING CODE REQUIREMENTS FOR MASONRY CONSTRUCTION" AND ACI 530.1 "SPECIFICATIONS FOR MASONRY CONSTRUCTION."

2. HOLLOW LOAD BEARING CONCRETE BLOCK SHALL CONFORM TO ASTM C90, TYPE 1, NORMAL WEIGHT. BLOCK UNITS SHALL BE TWO CELL, 50% SOLID WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.

3. SPECIFIED MASONRY DESIGN COMPRESSIVE STRENGTH, f'm = 1500 PSI.

4. MORTAR SHALL CONFORM TO ASTM C270, TYPE S. MINIMUM COMPRESSIVE STRENGTH SHALL BE 1800 PSI AT 28 DAYS.

5. GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.

6. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED BARS.

7. HORIZONTAL JOINT REINFORCEMENT SHALL BE LADDER TYPE, STANDARD CLASS, MILL GALVANIZED WITH 9 GAGE SIDE RODS AND 9 GAGE CROSS RODS. HORIZONTAL JOINT REINFORCEMENT SHALL BE PROVIDED AT EVERY OTHER HORIZONTAL JOINT UNLESS NOTED OTHERWISE. WHERE JOINT REINFORCEMENT IS SPLICED PROVIDE 12" MINIMUM LAP, INCLUDING CORNERS AND TEES. PREFABRICATED CORNERS AND TEES SHALL BE USED AT ALL WALL INTERSECTIONS.

8. PROVIDE CONTROL JOINTS IN CONCRETE MASONRY WALLS AS INDICATED OR, IF NOT INDICATED, AT A MAXIMUM SPACING OF 25' ON CENTER.

9. HOLLOW CONCRETE UNITS SHALL BE LAID IN RUNNING BOND UNLESS NOTED OTHERWISE. PROVIDE FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACES. EXPOSED JOINTS SHALL BE TOOLED CONCAVE, UNEXPOSED JOINTS SHALL BE STRUCK FLUSH.

10. CORNER BLOCKS AND END BLOCKS SHALL BE USED TO FINISH ALL 90 DEGREE CORNERS SAND WALL OPENINGS.

11. ALL BOND BEAM BLOCKS SHALL BE KNOCK-OUT TYPE BLOCKS. 41/2" MINIMUM GROUT MESH SHALL BE USED UNDER BOND BEAMS TO CONFINE GROUT FROM HOLLOW CORES.

12. GROUTING:
    - A. CELLS THAT ARE TO BE GROUTED SOLID SHALL BE ALIGNED TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL CELL. PROVIDE CLEANOUTS IN THE BOTTOM COURSE OF MASONRY FOR EACH GROUT POUR, AT EACH VERTICAL BAR.
    - B. GROUT SOLID ALL CELLS CONTAINING REINFORCING BARS OR OTHER ATTACHMENTS.
    - C. GROUT SOLID (3) CELLS MINIMUM BELOW LINTELS AND STEEL BEAMS BEARING ON MASONRY.
    - D. LOW LIFT GROUTS SHALL NOT EXCEED 5 FEET. GROUT POUR HEIGHT SHALL NOT EXCEED 10 FEET. PROVIDE CLEANOUTS IN THE BOTTOM COURSE OF MASONRY FOR EACH GROUT POUR, WHEN GROUT POUR EXCEEDS 5 FEET.
    - E. HIGH LIFT GROUTS SHALL NOT EXCEED 12'-8" AND THE FOLLOWING CONDITIONS MUST BE MET:
        - a. THE MASONRY MUST CURE FOR AT LEAST FOUR HOURS.
        - b. THE GROUT SLUMP MUST BE MAINTAINED BETWEEN 10 AND 11 INCHES.
        - c. THE WALL CANNOT CONTAIN INTERMEDIATE BOND BEAMS BETWEEN THE TOP AND BOTTOM OF THE POUR HEIGHT.
    - F. GROUTING SHALL BE STOPPED 11/2" BELOW THE TOP OF A COURSE TO FORM A KEY AT THE JOINT.
    - G. GROUTING OF MASONRY BEAMS OR LINTELS SHALL BE DONE IN ONE CONTINUOUS OPERATION.

13. MASONRY LINTELS:
    - A. PROVIDE A MINIMUM OF 8" BEARING AT EACH END OF EACH LINTEL. KNOCK-OUT SHALL BE PROVIDED. USE LINTEL-TYPE BLOCKS ONLY AT OPENINGS.
    - B. EXTEND BOTTOM BARS 24" BEYOND THE OPENING FOR #4 & #5 BARS AND 30" BEYOND THE OPENING FOR #6.

14. REINFORCING:
    - A. VERTICAL REINFORCING SHALL BE PLACED AT EACH JAMB OF EACH WALL OPENING, AT EACH WALL END, AT EACH SIDE OF WALL CONTROL JOINT, AT EACH WALL INTERSECTION.
    - B. SPLICED REINFORCING SHALL BE LAPPED 48 BAR DIAMETERS OR AS SHOWN ON DRAWINGS, WHICHEVER IS GREATER.
    - C. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4" FROM MASONRY AND SHALL BE HELD IN POSITION TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING FOUR (4) FEET.
    - D. FOUNDATION DOWELS MAY BE SLOPED NO MORE THAN 1h:6v TO ALIGN WITH WALL CAVITIES OR VERTICAL CMU CORES. PROVIDE DOWELS TO MATCH WALL REINFORCEMENT SIZE AND SPACING, UNLESS OTHERWISE NOTED ON THE DRAWINGS.
    - E. LOCATE ALL VERTICAL REINFORCEMENT CENTERED IN THE WALL UNLESS OTHERWISE DETAILED.
    - F. WHERE BOND BEAMS ARE DISCONTINUOUS, EXTEND REINFORCEMENT 48 BAR DIAMETERS BEYOND THE SPECIFIED TERMINATION.
    - G. WHERE BOND BEAMS ARE REQUIRED TO STEP BY 4"-12" VERTICALLY, REMOVE MASONRY AS REQUIRED TO MAINTAIN CONTINUITY OF REINFORCEMENT @ A SLOPE = 6 UNITS HORIZONTAL TO 1 UNIT VERTICAL.

15. FIELD PENETRATIONS THROUGH BLOCK WALLS SHALL NOT BE MADE THROUGH BOND BEAMS, LINTELS OR GROUTED CELLS.

16. DRAWINGS INDICATE DOWELS FOR SECURING REINFORCEMENT IN STRUCTURAL WALLS TO BE CAST IN CONCRETE. POST-INSTALLED DOWELS ARE ACCEPTABLE AS A SUBSTITUTION WHEN APPROVED BY THE STRUCTURAL ENGINEER OF RECORD, SUBJECT TO THE FOLLOWING REQUIREMENTS:
    - A. REQUIRED HOLE SIZE FOR "UNIFORM VERTICAL REINFORCEMENT" SHALL BE AS FOLLOWS:
        - a. #4 5/8"∅x4 1/4"
        - b. #5 3/4"∅x5 1/4"
    - B. REQUIRED HOLE SIZE FOR "JAMB REINFORCEMENT" SHALL BE AS FOLLOWS:
        - a. #4 5/8"∅x6 1/4"
        - b. #5 3/4"∅x8"
    - C. SET BARS IN EPOXY INJECTION ADHESIVE HILTI HIT-RE 500 OR AN APPROVED EQUAL. COMPLY WITH MANUFACTURER'S SPECIFICATIONS FOR PREPARATION & PLACEMENT REQUIREMENTS.

REV.	DATE	DESCRIPTION	BY	CHK
A	2025/06/19	RE-BID ADDENDUM 1	WCT	CDL

DRAWING ISSUE STATUS

ISSUED FOR RE-BID

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PROJECT

CONNOR SCHOOL RENOVATIONS

1581 VAN BUREN RD, CONNOR, ME 04736

TITLE

STRUCTURAL NOTES

STATE OF MAINE
WILLIAM C. TIMKEN
No. 18004
LICENSED PROFESSIONAL ENGINEER

DATE	SCALE
2025.06.19	12" = 1'-0"

DRAWN BY	DESIGNED BY	CHECKED BY
WCT	WCT	CDL

PROJECT No. 10377.028

DRAWING NO. **S002** REV. **A**

STRUCTURAL STATEMENT OF TESTS AND SPECIAL INSPECTIONS

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS PROJECT AS WELL AS THE NAME OF THE STRUCTURAL SPECIAL INSPECTION COORDINATOR (SSIC) AND THE IDENTITY OF OTHER APPROVED AGENCIES TO BE RETAINED FOR CONDUCTING THESE INSPECTIONS AND TESTS.

THE STRUCTURAL SPECIAL INSPECTION COORDINATOR SHALL KEEP RECORDS OF ALL STRUCTURAL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING CODE OFFICIAL (BCO) AND THE STRUCTURAL REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE (SRDP). DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE STRUCTURAL REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITIES.

INTERIM REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND THE STRUCTURAL REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AT AN INTERVAL DETERMINED BY THE SSIC AND THE BCO.

A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS, TESTING AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE BCO PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

PROJECT NAME:		CONNOR SCHOOL RENOVATIONS	
PROJECT LOCATION:		1581 VAN BUREN RD, CONNOR, ME 04736	
OWNER:		MAINE DEPT. OF EDUCATION	
BUILDING OFFICIAL:		TO BE DETERMINED	
DESIGN PROFESSIONAL: (STRUCTURAL ENGINEER)		WILLIAM TIMKEN, P.E.	
SEISMIC RESISTANCE:		SEISMIC DESIGN CATEGORY "B"	
WIND RESISTANCE:		EXPOSURE "B," BASIC WIND SPEED 102 MPH	
PREPARED BY:	WILLIAM TIMKEN, P.E.	DESIGN PROFESSIONAL SEAL:	
SIGNATURE	DATE		
OWNER'S AUTHORIZATION:		BUILDING CODE OFFICIAL'S ACCEPTANCE:	
SIGNATURE	DATE	SIGNATURE	DATE
PRINT NAME:		PRINT NAME:	

SCHEDULE OF TESTING AND INSPECTION AGENCIES

	SPECIAL INSPECTION AGENCIES	FIRM	ADDRESS, TELEPHONE, EMAIL
1	SPECIAL INSPECTIONS COORDINATOR	HALEY WARD	ONE MERCHANTS PLAZA, SUITE 701 BANGOR, MAINE 04401 F. (207)-989-4824
2	BUILDING INSPECTOR	TO BE DETERMINED	-
3	EARTHWORK INSPECTOR	TO BE DETERMINED	-
4	CONCRETE TESTING LABORATORY	TO BE DETERMINED	-
5	STRUCTURAL STEEL / WELD TESTING AGENCY	TO BE DETERMINED	-

NOTE: THE INSPECTORS AND TESTING AGENCIES SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL, PRIOR TO COMMENCING WORK.

CONTRACTOR'S RESPONSIBILITIES:

1. STRUCTURAL TESTS AND SPECIAL INSPECTIONS DO NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES AND OBLIGATIONS FOR QUALITY CONTROL OF THE WORK. THEIR OBLIGATIONS FOR SUPERVISING THE WORK, FOR ANY DESIGN WORK THAT IS INCLUDED IN THEIR SCOPE OF SERVICES OR FOR FULL COMPLIANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. FURTHERMORE, THE DETECTION OF OR FAILURE TO DETECT DEFICIENCIES IN THE WORK DURING TESTING AND INSPECTION CONDUCTED PURSUANT TO THIS PROGRAM SHALL NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITY TO CORRECT ALL DEFICIENCIES OR DEFECTS, WHETHER DETECTED OR UNDETECTED, IN ALL PARTS OF THE WORK, AND TO OTHERWISE COMPLY WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS.
2. JOB SITE SAFETY IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT PART OF THESE TESTS AND INSPECTIONS. MATERIALS AND ACTIVITIES TO BE TESTED AND INSPECTED DO NOT INCLUDE THE CONTRACTOR'S EQUIPMENT OR THE MEANS, METHODS AND PROCEDURES USED TO ERECT OR INSTALL THE MATERIALS OR ASSEMBLIES LISTED.
3. WHERE A STRUCTURAL COMPONENT OR SYSTEM IS SUBJECT TO TESTS AND INSPECTIONS AS DETERMINED BY THE BUILDING OFFICIAL, AND THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE FOR THE PROJECT HAS NOT BEEN RETAINED TO DESIGN OR TO PREPARE A PERFORMANCE SPECIFICATION FOR SAID COMPONENT OR SYSTEM, THE CONTRACTOR SHALL RETAIN A LICENSED PROFESSIONAL ENGINEER TO DESIGN SAID COMPONENT OR SYSTEM AND TO PROVIDE ANY REQUIRED TESTS AND INSPECTIONS.
4. THE CONTRACTOR SHALL PROVIDE FREE AND SAFE ACCESS TO THE WORK FOR ALL INDIVIDUALS WHO ARE PERFORMING THE TESTS OR INSPECTIONS. THE CONTRACTOR SHALL PROVIDE ALL LADDERS, SCAFFOLDING, STAGING, AND UP-TO DATE SAFETY EQUIPMENT, ALL IN GOOD AND SAFE WORKING ORDER, AND QUALIFIED PERSONNEL TO HANDLE AND ERECT THEM, AS MAY BE REQUIRED FOR SAFE ACCESS.
5. THE CONTRACTOR SHALL GIVE REASONABLE NOTICE TO THOSE PERFORMING INSPECTIONS AND TESTS OF WHEN THE VARIOUS PARTS OF THE WORK WILL BE READY FOR TESTING AND INSPECTION. THE CONTRACTOR SHALL OBTAIN INSTRUCTIONS FROM THE INSPECTION COORDINATOR AS TO WHAT IS REASONABLE NOTICE FOR THE VARIOUS ASPECTS OF THE WORK (TYPICALLY 48 HOURS), WHO IS TO BE NOTIFIED AND HOW.
6. THE OWNER RESERVES THE RIGHT TO BACK CHARGE THE CONTRACTOR FOR ADDITIONAL EXPENSE INCURRED BY THE OWNER FOR THE SERVICES OF THE INSPECTORS WHEN WORK IS NOT REASONABLY READY FOR INSPECTION IN ACCORDANCE WITH THE NOTICE PROVIDED BY THE CONTRACTOR. LIKEWISE, IF WORK IS REPEATEDLY FOUND DEFICIENT, COSTS FOR A THIRD INSPECTION AND BEYOND MAY BE REIMBURSED FROM THE CONTRACTOR.

QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS

THE CREDENTIALS AND QUALIFICATIONS OF ALL INDIVIDUALS PERFORMING SPECIAL INSPECTION AND TESTING ACTIVITIES ARE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL AND THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. CREDENTIALS SHALL BE PROVIDED FOR REVIEW, APPROVAL AND RECORD.

KEY FOR MINIMUM QUALIFICATIONS OF INSPECTION AGENTS: WHEN THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE DEEMS IT APPROPRIATE THAT THE INDIVIDUAL PERFORMING A STIPULATED TEST OR INSPECTION HAVE A SPECIFIC CERTIFICATION, LICENSE OR EXPERIENCE LEVEL AS INDICATED BELOW, SUCH DESIGNATION SHALL APPEAR WITH THE AGENCY NAME ON THE SCHEDULE.

ENGINEER	
PE/SE	STRUCTURAL ENGINEER – A LICENSED SE OR PE SPECIALIZING IN THE DESIGN OF BUILDING STRUCTURES
PE/GE	GEOTECHNICAL ENGINEER – A LICENSED PE SPECIALIZING IN SOIL MECHANICS AND FOUNDATIONS
EIT	ENGINEER-IN-TRAINING – A GRADUATE ENGINEER WHO HAS PASSED THE FUNDAMENTALS OF ENGINEERING EXAMINATION
EXPERIENCED TESTING TECHNICIAN	
ETT	EXPERIENCED TESTING TECHNICIAN - AN EXPERIENCE TESTING TECHNICIAN WITH A MINIMUM 5 YEARS EXPERIENCE WITH THE STIPULATED TEST.
AMERICAN CONCRETE INSTITUTE (ACI) CERTIFICATION	
ACI-CFTT	CONCRETE FIELD TESTING TECHNICIAN – GRADE 1
ACI-CCI	CONCRETE CONSTRUCTION INSPECTOR
ACI-LTT	LABORATORY TESTING TECHNICIAN – GRADE 1&2
ACI-STT	STRENGTH TESTING TECHNICIAN
AMERICAN WELDING SOCIETY (AWS) CERTIFICATION	
AWS-CWI	CERTIFIED WELDING INSPECTOR
AWS/AISC-SSI	CERTIFIED STRUCTURAL STEEL INSPECTOR
INTERNATIONAL CODE COUNCIL (ICC) CERTIFICATION	
ICC-SMSI	STRUCTURAL MASONRY SPECIAL INSPECTOR
ICC-SWSI	STRUCTURAL STEEL AND WELDING SPECIAL INSPECTOR
ICC-RCSI	REINFORCED CONCRETE SPECIAL INSPECTOR
NATIONAL INSTITUTE FOR CERTIFICATION IN ENGINEERING TECHNOLOGIES (NICET)	
NICET-CT	CONCRETE TECHNICIAN – LEVELS I, II, III & IV
NICET-ST	SOILS TECHNICIAN - LEVELS I, II, III & IV
NICET-GET	GEOTECHNICAL ENGINEERING TECHNICIAN - LEVELS I, II, III & IV

REQUIRED VERIFICATION AND INSPECTION OF SOILS

	VERIFICATION AND INSPECTION TASK	FREQUENCY	AGENT	QUALIF.
1	VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY AND ARE CONSISTENT WITH THE GEOTECHNICAL REPORT.	PERIODIC	TBD	PE/GE EIT, ETT
2	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED THE PROPER MATERIAL.	PERIODIC	TBD	PE/GE EIT, ETT
3	PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	PERIODIC	TBD	PE/GE EIT, ETT
4	TEST AND VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	CONTINUOUS	TBD	PE/GE EIT, ETT
5	INSPECT REMOVAL OF UNSUITABLE MATERIAL AND PREPARATION OF SUBGRADE PRIOR TO PLACEMENT OF CONTROLLED FILL.	CONTINUOUS	TBD	PE/GE EIT, ETT
6	PROPER TO PLACEMENT OF CONTROLLED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC	TBD	PE/GE EIT, ETT
7	APPROVE SUBGRADE PRIOR TO FORMING FOOTINGS, PREPARING SLABS-ON-GRADE AND PLACING CONCRETE.	PERIODIC	TBD	PE/GE EIT, ETT

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

	VERIFICATION AND INSPECTION TASK	FREQUENCY	AGENT	QUALIF.
1	INSPECTION OF REINFORCING STEEL INCLUDING SIZE, SPACING, COVER, LAPS. VERIFY THAT BARS ARE FREE OF FORM OIL OR OTHER DELETERIOUS MATERIALS. VERIFY THAT BARS ARE ADEQUATELY TIED AND SUPPORTED ON CHAIRS OR BOLSTERS	PERIODIC	TBD	PE/SE EIT
2	INSPECTION OF EMBEDDED STRUCTURAL STEEL ITEMS, SUCH AS COLUMN ANCHOR RODS, PRIOR TO AND DURING CONCRETE PLACEMENT. INSPECT SIZE, POSITIONING, EMBEDMENT AND CONCRETE CONSOLIDATION AROUND ANCHORS.	PERIODIC	TBD	PE/SE EIT
3	REVIEW BATCH TICKETS AND VERIFY COMPLIANCE WITH APPROVED MIX DESIGN. VERIFY THAT WATER ADDED AT THE SITE DOES NOT EXCEED THAT ALLOWED BY THE MIX DESIGN.	CONTINUOUS	TBD	ACI-CFTT, ACI-STT
4	AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, VERIFY SLUMP, AIR CONTENT AND TEMPERATURE.	CONTINUOUS	TBD	ACI-CFTT, ACI-STT
5	INSPECTION OF CONCRETE FOR PROPER PLACEMENT TECHNIQUES, INCLUDING HOT AND COLD WEATHER CONCRETING. VERIFY THAT CONVEYANCE AND DEPOSITING AVOIDS SEGREGATION OR CONTAMINATION AND PROPER CONSOLIDATION.	CONTINUOUS	TBD	ACI-CFTT, ACI-STT
6	INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	TBD	PE/SE EIT
7	INSPECT FORMWORK GEOMETRY, FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	TBD	PE/SE EIT
8	INSPECT THE TIMELY INSTALLATION OF SLAB CONTROL JOINTS AND LOCATION OF THICKENED SLABS.	PERIODIC	TBD	PE/SE EIT

REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

	VERIFICATION AND INSPECTION TASK	FREQUENCY	AGENT	QUALIF.
1	REVIEW SHOP FABRICATION CERTIFICATION AND QUALITY CONTROL PROCEDURES.	SUBMITTAL	TBD	PE/SE
2	VERIFY MATERIALS: REVIEW CERTIFIED MILL TEST REPORTS AND IDENTIFICATION MARKINGS ON WIDE-FLANGE SHAPES, HIGH-STRENGTH BOLTS, NUTS AND WELDING ELECTRODES.	SUBMITTAL	TBD	PE/SE
3	INSPECT INSTALLATION AND TIGHTENING OF HIGH-STRENGTH BOLTS IN BEARING CONNECTIONS.	PERIODIC	TBD	AWS/ AISC-SSI
4	VISUALLY INSPECT ALL WELDS. INSPECT PRE-HEAT, POST-HEAT AND SURFACE PREPARATION BETWEEN PASSES. VERIFY SIZE AND LENGTH OF FILLET WELDS.	PERIODIC	TBD	AWS-CWI
5	INSPECT STEEL FRAME FOR COMPLIANCE WITH STRUCTURAL DRAWINGS, INCLUDING BRACING, MEMBER CONFIGURATION AND CONNECTION DETAILS.	PERIODIC	TBD	PE/SE EIT

NOTE: SPECIAL INSPECTIONS AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360. FOR ADDITIONAL REQUIREMENTS REFERENCE CHAPTER N OF AISC 360.

REQUIRED VERIFICATION AND INSPECTION OF WOOD CONSTRUCTION

	VERIFICATION AND INSPECTION TASK	FREQUENCY	AGENT	QUALIF.
1	VERIFY WOOD STRUCTURAL PANEL SHEATHING FOR GRADE AND THICKNESS	PERIODIC	TBD	PE/SE
2	VERIFY THE NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES	PERIODIC	TBD	PE/SE
3	VERIFY THE NAIL OR STAPLE DIAMETER AND LENGTH	PERIODIC	TBD	PE/SE
4	VERIFY THE SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS.	PERIODIC	TBD	PE/SE
5	LOAD TESTS FOR JOIST/TRUSS HANGERS: PROVIDE EVIDENCE OF MANUFACTURER'S LOAD TEST IN ACCORDANCE WITH ASTM D1761 INCLUDING THE VERTICAL LOAD BEARING CAPACITY, TORSIONAL MOMENT CAPACITY, AND DEFLECTION CHARACTERISTICS WHEN THERE IS NO CALCULATED PROCEDURE RECOGNIZED BY THE CODE.	SUBMITTAL	TBD	PE/SE EIT

REQUIRED VERIFICATION AND LEVEL 1 INSPECTION OF MASONRY CONSTRUCTION

	VERIFICATION AND INSPECTION TASK	FREQUENCY	AGENT	QUALIF.
1	VERIFY STRENGTH: SAMPLE, PREPARE AND TEST GROUT SPECIMENS, MORTAR SPECIMENS, AND MASONRY PRISMS.	CONTINUOUS	TBD	ETT
2	INSPECT PROPORTIONING, MIXING AND RETEMPERING OF MORTAR AND GROUT.	CONTINUOUS	TBD	ETT
3	VERIFY MASONRY INSTALLATION SPECIFICATIONS. INSPECT SIZE, LAYOUT, BONDING AND PLACEMENT OF MASONRY UNITS.	CONTINUOUS	TBD	ETT
4	INSPECT CELLS PRIOR TO GROUTING AND VERIFY GROUTING PROCEDURES. INSPECT PLACEMENT AND CONSOLIDATION OF GROUT. INSPECT MASONRY CLEAN-OUTS FOR HIGH-LIFT GROUTING.	CONTINUOUS	TBD	ETT
5	INSPECT CONSTRUCTION OF MORTAR JOINTS INCLUDING TOOLING AND FILLING OF HEAD JOINTS.	CONTINUOUS	TBD	ETT
6	INSPECT ANCHORAGE OF MASONRY TO OTHER CONSTRUCTION, AND THE INSTALLATION OF EMBEDDED ITEMS.	PERIODIC	TBD	PE/SE EIT
7	VERIFY CURING AND PROTECTION PROCEDURES. VERIFY PROTECTION FOR COLD AND HOT WEATHER PROCEDURES. VERIFY THAT WALL CAVITIES ARE PROTECTED AGAINST PRECIPITATION.	PERIODIC	TBD	PE/SE EIT
8	INSPECT THE SIZE, QUANTITY, CONDITION AND PLACEMENT OF REINFORCING.	PERIODIC	TBD	PE/SE EIT
		CONTINUOUS	TBD	ETT

REV.	DATE	DESCRIPTION	BY	CHK
A	2025/06/19	RE-BID ADDENDUM 1	WCT	CDL

DRAWING ISSUE STATUS

ISSUED FOR RE-BID

HALEY WARD
ENGINEERING | ENVIRONMENTAL | SURVEYING
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PROJECT

CONNOR SCHOOL RENOVATIONS

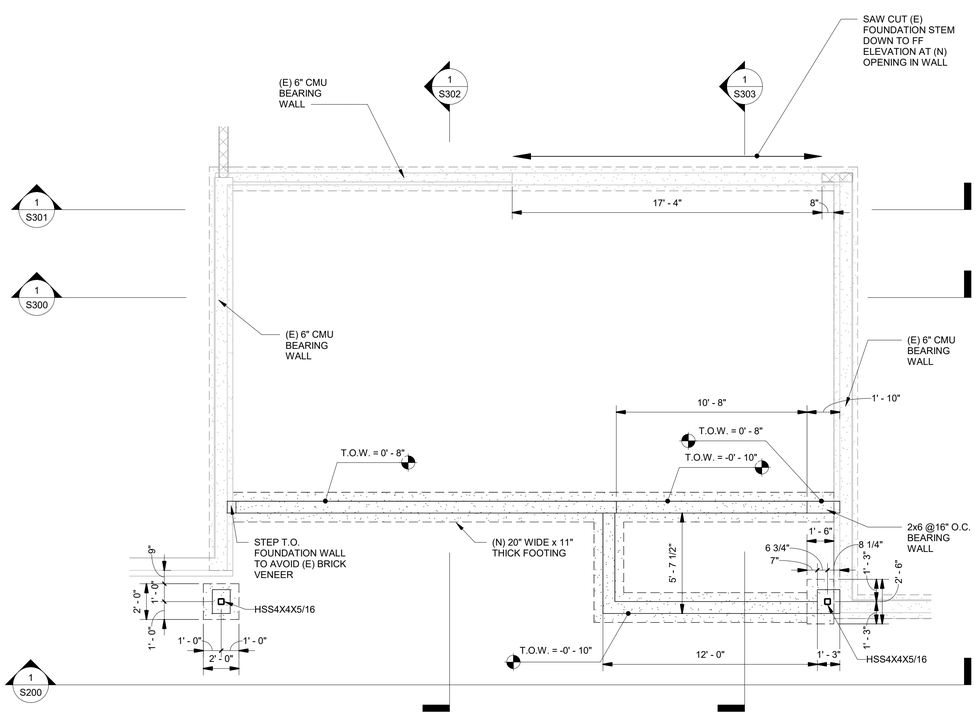
1581 VAN BUREN RD, CONNOR, ME 04736

TITLE

SPECIAL INSPECTIONS

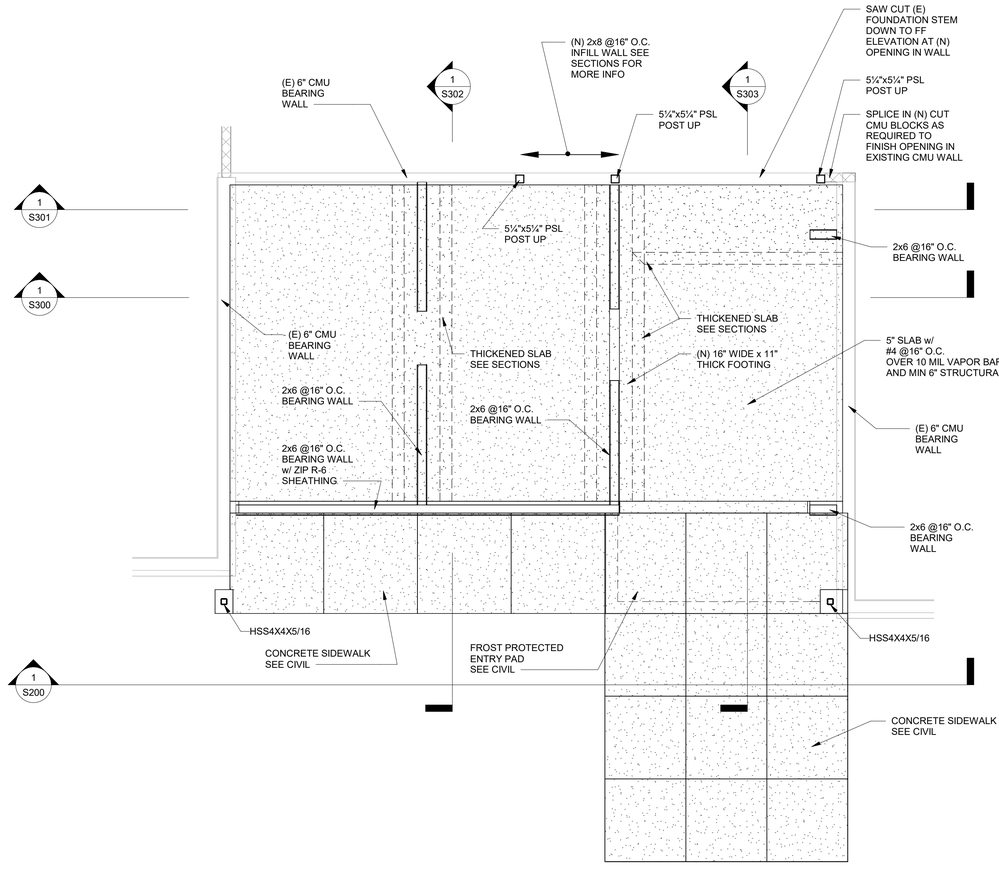
STATE OF MAINE
WILLIAM C. TIMKEN
No. 18004
LICENSED PROFESSIONAL ENGINEER

DATE	2025.06.19
SCALE	12" = 1'-0"
DRAWN BY	WCT
DESIGNED BY	WCT
CHECKED BY	CDL
PROJECT No.	10377.028
DRAWING NO.	S003
REV.	A



1 S101 FOUNDATION PLAN

SCALE: 1/4" = 1'-0"



2 S101 SLAB AND FRAMING PLAN

SCALE: 1/4" = 1'-0"

REV	DATE	DESCRIPTION	BY	CHK
A	2025/06/19	RE-BID ADDENDUM 1	WCT	CDL

DRAWING ISSUE STATUS

ISSUED FOR RE-BID

HALEY WARD
ENGINEERING | ENVIRONMENTAL | SURVEYING
One Merchants Plaza, Suite 701
Bangor, Maine 04401
207.989.4824
WWW.HALEYWARD.COM

PROJECT

CONNOR SCHOOL RENOVATIONS

1581 VAN BUREN RD, CONNOR, ME 04736

TITLE

FOUNDATION AND SLAB PLANS

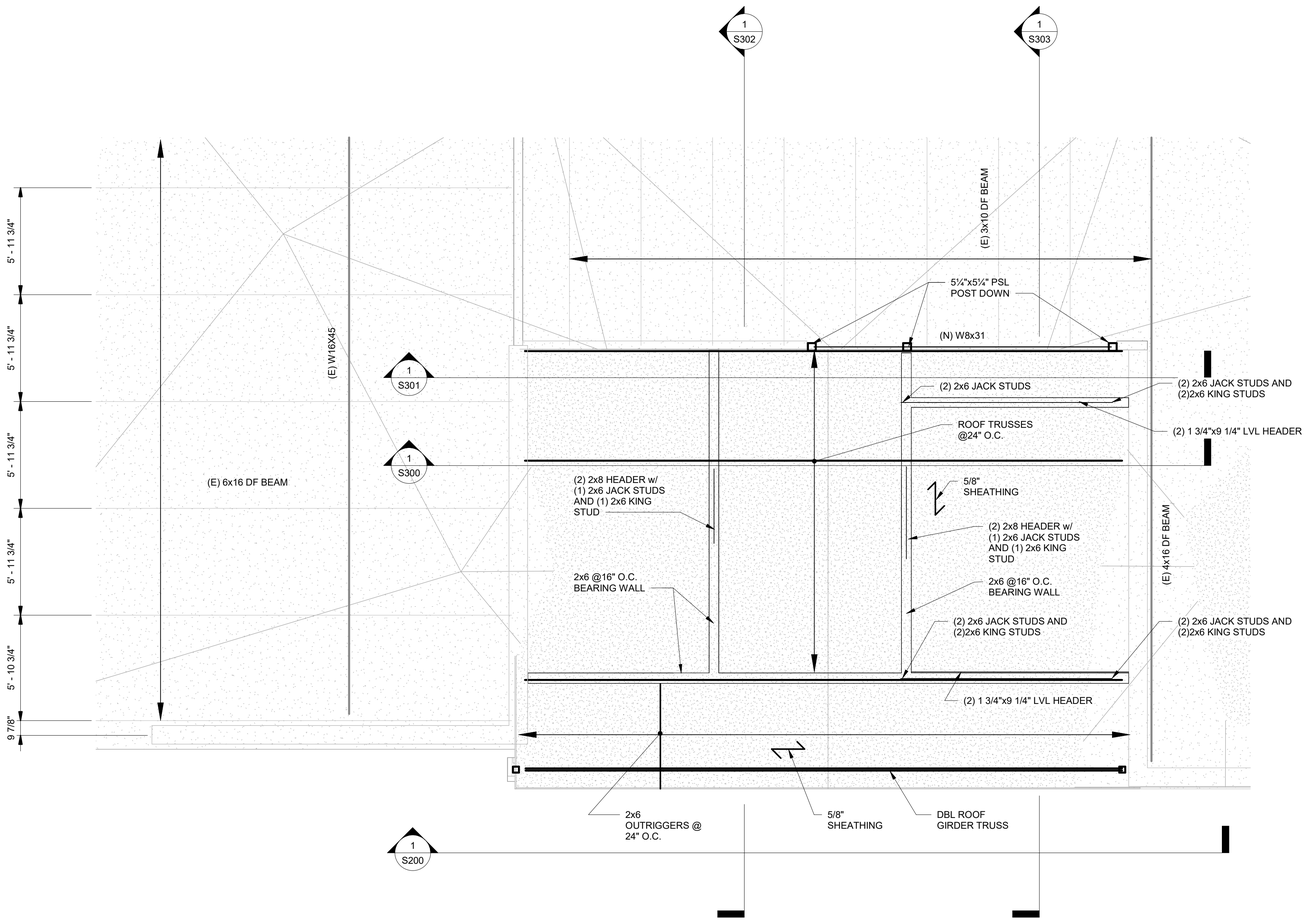
STATE OF MAINE
WILLIAM C. TIMKEN
No. 18004
LICENSED PROFESSIONAL ENGINEER

DATE	SCALE
2025.06.19	1/4" = 1'-0"

DRAWN BY	DESIGNED BY	CHECKED BY
WCT	WCT	CDL

PROJECT No. 10377.028

DRAWING NO. **S101** REV. **A**



1 S102 ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

REV.	DATE	DESCRIPTION	BY	CHK
A	2025/06/19	RE-BID ADDENDUM 1	WCT	CDL

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PROJECT

CONNOR SCHOOL RENOVATIONS

1581 VAN BUREN RD, CONNOR, ME 04736

TITLE

ROOF FRAMING PLANS

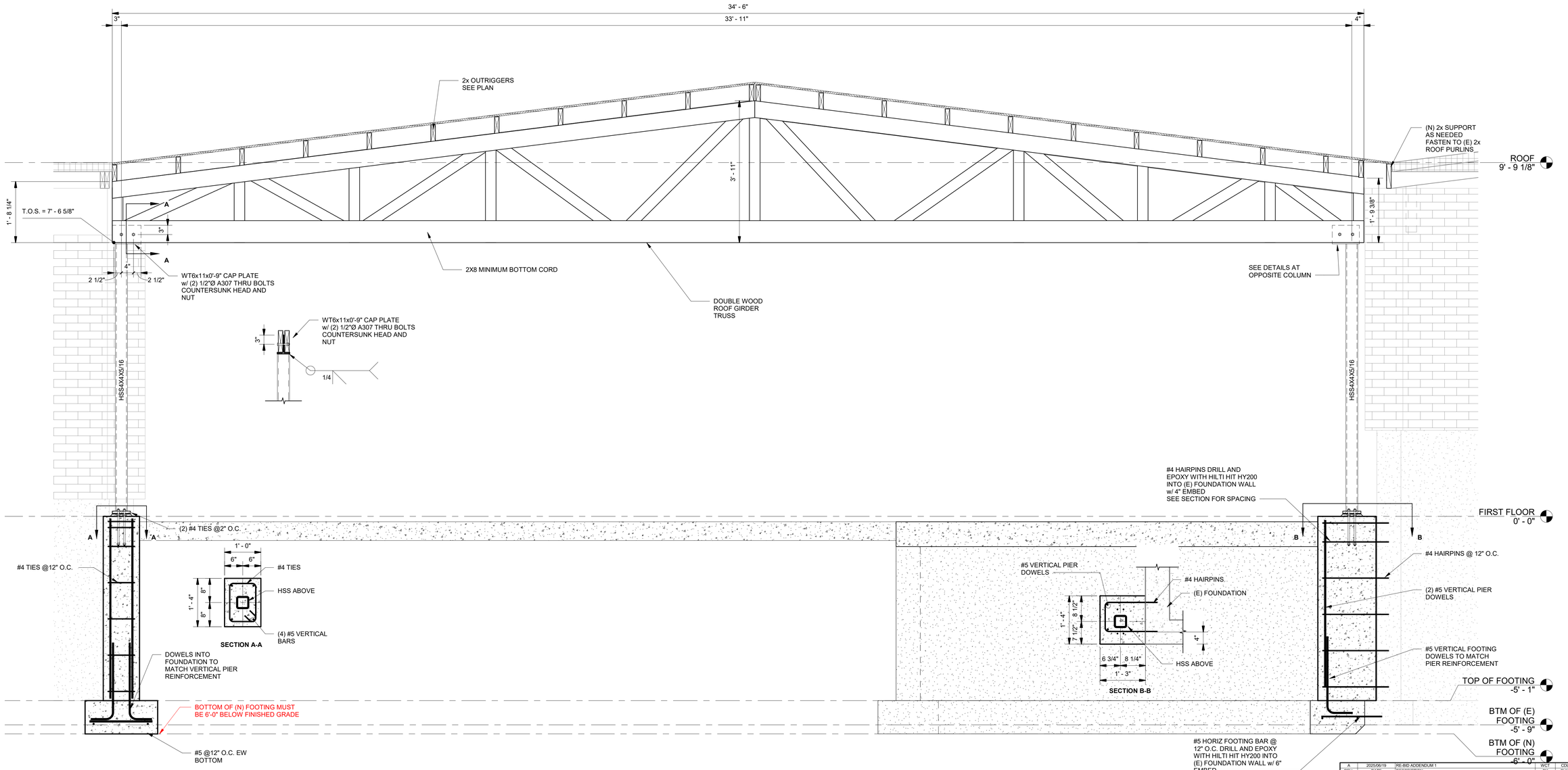
STATE OF MAINE
WILLIAM C. TIMKEN
No. 18004
LICENSED PROFESSIONAL ENGINEER

DATE	SCALE
2025.06.19	1/4" = 1'-0"

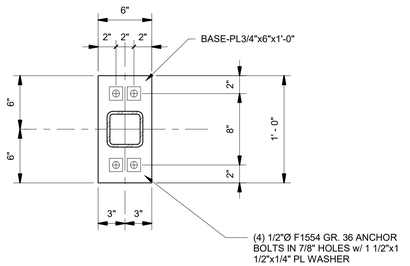
DRAWN BY	DESIGNED BY	CHECKED BY
WCT	WCT	CDL

PROJECT No. 10377.028

DRAWING NO. **S102** REV. **A**

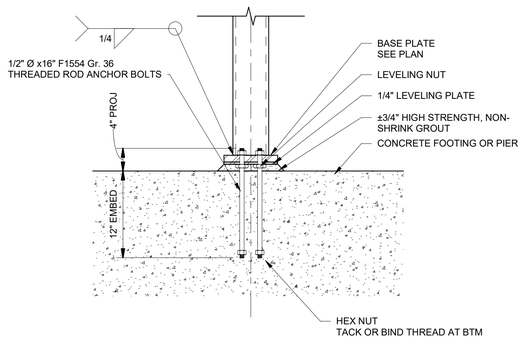


1 S200 **STRUCTURAL FRONT ELEVATION**
SCALE: 3/4" = 1'-0"



TYPICAL BP

TYPICAL BASEPLATE
SCALE: 1 1/2" = 1'-0"



TYPICAL STEEL COLUMN BASEPLATE CONNECTION
SCALE: 1 1/2" = 1'-0"

REV.	DATE	DESCRIPTION	BY	CHK
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CONNOR SCHOOL RENOVATIONS

1581 VAN BUREN RD, CONNOR, ME 04736

TITLE

STRUCTURAL FRONT ELEVATION

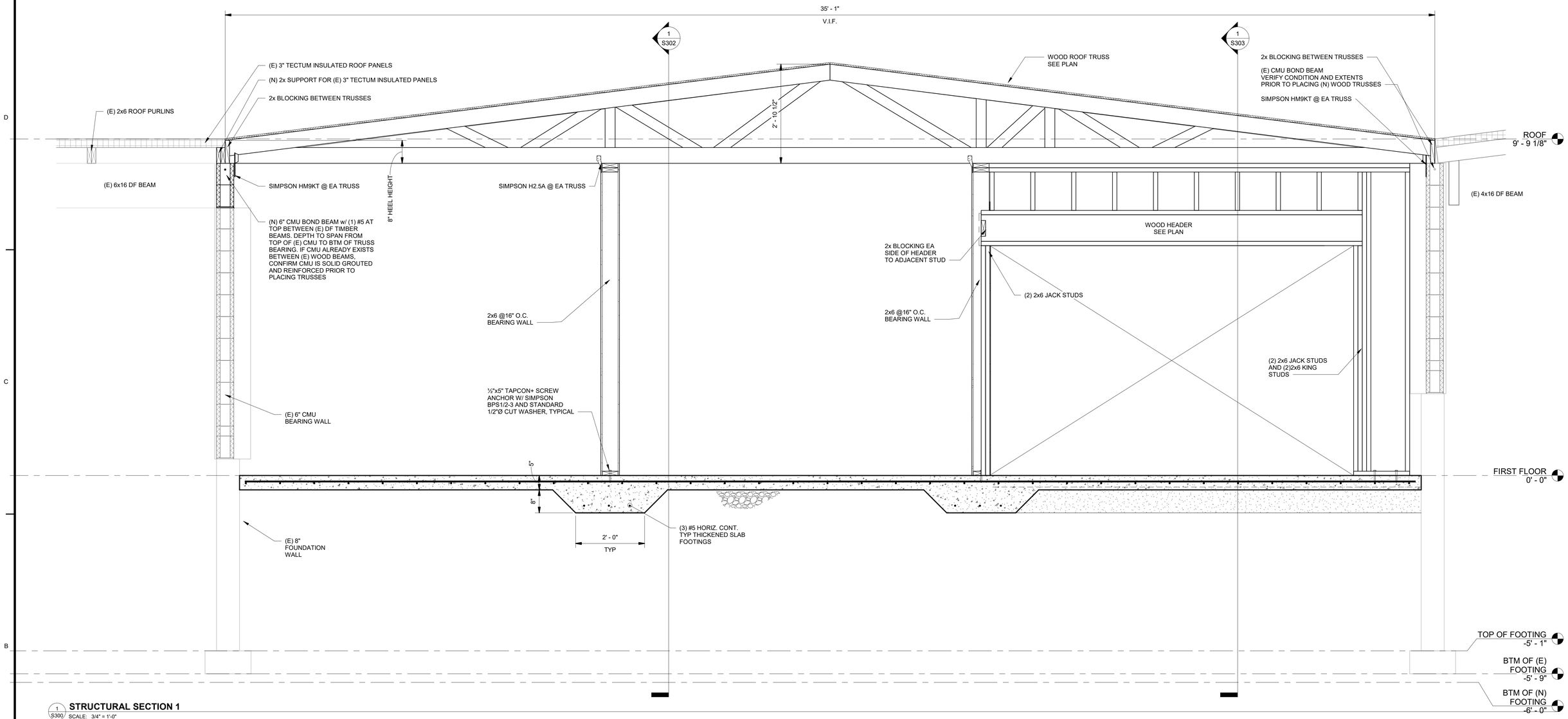
STATE OF MAINE
WILLIAM C. TIMKEN
No. 18004
LICENSED PROFESSIONAL ENGINEER

DATE	SCALE
2025.06.19	As indicated

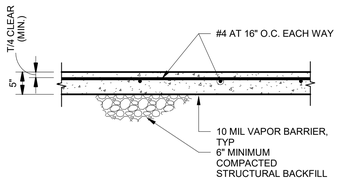
DRAWN BY	DESIGNED BY	CHECKED BY
WCT	WCT	CDL

PROJECT No. 10377.028

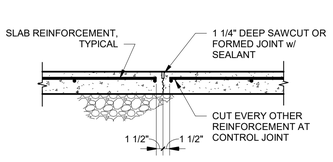
DRAWING NO. **S200** REV. **A**



1 S300 **STRUCTURAL SECTION 1**
SCALE: 3/4" = 1'-0"



TYPICAL INTERIOR SLAB DETAIL
SCALE: 3/4" = 1'-0"



CONTROL JOINT AT INTERIOR SLAB
SCALE: 3/4" = 1'-0"

REV.	DATE	DESCRIPTION	BY	CHK
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1581 VAN BUREN RD, CONNOR, ME 04736

TITLE

STRUCTURAL SECTION

STATE OF MAINE
WILLIAM C. TIMKEN
No. 18004
LICENSED
PROFESSIONAL ENGINEER

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2025.06.19	3/4" = 1'-0"

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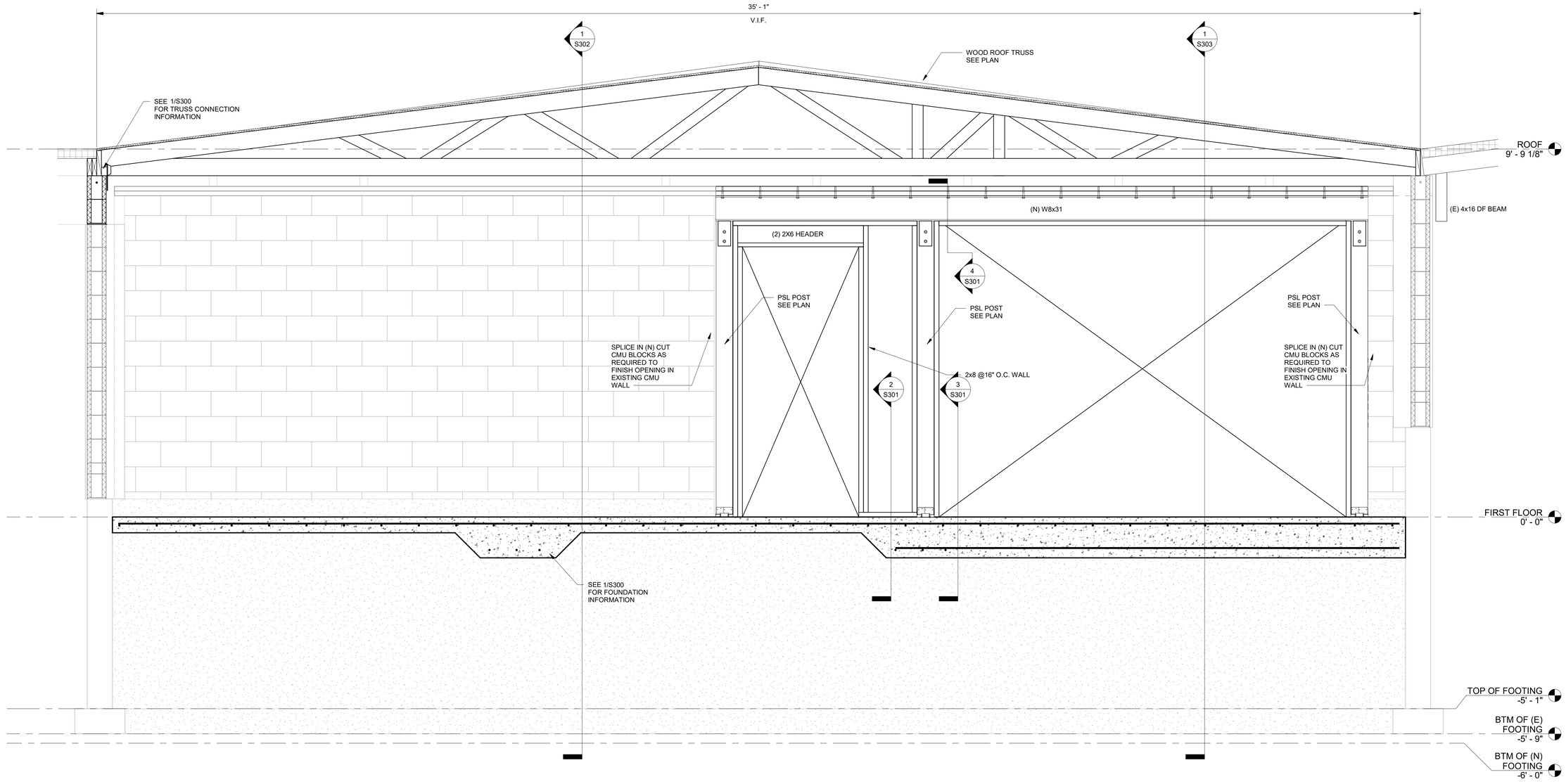
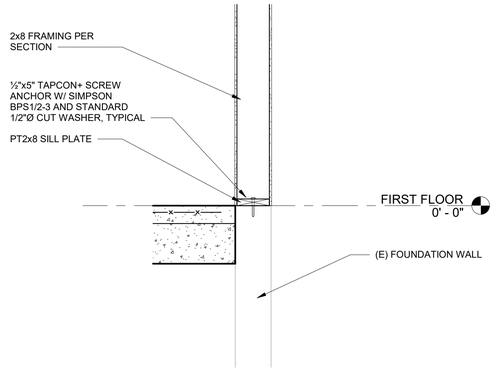
PROJECT No. 10377.028

DRAWING NO. **S300** REV. **A**



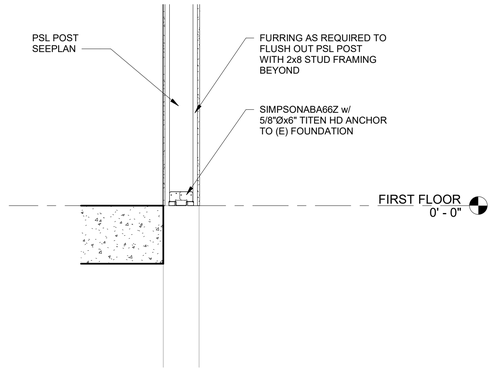
1 S301 STRUCTURAL SECTION 2

SCALE: 3/4" = 1'-0"



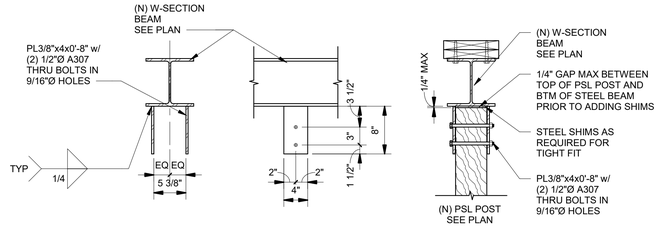
2 S301 (N) CMU WALL TO (E) FOUNDATION

SCALE: 3/4" = 1'-0"



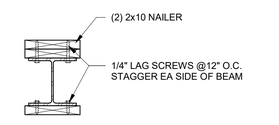
3 S301 (N) CMU WALL TO (E) FOUNDATION1

SCALE: 3/4" = 1'-0"



4 S301 STEEL BEAM TO PSL POST CONNECTION

SCALE: 1" = 1'-0"



NAILER TO STEEL BEAM CONNECTION

SCALE: 1" = 1'-0"

REV.	DATE	DESCRIPTION	BY	CHK
A	2025/06/19	RE-BID ADDENDUM 1	WCT	CDL

DRAWING ISSUE STATUS

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STRUCTURAL SECTION

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No. 18004
LICENSED PROFESSIONAL ENGINEER

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2025.06.19	As indicated

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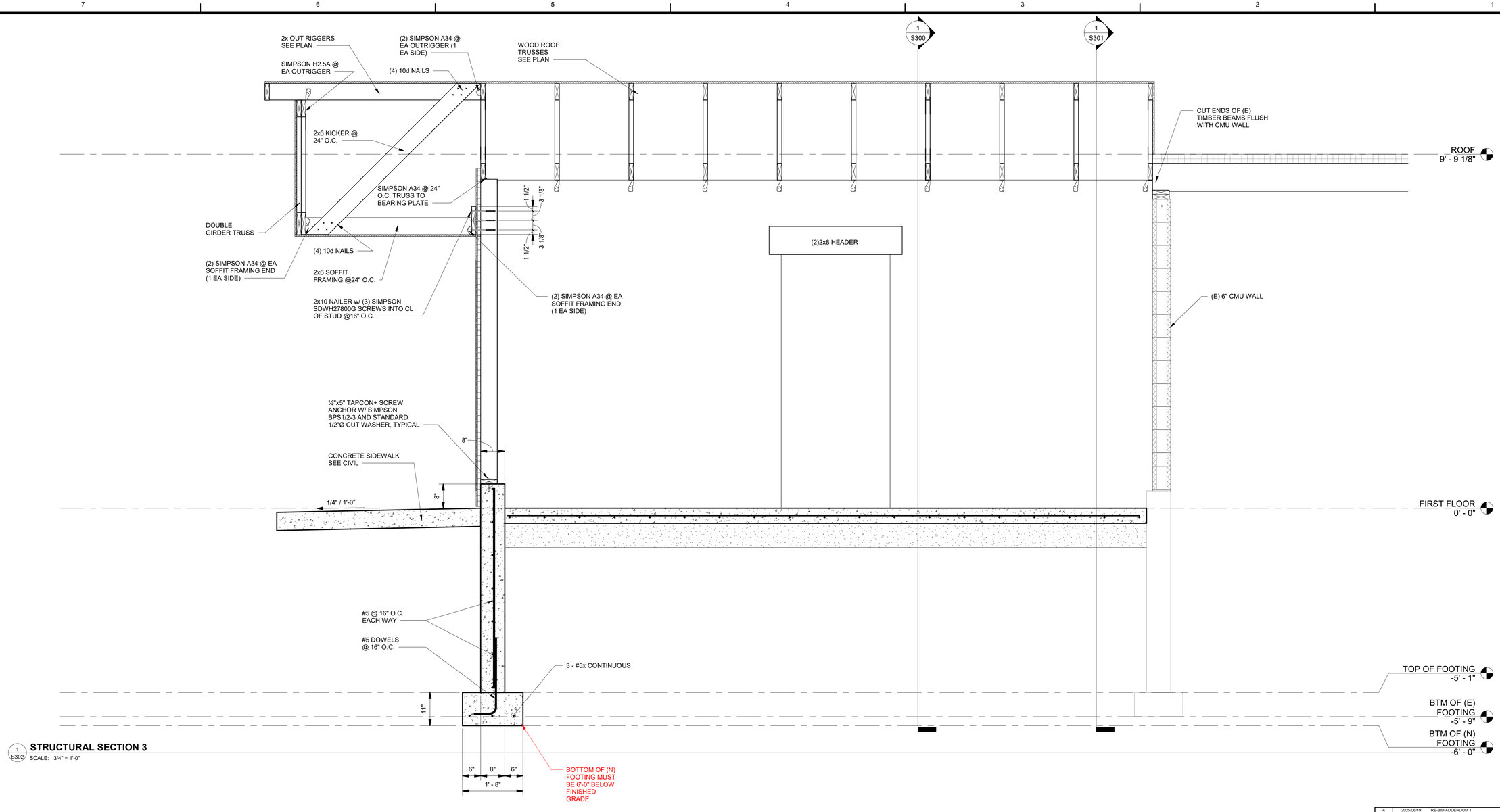
WCT	WCT	CDL

PROJECT No. 10377.028

DRAWING NO. **S301** REV. **A**



1 S302 **STRUCTURAL SECTION 3**
SCALE: 3/4" = 1'-0"

REV.	DATE	DESCRIPTION	BY	CHK.
A	2025/06/19	RE-BID ADDENDUM 1	WCT	CDL

DRAWING ISSUE STATUS

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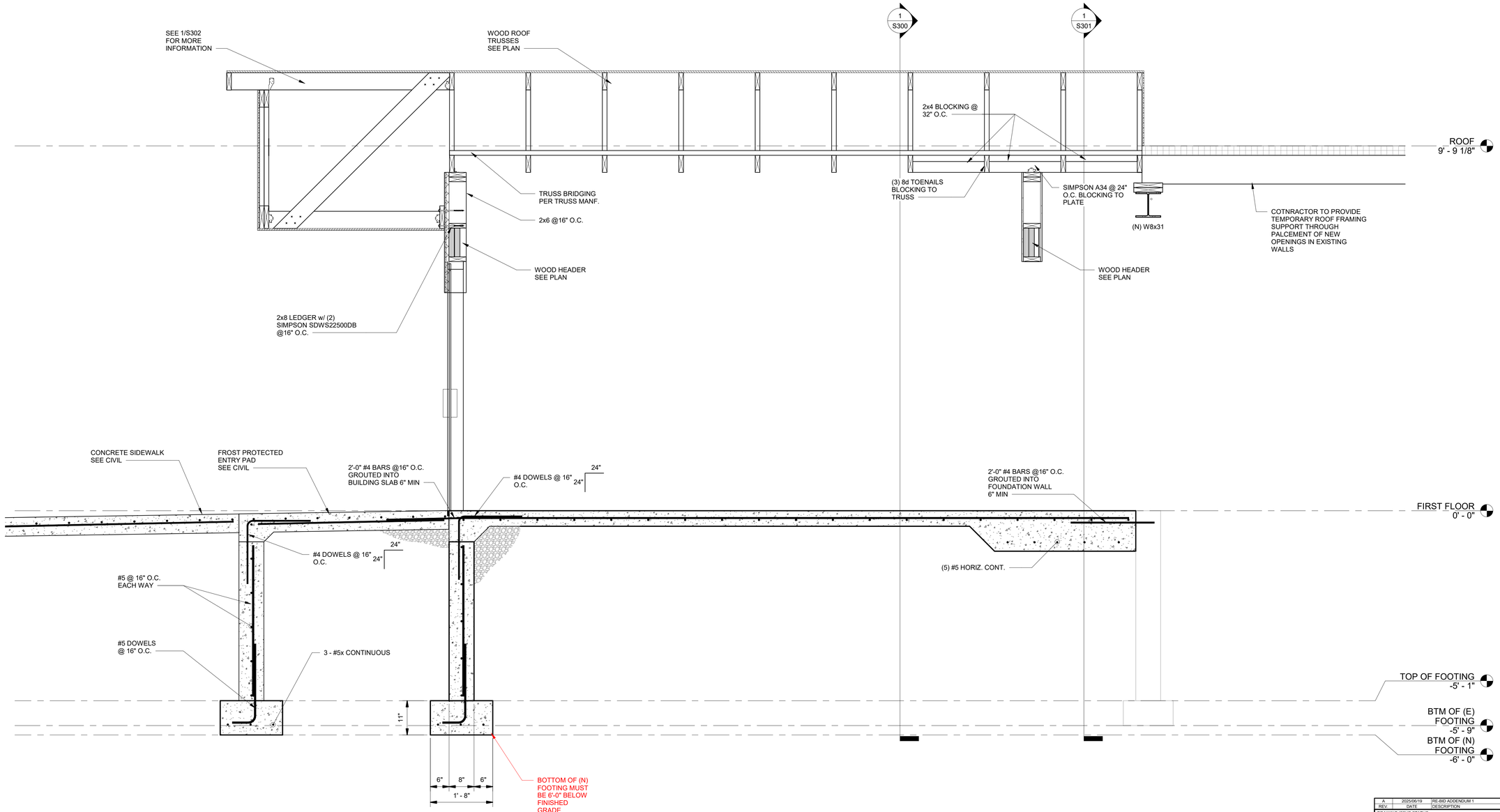
DATE	SCALE
2025.06.19	3/4" = 1'-0"

DRAWN BY	DESIGNED BY	CHECKED BY
WCT	WCT	CDL

PROJECT No. 10377.028

DRAWING NO. **S302** REV. **A**



1 S303 **STRUCTURAL SECTION 4**
SCALE: 3/4" = 1'-0"

REV.	DATE	DESCRIPTION	BY	CHK
A	2025/06/19	RE-BID ADDENDUM 1	WCT	CDL

DRAWING ISSUE STATUS

ISSUED FOR RE-BID

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TITLE

STRUCTURAL SECTION

STATE OF MAINE
WILLIAM C. TIMKEN
No. 18004
LICENSED PROFESSIONAL ENGINEER

DATE	SCALE
2025.06.19	3/4" = 1'-0"
DRAWN BY: WCT	DESIGNED BY: WCT	CHECKED BY: CDL
PROJECT No. 10377.028	
DRAWING NO. S303	REV. A